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U.S. GEOLOGICAL SURVEY

SEISMIC ENGINEERING DATA REPORT

STRONG-MOTION EARTHQUAKE ACCELEROGRAMS

DIGITIZATION AND ANALYSIS

1971 RECORDS

OPEN FILE REPORT

No. <u>76-609</u>

This report is preliminary and has not been edited or reviewed for conformity with Geological Survey standards and nomenclature

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Any opinions, findings, conclusions or recommendations expressed in this publication are those of the author(s) and do not necessarily reflect the views of the National Science Foundation.

Brady, AG

PREFACE

This is the first of a series of reports planned to include the results of digitization and routine analyses of strong-motion earthquake accelerograms published by the U.S. Geological Survey. Serving as a model for this effort is the collection of data reports published by the Earthquake Engineering Research Laboratory of the California Institute of Technology during the years 1969 - 1975 and covering the significant records of the period from 1933 up to the San Fernando earthquake of February 9, 1971. This report covers the significant records of 1971 subsequent to the San Fernando earthquake. The following five records are included:

- Isabella Dam, California (auxiliary dam abutment), March 8, 1971.
- 2. Adak, Alaska, U.S. Naval Base (seismic vault), May 1, 1971.
- 3. Santiago, Chile, (University of Chile), July 9, 1971.
- Ferndale City Hall, California, (ground level pier),
 September 12, 1971.
- 5. Lima, Peru, (Instituto Geofisico), November 29, 1971.

Seismic Engineering Branch U.S. Geological Survey 345 Middlefield Road Menlo Park, CA 94025

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INTRODUCTION TO THE DATA MANAGEMENT PROJECT

A five year project for the continued operation and development of the national program in strong-motion instrumentation and data management was initiated in 1974 through an inter-agency agreement between the National Science Foundation and the U.S. Geological Survey. The present operation has overail direction and funding provided by the National Science Foundation while day-to-day management is provided by the U.S. Geological Survey. The total program is divided into four projects: Project Management, Network Design, Network Operations, and Data Management. The first provides the administrative function for the other three, whose functions are indicated by their names.

Data Management is concerned with archiving the original records, processing the significant records, and disseminating the resulting data and other information to the user community. One objective of data management is to develop a complete and unified processing system for the strong-motion data recorded by the networks of instruments. This report contains the results of some of the processing, the details of which appear in subsequent sections.

An arbitrarily selected peak amplitude of 5% g has been chosen as the main criterion for a record to be considered significant, if it is a ground level record, and 10% g if an upper story structural record. This holds for the 1971 post San Fernando records of this report and may be altered for future reports if the number of records designated "significant" by the 5% criterion becomes greater (or less), than the current staff and program can expeditiously handle. The current plan is for a report of perhaps up to three or four times this size to be published for each of the years subsequent to 1971.

One aspect of the archiving system for the significant records is the preparation of full-size contact film negatives from the original records which were recorded in the field on photographic paper or film. This reproduction

process is carried out under the supervision of the staff of the Seismic Engineering Branch by one of several commercial photographic concerns in the Bay Area, using the original records. From these film negatives, contact prints are made on Mylar-based film, either translucent frosted film for subsequent hand digitizing, (e.g., 6 in. or 12 in. records), or clear film for automatic digitizing procedures (e.g., 70 mm records). Measurements have shown that these prints differ in size from the film negatives by less than 0.1%, and the distortion involved in going from the original paper record to the contact negative is no larger than this. The Mylar film is mechanically strong, dimensionally stable, and affords excellent optical contrast. Experiments on photographic development techniques have resulted in standard methods for producing an optimum balance between contrast and trace width.

For 6 in. and 12 in. photographic paper records a choice is made, depending on the convenience of the digitizer, whether to digitize from the original or a contact print. For 70 mm or 35 mm film records, digitizing by hand requires enlargement of the original. The enlargement is 2X or 3X for 70 mm film, and 3X for 35 mm film, these being a compromise between the resultant effective sensitivity of the acceleration traces, the original length, and the size of the vacuum table carrying the copying film. Without compromise is the requirement that the enlargement be performed in one step, using a lens of absolute minimum distortion, and viewing the entire original length in one frame.

<u>Digitizing options.</u> All the records analyzed in this report have been digitized at Dynamics Graphics, in Berkeley, California, on a Calma digitizing system. This includes a hand-held restrained cursor moving on a 60"-long table with resolution of 1000 points per inch and an RMS error of perhaps 0.003". Digitized output, selected at approximately 50 points per second of record time, is recorded on magnetic tape compatible with the U.C. Berkeley 6600 computer.

For 70 mm film, another digitizing facility is currently used. The I/OMetrics Corporation in Sunnyvale, California has a trace-following, laser scanning system, capable of handling a record in 12 cm sections, with a resolution of 1 micron (10^{-6} meter) and an RMS error of the order of 10 micron. Digitized output at approximately 500 points per cm of record path length is recorded on computer compatible magnetic tape. A projected report will cover the detailed investigation of the accuracies of these two systems, and of other systems which may be called on in the future, for example, Environmental Data Service in Boulder, Colorado, and the Earthquake Engineering Center at Stanford University, California.

STANDARD DATA PROCESSING FOR ACCELEROGRAMS

All processing of the raw digital tapes is done at the U.C. Berkeley computer center and Lawrence Berkeley Laboratory facilities. The programs developed at Caltech during the years 1968 to 1972 (Trifunac and Lee, 1973), have been adapted for use with the CDC 6C00 and 7000 computer systems. The total programming package includes the following phases:

Phase 1: processing to obtain "uncorrected" acceleration data.

- Phase 2: introduction of instrument correction and baseline correction to obtain corrected acceleration, velocity, and displacement.
- Phase 3: calculation of response spectra and Fourier amplitude spectra at the same values of period.
- Phase 4: note that the results of the fast Fourier spectra have not been adapted for inclusion in this report.
- Phase 5: calculation of time-dependent spectra.

<u>Phase 1 - Uncorrected Accelerations.</u> This first phase of processing presents the digitized accelerograms of strong earthquake ground motions as processed from records obtained from the strong-motion accelerograph network maintained by the U.S. Geological Survey. No base-line or instrumental corrections or adjustments have been made at this stage - the data may thus be regarded as "uncorrected" in the sense that no modifications have been introduced which involve any hypotheses as to the character of the ground motions or instruments involved. This digitized data is thus believed to be as close a representation of the original, raw information as it is feasible to achieve with digital processing.

The records have been digitized on an unequal time basis, which gives, if the points are well chosen, the best definition of the trace for a given number of data points. All visible local peaks and changes of slope have been picked,

along with as many intermediate points as are needed to maintain an average number of points per second of record of at least 50 throughout the accelerograms. If equispaced data is needed for a particular computer program, it is a relatively simple matter to set up an interpolation program as the first step in the computation.

Considerable thought has been given to the length of record to be digitized. Although the actual strong-motion portion of the record is the most important part for engineering purposes, a sufficient length of the later lower amplitude accelerations have been included to permit studies of long-period characteristics. Such studies are, of course, limited by other factors of instrument characteristics and accuracy. For certain investigations for which the longer-period components are not important, only a fraction of the whole digitized record might be used.

Most records contain several traces produced by "fixed" mirrors rigidly attached to the accelerograph frame. In some cases, these fixed traces depart measurably from straight lines, usually involving long-period components to be ascribed to paper distortion, motions of the paper in the drive mechanism, etc. For all records on which fixed traces are present, the fixed traces are digitized at intervals of the order of one half-second, smoothed by weighted averaging over every three consecutive points, and subtracted from the accelerometer traces as a first step in the data processing.

Timing marks on the record were also digitized and smoothed by a 1/4, 1/2, 1/4 running average to form the basic time coordinate.

To fix the particular values of the digitized ordinates, some more-or-less arbitrary decision has been made as to the position of a straight reference line. When the record is placed on the table of the digitizing machine, it is lined up with the horizontal axis of the machine as closely as can be judged by eye. For this purpose the fixed traces serve as useful guides, as do the zero

trace sections at the beginning of the record before the triggering of the instrument. It must be realized, however, that imperceptible shifts of the horizontal axis in translation or rotation lead to large deviations in double integrated displacement curves, so some technique which assures a uniform result is needed. For this purpose, the following procedures have been adopted. The film record is first placed on the digitizing table with the horizontal axis lined up by eye parallel to an estimated zero axis. If the record trace and the fixed mirror traces have been digitized without moving the record on the table of the digitizing machine, then the subtraction of the two traces will correct for any slight rotation of the record on the table, so that only translation of the horizontal axis is required. This axis is therefore translated to a position which makes the integral of the digitized acceleration curve, over the length of the record, zero. This is, in principle, the same as making the mean acceleration value zero, or making the sum of the squares of the deviations from the horizontal axis a minimum. Although this means physically that the change in ground velocity from beginning to end of the record is zero, which is actually not the case for records triggered by the excitation itself, this method for selecting the horizontal axis position would seem to be the most logical choice for a standard procedure.

For those few records for which fixed traces are not available, or for which the record has been moved on the table between the digitizing of the record trace and the fixed trace, the horizontal axis is not only first translated to make the mean zero as above, but then a very small rotation is introduced to make the sum of the squares of the deviations from the zero line a minimum. This removes the effects of any slight rotational misalignment without interfering with the basic data.

It is believed that the above data processing techniques represent a

minimum adjustment of the data, which consequently may be referred to as the basic "uncorrected" data.

Computer plots of the uncorrected data are included in a later section of this report. The three components of each record are shown, except for those records where fewer than three components were digitized. The scales on the two axes are standard throughout, wherever possible, so that a quick glance shows immediately the relative amplitudes, and whether or not there were particularly high frequency or long period ground motions present.

The components mentioned, e.g. S44W, N46W, Up, indicate the direction of the transducer pendulum motion when moved by hand, for the trace to be deflected upwards on the record is it is viewed in the normal way with time increasing from left to right and the emulsion side up. This has been the standard practice since the strong-motion program began in the early thirties under the name of the Seismological Field Survey. A true ground acceleration, during earthquake motion, will be positive (or upwards) on these plots when in the opposite direction to these component labels.

<u>Phase 2. - Corrected accelerations.</u> The processing necessary to correct accelerograms includes the corrections for instrumental response and true baseline. Preliminary smoothing is carried out first. From the uncorrected accelerograms, digitized at unequally spaced points, equally spaced data with 100 points per second are interpolated. This is low-pass filtered using an Ormsby filter (Ormsby, 1961), having a cutoff frequency of 25 cps and a roll-off termination frequency of 27 cps. Decimation follows, selecting every second point, resulting in smoothed data with 50 points per second, corresponding to a Nyquist frequency of 25 cps.

The instrument correction affects particularly those frequency components higher than about 1/2 of the natural frequency of the instrument transducer,

and is particularly important towards the upper limit of the frequency band retained, i.e., 25 cps. The correction is performed using the standard secondorder differential equation governing oscillator motion together with the natural frequency and fraction of critical damping obtained from calibration tests of each accelerograph component. Differentation of the instrumental response that is required for this correction is carried out by the simplest central difference method.

The baseline correction removes from the accelerogram all Fourier components with periods longer than an upper period limit chosen on the basis of careful tests. The standard upper limit for hand digitized paper records has a cut-off period of 14 sec, and roll-off termination at 20 sec — the corresponding frequencies are 0.07 and 0.05 cps. This correction procedure, developed in the Caltech standard data processing project, replaces the earlier systems of baseline correction that consisted of the least squares fitting of a constant, a straight line, or a parabola, etc. The details may be found in the reports from the Caltech data processing project (see References, Phase 2) but in general terms the procedure is made up of the following steps:

- Least square fit a straight line to the uncorrected acceleration as an initial baseline and initial correction to the acceleration.
- Compute the velocity, assuming a zero initial value, and least square fit a straight line to the velocity.
- 3. Add the slope of this fitted line to the acceleration obtained in scep 1.
- 4. Low-pass filter the acceleration of step 3 with a running mean filter, decimate it, and low-pass again with an Ormsby filter using the cut-off and roll-off termination frequencies (.07 and .05 cps) given above. The remaining very long period components form the new baseline, which is subtracted from the acceleration of step 3. This complete step consequently results in a high-pass filtered acceleration.

- 5. Again compute the velocity, assuming zero initial values, and least square fit a straight line to the velocity.
- Incorporate the slope of this fitted line into the acceleration of step 4.
- 7. Apply the complete step 4 to the velocity of step 5, resulting in a high-pass filtered velocity and a particular initial value of the velocity.
- 8. Compute the displacement from the velocity of step 7, assuming a zero initial value, and apply the high-pass filter of step 4 to this displacement, which also results in a particular initial value of the displacement.

The instrument correction and baseline correction outlined above result in a corrected accelerogram which represents the acceleration of the instrument support in the frequency band between 0.07 cps and 25 cps. The accuracy of the long period Fourier components of displacement has been extensively investigated (Hanks, 1973) using results obtained from the San Fernando earthquake. The accuracy is estimated to be somewhat better than 1 cm in the period range of 5-8 seconds, approximately 2 cm at periods near 10 seconds, and several (2-4) cm in the 10-15 second period range. It is evident here also in some of the displacement plots of stage 2 processing that some long period noise is still present. There are no components remaining with periods longer than the cut-off period of 14 seconds (and roll-off termination at 20 seconds), but it is evident that for short records and some long records this cut-off period is in fact too long if displacement errors are to be kept smaller than the estimates indicated above. The cause for this lies in the presence of noise arising primarily from random digitization noise, independent of the acceleration amplitudes. The very low amplitude of long period accelerations throughout the record therefore

gives a low signal-to-noise ratio at these long periods. With this in mind, the displacement plots indicate that a cut-off period lower than 14 seconds could perhaps be used to remove these components from the acceleration data. Routine processing of subsequent records will include the ability to make a preliminary selection of a cut-off period optimal for a particular record by viewing displacements during computing runs and phase 2 processing will incorporate this selection.

Computer printout of the corrected data of phase 2 are included in a later section of this report. The corrected acceleration is listed in units of mm/sec/sec, so that space-saving integers can be used, retaining the precision of the original digitizing machine, whose least count corresponds to about 3 mm/sec/sec. The equal time interval is 0.02 seconds. Identification labels. instrument characteristics, peak values, and initial values are included in the headings for each component. Plots of acceleration, velocity, and displacement are shown with axes and scales selected suitably to show the entire record. Peak values are indicated. The definition of component direction has been described previously for the phase 1 plots. For the phase 2 plots, the same component name is retained. But for the corrected data, in order for the component name to indicate the direction of positive ground acceleration, velocity, and displacement, the signs on the vertical axes have been changed. All ground motions, both portrayed in the plots and listed in tables, are now positive when in the direction of the named component direction, and such a positive motion is plotted downwards. As examples of this convention, if the vertical component is labelled "Down", then a phase 2 plot will present an actual downward ground displacement (or velocity, or acceleration) with a curve that is down below the axis. If a horizontal component is labelled "East", a phase 2 plot will present an eastward ground displacement with a curve that is down below the axis.

<u>Phase 3 - Response spectra.</u> An introduction to response spectrum techniques as they are commonly applied in the earthquake engineering field may be found in the references, particularly the Caltech report, Volume IIIA. The following remarks apply primarily to the influence of the data processing techniques on the computation of these spectra.

The component directions have been described in the earlier sections of phase 1 and 2. The spectral calculations of phase 3 are concerned with absolute values of response and the particular component sense is thus immaterial.

The earthquake response spectrum was originally introduced as a means of characterizing the response of structures to the exciting ground motion, and hence such spectra are calculated from ground motions. There are many instances, however, in which a structural motion is the excitation, for example, of mechanical equipment or other objects attached at upper story locations in buildings. In such cases it may be useful to calculate a response spectrum based on an acceleration-time function which is in itself a response to the earthquake ground motion. Such response spectra of the motion of a particular structure will ordinarily contain predominant peaks corresponding to structural frequencies and will consequently appear very different from ground spectra.

Response spectra calculated from the corrected accelerograms of phase 2 are presented in the data section of the report. They represent the best combination of accuracy and frequency range that it is feasible to achieve with the currently available instrumentation and data processing. As future investigations improve these data processing techniques, and as more refined instruments make it possible to attain higher levels of accuracy and a wider frequency range, such calculations can no doubt be extended to take advantage of such improvements.

The long period noise described in the section on phase 2, and evident in

some of the displacement plots appearing in this report, have an effect on the appearance of the phase 3 plots. Only experience can help in determining if the high spectral amplitudes at long periods correspond to the evident long period oscillations in some of the displacement plots. The extent to which long period noise plays a part in this behavior also becomes clear with experience and is discussed in the references under phase 2.

A number of accelerograms have been obtained from instruments located in upper floors of buildings, and upper levels in dams, and it has been decided to include response spectra computed from these structural responses in the same form as the response spectra from ground level records. Such building spectra will be useful in estimating the response of equipment and contents to the motions of particular buildings excited by earthquakes. The reader is cautioned to keep this point in mind and to avoid such building spectra when studying the characteristics of the ground motion itself.

The phase 3 plots and printouts are presented later in this report. For each component there are two figures showing response spectra and two pages of tables containing the spectral ordinates. These are described in detail in the following paragraphs.

The first plot is that of the true relative velocity response spectrum, RV, with an identifying descriptive title. The five continuous line plots correspond to damping values of 0, 2, 5, 10 and 20 percent of critical and these curves will usually be easily distinguishable, with the zero damped curves having the greatest ordinates.

The dashed curve on this plot is the unsmoothed Fourier amplitude spectrum, FAS, calculated at the same periods as the relative velocity response spectra. The ordinates for the spectra are in units of cm/sec, in accordance with current scientific practice, while the scale is chosen to fill the available space.

The periods extend to 15 seconds, close to the long period cut-off point in the corrected data of phase 2, but the axis is divided into two separated linear portions. From 0 to 3 seconds takes up three-quarters of the axis, and from 3 to 15 seconds takes up the remainder.

The second plot is that of the pseudo velocity response spectrum, PSRV, together with the relative displacement spectrum, RD, and the pseudo acceleration spectrum, PSAA, in the tripartite logarithmic plot versus period. This convenient plot is made possible by the relationships between PSRV, RD, and PSAA:

 $PSRV = (2\pi/T) RD$ $PSAA = (2\pi/T)^2 RD$

The units used are cm/sec, cm, and g.

The two pages of tables contain values of the ordinates for the previous plots. After the titles there are arranged, in columns, the periods (PER), Fourier amplitude spectrum (FAS), and then sets of four columns containing RD, RV, absolute acceleration spectra (AA), and PSRV for all of the five damping values. The units are indicated and each value is followed by a multiplicative power of 10, e.g., 432-1 represents a value of 43.2.

<u>Phase 4 Fourier spectra</u>. As indicated earlier in this section, the Fourier spectra calculated by fast Fourier transform subroutines are not included in this report. They will be incorporated in a subsequent report devoted solely to Fourier spectra.

<u>Phase 5. Time-dependent Spectral Analysis.</u> The response spectrum was first introduced into earthquake engineering by Benioff (1934) and refined by Biot (1941). With improvement and refinement by others (Alford et al., 1951; Housner et al., 1953; Huuson, 1956), this technique has become an important tool in the design of earthquake resistant structures when dealing with buildings of simple design and special structures such as elevated water tanks. The response of

each mode of multi-degree-of-freedom systems such as tall buildings, chimneys or towers can be calculated utilizing the same equation of motion used to obtain response spectra. Each modal response can then be superposed to obtain the total response of the system (Nerchant and Hudson, 1962). When the approximate design method utilizing response spectra is not sufficiently accurate and the more involved and costly technique of time-history dynamic response is needed, a response spectrum can be used for the preliminary design. Considering the importance of the response spectrum in seismic engineering, a maximum amount of information should be extracted from it. One method involves the study of response as a function of time (Perez, 1973a; Trifunac, 1971; Hays et al., 1973; Blume and Associates, 1973).

The response spectrum is a plot of the maximum response of a single-degreeof-freedom oscillator, subjected to the particular earthquake, for a given damping factor and for a spectrum of frequencies. However, this focusing on the maximum value ultimately ignores any relationship that exists between the time history of the response and the ground motion. In particular, no information is retained on the difference in time between the strongest part of the ground motion and the strongest part of the resulting response. This type of relationship can be qualitatively investigated using time-dependent spectral analysis, which helps in understanding the effects of high levels of ground acceleration on the response spectrum (Perez, 1973b). The understanding of high levels of ground acceleration is critical, as values have been recorded as high as 1.25 g for Pacoima Dam in 1971 (Trifunac and Hudson, 1971), and 0.7 g for Melendy Ranch in 1972 (Morrill et al., 1974). It can be expected that higher peak accelerations will continue to be recorded.

Important structural engineering information can be obtained by studying in detail the length of time that the general level of velocity response, as

indicated by its envelope, is greater than particular predetermined levels. Although certain levels of shaking may do minimal damage to structures at the onset of an earthquake, prolonged shaking at those levels could cause extensive damage due to progressive failure. At the present time, the correlation of building damage versus levels of response and their respective time duration has not been developed. However, attempts in these two areas are being made by several investigators. For example, Matthiesen and Rojahn (1972) have made estimates of threshold structural damage levels for various classes of buildings. The structural response duration at different levels and its relation to structural damage by low-cycle fatigue has been studied by Kasiraj and Yao (1968); Suidan and Eubanks (1973) have studied the cumulative fatigue damage in seismic structures; Popov and Bertero (1973) have studied cyclic loading of steel beams and connections.

<u>The Velocity Response Envelope Spectrum (VRES)</u>. This section describes the contour plots of the velocity levels attained by the velocity response envelopes. The original response spectrum is based on the response of the single-degree-of-freedom, viscously-damped, linear oscillator subjected to earthquake ground motion. Such an oscillator acts as a narrow-band filter which amplifies the input frequencies centered around the natural frequency of the oscillator (Trifunac, 1971). To study the response as it varies with time, the envelope of the response is used instead of the actual response. The envelope contains all of the important information required to calculate the maximum relative velocity as normally defined, while maintaining the history of the response as it varies in time. For any particular oscillator those times at which the envelope of the oscillator response rises above or falls below predetermined velocity levels are noted and the results are plotted in the VRES two-dimensional contour plots. The plots are shaded for clarity.

The following method was used to calculate the VRES as a two-dimensional function of time and natural period. The oscillator response was computed for 41 natural periods. The periods selected were: 0.05, 0.075 seconds; from 0.1 to 1.0 s.conds at 0.05 second intervals; from 1.0 to 2.0 seconds at 0.1 second intervals; and from 2.0 to 4.0 seconds at 0.2 second intervals. This scheme was chosen to obtain an appropriate density distribution at the higher frequency end of the spectrum. For each period the response envelope was approximated by connecting the absolute value of the local peaks of the response curve. The envelope curve was then interpolated at 160 equal intervals regardless of length of time (i.e. if length of time was 16 sec, then interpolation interval was 1/10 sec). Levels were chosen according to the maximum response, with no more than six levels, regardless of amplitude.

These 41 periods, with their respective VRES calculated at equal time intervals, represent a rectangular grid of spectral values. Contours of equal amplitude were produced by plotting interpolated values from the grid, giving a contour map of the VRES amplitude values as a function of time and period. The maximum relative velocity response spectrum is plotted to the right of each contour map. The contour map shows the peaks and valleys of the VRES as a function of time and period, while the maximum relative velocity response spectrum shows the silhouette of the peaks. The input acceleration is plotted below the contour map to show the relationship between acceleration and the VRES as they vary in time.

In most instances, connecting the local peaks of the response is a good approximation to the response envelope (Perez, 1974). In some instances, the high frequency content in the response gives an envelope of these high frequencies and not of the period being analyzed. A better approximation to the envelope of the response would be obtained by filtering out frequencies appreciably higher

than the ones being studied, without altering either the phase or the magnitude of the response of a given oscillator. Since filtering would eliminate the high frequency information that exists in the response of longer period oscillators, and since the VRES plots can easily be inspected when these high frequencies are present, no smoothing of the response is contemplated at the present time.

<u>Displacement and Pseudo-absolute Acceleration Response Envelope Spectrum</u> (DRES AND ARES) Time-dependent spectral analysis may be expressed not only as relative velocity (VRES), but also as relative displacement (DRES) and absolute acceleration (ARES). Relative displacement is important because the shear force exerted by the columns of a structure on the ground are directly proportional to the relative displacement. The absolute acceleration is a measure of the seismic forces acting on the mass of a structure.

A partial solution to obtain VRES, DRES, and ARES is to approximate these quantities through a calculation commonly used by structural engineers. If the response is assumed to be approximately sinusoidal, then the displacement response can be approximated by dividing the velocity response by $\omega_0 = 2\pi/T_0(T_0)$ is the natural period of the oscillator) and the absolute acceleration response can be approximated by multiplying the velocity by ω_0 . Because velocity response is calculated as a function of time for specified levels, a nomograph may be constructed with curves relating velocity to the corresponding values of displacements and pseudo-absolute accelerations. The method is somewhat similar to the tripartite logarithmic plots used by structural engineers. The curves of the nomograph are based on the different levels of the relative velocity response envelope as a function of time. Note the ordinate of the nomograph or the right of the VRES plots is drawn to the same scale as the contour map and the maximum velocity response spectrum. The left side of the nomograph's contour level represents the lower velocity ranges. An example of using the nomograph is as follows:

for a velocity level of 2.5 cm/sec and 3.0 second period, the equivalent pseudo-absolute acceleration is about 0.005 g; for the same velocity level and same period, the equivalent displacement is about 1.2 cm.

The damping of structures undergoing small amplitude oscillations is generally found to be in the range of 1 to 5 percent critical damping; for structures behaving in the nonlinear and plastic range, the damping can be 10 percent or more (Trifunac, 1971). In this analysis, 5 percent critical damping was used in calculating the VRES.

Since the period range for the VRES is between 0.05 and 4.0 seconds, harmonic theory indicates that the response delay to any significant pulse should be no more than 0.0125 to 1 second (i.e., period/4). However, an examination of the VRES and the acceleration record incorporated alongside shows that the maximum acceleration and the maximum velocity response need not be separated by this delay time interval (Perez, 1974).

<u>Time Duration Spectrum of the Response Envelope.</u> From an engineering point of view, it is important to study not only the peak response and time of occurrence, but also the time duration above a given level of response (Perez, 1973a). The time duration spectrum is defined as the cumulative total time that the VRES equalled or exceeded a given level during the entire acceleration record. In the time duration spectra plots, note that levels of response chosen are identical to those chosen for the computation of the VRES plots. The total time duration of different amplitude levels of the VRES can also be expressed in terms of the number of cycles that occurred at or above a particular level. Due to the filtering properties of a simple harmonic oscillator, the period of the velocity response is approximately equal to the natural period of the oscillator. Therefore, by dividing the duration by the period of the oscillator, a family of straight lines indicating the number of cycles for a given velocity response level can be generated.

The nomograph in the time duration spectra may be used to convert envelope levels of the velocity response to corresponding envelope levels of displacement and pseudo-absolute acceleration response. Because approximate sinusoidal motion is assumed for the velocity response, the time duration and the number of cycles for a given amplitude level also hold true for the corresponding levels of the displacement and the pseudo-absolute acceleration response envelope.

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SUMMARY OF RECORDS INCLUDED IN THIS REPORT

The data on the records and the earthquakes providing them are summarized in Tables 1, 2, and 3. Additional information on some of the records follows.

1037 Isabella Aux. Dam:

Five records were recovered from the Isabella array, although only the one from the abutment of the auxilliary dam was considered significant, with an initial scaling of the peak acceleration of 0.11 g.

4400 Santiago, Chile:

Only one trace was legible enough to digitize, the NIOW horizontal trace. It was considered impossible to recover the other two traces with sufficient confidence in their accuracy.

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EAR'	THQUAKE	DATA
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No.	Date, Location	Origin time Local and GMT	Epicenter	Depth	Magritude	Max MMI	Felt Area
1.	8 Mar 1971	15:08:07.7 PST	35°40.0'N	6 km	4.1 (PAS)	v	10,400 km ²
	Central Calif.	(23:08 GMT)	118°24.2'W	(PAS)	4.7 (BRK)		(4000 mi ²)
2.	1 May 1971	20:08:27.3 AST	51.4°N	43 km	7.1	VI (BSSA)	_
	Andreanof				(M _s , ERL)		
	Islands	(2 May, 06:08 GMT)) 177.2°W	(ERL)	6.8 (PAS)		
					7.0 (BRK)	IV (JSE)	
					6.0 (ERL)		
3.	8 July 1971	23:03:18.7 Local	32 . 5°S	58 km	6.6 (ERL)	X	
	Off Central	(9 July, 03:03 GMT	ſ) 71.2°₩	(ERL)	7.5 (PAS)		
	Chile coast				7.5 (BRK)		
4.	12 Sep 1971	11:32:38.0 PST	41°17.9'N	20 km	4.6 (BRK)	v	7800 km ²
	Northern	(19:32:38.0 GMT)	123°40.4'W	(ERL)	4.5 (BRK)		(3000 mi ²)
	Calif.	(19:32:33.9, ERL)	(41.5°N,		4.9 (ERL)		
			123.7°W,	ERL)			
5	29 Nov 1971		11.2°S		5.3		
	Off Peru coast		77.8°W				

Notes:

PAS - Seismological Laboratory, Caltech, Pasadena ERL - Environmental Research Laboratories, "Preliminary Determination of Epicenters". BRK - Seismographic Station, UC Berkeley. BSSA - "Bulletin Seis. Soc. Amer." USE - "United States Earthquakes," NOAA.

Table 2

EARTHQUAKE RECORD INFORMATION

No.	<u>Station</u> Name	Site Charact.	Structure Type/size	Instr. Location	Eq. No.	Epi. Dist. (km)	MMI at Site	Dig'd Length (sec)	Total Record Length (sec)
1037	Isabella Aux. Dam;	_	earth dam	abutment	(A) 1		v	9	9
2701	Adak, Alaska, U.S. Naval Base	basalt	instrument shelter (C)	ground level	2	70	IV (USE) VI (BSSA)	25	49
44 00	Santiago, Chile, Engineering Bldg, UC		3-story bldg.	ba semen t	3	120	VI	50	125
1023	Ferndale, Old City Hall, Brown St.	Alluvium	2-story bldg.	ground floor	4		IV	20	78
4302	Lima, Peru, Geophysical Institute, Avenida Arequipa		l-story bldg.	ground level	5	125	-	40	62

Notes: (A) - in small prefab. bldg. (C) - in concrete vault (USE) - "US Earthquakes," NOAA (BSSA) - "Bulletin," Seis. Soc. Am.

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Table 3

INSTRUMENT CHARACTERISTICS

	Station Identification			Instr. Components		Period	Damping
No.	Name	Coord	owner Serial no	•	cm/g	sec	fraction
1037	Isabella Aux. Dam	35.64N 118.47W	RFT 250 AE-112	L 36 N14E V 63 Down T 33 N76W	1.91 1.95 1.82	.047 .047 .048	.55 .55 .59
		Installed:	1-19-68;	Removed - No			
2701	Adak, Alaska US Naval Base	51.88N 176.58W	AR 240 FS 141	L 235 N V 409 Down T 232 W	8.2 6.5 7.5	.055 .051 .055	.59 .59 .59
		Installed:	1-24-67;	Removed - No			
4400	Santiago, Chile Engr. Bldg, UC	33.47S 70.67W	S-M FS 50	V 271 Up L 272 S80W T 273 N10W	N N 13.0	ot digiti ot digiti N62	zed zed 57
		Installed:	0-0-44;	Removed - No		IUUE	•••
1023	Ferndale Old City Hall Brown St.	40.58N 124.26W	S-₩ FS-23	V 247 Up L 248 S44W T 249 N24W Bonoved – No	13.8 13.3 12.3	.068 .067 .065	.57 .57 .59
		Instatteu:	5-0-35%	Removed - NU			
4302	Lima, Peru Instituto Geofisico Avenida Areguipa	12.075 77.04W	S-M FS 44	V 205 Up L 204 N82W T 203 N08E	12.4 12.9 12.9	.065 .065 .066	.55 .61 .55
	••••	Installed:	0-0-44;	Removed - No			

Notes: AE: Corps of Engineers, US Army FS: Seismic Engineering Branch, USGS RFT 250: Teledyne AR 240: Teledyne









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CORRECTED ACCELERATION ORDINATES

INSTR PERIOD = .0510 SEC DARFING = .990 PEAK VALS ACLN = -50.3 CM/SEC/SEC AT 6.06 SEC VELD = 3.7 CM/SEC AT 6.00 SEC DISP = -3.2 CM AT 12.48 SEC LENGTH OF NECOND = 24.60 SEC INTIAL VELD = .27483 CM/SEC INTIAL DISP = .21637 CM

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TIME DURATION SPECTRUM OF THE RESPONSE ENVELOPE

UNCORRECTED ACCELEROGRAM LIMA, PERU, INSTITUTU GEOFISICO, 1/29/71 N 8 2 W 2, ACCELERATION IN C/10 N 0 8 E - 1 - 2 UΡ - 1 - 2 L 0 TIME IN SECONDS LIMA, PERU, INSTITUTO GEOFISICO, 11/29/71 N 8 2 W ACCELERATION IN C/10 - 2 NOBE UΡ

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ACCELERATION ORDINATE CORRECTED

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 ****;** 10 AT .04 SEC Connected Data LIMA, PENU, INSTITUTO -LIMA, PENU, INSTITUTO -ACLN = 53.4 CM/SEC/SEC AT . ON LENGTH OF RECOND = 90.10 SEC 2010 INSTRUMENT AND BASELINE CORRECTEI **2.2019 2.201** 2 VALS ENSTA PEAK

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TIME DURATION SPECTRUM OF THE RESPONSE ENVELOPE



