# DYNAMIC ANALYSIS OF ELECTROHYDRAULIC SHAKING TABLES

by

Dixon Rea

S. Abedi-Hayati

and

Y. Takahashi

Report to National Science Foundation

Report No. UCB/EERC-77/29 Earthquake Engineering Research Center College of Engineering University of California Berkeley, California

December 1977

	• .			
BIBLIOGRAPHIC DATA SHEET	1. Report No. NSF/RA-770529	2.	3. Recipier PB	11 <sup>°</sup> s Accession-No. 282 569
4. Title and Subtitle Dyna: Chak	nic Analysis of Electrohyd	raulic	5. Report D Decemb	oer 1977
Sliak	Ing Tables		6.	
7. Author(s) Dixon Rea,	S. Abedi-Hayati and Y. Ta	kahashi	8. Performi No.	ng Organization Rept. 17/29
9. Performing Organization Earthquake Engin	Name and Address eering Research Center		10. Project	/Task/Work Unit No.
University of Ca 47 Street and Ho Richmond, Califo	lifornia, Richmond Field S ffman Blvd. rnia 94804	tation	11. Contrac	t/Grant No.
12. Sponsoring Organization	Name and Address		13. Type of	Report & Períod
National Science Foundation			Covered	<b>j</b>
Washington,	D.C. 20550		14.	
15. Supplementary Notes		*****		
		:		
Mathematica by adjusting parameters experimental freque effects of a reson of shaking tables motions. It was foun quency response of by the control syst the structure to the structure with difficulties in d	I models were formulated heters to obtain the best hency responses. The math hant structure and of foun and on the ability of sha hd that the magnitudes of a shaking table are sens stem. In addition, the ma the mass of the shaking ta h respect to the table. A	for both tables, correspondence b ematical models dation complienc king tables to r the peak and not itive to the amo gnitudes depend ble and to the t lthough the peak esponse of struct	and the model etween the com were then used e on the frequ eproduce earth on distortions ount of force f on the ratio c ransmissibilit and notch eff	s were refined puted and to study the ency responses quake-type in the fre- eedback employed of the mass of y function of ect may cause of shaking
difficulties in d tables, it has li earthquake-type m	stermining the frequency f ttle effect on the accurac otions.	y to which a sha	king table car	reproduce
It was fou shaking table onl amount which depe the table	nd that foundation complie y at low frequencies, and nds on the transmissibilit	nce affects the the magnitude of y function of th	frequency resp the effect is ne foundation w	oonse of a s limited to an with respect to
		<b>i</b>		
18. Availability Statement		19. Sec Re	curity Class (This port)	21.
Poloaco	Unlimited	20. Sec	UNCLASSIFIED curity Class (This	22. Price
verease		Pa	8e	IAA.I AA.

FORM NTIS-35 (REV. 10-73) ENDORSED BY ANSI AND UNESCO.

THIS FORM MAY BE REPRODUCED

USCOMM-DC 8265-P74

. . •

#### ABSTRACT

The frequency response characteristics of two shaking tables have been determined experimentally. The lighter table, weighing 2,000 lb (900 kg), was used primarily to determine the effects of a resonant structure on a shaking table's frequency response. The heavier table, weighing 100,000 lb (45,300 kg), was used primarily to determine the effects of foundation compliance on a shaking table's frequency response.

Mathematical models were formulated for both tables, and the models were refined by adjusting parameters to obtain the best correspondence between the computed and experimental frequency responses. The mathematical models were then used to study the effects of a resonant structure and of foundation compliance on the frequency responses of shaking tables and on the ability of shaking tables to reproduce earthquake-type motions.

It was found that the magnitudes of the peak and notch distortions in the frequency response of a shaking table are sensitive to the amount of force feedback employed by the control system. In addition, the magnitudes depend on the ratio of the mass of the structure to the mass of the shaking table and to the transmissibility function of the structure with respect to the table. Although the peak and notch effect may cause difficulties in determining the frequency response of structures by means of shaking tables, it has little effect on the accuracy to which a shaking table can reproduce earthquake-type motions.

It was found that foundation compliance affects the frequency response of a shaking table only at low frequencies, and the magnitude

iii

of the effect is limited to an amount which depends on the transmissibility function of the foundation with respect to the table.

# Preceding page blank

# ACKNOWLEDGMENT

The work described in this report was supported by the National Science Foundation.

Preceding page blank

## TABLE OF CONTENTS

	2age
ABSTRACT	iii
ACKNOWLEDGMENT	vii
TABLE OF CONTENTS	ix
LIST OF FIGURES	xi
1. INTRODUCTION	1
2. EXPERIMENTAL FREQUENCY RESPONSES	3
2.1 2,000 lb Shaking Table	3
2.2 100,000 lb Shaking Table	5
3. ANALYSIS OF CONTROL SYSTEM FOR 2,000 lb SHAKING TABLE	11
3.1 Frequency Response of 2,000 lb Shaking Table	11
3.2 Effects of Test Specimen Reaction on 2,000 lb Shaking Table	18
4. ANALYSIS OF CONTROL SYSTEM FOR 100,000 1b SHAKING TABLE	23
4.1 Effect of Flexibility in Couplings and Foundation	24
5. CONCLUSION	27
REFERENCES	29

Preceding page blank

\_\_\_\_\_

# LIST OF FIGURES

Figure		Page
1.1	Limitations on the dynamic performance of a particular shaking table	30 •
2.1	The 2,000 lb shaking table	. 31
2.2	Schematic diagram of shaking table	. 32
2.3	Displacement frequency response of 2,000 lb table loaded with rigid mass of 2,000 lb	. 33
2.4	Structure and table frequency responses obtained simultaneously	. 34
2.5	Frequency responses of 2,000 lb table with resonant structure and varying amounts of force feedback	. 35
2.6	The 100,000 lb shaking table	. 36
2.7	Actuator locations	. 37
2.8	Translational frequency responses	. 38
2.9	Foundation transmissibilities	. 38
3.1	Servovalve-actuator hydraulic system	. 39
3.2	Idealized servovalve-actuator system	. 40
3.3	Linearized servovalve-actuator system	. 41
3.4	Shaking table closed loop control system	. 42
3.5	Simplified control system	43
3.6	Computed frequency responses: shaking table only	. 44
3.7	Shaking table with structure	. 45
3.8	Computed frequency responses: shaking table plus structure	. 46
3.9	Computed frequency responses for different effective table masses	. 47
3.10	Checks on fidelity of earthquake simulations	. 48
3.11	Computed vertical frequency response for 100,000 lb table	. 49
3.12	Shaking table with flexibly coupled structure and foundation	. 50

Preceding page blank

#### 1. INTRODUCTION

Shaking tables are being employed with increasing frequency in earthquake engineering to study the effects of earthquake type excitations on models of structures and small structures constructed from realistic structural components<sup>(1)</sup>. The objective of these tests is to determine the responses of the test specimens to earthquake type excitations, especially in cases where the intensity of the excitation is strong enough to cause nonlinear behavior in the specimen.

Shaking tables designed specifically for use in earthquake engineering are generally driven by high-performance hydraulic rams, or actuators, equipped with servo-valves. In these systems, the position of the actuator piston is controlled by means of an electronic closed-loop displacement feedback system. The basic displacement feedback is supplemented by velocity and force feedback signals to improve performance characteristics.

There are physical limitations to the intensity of motions shaking tables may undergo. Generally, at low frequencies the limitations are displacements imposed by the actuator strokes, at intermediate frequencies the limitations are velocities imposed by the maximum flow capacities of the servo-valves or pumps, and at higher frequencies the limitations are accelerations imposed by the force capacities of the actuators. These limitations for a particular shaking table are depicted in Fig. 1.1.

Within their physical limitations, shaking tables loaded only with rigid masses will exhibit frequency responses typical of secondorder systems. If the control system were open loop, the corner

frequency would occur at the oil column resonant frequency which is a function of the total mass being driven and the compliance of the oil contained in the actuators' cylinders. However, in closed-loop systems the corner frequency may be significantly lower than the oil column resonant frequency. The form of the frequency response limitations is also depicted in Fig. 1.1.

A shaking table loaded with a structure may have a distortion in its frequency response at the resonant frequency of the structure<sup>(2)</sup>. In addition, compliance in the actuator supports, which is likely in the case of large shaking tables, may adversely affect the frequency response<sup>(3)</sup>. Since these two effects may impair the performance of a shaking table, the degree to which they can alter frequency response characteristics has been investigated by a combination of experimental and analytical procedures.

Experimentally, the effects of a resonant structure on the frequency response characteristics of a shaking table can be investigated most conveniently by means of a relatively light table. In contrast, the investigation of foundation compliance needs a relatively heavy table. Thus two tables, one weighing 2,000 lb (900 kg) and the other 100,000 lb (45,300 kg) were used in the experimental phase of the investigation.

In the analytical phase of the investigation mathematical models were formulated for both tables, and the models were refined by altering their parameters in order to match the computed frequency responses with the experimental ones. The effects of a resonant structure and foundation compliance on the frequency response of shaking tables and on their ability to reproduce earthquake type motions were then studied by means of the mathematical models.

### 2. EXPERIMENTAL FREQUENCY RESPONSES

Experimental frequency responses were determined for two shaking tables. The lighter shaking table, weighing 2,000 lb (900 kg), was used to determine the effects of load resonance on the frequency response since a large ratio of structure mass to table mass could be achieved easily. The effects of foundation compliance on frequency response were investigated experimentally by means of a 100,000 lb (45,300 kg) shaking table because then the table weight is large enough relative to the foundation weight that some significant effects might be expected.

### 2.1 2,000 lb Shaking Table

A shaking table with one horizontal direction of motion only, driven by a single hydraulic actuator, and loaded by a single degree of freedom test structure, is shown in Fig. 2.1. The shaking table is a rectangular steel platform with overall dimensions of 10 ft x 7 ft (3 m x 2 m) weighing 2,000 lb. The longer sides are formed by two 10 x 12 WF beams which are connected transversely by four 6 ft (1.8 m) lengths of rectangular tubing. Four 10 ft (3 m) lengths of 6 x 2 in. (150 x 50 mm) channel sections are welded across the bottom faces of the rectangular tubes and run the length of the platform. The table is supported on two V and two single Thompson linear bearings. Each bearing runs on an 18 in. (450 mm) length of 2 in. (50 mm) diameter bar which is grouted into a 3 x 1 1/2 in. (75 x 38 mm) channel bolted to a strong floor.

The single degree of freedom structure shown mounted on the table in Fig. 2.1 consists of a platform supported by four columns.

The platform, which is identical to the shaking table, weighs 2,000 lb (900 kg). The far two columns have pillow block bearings at both ends and thus do not contribute to the horizontal stiffness of the single degree of freedom structure. The near two columns also have pillow block connections at their tops but are bolted rigidly to the shaking table and provide the horizontal stiffness of the single degree of freedom structure by bending about their weak axes.

The table is driven by an hydraulic actuator having an effective area of 25.4 in<sup>2</sup> (164 cm<sup>2</sup>) and a stroke of 12 (±6) in.  $(300(\pm 150)$  mm). The actuator is equipped with a two stage servo-valve that can feed oil to the actuator at a rate up to 175 gallons (0.67 m<sup>3</sup>) per minute. The position of the actuator's piston is controlled by means of a closed loop feedback system. In addition to the primary displacement feedback signal, secondary feedback signals consisting of actuator force and piston velocity are used for stabilizing the primary feedback loop. A schematic diagram of the control system is shown in Fig. 2.2. MTS Systems supplied the hydraulic actuator and its associated hydraulic and electronic components.

Frequency responses were determined for the 2,000 lb (900 kg) shaking table loaded with a rigid mass weighing 2,000 lb (900 kg) and for the shaking table loaded with the single degree of freedom structure shown in Fig. 2.1. Displacement frequency responses for the table loaded with the rigid mass and for amplitudes of 0.025, 0.05, and 0.1 in. (0.6, 1.3 and 2.5 mm) are shown in Fig. 2.3. The resonant or corner frequency decreases from 24 cps (24 Hz) for the smallest amplitude to 18 cps (18 Hz) for the largest amplitude.

An acceleration frequency response of the single degree of freedom structure shown on the table in Fig. 2.1 is shown in Fig. 2.4(a).

This frequency response was observed while the shaking table frequency response, shown in Fig. 2.4(b), was being observed. The table's frequency response contains a peak and notch at the same frequency as the resonant frequency of the structure. This effect is caused by the reaction loads of the structure on the table and is sensitive to the amount of force feedback used to stabilize the primary displacement feedback loop, see Fig. 2.2. The effect of varying amounts of force feedback on the table's frequency response is illustrated in Fig. 2.5.

## 2.2 100,000 lb Shaking Table

The 100,000 lb (45,300 kg) shaking table, shown in Fig. 2.6(a), is constructed from a combination of reinforced and prestressed concrete  $^{(4)}$ . Structurally, it may be considered as a 1 ft (300 mm) thick 20 ft (6 m) square plate. The plate is stiffened by heavy central transverse ribs that are 1 ft (300 mm) wide and extend 1 ft 9 in. (525 mm) below the bottom surface of the plate, and by lighter diagonal ribs that are also 1 ft (300 mm) wide and extend 4 in. (100 mm) below the bottom surface of the table. The hydraulic actuators that drive the table horizontally are attached to the table by means of one of the transverse ribs. The vertical actuators, as well as test structures, are attached to the table by means of prestressing rods located in 2 in. (50 mm) diameter pipes that run vertically through the table on a 3 ft (1 m) square grid.

The shaking table is driven horizontally by three 50 kip (220 kN) hydraulic actuators, one of which is shown in Fig. 2.6(b), and vertically by four 25 kip (110 kN) hydraulic actuators, one of which is shown in Fig. 2.6(c). The actuators have swivel joints at both ends so that they rotate about the foundation swivel joints as the table

moves. The total length of each horizontal actuator, including swivel joints, is 10 ft 6 in. (3.2 m), and the total length of each vertical actuator is 8 ft 8 in. (2.7 m). The length of the actuators helps to decouple the vertical and horizontal motions of the table, and further decoupling is accomplished by electronic means. The actuators are located in a pit beneath the shaking table as shown in Fig. 2.7.

The horizontal actuators are equipped with 175 gpm  $(0.67 \text{ m}^3/\text{min})$ servo-valves and the vertical actuators with 90 gpm  $(0.34 \text{ m}^3/\text{min})$  servovalves. The flow rate of the servo-valves limits the maximum velocities in the horizontal and vertical directions to 20 in/sec (500 mm/sec) and 15 in./sec (380 mm/sec), respectively. The strokes are 12 in. (±6) (300 mm(±150)) for the horizontal actuators and 4 in. (±2)(100 mm(±50)) for the vertical actuators. However, the horizontal actuators will be limited to displacements of ± 5 in. (126 mm) to improve the resolution of table motion in the horizontal direction.

In operation, the air in the pit within the foundation and beneath the shaking table is pressurized so that the total dead weight of the table and the test structure is balanced by the difference in air pressure between the air in the pit and the air above the shaking table. The pit entrance is sealed by two air-tight doors that provide a lock chamber and, thus, access to the pit while the air in the pit is pressurized. The l ft (300 mm) gap between the shaking table and the interior foundation walls is sealed by a 24 in. (600 mm) wide strip of vinyl covered nylon fabric. The fabric, in its inflated position, can be seen in Fig. 2.6(a). A differential air pressure of 1.55 psi (10.7 kN/m<sup>2</sup>) is required to balance the dead weight of the shaking table alone; the maximum air pressure is not expected to exceed 4 psi.

The actuator forces are counteracted by a massive foundation, which is a reinforced concrete structure in the form of an open box with 5 ft (1.5 m) thick sides. The outside dimensions of the box are 32 ft x 32 ft x 15 ft (10 m x 10 m x 4.5 m), and the inside dimensions are 22 ft x 22 ft x 10 ft (7 m x 7 m x 3 m). The shaking table forms a closure for the box; the top of the shaking table being flush with the top of the foundation walls which in turn are flush with the floor slab of the building housing the shaking table, see also Fig. 2.6(a). The foundation weighs 1,580 kips (6.6 MN).

The electronic control system for the shaking table, which was supplied by MTS Systems Corporation, Minneapolis, Minnesota, who also supplied the hydraulic actuators, is based on controlling five degrees of freedom of the shaking table<sup>(5)</sup>. The sixth degree of freedom, translation perpendicular to the direction of the horizontal translational degree of freedom, is controlled by a sliding mechanism. Transducers are installed in each actuator to measure displacements and forces. From the displacement signals, feedback signals representing the average horizontal and vertical displacement, the pitch, roll and yaw (or twist) are derived on the assumption that the table is a rigid body. Corresponding force signals are also derived that are used to supplement the primary displacement feedback signals. Normally the pitch, roll and yaw command signals are zero, and the horizontal and vertical command signals represent translational displacement time histories of an earthquake record.

Frequency response functions for vertical and horizontal motions of the shaking table are shown in Fig. 2.8. The gain factors exhibit varying degrees of flatness and peaking because the control settings

were different for each frequency response measurement. The control system is quite sensitive to gain settings of the primary loops in the translational degrees of freedom, and to the amount of force stabilization in the pitch degree of freedom. A particular frequency response could be improved slightly by searching for an optimum control setting. However, since such adjustments will be difficult to perform with a test structure on the shaking table, the curves should be regarded as typical.

The frequency response functions indicate closed loop resonant frequencies of about 8 cps (Hz). These resonant frequencies are about 50% of the open-loop or oil column resonant frequencies which have been determined to be 15 and 16 cps (Hz) for vertical and horizontal motions, respectively. Although the closed loop resonant frequencies may be increased by increasing the loop gain, the improvement is small before the system becomes unstable.

The foundation transmissibility functions have been established by operating the table under harmonic motion of constant acceleration amplitude and varying frequency. The transmissibility functions for vertical and horizontal motions are shown in Fig. 2.9(a) and 2.9(b) respectively. The gain factors show that at frequencies below 10 cps (10 Hz) the soil stiffness is predominant in counteracting actuator forces, while at frequencies above 20 cps (20 Hz) the inertia mass of the foundation becomes predominant in counteracting the actuator forces. At frequencies between 10 and 20 cps (10 and 20 Hz), there is a transition zone where soil stiffness and foundation inertia are combining to counteract the actuator forces. In the vertical direction of motion the ratio of foundation acceleration to table acceleration

reaches 4% at a frequency of 24 cps (24 Hz). At 24 cps (24 Hz) the ratio appears to be rapidly approaching its limiting value of 6.3%, which is the ratio of table weight to foundation weight. The ratio of foundation acceleration to table acceleration for horizontal motion reaches the limit of 6.3% at 25 cps (25 Hz) and will probably exceed this value because the actuator forces are applied in a plane above the center of gravity of the foundation, see Fig. 2. Thus the foundation pitches as well as translates under the action of the horizontal actuators. The foundation acceleration measurements for Fig. 2.9(b) were made at the level of the horizontal actuators.

Soil of greater stiffness would have improved the foundation transmissibility functions over the complete frequency range  $^{(6)}$ . Since there are resonances in the transmissibility functions at about 8 cps (8 Hz), a lighter foundation would have improved the transmissibility functions in the frequency region below 10 cps (10 Hz) while making them worse at higher frequencies.

3. ANALYSIS OF CONTROL SYSTEM FOR 2,000 1b SHAKING TABLE

Servovalves control the flow of fluid through orifices and therefore they are inherently nonlinear devices. The nonlinear differential equations governing the behavior of systems incorporating servovalves may be solved directly by means of digital computers. But such analyses are expensive, and they do not easily impart a physical understanding of how a system behaves. On the other hand linear analyses, although valid only for small excursions about some operating point, are easily interpreted in terms of physical behavior. Since linear analyses for excursions about the zero position have been found adequate to describe the behavior of many electrohydraulic systems, a mathematical model of the 2,000 lb (900 kg) shaking table was formulated for such analyses. Frequency responses for the mathematical model are then compared with experimental frequency responses of the 2,000 lb (900 kg) shaking table. Finally, the response of the shaking table to earthquake type excitations when it is loaded with single degree of freedom linear and nonlinear structures is discussed.

# 3.1 Frequency Response of 2,000 1b Shaking Table

A schematic diagram of a rigid mass shaking table driven by an hydraulic actuator and a two stage servovalve is shown in Fig. 3.1. The equations governing such a system incorporating a single state servovalve have been derived by Merritt<sup>(7)</sup>. Following Merritt's assumptions the equations for the system shown in Fig. 3.1 may be written as follows:

$$m_{p} \ddot{x} + c_{p} \dot{x} + k_{p} \dot{x} = F_{i}$$
(3.1)

**Preceding page blank** 

$$q_{lp} = f_{p}(x, P_{ls})$$
(3.2)

$$q_{lp} = x_{s} A_{s} + \frac{V_{s}}{4\beta} P_{ls}$$
(3.3)

$$m_{s} \ddot{x}_{s} + c_{s} \dot{x}_{s} + k_{s} x_{s} = A_{s} P_{ls} = F_{s}$$
(3.4)

$$q_{ls} = f_s(x_s, P_{la})$$
(3.5)

$$q_{ls} = \dot{x}_{t} A_{a} + \frac{V_{a}}{4\beta} \dot{P}_{la}$$
(3.6)

$$m_t \ddot{x}_t + c_t \dot{x}_t = A_a P_a = F_a$$
(3.7)

where

- c, c, c, t = viscous damping coefficients for the pilot spool, slave spool, and actuator piston, respectively.
- k<sub>p</sub>, k<sub>s</sub> = stiffness coefficients tending to center the pilot and slave spools, respectively. The pilot spool coefficient is derived partly from a mechanical spring and partly from fluid flow, whereas the slave spool coefficient is derived entirely from fluid flow.

f , f = nonlinear functions governing the pilot and slave
 stage load flows.

$$P_{ls}$$
,  $P_{la}$  = differential pressure across slave spool and actuator piston, respectively.

$$\beta$$
 = bulk modulus of fluid.

Equations (3.1) through (3.7) are depicted in block diagram form in Fig. 3.2 where S denotes the Laplacian operator.

Normally, two stage servovalves have some form of feedback from the slave stage to the pilot stage, and the servovalve in the control system of the 2,000 lb (900 kg) shaking table feeds back a signal from a displacement transducer that is proportional to slave spool position  $x_s$ . The effect of such feedback is to linearize the behavior of the servovalve so that the slave spool position is proportional to the applied force in the system's operating frequency range:

$$\mathbf{x}_{s} = \mathbf{k} \mathbf{F}_{i} \tag{3.8}$$

In addition, for small slave spool excursions about its central position,

the nonlinear function f of equation (3.5) may be linearized by means of a Taylor series expansion of the form (see ref. (7))

$$q_{ls} = K_q x_s - K_c P_{la}$$
(3.9)

where

 $K_q$  = slave stage flow gain, and  $K_c$  = slave stage flow-pressure coefficient.

Equations (3.1) through (3.7) can be replaced by equations (3.6) through (3.9) and the latter equations solved to determine the transfer function relating the position of the actuator piston to the force applied to the pilot spool:

$$\frac{\mathbf{x}_{t}}{\mathbf{F}_{i}} = \frac{\mathbf{k} \mathbf{K}_{q} \mathbf{A}_{a}}{s \left(\frac{\mathbf{v}_{a} \mathbf{m}_{t}}{4\beta} s^{2} + \left(\frac{\mathbf{v}_{a} \mathbf{c}_{t}}{4\beta} + \mathbf{K}_{c} \mathbf{m}_{t}\right) s + \left(\mathbf{A}_{a}^{2} + \mathbf{K}_{c} \mathbf{c}_{t}\right)\right)}$$
(3.10)

Since K c is negligible in comparison to  $A_a^2$ , see ref. (7), and assuming

$$\mathbf{F}_{\mathbf{i}} = \mathbf{k}' \mathbf{x}_{\mathbf{i}} \tag{3.11}$$

where

x, = command table displacement

k' = an electronic amplifier gain,

then the transfer function relating  $x_t$  and  $x_i$  is

$$\frac{x_{t}}{x_{i}} = \frac{\omega_{0}^{2} k' k K_{q} / A_{a}}{s \left(s^{2} + 2 \zeta_{0} \omega s + \omega_{0}^{2}\right)}$$
(3.12)

$$\omega_0^2 = \frac{4 \beta A_a^2}{V_a m_t}, \qquad (3.13)$$

and

$$\zeta_{0} = \frac{c_{t}}{2 m_{t} \omega_{0}} + \frac{4 \beta K_{c}}{2 V_{a} \omega_{0}}.$$
 (3.14)

The open loop natural frequency  $\omega_0$  is commonly referred to as the oil column resonant frequency. The oil column resonant frequency for the 2,000 lb (900 kg) shaking table loaded with a 2,000 lb (900 kg) weight (m<sub>t</sub> = 4,000/386 lb - sec<sup>2</sup>/in. (1,800/9.81 kg - sec<sup>2</sup>/m),  $A_a = 25.4 \text{ in}^2$  (164 cm<sup>2</sup>),  $V_a = 25.4 \times 12.5 \text{ in}^3$  (164 x 31.8 cm<sup>3</sup>), and assuming  $\beta = 2 \times 10^5$  psi (1.4 x  $10^6 \text{ kN/m}^2$ ) is 396 rad/sec or 63 cps (63 Hz).

The damping factor for the open loop system cannot be evaluated reliably from equation (3.13) because neither  $c_t$  nor  $K_c$  are known accurately, and there are other sources of damping that have been neglected in the analyses. However, the effect of  $K_c$ , the slave stage flow-pressure coefficient, is to increase the equivalent damping of the open loop system; and the effects of  $c_t$  and  $K_c$  and other sources of damping may be incorporated into an equivalent viscous damping coefficient is given by

$$c_e = 2 \zeta_e \omega_0 m_t . \tag{3.15}$$

Thus equations (3.7) and (3.9) become, respectively,

$$m_t \ddot{x}_t + c_e \dot{x}_t = A_a P_{la} = F_a$$
(3.7a)

$$q_{ls} = K_{q} x_{s}$$
(3.9a)

and equations (3.6), (3.7a), (3.8), (3.9a), and (3.11) are the equivalent linearized equations for the open loop system operating about its central position. These equations are depicted in block diagram form in Fig. 3.3.

Electrohydraulic shaking tables employ closed loops for control in which the position of the actuator piston (which is the same as the table's position assuming they are rigidly coupled) is the primary feedback and the force the actuator is exerting and the velocity of the piston are supplementary feedback signals. These feedback signals are shown added to a modified linearized open loop system in Fig. 3.4. The transfer function relating actual table displacement and command displacement for the closed loop system is

$$\frac{x_{t}}{x_{i}} = \frac{K}{\frac{V_{a} m_{t}}{4\beta A_{a}} s^{3} + \left(\frac{V_{a} c_{e}}{4\beta A_{a}} + K k_{ff} m_{t}\right) s^{2} + \left(A_{a} + K k_{ff} c_{e} + K k_{vf}\right) s + K k_{d}}$$
......(3.16)

where

 $K = k' k K_{q}$ 

k<sub>ff</sub> = gain of force feedback

k<sub>vf</sub> = gain of velocity feedback, and

 $k_{df}$  = gain of displacement feedback.

In practical shaking tables, the equivalent viscous damping coefficient is small and velocity feedback does not have much effect. Thus  $c_{\rho}$  and  $k_{vf}$  may both be assumed equal to zero in equation (3.16),

anđ

and since the table displacement is required to equal the command displacement  $k_{df}$  is unity. Therefore equation (3.16) simplifies to

$$\frac{x_{t}}{x_{i}} = \frac{K}{\left(\frac{VS^{3}}{4\beta A} + Kk_{ff}S^{2}\right)m_{t} + AS + K}$$
(3.17)

where the subscript a has been dropped from both V and A. The associated block diagram is depicted in Fig. 3.5.

The frequency response function  $H(i\omega)$  of the closed loop system may be obtained by substituting  $i\omega$  for S in equation (3.17) so

$$H(i\omega) = \frac{K}{K\left(1 - k_{ff} m_{t} \omega^{2}\right) + i\left(A \omega - \frac{V m_{t}}{4\beta A}\omega^{3}\right)}$$
(3.18)  
$$= |H(i\omega)| e^{-i\phi(\omega)}.$$
(3.19)

The system depicted in Fig. 3.5 was simulated on a digital computer and the gain factor  $|H(i\omega)|$  and the phase factor,  $\phi(\omega)$ , were determined. The values of A,  $m_t$ , V, and  $\beta$  were made 25.4, 4,000/g, 318, and 2 x 10<sup>5</sup>, respectively, so that the system would represent the 2,000 lb (900 kg) shaking table loaded with a 2,000 lb (900 kg) mass for which frequency responses were presented in Fig. 2.3. The parameters K and  $k_{ff}$  were varied in order to obtain values that would reproduce these experimental frequency responses.

The effects of varying K while maintaining  $k_{ff}$  constant and of varying  $k_{ff}$  while maintaining K constant are shown in Fig. 3.6(a) and (b), respectively. The effectiveness of force feedback in controlling the resonant response of the system can be seen in Fig. 3.6(b). Once the force feedback reaches an adequate level it effectively prevents

resonance at the oil-column resonant frequency. However, it also has the adverse effect of reducing the corner frequency of the frequency response function. It is apparent that values of K in the range 2,000 to 3,000 and values of  $k_{ff}$  in the range 4 x 10<sup>-6</sup> to 8 x 10<sup>-6</sup> will produce frequency responses similar to those obtained experimentally.

3.2 Effects of Test Specimen Reaction on 2,000 lb Shaking Table

The mathematical model of the 2,000 lb (900 kg) shaking table was modified to incorporate a single degree of freedom structure attached to the shaking table, and the block diagram of the model is shown in Fig. 3.7. The transfer function relating table displacement to the input is

$$\frac{x_{t}}{x_{i}} = \frac{K}{\left(\frac{VS^{3}}{4\beta A} + K K_{ff} S^{2}\right) m_{t} \left(1 + \frac{m}{m_{t}} \frac{x_{s}}{x_{t}}\right) + AS + K}$$
(3.20)

The system shown in Fig. 3.7 was simulated on a digital computer. The parameters in the system were selected so that it would represent the single degree of freedom structure and shaking table to which the experimental frequency response of Fig. 2.4 pertains.

Frequency responses in the form of gain factors versus frequency for the system are shown in Fig. 3.8, and peaks and notches similar to those shown in Fig. 2.4(b) and Fig. 2.5 are evident. The effect of the amount of force feedback on the magnitudes of the peak and notch is shown in Fig. 3.8(a). Increasing the amount of force feedback increases the magnitudes of the peak and notch. Similarly, as shown in Fig. 3.8(b), the magnitudes of the peak and notch increase as the ratio of the mass of the structure to the mass of the table  $(m_g/m_{+})$  increases. The

magnitudes of the peak and notch also increase if the damping capacity of the structure decreases as shown in Fig. 3.8(c).

The cause of the peak and notch may be seen by examining equation 3.20. This transfer function is similar to the transfer function for the table alone, equation 3.17, except that the table mass  $m_t$  has been replaced by an effective table mass  $m_p$  where

$$m_{e} = m_{t} \left( 1 + \frac{s}{m_{t}} \frac{s}{s_{t}} \right) .$$
 (3.21)

The effective mass depends on the ratio of the masses of the structure and table,  $m_s/m_t$ , and on the ratio  $x_s/x_t$ , which is the transmissibility function relating the absolute displacements of the structure and table:

$$\frac{x}{x_{t}} = T(\omega) = \frac{1 + i 2 \zeta \omega/\omega_{n}}{1 - (\omega/\omega_{n})^{2} + i 2 \zeta \omega/\omega_{n}}$$
(3.22)

where

$$\omega_n$$
 = circular natural frequency of the structure and

 $\zeta$  = damping factor of the structure.

The transmissibility function is approximately unity for frequencies up to 70% of the natural frequency of the structure, and it is approximately zero for frequencies greater than 1.5 times the natural frequency. Thus at the lower frequencies the effective table mass is

$$m_{e} = m_{t} \left( 1 + \frac{m_{s}}{m_{t}} \right) = m_{t} + m_{s}$$
(3.23)

and at the higher frequencies it becomes

$$m_{e} = m_{t}.$$
 (3.24)

Near the natural frequency of the structure the transmissibility function varies rapidly in magnitude and phase depending on the damping factor of the structure. At frequencies just less than the natural frequency the effective table mass is given approximately by

$$m_{e} = m_{t} \left( 1 + \frac{m_{s}}{m_{t}} \frac{1}{2\zeta} \right) ,$$
 (3.25)

and at frequencies just greater than the natural frequency it is given approximately by

$$m_e = m_t \left( 1 - \frac{m_s}{m_t} \frac{1}{2\zeta} \right) \quad .$$

If  $m_s/2\zeta$  is large compared with  $m_t$ , which will be the case when the mass of the structure is nearly as large as, or larger than, the mass of the table and the damping capacity of the structure is small, then the effective table mass just below and just above the natural frequency is  $m_c/2\zeta$  and  $-m_c/2\zeta$ , respectively.

A shaking table loaded with a resonant structure appears to the control system as a mass which varies with frequency. However, as shown in Fig. 3.9, the gain factor for a shaking table without load is a function of the table mass. The curves in this figure were obtained by assuming different values, including a negative value, for the mass of the shaking table in the system shown in Fig. 3.5. At low frequencies the effective table mass for a table weighing 2,000 lb (900 kg) loaded with a structure weighing 2,000 lb (900 kg) is 4,000/g(1800 kg/g) and the frequency response will be curve 2 in Fig. 3.9. Just below the natural frequency of the structure the effective table mass may increase due to resonance to 20,000/g (9,000 kg/g) and the frequency response changes from curve 2 to curve 3, Fig. 3.9. Just above the natural frequency, the effective table mass may become a negative 20,000/g (9,000 kg/g) and curve 4 is the appropriate frequency response. Finally, at higher frequencies the effective mass becomes 2,000/g (900 kg/g), and curve 1 is the appropriate frequency response. Thus a peak and notch are formed in the frequency response function as illustrated by the dotted line in Fig. 3.9.

In practice most shaking tables must use force feedback to control the table's resonant response and thus, when loaded with a resonant structure, their frequency response contains a peak and notch. Because of this effect, caution must be exercised when determining the frequency response characteristics of structures by means of shaking tables.

In order to determine the effect of the peak and notch in the frequency response on the ability of the shaking table to simulate earthquake motions, the acceleration time history of the N-S component of the El Centro (1940) earthquake was doubly integrated to obtain a displacement command signal. The command signal was then fed to the computer model of the shaking table loaded with a resonant structure as shown in Fig. 3.7. The amount of force feedback in the model was intentionally, made sufficient to produce a significant peak and notch in the frequency response of the shaking table.

The commanded El Centro acceleration time history is compared with the simulated earthquake motion in Fig. 3.10(a), and, although the

acceleration peaks tend to be slightly smaller in the simulated earthquake, the fidelity of the simulation is excellent. As a further check on the fidelity of the simulation the structure's response to the simulated shaking table motion and to the commanded motion are compared in Figs. 3.10(b) and (c). Figure 3.10(b) shows the response of a structure in which the spring characteristic remained linear throughout the acceleration time history. Figure 3.10(c) shows the response of a structure in which the spring characteristic was bi-linear hysteretic and the yield force was low enough so that the response of the structure was nonlinear during the simulation. In the case of linear behavior, Fig. 3.10(b), the response of the structure to the simulation is slightly smaller than the response of the structure to the commanded motion, but in the case of nonlinear behavior, Fig. 3.10(c), there are no discernible differences other than the inevitable phase shift.
4. ANALYSIS OF CONTROL SYSTEM FOR 100,000 1b SHAKING TABLE

The 100,000 lb (45,300 kg) shaking table is driven vertically by four 25 kip (110 kN) hydraulic actuators and horizontally by three 50 kip (220 kN) hydraulic actuators. The vertical actuators are controlled electronically to operate in phase, and the horizontal actuators are also synchronized independently of the vertical actuators. The actuators are sufficiently long so that horizontal and vertical motions are essentially uncoupled. Assuming the synchronization circuits have no effect on the main control loops, the horizontal and vertical actuators may each be treated as a single independent actuator, and the analytical model of Fig. 3.5 may be used for either vertical or horizontal motion.

The frequency response of the analytical model representing vertical motion of the 100,000 lb shaking table was determined by digital computer. The area of the equivalent vertical actuator was made equal to the sum of the areas of the four 25 kip (110 kN) actuators,  $38.44 \text{ in}^2$  (248 cm<sup>2</sup>). The volume of oil in the actuators was made equal to the total volume of oil in the four 25 kip (110 kN) actuators, 192.20 in<sup>3</sup> (3150 cm<sup>3</sup>). The bulk modulus of the oil was chosen so that the model would have the same oil column resonant frequency, 16 cps (16 Hz), as the 100,000 lb (45,300 kg) shaking table. The amounts of main loop gain and force feedback were varied in order to achieve optimum frequency responses.

An optimum frequency response for the analytical model of the 100,000 lb (45,300 kg) shaking table is shown in Fig. 3.11. The closed loop resonant frequency of this system is approximately 8 cps (8 Hz)

which is about half of the oil column or open loop resonant frequency. The computed frequency response is very similar to the one shown in Fig. 2.8(a) that was obtained for the actual table.

4.1 Effects of Flexibility in Couplings and Foundation

In the analytical model shown in Fig. 3.5 it has been assumed that the piston is coupled rigidly to a rigid shaking table, and that the cylinder of the actuator is attached rigidly to a rigid foundation. In practice, the couplings, shaking table, and foundation are not rigid. When the flexibilities associated with these components are introduced into the block diagram of Fig. 3.7, the analytical model shown in Fig. 3.12 is obtained. In the model shown in Fig. 3.12, the subscript g refers to the foundation or ground, the subscript c refers to the cylinder and its coupling to the foundation, the subscript p refers to the piston, the subscript t refers to the table and its coupling to the piston, and the subscript s refers to the structure. The coupling between the piston and shaking table includes the flexibility of the shaking table. The primary feedback, for both the 2,000 lb (900 kg) and 100,000 lb (45,300 kg) shaking tables, is the displacement between the piston and cylinder of the actuator.

Since the primary function of the shaking table is to reproduce an absolute table motion, the transfer function of interest is that relating absolute table displacement to input, which is

$$\frac{\mathbf{x}_{t}}{\mathbf{x}_{i}} = \frac{K}{\left(\frac{\mathbf{v} \mathbf{s}^{3}}{4\beta \mathbf{A}} + K \mathbf{k}_{ff} \mathbf{s}^{2}\right) \mathbf{m}_{t} \left(1 + \frac{\mathbf{m}}{\mathbf{m}_{t}} \frac{\mathbf{x}}{\mathbf{x}_{t}} + \frac{\mathbf{m}}{\mathbf{m}_{t}} \frac{\mathbf{x}}{\mathbf{x}_{t}}\right) + \left(\mathbf{A} \mathbf{s} + K\right) \left(\frac{\mathbf{x}_{p} - \mathbf{x}_{c}}{\mathbf{x}_{t}}\right)}$$
(4.1)

This transfer function is of the same basic form as the transfer

function for the simplified analytical model used previously, with an effective table mass given by

$$m_{e} = m_{t} \left( 1 + \frac{p}{m_{t}} \frac{p}{x_{t}} + \frac{m}{m_{t}} \frac{x}{x_{t}} \right)$$

$$(4.2)$$

and with the last two terms of the numerator (A S + K) multiplied by  $(x_p-x_c)/x_t$ .

In the case of the 100,000 lb (45,300 kg) table, the coupling between the piston and shaking table and the shaking table itself are sufficiently rigid that the transmissibility function  $x_p/x_t$  is close to unity within the operating frequency of the table. Then, since  $m_p/m_t$  is very small, the effective mass reduces to

$$m_{e} = m_{t} \left( 1 + \frac{s}{m_{t}} \frac{s}{x_{t}} \right)$$
(4.3)

which is the same as that discussed previously for the 2,000 lb table in section 3.2. Thus the 100,000 lb (45,300 kg) table when loaded with a resonant structure will also exhibit a peak and notch in its frequency response function.

Since the transmissibility function  $x_p/x_t$  is approximately unity, the multiplier of the last two terms of the numerator (A S + K) in equation (4.1) becomes  $(1 - x_c/x_t)$ . Also, since the cylinders of the actuators in the 100,000 lb (45,300 kg) table are prestressed onto the foundation, flexibility in these couplings is negligible so that  $x_c = x_g$ . Thus the multiplier becomes  $(1 - x_g/x_t)$  and the transmissibility function  $x_g/x_t$  is shown for vertical motion of the 100,000 lb (45,300 kg) table in Fig. 2.9(a). The transmissibility function  $x_g/x_t$  ranges from zero to a maximum value of 0.06, and thus the multiplier  $(1 - x_g/x_t)$  can only range between 1 and 0.94. Thus foundation flexibility in the 100,000 lb (45,300 kg) shaking table can reduce the magnitude of the terms (A S + K) in the numerator of equation 4.1, by up to 6%. However, since these terms are significant only at low frequencies, the effects of foundation flexibility on the performance of the 100,000 lb (45,300 kg) table are negligible.

# 5. CONCLUSION

Mathematical models have been formulated that describe adequately the small amplitude dynamic behavior of electro-hydraulic shaking tables. The accuracy of the models has been confirmed by comparing the computed frequency response functions for the models with experimental frequency responses for a 2,000 lb (900 kg) and a 100,000 lb (45,300 kg) shaking table. The models simulated the corner frequency characteristic and the peak-and-notch distortion caused by a resonant structure on the shaking table.

Force feedback from the actuator was found to have a significant influence on the frequency response characteristics of shaking tables. Once the force feedback reaches an adequate level it effectively prevents resonance at the oil-column resonant frequency. However, force feedback also reduces the corner frequency and leads to the peak-andnotch distortion in the frequency response.

In addition to the amount of force feedback, the magnitudes of the peak and of the notch are sensitive to the ratio of the mass of the structure to the mass of the shaking table and to the transmissibility function of the structure. A shaking table loaded with a resonant structure appears to the control system as a mass whose magnitude varies with frequency. The variation has the form of the transmissibility function of the structure which has a maximum amplitude and change of phase at the resonant frequency. The transmissibility function increases the effective mass just below the resonant frequency of the structure producing the peak in the frequency response. Because of the phase change the transmissibility function reduces the effective mass just above the resonant frequency response.

The peak and notch distortion in the frequency response results in difficulties in determining the frequency response of structures by means of shaking tables. Corrections for this distortion need to be made either during the experimental work or later in the analysis of the data. However, it was found that the peak and notch distortion had little effect on the accuracy to which a shaking table could reproduce earthquake type motions.

The effect of foundation compliance on the frequency response characteristics of shaking tables was also examined. It was found that foundation compliance can only affect the frequency response at low frequencies, and the magnitude of the effect is limited to an amount which depends on the transmissibility function of the foundation with respect to the table.

### REFERENCES

- 1. Rea, D. and J. Penzien, "Structural Research Using an Earthquake Simulator," Proceedings of the Structural Engineers Association of California Conference, Monterey, California, October 1972.
- Takahashi, Y., D. Rea, and S. Abedi-Hayati, "Effects of Test Specimen Reaction Loads on Shaking Tables," Proceedings of the Fifth World Conference on Earthquake Engineering, Rome, Italy, June 1973.
- Rea, D. and J. Penzien, "Dynamic Response of a 20 ft x 20 ft Shaking Table," Proceedings of the Fifth World Conference on Earthquake Engineering, Rome, Italy, June 1973.
- 4. Rea, Dixon, "Design and Construction of a 20 ft x 20 ft Prestressed Concrete Shaking Table," Proceedings of the FIP Symposium on Prestressed Concrete in Seismic Structures, Tbilisi, Georgia, USSR, September 1972.
- 5. Larson, R. L., Programming and Control of Large Vibration Tables in Uniaxial and Biaxial Motions," 42nd Symposium on Shock and Vibration, U.S. Naval Station, Key West, Florida, November 1971.
- 6. Rea, D., "The Berkeley Shaking Table," Proceedings of a Workshop on Simulation of Earthquake Effects on Structures published by the National Academy of Engineering, Washington, D.C., 1974.
- Merritt, H. E., "Hydraulic Control Systems," J. Wiley, New York, 1967.



FIG. 1.1 LIMITATIONS ON THE DYNAMIC PERFORMANCE OF A PARTICULAR SHAKING TABLE

\_ \_ \_ -













FIG. 2.4 STRUCTURE AND TABLE FREQUENCY RESPONSES OBTAINED SIMULTANEOUSLY



FIG. 2.5 FREQUENCY RESPONSES OF 2000 1b TABLE WITH RESONANT STRUCTURE AND VARYING AMOUNTS OF FORCE FEEDBACK



(a) GENERAL VIEW



- (b) above, A HORIZONTAL ACTUATOR
  - (c) right, A VERTICAL ACTUATOR





FIG. 2.7 ACTUATOR LOCATIONS



FIG. 2.8 TRANSLATIONAL FREQUENCY RESPONSES



FIG. 2.9 FOUNDATION TRANSMISSIBILITIES





FIG. 3.2 IDEALIZED SERVOVALVE-ACTUATOR SYSTEM

40

......







FIG. 3.4 SHAKING TABLE CLOSED LOOP CONTROL SYSTEM



FIG. 3.5 SIMPLIFIED CONTROL SYSTEM



(a) Effects of varying loop gain



FIG. 3.6 COMPUTED FREQUENCY RESPONSES: SHAKING TABLE ONLY



FIG. 3.7 SHAKING TABLE WITH STRUCTURE



FIG. 3.8 COMPUTED FREQUENCY RESPONSES: SHAKING TABLE PLUS STRUCTURE



FIG. 3.9 COMPUTED FREQUENCY RESPONSES FOR DIFFERENT EFFECTIVE TABLE MASSES



El Centro and simulated El Centro motion

FIG. 3.10 CHECKS ON FIDELITY OF EARTHQUAKE SIMULATION





FIG. 3.11 COMPUTED VERTICAL FREQUENCY RESPONSE FOR 100,000 1b TABLE



FIG. 3.12 SHAKING TABLE WITH FLEXIBLY COUPLED STRUCTURE AND FOUNDATION

# EARTHQUAKE ENGINEERING RESEARCH CENTER REPORTS

#### EARTHQUAKE ENGINEERING RESEARCH CENTER REPORTS

NOTE: Numbers in parenthesis are Accession Numbers assigned by the National Technical Information Service; these are followed by a price code. Copies of the reports may be ordered from the National Technical Information Service, 5285 Port Royal Road, Springfield, Virginia, 22161. Accession Numbers should be quoted on orders for reports (PB -----) and remittance must accompany each order. Reports without this information were not available at time of printing. Upon request, EERC will mail inquirers this information when it becomes available.

- EERC 67-1 "Feasibility Study Large-Scale Earthquake Simulator Facility," by J. Penzien, J.G. Bouwkamp, R.W. Clough and D. Rea - 1967 (PB 187 905)A07
- EERC 68-1 Unassigned
- EERC 68-2 "Inelastic Behavior of Beam-to-Column Subassemblages Under Repeated Loading," by V.V. Bertero 1968 (PB 184 888)A05
- EERC 68-3 "A Graphical Method for Solving the Wave Reflection-Refraction Problem," by H.D. McNiven and Y. Mengi-1968 (PB 187 943)A03
- EERC 68-4 "Dynamic Properties of McKinley School Buildings," by D. Rea, J.G. Bouwkamp and R.W. Clough 1968 (PB 187 902)A07
- EERC 68-5 "Characteristics of Rock Motions During Earthquakes," by H.B. Seed, I.M. Idriss and F.W. Kiefer 1968 (PB 188 338)A03
- EERC 69-1 "Earthquake Engineering Research at Berkeley," 1969 (PB 187 906)All
- EERC 69-2 "Nonlinear Seismic Response of Earth Structures," by M. Dibaj and J. Penzien 1969 (PB 187 904)A08
- EERC 69-3 "Probabilistic Study of the Behavior of Structures During Earthquakes," by R. Ruiz and J. Penzien 1969 (PB 187 886)A06
- EERC 69-4 "Numerical Solution of Boundary Value Problems in Structural Mechanics by Reduction to an Initial Value Formulation," by N. Distefano and J. Schujman - 1969 (PB 187 942)A02
- EERC 69-5 "Dynamic Programming and the Solution of the Biharmonic Equation," by N. Distefano 1969 (PB 187 941)A03
- EERC 69-6 "Stochastic Analysis of Offshore Tower Structures," by A.K. Malhotra and J. Penzien 1969 (PB 187 903) A06
- EERC 69-7 "Rock Motion Accelerograms for High Magnitude Earthquakes," by H.B. Seed and I.M. Idriss 1969 (PB 187 940) A02
- EERC 69-8 "Structural Dynamics Testing Facilities at the University of California, Berkeley," by R.M. Stephen. J.G. Bouwkamp, R.W. Clough and J. Penzien - 1969 (PB 189 111)A04
- EERC 69-9 "Seismic Response of Soil Deposits Underlain by Sloping Rock Boundaries," by H. Dezfulian and H.B. Seed 1969 (PB 189 114)A03
- EERC 69-10 "Dynamic Stress Analysis of Axisymmetric Structures Under Arbitrary Loading," by S. Ghosh and E.L. Wilson 1969 (PB 189 026)Al0
- EERC 69-11 "Seismic Behavior of Multistory Frames Designed by Different Philosophies," by J.C. Anderson and V. V. Bertero 1969 (PB 190 662)A10
- EERC 69-12 "Stiffness Degradation of Reinforcing Concrete Members Subjected to Cyclic Flexural Moments," by V.V. Bertero, B. Bresler and H. Ming Liao 1969 (PB 202 942)A07
- EERC 69-13 "Response of Non-Uniform Soil Deposits to Travelling Seismic Waves," by H. Dezfulian and H.B. Seed 1969 (PB 191 023)A03
- EERC 69-14 "Damping Capacity of a Model Steel Structure," by D. Rea, R.W. Clough and J.G. Bouwkamp 1969 (PB 190 663) A06
- EERC 69-15 "Influence of Local Soil Conditions on Building Damage Potential during Earthquakes," by H.B. Seed and I.M. Idriss 1969 (PB 191 036)A03
- EERC 69-16 "The Behavior of Sands Under Seismic Loading Conditions," by M.L. Silver and H.B. Seed 1969 (AD 714 982) A07
- EERC 70-1 "Earthquake Response of Gravity Dams," by A.K. Chopra-1970 (AD 709 640)A03
- EERC 70-2 "Relationships between Soil Conditions and Building Damage in the Caracas Earthquake of July 29, 1967." by H.B. Seed, I.M. Idriss and H. Dezfulian - 1970 (PB 195 762)A05
- EERC 70-3 "Cyclic Loading of Full Size Steel Connections," by E.P. Popov and R.M. Stephen 1970 (PB 213 545)A04
- EERC 70-4 "Seismic Analysis of the Charaima Building, Caraballeda, Venezuela," by Subcommittee of the SEAONC Research Committee: V.V. Bertero, P.F. Fratessa, S.A. Mahin, J.H. Sexton, A.C. Scordelis, E.L. Wilson, L.A. Wyllie, H.B. Seed and J. Penzien, Chairman - 1970 (PB 201 455)A06



- EERC 70-5 "A Computer Program for Earthquake Analysis of Dams," by A.K. Chopra and P. Chakrabarti 1970 (AD 723 994) A05
- EERC 70-6 "The Propagation of Love Waves Across Non-Horizontally Layered Structures," by J. Lysmer and L.A. Drake 1970 (PB 197 896)A03
- EERC 70-7 "Influence of Base Rock Characteristics on Ground Response," by J. Lysmer, H.B. Seed and P.B. Schnabel 1970 (PB 197 897)A03
- EERC 70-8 "Applicability of Laboratory Test Procedures for Measuring Soil Liquefaction Characteristics under Cyclic Loading," by H.B. Seed and W.H. Peacock - 1970 (PB 198 016)A03
- EERC 70-9 "A Simplified Procedure for Evaluating Soil Liquefaction Potential," by H.B. Seed and I.M. Idriss 1970 (PB 198 009)A03
- EERC 70-10 "Soil Moduli and Damping Factors for Dynamic Response Analysis," by H.B. Seed and I.M. Idriss ~ 1970 (PB 197 869)A03
- EEEC 71-1 "Koyna Earthquake of December 11, 1967 and the Performance of Koyna Dam," by A.K. Chopra and P. Chakrabarti 1971 (AD 731 496)A06
- EERC 71-2 "Preliminary In-Situ Measurements of Anelastic Absorption in Soils Using a Prototype Earthquake Simulator," by R.D. Borcherdt and P.W. Rodgers - 1971 (PB 201 454)A03
- EERC 71-3 "Static and Dynamic Analysis of Inelastic Frame Structures," by F.L. Porter and G.H. Powell 1971 (PB 210 135)A06
- EERC 71-4 "Research Needs in Limit Design of Reinforced Concrete Structures," by V.V. Bertero 1971 (PB 202 943)A04
- EERC 71-5 "Dynamic Behavior of a High-Rise Diagonally Braced Steel Building," by D. Rea, A.A. Shah and J.G. Bouwhapp 1971 (PB 203 584)A06
- EERC 71-6 "Dynamic Stress Analysis of Porous Elastic Solids Saturated with Compressible Fluids," by J. Ghaboussi and E. L. Wilson - 1971 (PB 211 396)A06
- EERC 71-7 "Inelastic Behavior of Steel Beam-to-Column Subassemblages," by H. Krawinkler, V.V. Bertero and E.P. Popov 1971 (PB 211 335)A14
- EERC 71-8 "Modification of Seismograph Records for Effects of Local Soil Conditions," by P. Schnabel, H.B. Seed and J. Lysmer - 1971 (PB 214 450)A03
- EERC 72-1 "Static and Earthquake Analysis of Three Dimensional Frame and Shear Wall Buildings," by E.L. Wilson and H.H. Dovey - 1972 (PB 212 904)A05
- EERC 72-2 "Accelerations in Rock for Earthquakes in the Western United States," by P.B. Schnabel and H.B. Seed 1972 (PB 213 100)A03
- EERC 72-3 "Elastic-Plastic Earthquake Response of Soil-Building Systems," by T. Minami 1972 (PB 214 868)A08
- EERC 72-4 "Stochastic Inelastic Response of Offshore Towers to Strong Motion Earthquakes," by M.K. Kaul-1972 (PB 215 713)A05
- EERC 72-5 "Cyclic Behavior of Three Reinforced Concrete Flexural Members with High Shear," by E.P. Popov, V.V. Bertero and H. Krawinkler - 1972 (PB 214 555)A05
- EERC 72-6 "Earthquake Response of Gravity Dams Including Reservoir Interaction Effects," by P. Chakrabarti and A.K. Chopra 1972 (AD 762 330)A08
- EERC 72-7 "Dynamic Properties of Pine Flat Dam," by D. Rea, C.Y. Liaw and A.K. Chopra-1972 (AD 763 928)A05
- EERC 72-8 "Three Dimensional Analysis of Building Systems," by E.L. Wilson and H.H. Dovey 1972 (PB 222 438) A06
- EERC 72-9 "Rate of Loading Effects on Uncracked and Repaired Reinforced Concrete Members," by S. Mahin, V.V. Bertero, D. Rea and M. Atalay - 1972 (PB 224 520)A08
- EERC 72-10 "Computer Program for Static and Dynamic Analysis of Linear Structural Systems," by E.L. Wilson, K.-J. Bathe, J.E. Peterson and H.H.Dovey 1972 (PB 220 437)A04
- EERC 72-11 "Literature Survey Seismic Effects on Highway Bridges," by T. Iwasaki, J. Penzien and R.W. Clough 1972 (PB 215 613)A19
- EERC 72-12 "SHAKE-A Computer Program for Earthquake Response Analysis of Horizontally Layered Sites," by P.B. Schnabel and J. Lysmer - 1972 (PB 220 207)A06
- EERC 73-1 "Optimal Seismic Design of Multistory Frames," by V.V. Bertero and H. Kamil 1973
- EERC 73-2 "Analysis of the Slides in the San Fernando Dams During the Earthquake of February 9, 1971," by H.B. Seed, K.L. Lee, I.M. Idriss and F. Makdisi ~ 1973 (PB 223 402)A14

- EERC 73-3 "Computer Aided Ultimate Load Design of Unbraced Multistory Steel Frames," by M.B. El-Hafez and G.H. Powell 1973 (PB 248 315)A09
- EERC 73-4 "Experimental Investigation into the Seismic Behavior of Critical Regions of Reinforced Concrete Components as Influenced by Moment and Shear," by M. Celebi and J. Penzien - 1973 (PB 215 884)A09
- EERC 73-5 "Hysteretic Behavior of Epoxy-Repaired Reinforced Concrete Beams," by M. Celebi and J. Penzien 1973 (PB 239 568)A03
- EERC 73-6 "General Purpose Computer Program for Inelastic Dynamic Response of Plane Structures," by A. Kanaan and G.H. Powell-1973 (PB 221 260)A08
- EERC 73-7 "A Computer Program for Earthquake Analysis of Gravity Dams Including Reservoir Interaction," by P. Chakrabarti and A.K. Chopra-1973 (AD 766 271)A04
- EERC 73-8 "Behavior of Reinforced Concrete Deep Beam-Column Subassemblages Under Cyclic Loads," by O. Küstü and J.G. Bouwkamp 1973 (PB 246 117)Al2
- EERC 73-9 "Earthquake Analysis of Structure-Foundation Systems," by A.K. Vaish and A.K. Chopra 1973 (AD 766 272)A07
- EERC 73-10 "Deconvolution of Seismic Response for Linear Systems," by R.B. Reimer 1973 (PB 227 179)A08
- EERC 73-11 "SAP IV: A Structural Analysis Program for Static and Dynamic Response of Linear Systems," by K.-J. Bathe, E.L. Wilson and F.E. Peterson - 1973 (PB 221 967)A09
- EERC 73-12 "Analytical Investigations of the Seismic Response of Long, Multiple Span Highway Bridges," by W.S. Tseng and J. Penzien - 1973 (PB 227 816)Al0
- EERC 73-13 "Earthquake Analysis of Multi-Story Buildings Including Foundation Interaction," by A.K. Chopra and J.A. Gutierrez 1973 (PB 222 970)A03
- EERC 73-14 "ADAP: A Computer Program for Static and Dynamic Analysis of Arch Dams," by R.W. Clough, J.M. Raphael and S. Mojtahedi ~ 1973 (PB 223 763)A09
- EERC 73-15 "Cyclic Plastic Analysis of Structural Steel Joints," by R.B. Pinkney and R.W. Clough 1973 (PB 226 843)A08
- EERC 73-16 "QUAD-4: A Computer Program for Evaluating the Seismic Response of Soil Structures by Variable Damping Finite Element Procedures," by I.M. Idriss, J. Lysmer, R. Hwang and H.B. Seed - 1973 (PB 229 424)A05
- EERC 73-17 "Dynamic Behavior of a Multi-Story Pyramid Shaped Building," by R.M. Stephen, J.P. Hollings and J.G. Bouwkamp 1973 (PB 240 718)A06
- EERC 73-18 "Effect of Different Types of Reinforcing on Seismic Behavior of Short Concrete Columns," by V.V. Bertero, J. Hollings, O. Küstü, R.M. Stephen and J.G. Bouwkamp - 1973
- EERC 73-19 "Olive View Medical Center Materials Studies, Phase I," by B. Bresler and V.V. Bertero 1973 (PR 235 986)A06
- EERC 73-20 "Linear and Nonlinear Seismic Analysis Computer Programs for Long Multiple-Span Highway Bridges," by W.S. Tseng and J. Penzien 1973
- EERC 73-21 "Constitutive Models for Cyclic Plastic Deformation of Engineering Materials," by J.M. Kelly and P.P. Gillis 1973 (PB 226 024)A03
- EERC 73-22 "DRAIN 2D User's Guide," by G.H. Powell 1973 (PB 227 016)A05
- EERC 73-23 "Earthquake Engineering at Berkeley 1973," (PB 226 033)All
- EERC 73-24 Unassigned
- EERC 73-25 "Earthquake Response of Axisymmetric Tower Structures Surrounded by Water," by C.Y. Liaw and A.K. Chopra 1973 (AD 773 052)A09
- EERC 73-26 "Investigation of the Failures of the Olive View Stairtowers During the San Fernando Earthquake and Their Implications on Seismic Design," by V.V. Bertero and R.G. Collins - 1973 (PB 235 106)A13
- EERC 73-27 "Further Studies on Seismic Behavior of Steel Beam-Column Subassemblages," by V.V. Bertero, H. Krawinkler and E.P. Popov - 1973 (PB 234 172)A06
- EERC 74-1 "Seismic Risk Analysis," by C.S. Oliveira 1974 (PB 235 920)A06
- EERC 74-2 "Settlement and Liquefaction of Sands Under Multi-Directional Shaking," by R. Pyke, C.K. Chan and H.B. Seed 1974
- EERC 74-3 "Optimum Design of Earthquake Resistant Shear Buildings," by D. Ray, K.S. Pister and A.K. Chopra 1974 (PB 231 172)A06
- EERC 74-4 "LUSH A Computer Program for Complex Response Analysis of Soil-Structure Systems," by J. Lysmer, T. Udaka, H.B. Seed and R. Hwang-1974 (PB 236 796)A05

- 54
- EERC 74-5 "Sensitivity Analysis for Hysteretic Dynamic Systems: Applications to Earthquake Engineering," by D. Ray 1974 (PB 233 213)A06
- EERC 74-6 "Soil Structure Interaction Analyses for Evaluating Seismic Response," by H.B. Seed, J. Lysmer and R. Hwang 1974 (PB 236 519)A04
- EERC 74-7 Unassigned
- EERC 74-8 "Shaking Table Tests of a Steel Frame A Progress Report," by R.W. Clough and D. Tang-1974 (PB 240 969)A02
- EERC 74-9 "Hysteretic Behavior of Reinforced Concrete Flexural Members with Special Web Reinforcement," by V.V. Bertero, E.P. Popov and T.Y. Wang 1974 (PB 236 797)A07
- EURC 74-10 "Applications of Reliability-Based, Global Cost Optimization to Design of Earthquake Resistant Structures," by E. Vitiello and K.S. Pister - 1974 (PB 237 231)A06
- EERC 74-11 "Liquefaction of Gravelly Soils Under Cyclic Loading Conditions," by R.T. Wong, H.B. Seed and C.K. Chan 1974 (PB 242 042)A03
- EERC 74-12 "Site-Dependent Spectra for Earthquake-Resistant Design," by H.B. Seed, C. Ugas and J. Lysmer 1974 (PB 240 953)A03
- EERC 74-13 "Earthquake Simulator Study of a Reinforced Concrete Frame," by P. Hidalgo and R.W. Clough 1974 (PB 241 94)A13
- EERC 74-14 "Nonlinear Earthquake Response of Concrete Gravity Dams," by N. Pal 1974 (AD/A 006 583)A06
- EERC 74-15 "Modeling and Identification in Nonlinear Structural Dynamics I. One Degree of Freedom Models," by N. Distefano and A. Rath - 1974 (PB 241 548)A06
- EERC 75-1 "Determination of Seismic Design Criteria for the Dumbarton Bridge Replacement Structure,Vol.I: Description, Theory and Analytical Modeling of Bridge and Parameters," by F. Baron and S.-H. Pang - 1975 (PB 259 407)A15
- EERC 75-2 "Determination of Seismic Design Criteria for the Dumbarton Bridge Replacement Structure, Vol. II: Numerical Studies and Establishment of Seismic Design Criteria," by F. Baron and S.-H. Pang - 1975 (PB 259 408)All (For set of EERC 75-1 and 75-2 (PB 259 406))
- EERC 75-3 "Seismic Risk Analysis for a Site and a Metropolitan Area," by C.S. Oliveira 1975 (PB 248 134)A09
- EERC 75-4 "Analytical Investigations of Seismic Response of Short, Single or Multiple-Span Highway Bridges," by M.-C. Chen and J. Penzien - 1975 (PB 241 454)A09
- EERC 75-5 "An Evaluation of Some Methods for Predicting Seismic Behavior of Reinforced Concrete Buildings," by S.A. Mahin and V.V. Bertero - 1975 (PB 246 306)Al6
- EERC 75-6 "Earthquake Simulator Study of a Steel Frame Structure, Vol. I: Experimental Results," by R.W. Clough and D.T. Tang-1975 (PB 243 981)A13
- EERC 75-7 "Dynamic Properties of San Bernardino Intake Tower," by D. Rea, C.-Y. Liaw and A.K. Chopra 1975 (AD/A008 406) A05
- EERC 75-8 "Seismic Studies of the Articulation for the Dumbarton Bridge Replacement Structure, Vol. I: Description, Theory and Analytical Modeling of Bridge Components," by F. Baron and R.E. Hamati - 1975 (PB 251 539)A07
- EERC 75-9 "Seismic Studies of the Articulation for the Dumbarton Bridge Replacement Structure, Vol. 2: Numerical Studies of Steel and Concrete Girder Alternates," by F. Baron and R.E. Hamati - 1975 (PB 251 540)Al0
- EERC 75-10 "Static and Dynamic Analysis of Nonlinear Structures," by D.P. Mondkar and G.H. Powell 1975 (PB 242 434)A08
- EERC 75-11 "Hysteretic Behavior of Steel Columns," by E.P. Popov, V.V. Bertero and S. Chandramouli 1975 (PB 252 365)All
- EERC 75-12 "Earthquake Engineering Research Center Library Printed Catalog," 1975 (PB 243 711)A26
- EERC 75-13 "Three Dimensional Analysis of Building Systems (Extended Version)," by E.L. Wilson, J.P. Hollings and H.H. Dovey - 1975 (PB 243 989)A07
- EERC 75-14 "Determination of Soil Liquefaction Characteristics by Large-Scale Laboratory Tests," by P. De Alba, C.K. Chan and H.B. Seed - 1975 (NUREG 0027) A08
- EERC 75-15 "A Literature Survey Compressive, Tensile, Bond and Shear Strength of Masonry," by R.L. Mayes and R.W. Clough - 1975 (PB 246 292)Al0
- EERC 75-16 "Hysteretic Behavior of Ductile Moment Resisting Reinforced Concrete Frame Components," by V.V. Bertero and E.P. Popov - 1975 (PB 246 388)A05
- EERC 75-17 "Relationships Between Maximum Acceleration, Maximum Velocity, Distance from Source, Local Site Conditions for Moderately Strong Earthquakes," by H.B. Seed, R. Murarka, J. Lysmer and I.M. Idriss - 1975 (PB 248 172)A03
- EERC 75-18 "The Effects of Method of Sample Preparation on the Cyclic Stress-Strain Behavior of Sands," by J. Mulilis, C.K. Chan and H.B. Seed - 1975 (Summarized in EERC 75-28)

- EERC 75-19 "The Seismic Behavior of Critical Regions of Reinforced Concrete Components as Influenced by Moment, Shear and Axial Force," by M.B. Atalay and J. Penzien - 1975 (PB 258 842)All
- EERC 75-20 "Dynamic Properties of an Eleven Story Masonry Building," by R.M. Stephen, J.P. Hollings, J.G. Bouwkamp and D. Jurukovski 1975 (PB 246 945)A04
- EERC 75-21 "State-of-the-Art in Seismic Strength of Masonry An Evaluation and Review," by R.L. Mayes and R.W. Clough 1975 (PB 249 040)A07
- EERC 75-22 "Frequency Dependent Stiffness Matrices for Viscoelastic Half-Plane Foundations," by A.K. Chopra, P. Chakrabarti and G. Dasgupta 1975 (PB 248 121)A07
- EERC 75-23 "Hysteretic Behavior of Reinforced Concrete Framed Walls," by T.Y. Wong, V.V. Bertero and E.P. Popov 1975
- EERC 75-24 "Testing Facility for Subassemblages of Frame-Wall Structural Systems," by V.V. Bertero, E.P. Popov and T. Endo - 1975
- EERC 75-25 "Influence of Seismic History on the Liquefaction Characteristics of Sands," by H.B. Seed, K. Mori and C.K. Chan 1975 (Summarized in EERC 75-28)
- EERC 75-26 "The Generation and Dissipation of Pore Water Pressures during Soil Liquefaction," by H.B. Seed, P.P. Martin and J. Lysmer - 1975 (PB 252 648)A03
- EERC 75-27 "Identification of Research Needs for Improving Aseismic Design of Building Structures," by V.V. Bertero 1975 (PB 248 136)A05
- EERC 75-28 "Evaluation of Soil Liquefaction Potential during Earthquakes," by H.B. Seed, I. Arango and C.K. Chan 1975 (NUREG 0026)A13
- EERC 75-29 "Representation of Irregular Stress Time Histories by Equivalent Uniform Stress Series in Liquefaction Analyses," by H.B. Seed, I.M. Idriss, F. Makdisi and N. Banerjee - 1975 (PB 252 635)A03
- EERC 75-30 "FLUSH A Computer Program for Approximate 3-D Analysis of Soil-Structure Interaction Problems," by J. Lysmer, T. Udaka, C.-F. Tsai and H.B. Seed 1975 (PB 259 332)A07
- EERC 75-31 "ALUSH A Computer Program for Seismic Response Analysis of Axisymmetric Soil-Structure Systems," by E. Berger, J. Lysmer and H.B. Seed 1975
- EERC 75-32 "TRIP and TRAVEL Computer Programs for Soil-Structure Interaction Analysis with Horizontally Travelling Waves," by T. Udaka, J. Lysmer and H.B. Seed - 1975
- EERC 75-33 "Predicting the Performance of Structures in Regions of High Seismicity," by J. Penzien 1975 (PB 248 130)A03
- EERC 75-34 "Efficient Finite Element Analysis of Seismic Structure Soil Direction," by J. Lysmer, H.B. Seed, T. Udaka, R.N. Hwang and C.-F. Tsai 1975 (PB 253 570)A03
- EERC 75-35 "The Dynamic Behavior of a First Story Girder of a Three-Story Steel Frame Subjected to Earthquake Loading," by R.W. Clough and L.-Y. Li - 1975 (PB 248 841)A05
- EERC 75-36 "Earthquake Simulator Study of a Steel Frame Structure, Volume II Analytical Results," by D.T. Tang 1975 (PB 252 926)Al0
- EERC 75-37 "ANSR-I General Purpose Computer Program for Analysis of Non-Linear Structural Response," by D.P. Mondkar and G.H. Powell - 1975 (PB 252 386)A08
- EERC 75-38 "Nonlinear Response Spectra for Probabilistic Seismic Design and Damage Assessment of Reinforced Concrete Structures," by M. Murakami and J. Penzien - 1975 (PB 259 530)A05
- EERC 75-39 "Study of a Method of Feasible Directions for Optimal Elastic Design of Frame Structures Subjected to Earthquake Loading," by N.D. Walker and K.S. Pister - 1975 (PB 257 781)A06
- EERC 75-40 "An Alternative Representation of the Elastic-Viscoelastic Analogy," by G. Dasgupta and J.L. Sackman 1975 (PB 252 173)A03
- EERC 75-41 "Effect of Multi-Directional Shaking on Liquefaction of Sands," by H.B. Seed, R. Pyke and G.R. Martin 1975 (PB 258 781)A03
- EERC 76-1 "Strength and Ductility Evaluation of Existing Low-Rise Reinforced Concrete Buildings Screening Method," by T. Okada and B. Bresler - 1976 (PB 257 906)All
- EERC 76-2 "Experimental and Analytical Studies on the Hysteretic Behavior of Reinforced Concrete Rectangular and T-Beams," by S.-Y.M. Ma, E.P. Popov and V.V. Bertero 1976 (PB 260 843)Al2
- EERC 76-3 "Dynamic Behavior of a Multistory Triangular-Shaped Building," by J. Petrovski, R.M. Stephen, E. Gartenbaum and J.G. Bouwkamp - 1976
- EERC 76-4 "Earthquake Induced Deformations of Earth Dams," by N. Serff and H.B. Seed 1976

- 56
- EERC 76-5 "Analysis and Design of Tube-Type Tall Building Structures," by H. de Clercq and G.H. Powell 1976 (PB 252 220) Alo
- EERC 76-6 "Time and Frequency Domain Analysis of Three-Dimensional Ground Motions, San Fernando Earthquake," by T. Kubo and J. Penzien (PB 260 556)All
- EERC 76-7 "Expected Performance of Uniform Building Code Design Masonry Structures," by R.L. Mayes, Y. Omote, S.W. Chen and R.W. Clough - 1976
- FERC 76-8 "Cyclic Shear Tests on Concrete Masonry Piers," Part I Test Results," by R.L. Mayes, Y. Omote and R.W. Clough - 1976 (PB 264 424)A06
- EERC 76-9 "A Substructure Method for Earthquake Analysis of Structure Soil Interaction," by J.A. Gutierrez and A.K. Chopra 1976 (PB 257 783)A08
- EERC 76-10 "Stabilization of Potentially Liquefiable Sand Deposits using Gravel Drain Systems," by H.B. Seed and J.R. Booker-1976 (PB 258 820)A04
- EERC 76-11 "Influence of Design and Analysis Assumptions on Computed Inelastic Response of Moderately Tall Frames," by G.H. Powell and D.G. Row 1976
- EERC 76-12 "Sensitivity Analysis for Hysteretic Dynamic Systems: Theory and Applications," by D. Ray, K.S. Pister and E. Polak 1976 (PB 262 859)A04
- EERC 76-13 "Coupled Lateral Torsional Response of Buildings to Ground Shaking," by C.L. Kan and A.K. Chopra 1976 (PB 257 907)A09
- EERC 76-14 "Seismic Analyses of the Banco de America," by V.V. Bertero, S.A. Mahin and J.A. Hollings 1976
- EERC 76-15 "Reinforced Concrete Frame 2: Seismic Testing and Analytical Correlation," by R.W. Clough and J. Gidwani 1976 (PB 261 323)A08
- EERC 76-16 "Cyclic Shear Tests on Masonry Piers, Part II Analysis of Test Results," by R.L. Mayes, Y. Omote and R.W. Clough - 1976
- EERC 76-17 "Structural Steel Bracing Systems: Behavior Under Cyclic Loading," by E.P. Popov, K. Takanashi and C.W. Roeder 1976 (PB 260 715)A05
- EERC 76-18 "Experimental Model Studies on Seismic Response of High Curved Overcrossings," by D. Williams and W.G. Godden 1976
- EERC 76-19 "Effects of Non-Uniform Seismic Disturbances on the Dumbarton Bridge Replacement Structure," by F. Baron and R.E. Hamati 1976
- EERC 76-20 "Investigation of the Inelastic Characteristics of a Single Story Steel Structure Using System Identification and Shaking Table Experiments," by V.C. Matzen and H.D. McNiven - 1976 (PB 258 453)A07
- EERC 76-21 "Capacity of Columns with Splice Imperfections," by E.P. Popov, R.M. Stephen and R. Philbrick 1976 (PB 260 378)A04
- EERC 76-22 "Response of the Olive View Hospital Main Building during the San Fernando Earthquake," by S. A. Mahin, R. Collins, A.K. Chopra and V.V. Bertero - 1976
- EERC 76-23 "A Study on the Major Factors Influencing the Strength of Masonry Prisms," by N.M. Mostaghel, R.L. Mayes, R. W. Clough and S.W. Chen 1976
- EERC 76-24 "GADFLEA A Computer Program for the Analysis of Pore Pressure Generation and Dissipation during Cyclic or Earthquake Loading," by J.R. Booker, M.S. Rahman and H.B. Seed - 1976 (PB 263 947)A04
- EERC 76-25 "Rehabilitation of an Existing Building: A Case Study," by B. Bresler and J. Axley 1976
- EERC 76-26 "Correlative Investigations on Theoretical and Experimental Dynamic Behavior of a Model Bridge Structure," by K. Kawashima and J. Penzien 1976 (PB 263 388)All
- EERC 76-27 "Earthquake Response of Coupled Shear Wall Buildings," by T. Srichatrapimuk 1976 (PB 265 157)A07
- EERC 76-28 "Tensile Capacity of Partial Penetration Welds," by E.P. Popov and R.M. Stephen 1976 (PB 262 899)A03
- EERC 76-29 "Analysis and Design of Numerical Integration Methods in Structural Dynamics," by H.M. Hilber 1976 (PB 264 410)A06
- EERC 76-30 "Contribution of a Floor System to the Dynamic Characteristics of Reinforced Concrete Buildings," by L.J. Edgar and V.V. Bertero 1976
- EERC 76-31 "The Effects of Seismic Disturbances on the Golden Gate Bridge," by F. Baron, M. Arikan and R.E. Hamati 1976
- EERC 76-32 "Infilled Frames in Earthquake Resistant Construction," by R.E. Klingner and V.V. Bertero 1976 (PB 265 892)Al3
- UCB/EERC-77/01 "PLUSH A Computer Program for Probabilistic Finite Element Analysis of Seismic Soil-Structure Interaction," by M.P. Romo Organista, J. Lysmer and H.B. Seed - 1977
- UCB/EERC-77/02 "Soil-Structure Interaction Effects at the Humboldt Bay Power Plant in the Ferndale Earthquake of June 7, 1975," by J.E. Valera, H.B. Seed, C.F. Tsai and J. Lysmer - 1977 (PB 265 795)A04
- UCB/EERC-77/03 "Influence of Sample Disturbance on Sand Response to Cyclic Loading," by K. Mori, H.B. Seed and C.K. Chan - 1977 (PB 267 352)A04
- UCB/EERC-77/04 "Seismological Studies of Strong Motion Records," by J. Shoja-Taheri - 1977 (PB 269 655)Al0
- UCB/EERC-77/05 "Testing Facility for Coupled-Shear Walls," by L. Li-Hyung, V.V. Bertero and E.P. Popov - 1977
- UCB/EERC-77/06 "Developing Methodologies for Evaluating the Earthquake Safety of Existing Buildings," by No. 1 - B. Bresler; No. 2 -B. Bresler, T. Okada and D. Zisling; No. 3 - T. Okada and B. Bresler; No. 4 - V.V. Bertero and B. Bresler - 1977 (PB 267 354)A08
- UCB/EERC-77/07 "A Literature Survey Transverse Strength of Masonry Walls," by Y. Omote, R.L. Mayes, S.W. Chen and R.W. Clough - 1977
- UCB/EERC-77/08 "DRAIN-TABS: A Computer Program for Inelastic Earthquake Response of Three Dimensional Buildings," by R. Guendelman-Israel and G.H. Powell - 1977 (PB 270 693)A07
- UCB/EERC-77/09 "SUBWALL: A Special Purpose Finite Element Computer Program for Practical Elastic Analysis and Design of Structural Walls with Substructure Option," by D.Q. Le, H. Peterson and E.P. Popov - 1977 (PB 270 567)A05
- UCB/EERC-77/10 "Experimental Evaluation of Seismic Design Methods for Broad Cylindrical Tanks," by D.P. Clough
- UCB/EERC-77/11 "Earthquake Engineering Research at Berkeley 1976," 1977
- UCB/EERC-77/12 "Automated Design of Earthquake Resistant Multistory Steel Building Frames," by N.D. Walker, Jr. - 1977
- UCB/EERC-77/13 "Concrete Confined by Rectangular Hoops Subjected to Axial Loads," by D. Zallnas, V.V. Bertero and E.P. Popov - 1977
- UCB/EERC-77/14 "Seismic Strain Induced in the Ground During Earthquakes," by Y. Sugimura - 1977
- UCB/EERC-77/15 "Bond Deterioration under Generalized Loading," by V.V. Bertero, E.P. Popov and S. Viwathanatepa - 1977

UCB/EERC-77/16	"Computer Aided Optimum Design of Ductile Reinforced Con- crete Moment Resisting Frames," by S.W. Zagajeski and V.V. Bertero - 1977
UCB/EERC-77/17	"Earthquake Simulation Testing of a Stepping Frame with Energy-Absorbing Devices," by J.M. Kelly and D.F. Tsztoo 1977
UCB/EERC-77/18	"Inelastic Behavior of Eccentrically Braced Steel Frames under Cyclic Loadings," by C.W. Roeder and E.P. Popov - 1977
UCB/EERC-77/19	"A Symplified Procedure for Estimating Earthquake-Induced Deformations in Dams and Embankments," by F.I. Makdisi and H.B. Seed - 1977
UCB/EERC-77/20	"The Performance of Earth Dams during Earthquakes," by H.B. Seed, F.I. Makdisi and P. de Alba - 1977
UCB/EERC-77/21	"Dynamic Plastic Analysis Using Stress Resultant Finite Element Formulation," by P. Lukkunapvasit and J.M. Kelly 1977
UCB/EERC-77/22	"Preliminary Experimental Study of Seismic Uplift of a Steel Frame," by R.W. Clough and A.A. Huckelbridge - 1977
UCB/EERC-77/23	"Earthquake Simulator Tests of a Nine-Story Steel Frame with Columns Allowed to Uplift," by A.A. Huckelbridge - 1977
UCB/EERC-77/24	"Nonlinear Soil-Structure Interaction of Skew Highway Bridges," by MC. Chen and Joseph Penzien - 1977
UCB/EERC-77/25	"Seismic Analysis of an Offshore Structure Supported on Pile Foundations," by D.DN. Liou - 1977
UCB/EERC-77/26	"Dynamic Stiffness Matrices for Homogeneous Viscoelastic Half-Planes," by G. Dasgupta and A.K. Chopra - 1977
UCB/EERC-77/27	"A Practical Soft Story Earthquake Isolation System," by J.M. Kelly and J.M. Eidinger - 1977

UCB/EERC-77/28 "Seismic Safety of Existing Buildings and Incentives for Hazard Mitigation in San Francisco: An Exploratory Study," by A.J. Meltsner - 1977

UCB/EERC-77/29 "Dynamic Analysis of Electrohydraulic Shaking Tables," by D. Rea, S. Abedi-Hayati and Y. Takahashi - 1977

58

.

For sale by the National Technical Information Service, U. S. Department of Commerce, Springfield, Virginia 22161.

See back of report for up to date listing of EERC reports.