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Stanford University

**SEISMIC HAZARD MAPPING  
FOR GUATEMALA**



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by  
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## PREFACE

This is the first report of the study on Seismic Risk for Guatemala. This phase of the study includes the collection and description of geological and seismological environment and the use of that information in developing iso-acceleration maps of Guatemala. This phase of the study gives results on future probable seismic hazard for Guatemala which can be utilized by engineers in formulating the seismic codes, by planners in future developments and by the insurance companies in evaluating the seismic risks and rates. Various state-of-the-art models are used to develop this hazard information.

Phase II of the project will involve the following topics.

- A thorough study of the 1973 UBC, the 1976 UBC and the ATC-3 work currently being completed. The above codes will be evaluated with the seismicity of Guatemala in mind. Such an evaluation will permit the engineers and planners in Guatemala to appreciate and understand the current code levels and their relationships with future seismic loading demands.
- A detailed discussion on the purpose and effects of earthquake codes will be presented. This will include the history of earthquake loading criteria, the relation of design loads and quality of structures, the objectives and qualities of workable seismic codes and the role of design detailing and design forces in providing a safe economic construction.

- Introduction to the Proposed Seismic Design Provisions.

This will take into account the current codes, their advantages, their shortcomings and the available solutions to eliminate or reduce the shortcomings. This part of the study will explain in detail the concept of acceptable risk and the associated loading levels from the hazard maps of Guatemala. The study will also develop the shape and levels of various design spectra. These spectra will include the site characteristics information.
- A detailed description of the type of structural systems, their effects on the design level and a step-by-step design procedure will be developed.
- Based on the spectral approach of seismic design, a simplified equivalent static load method, similar to the 1976 UBC method of design, will be developed. It should be emphasized here that a workable code should have the following four ingredients.
  - 1) Simplicity.
  - 2) Rationality.
  - 3) Freedom to use responsible ingenuity for special structures.
  - 4) Reward and encouragement for using dynamic analysis when merited by the complexity of a given structure.

In developing the methodology in part II of this study, the above four ingredients will be kept in mind.
- Finally, a detailed comparison with the proposed methodology and the 1976 UBC will be made. This will be done with the seismic environment of Guatemala in mind.

It is estimated at this time that the results of phase II study will be submitted before the end of September 1977.



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Seismic Hazard Mapping  
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## CHAPTER I

### INTRODUCTION

#### Scope

In this chapter the general background under which this study was initiated is presented. The concepts of seismic hazard and risk are described. The main objectives of the current study are presented.

.....

#### I-1 Introduction

On February 4, 1976 at 3:01 A.M. (local Guatemalan time), central Guatemala experienced a major earthquake. The surface-wave magnitude ( $M_s$ ) of this earthquake was 7.5 and the body wave magnitude ( $M_b$ ) was 5.8. The hypocentral location was 15.32°N latitude and 89.08°W longitude with a focal depth of about 5 kilometers. Various aftershocks followed the main event for days and weeks. It is estimated that millions of dollars of damage and thousands of lives were lost. The resulting economic hardships and the disruption in the way of life can not be fully estimated.

In the chaos of rescue, public care and debris removal which are the usual results of such earthquakes, it is very difficult for public officials and engineers to be concerned about why some buildings survive and others collapse. The building efforts which follow such events bring up many questions. Are existing design requirements adequate? What are the future seismic hazards? What should be the acceptable level of seismic risk? How should the information on future seismic hazards and acceptable risk be translated into a rational, simple and acceptable design methodology? Should similar land uses be permitted in the future for areas which suffered major damage? These and numerous other questions become especially relevant

after a significant damaging event. The loss of life and damage brought about by the earthquake-induced landslides in Guatemala City demonstrated the need for proper land use planning.

The decision processes which lead to the answers for the numerous questions posed in the previous paragraph are a complex mixture of socio-economic constraints, political expediency and the general engineering knowhow. In times when no significant earthquake events have taken place, the decision making processes go on at a slow rate, while the decisions immediately after the damaging event are often based on expediency and, at times, on incomplete and irrational analysis. The attention is only focused on collapsed or heavily damaged structures and the reaction is that these failures should never be allowed to happen. The public, through their representative officials, demand doubling or tripling of the existing design levels -- with resulting higher costs and delayed construction.

Under this barrage of emergency actions, public pressure and all sorts of "consulting advice" from experts from all over the world, it is remarkable that the engineers, planners and decision makers in Guatemala decided to initiate a systematic study of seismic hazard and risk analysis with long range perspective. Such a study has two basic goals.

- 1) To develop a seismic hazard map for Guatemala, based on all the available seismological and geological information.
- 2) Based on the information of future seismic hazards in various parts of the country and based on the concept of acceptable level of risk, develop a rational and simple design methodology for lateral load resistance.

This report is the result of achieving the first goal mentioned above. The study was conducted at Stanford University and supported by the following organizations in Guatemala.

1. Cámara Guatemalteca de la Construcción
2. Universidad de San Carlos de Guatemala
3. Banco Nacional de la Vivienda
4. Instituto Nacional de Electrificación
5. Banco de Guatemala

In a second report (to be published in the future) the probabilistic hazard information will be used to develop a design methodology which if implemented could help in reducing the future seismic risk to an acceptable level. A detailed comparison between the suggested method and the current and proposed California codes will also be presented.

The results of seismic hazard evaluation presented in this report should be used as a base for planning and decision making in Guatemala. Such a study can also be utilized by land use planners, investors and insurance companies. In general, the results of this study (Phase I) can provide professionals in Guatemala with tools and procedures for risk analysis and planning.

#### I-2 Some Basic Concepts Concerning Hazard and Risk

In order to convey the importance of seismic hazard and risk analysis to the reader, some basic notions are presented in this section. In the earthquake engineering literature, there is in general, ambiguity regarding two words. They are: Hazard and Risk. Seismic hazard is regarded by many to be synonymous with seismic risk. Earthquake engineers and planners use these two words loosely and interchangeably in their work. There is some danger in this ambiguity since these two words within the context of earthquake engineering have different meanings.

Seismic hazard is defined as "expected occurrence of a future adverse seismic event".

Seismic risk is defined as "expected consequences of a future seismic event".

Consequences may be life loss, injury, economic loss, function loss and damage. Expected hazard and expected risk have an implication of future uncertainty. Hence, it is not surprising that principles of probabilistic forecasting and decision making are essential in any seismic hazard or seismic risk analysis.

In a recent report to the United States Congress (1972) by the U.S. Executive Office of the President, Office of Emergency Preparedness, the following two recommendations were made.

- 1) The development of seismic hazard maps is an essential first step in hazard reduction and preparedness planning.
- 2) The greatest potential for reducing the loss of life and property from earthquakes lies in restructuring the use of land in high risk areas and in imposing appropriate structural engineering and materials standards both upon new and existing buildings.

As can be seen from above, it is essential that a seismic hazard map be prepared for Guatemala as a first step.

Finally, it should be kept in mind that the work and results presented here depend on the available data base and information. The reliability of results are at best as good as the reliability of the data on which the results are based. It is very easy to attack and criticize any work from the point of view of data reliability. However, it is very difficult to obtain long-range reliable data. The best available information through various organizations and researchers have been used in this study. The forecasts and predictions are based on those data. However, if in the future more reliable data are available, the model can easily accommodate the inclusion of new information and update the results. At this time, the

authors of this report feel that the results presented here represent the "best available" estimates of the future forecasts.



CHAPTER II  
GEOLOGIC SETTING OF GUATEMALA

Scope

In this chapter the general geologic and seismologic setting of Guatemala and surrounding regions is presented. A description of the major plates and plate boundaries is given along with their relationship to seismically active zones. Guatemala has been divided into five geologic and structural regions which are individually discussed. The volcanic activity is described as are major faults and structural trends. This chapter concludes with some suggestions relating to seismic zoning.

II-1 Introduction

Guatemala is located within the circumpacific "Ring of Fire", a zone of intense seismic and volcanic activity. The modern plate tectonics model explains this high level of activity as being associated with the motions of crustal plates and the interactions at their boundaries. Most of Central America lies on the western portion of the Caribbean Plate. However, Guatemala is situated athwart the boundary between the Caribbean Plate to the south and the North American Plate to the north (see Figure 2-1). This boundary is marked by the generally east-west trending Cayman (or Bartlett) trough, a deep submarine fault valley over 4 km. deep. To the west, the landward extension of this trough coincides with the Motagua and Polochic fault zones. The motion along this boundary is predominantly left lateral (opposite side moving to the left) movement (Molnar and Sykes, 1969).

West of the Caribbean Plate lies the Cocos Plate, the boundary between the two being the Middle America Trench. The Cocos Plate is moving generally northeast and being thrust under the Caribbean Plate (Molnar and Sykes, 1969; Jordan, 1975). The Middle America Trench is due to this underthrusting, or subduction, of the Cocos Plate beneath the Caribbean Plate. Other

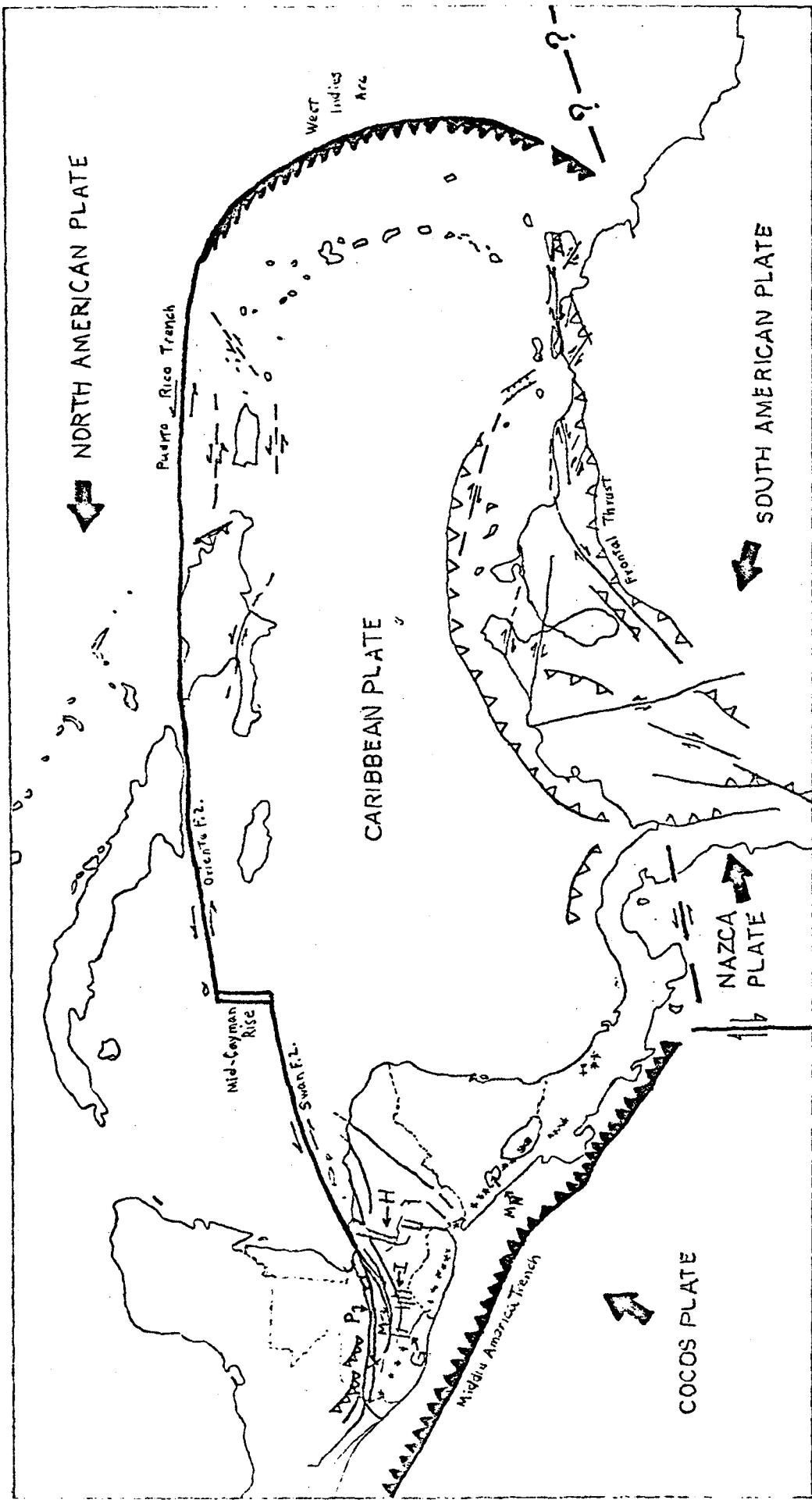


Figure 2-1. Structure and boundaries of the Caribbean Plate.  
 P--Polochic fault zone, M--Motagua fault zone,  
 G--Guatemala City graben, I--Ipala graben, H--Honduras  
 depression, MN--Managua, Nicaragua  
 (From Billieau, 1976, modified from Jordan, 1975)

features normally associated with a "subducting" plate boundary are also observed along the western and central portions of Central America, including a northwesterly, arcuate trending chain of andesitic stratovolcanoes, a band of shallow to intermediate depth earthquake foci, and an oceanic trench as previously mentioned.

The Middle America Trench is a four to five kilometer deep depression located approximately 100 kilometers west of Central America, extending from southern Mexico southward to Costa Rica. Along the landward margin of the trench is a band of intense shallow seismic activity, with earthquake foci increasing in depth to the northeast, defining a northeast dipping slab-like zone. This zone of friction is commonly known as the Benioff Zone.

A chain of Quaternary and active volcanoes parallels the trench and is located about 100 to 200 kilometers northeast of the trench and directly above the earthquakes with focal depths of about 100 to 200 kilometers (intermediate depth foci).

Guatemala is located due east of the junction of the Cocos, Caribbean and North American Plates. This feature is known as a triple junction. The tectonics associated with triple junctions is often quite complex and confusing.

### II-2 Nuclear Central America:

Schuchert (1935) names "the ancient folded and faulted mountain land of Central America" Nuclear Central America. This portion of Northern Central America includes Chiapas of Mexico, Guatemala, Belize, Honduras, El Salvador and much of northern Nicaragua. Guatemala itself can be further subdivided into five "morphotectonic" units, as described by Dengo and Bohnenberger (1969). These morphotectonic units were "established on the basis of their internal constitution and external relief" (Guzman and DeCserna, 1961), and are a combination of the physiographic and tectonic

features of the various subdivisions. The five subdivisions are the Pacific Coastal Plain, the Pacific Volcanic Chain, the Volcanic Ranges and Plateaus, the Sierras of Northern Central America and the Yucatan Platform (see Figure 2-2). Other investigators, such as Bonis (1967), Schuchert (1935), and Brineman and Vinson (1961), have used various different divisions and classification systems. However, the classification system used herein was selected because it appeared to be the most complete and comprehensive.

Pacific Coastal Plain:

The Pacific Coastal Plain is a narrow coastal plain about 50 kilometers wide, located along the Pacific side of Northern Central America, extending from El Salvador northward into southern Mexico. This coastal plain is composed of a thick accumulation of sands, gravels, pumice and bouldary laharic deposits (mudflows of volcaniclastic materials), most of which were derived from the adjacent volcanic chain and highlands. The boundary between this unit and the Pacific Volcanic Chain is thought to be a series of major faults which have been buried under alluvial deposits (Agos, 1958), based on the interpretation of airborne magnetometer data.

Pacific Volcanic Chain:

A chain of active and Quaternary andesitic volcanic cones extends from southernmost Mexico, across Guatemala into El Salvador, paralleling the Pacific Coast with a generally northwest trend.

This precipitous volcanic chain stretches longitudinally across much of Central America, and can be divided into several short segments (see Figure 2-3) based on prominent volcanic lineaments, seismicity and quaternary structures (including faulting) (Carr, 1976; Carr, Stoiber and Drake, 1974). The boundaries separating these different segments are often zones of transcurrent faulting, and commonly appear to be tectonically very active.

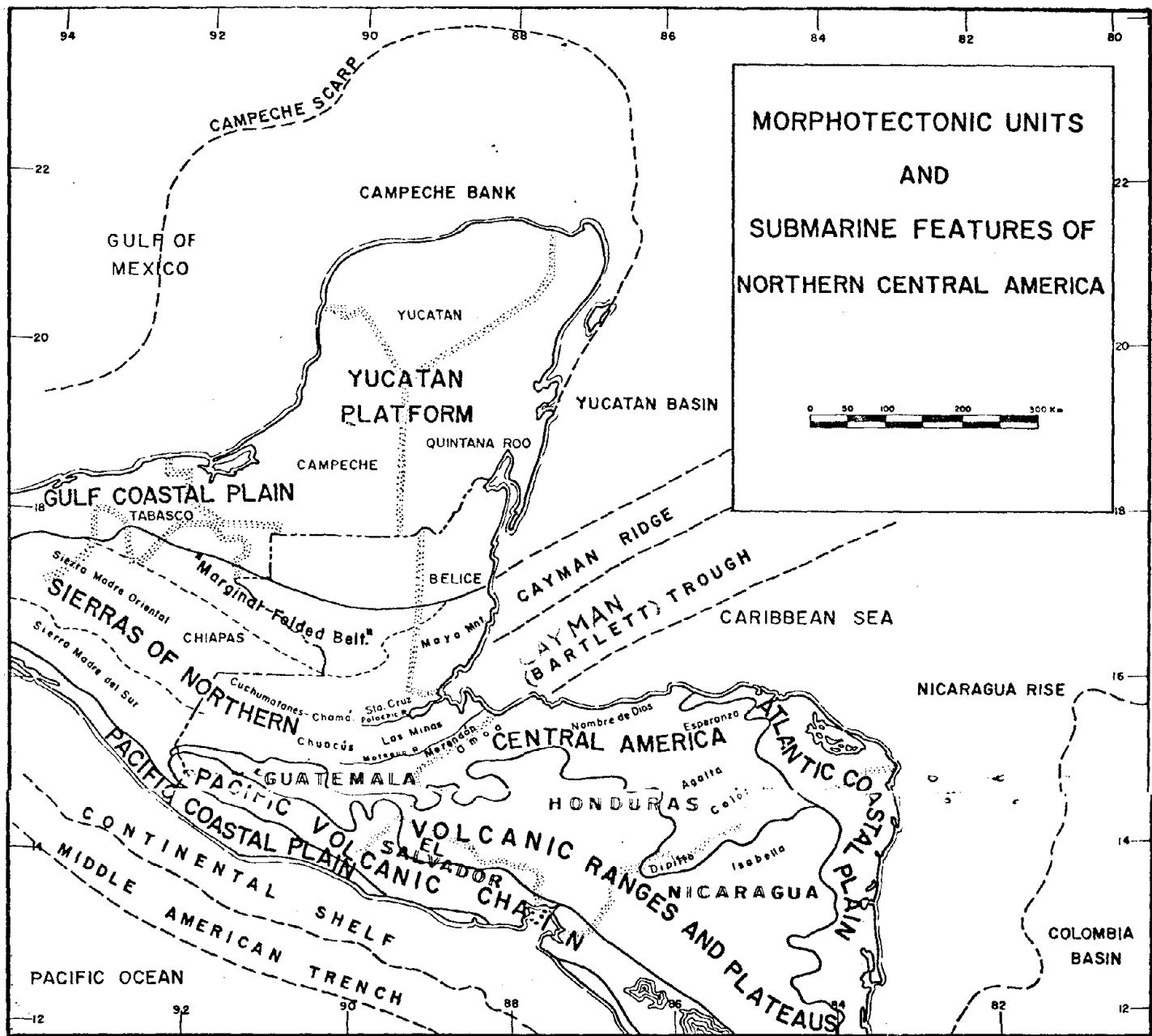
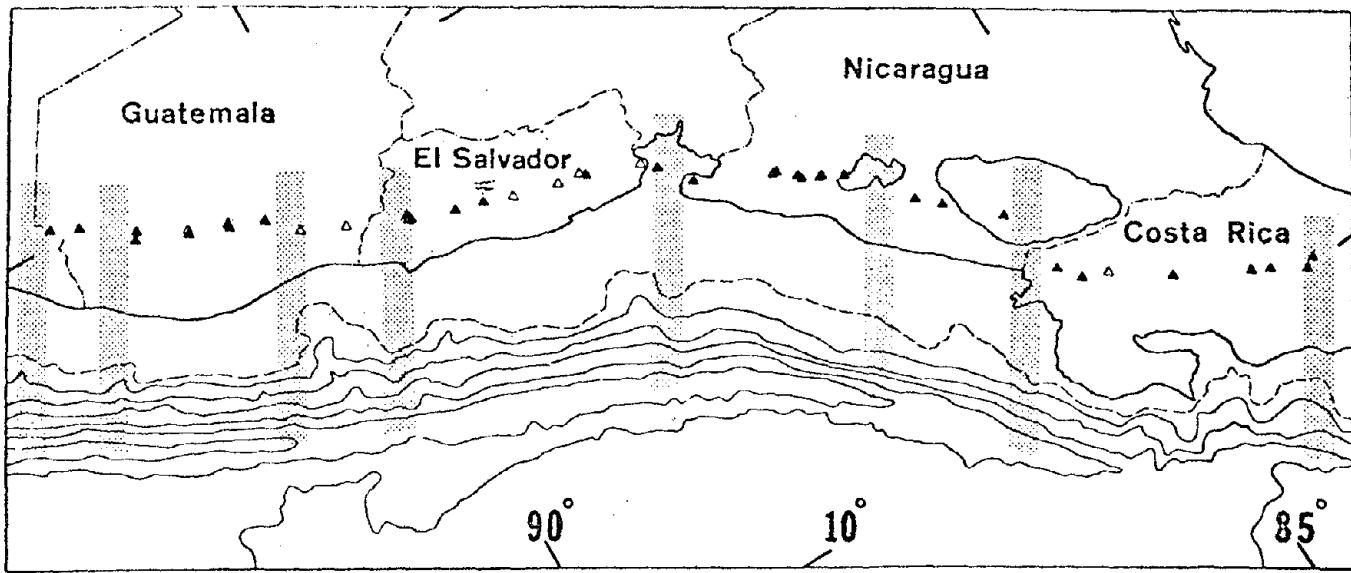
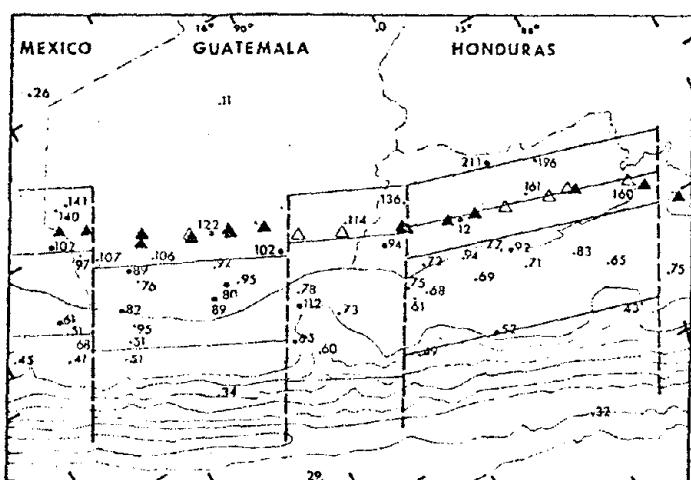


Figure 2-2. Map showing Morphotectonic units and Submarine features of Northern Central America  
(From Dengo & Bohnenberger, 1969)



Volcanic segments of Central America. Solid triangles are volcanoes with historic eruptions (from Mooser et al. 1958), with the addition of Arenal Volcano in Costa Rica. Open triangles are volcanoes with solfatara activity (from Mooser et al., 1958), with the addition of Moyuta Volcano in southeastern Guatemala and the deletion of Zuñil. Bathymetry is from Fisher (1961); the dashed contour is the 100-fathom contour; the next contour is the 500-fathom interval. Stippled areas represent boundaries between adjacent segments.

(From Carr, Stoiber & Drake, 1974)



Seismicity of northern Central America. Focal depths in kilometres are written next to epicenters (dots). Circled dots are epicenters of 15 earthquakes used in JHD. Dashed lines are proposed discontinuities in the inclined seismic zone. Straight lines are isobaths of the deep seismic zone. Contour interval is 50 km and contour nearest the Middle America Trench is the 50-km contour. Unlabeled country is El Salvador. Solid triangles are volcanoes with historic eruptions, and open triangles are volcanoes with solfatara activity (Stoiber and Carr, 1973). Bathymetry is from Fisher (1961). Contour nearest the coast is the 100-fm contour. Next contour is the 500-fm contour, and subsequent contours have a 500-fm interval.

(From Carr, 1976)

Figure 2-3

Volcanic Ranges and Plateaus:

The Volcanic Ranges and Plateaus, or Volcanic Highlands as described by Williams (1960), form the southern part of Northern Central America, extending across southern Guatemala from the Pacific Volcanic Chain eastward into western Honduras and northern El Salvador (see Figure 2-2). The Tertiary (Miocene to Pliocene, or 25 to one million years old) rocks of the highlands are partly lava flows, ranging from basalt to rhyolite, but are more commonly rhyolitic and dacitic pumice deposits with interbedded fluviatile (river deposited) tuffaceous sediments and a few lenses of diatomite (a deposit formed from silica rich microscopic organisms) (Williams, 1960). Most of these deposits are products of fissure eruptions, rather than of large composite cones, which developed later during Quaternary times. The volcanic deposits overlie a rugged surface cut in mid-Cretaceous plutonic rocks (diorites, granodiorites and granites), early Cretaceous limestones and older metamorphic rocks (serpentinites, mica schists and minor phyllitic shales, marbles, quartzites, and calc-silicate contact rocks).

During and after late Pliocene time, these volcanic deposits were faulted and folded, the trends being generally north-south in the north eastern portion and more northeast-southwest to east-west throughout the rest of the highlands.

Quaternary volcanism consists mainly or wholly of basaltic volcanoes and abundant basaltic cinder cones, occurring as parasitic cones on the flanks of the volcanoes, and as independent features often aligned along faults (Williams, McBirney and Dengo, 1964). Quaternary pumice is relatively scarce, compared to that discharged by the volcanic chain to the west, and to those deposits produced during the period of Tertiary volcanism. The Quaternary volcanoes of this region are widely scattered

along fissure systems, often lying on north-south trending faults (Williams, McBirney and Dengo, 1964).

Sierras of Northern Central America:

An arcuate series of subparallel high mountain ranges, convex southward, extends across central Guatemala from Chiapas, Mexico to the Caribbean Sea (see Figure 2-2). The northern ranges of this series appear to be structurally continuous with the submarine topographic high, known as the Cayman Ridge (Dengo and Bohnenberger, 1969). Also the submarine Cayman (Bartlett) trough is continuous structurally with some of the longitudinal valleys which separate several of the mountain ranges, namely the long, narrow fault controlled Polochic and Motagua Valleys.

These mountain ranges can be subdivided, as suggested by Dengo and Bohnenberger (1969) into a northern and a southern group of mountain ranges. The northern group includes the northwest trending Sierra Cuchumatanes, the east-west trending Sierra de Chama and the generally east-northeast trending Sierra de Santa Cruz Range. The Maya Mountains of Belize are also included in this northern group of mountains. The southern group of ranges includes the Sierra de Chuacus, which trends nearly east-west, the east-northeast trending Sierra de las Minas and Sierra de Merendon Ranges and the El Mico Mountains. The Sierra de Santa Cruz and Sierra de las Minas ranges are separated by the Polochic Valley, whereas the Sierra de las Minas and the Sierra de Chuacus are separated from the Sierra de Merendon by the Motagua Valley.

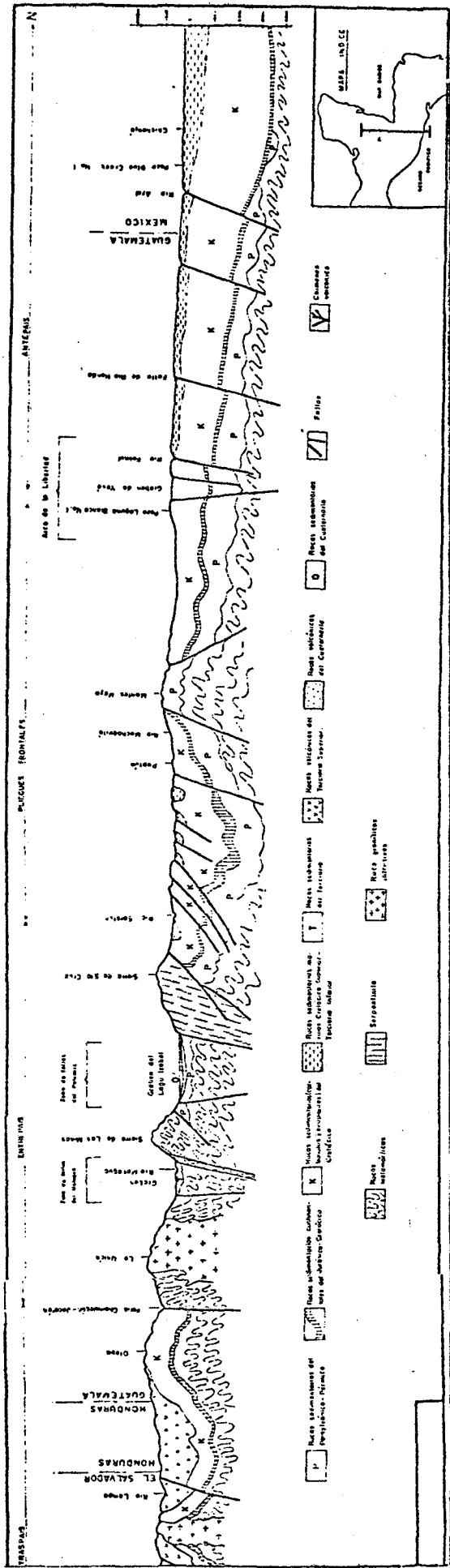
The northern group of mountain ranges are composed primarily of Permian (270 to 220 million years old) and Cretaceous (135 to 70 million years old) limestone which now forms a "series of parallel, tightly folded ranges thrust faulted toward the north and modified by later normal

"faults" (Dengo and Bohnenberger, 1969) (see Figure 2-4). These thrust faults trend subparallel to the arcuate trend of the mountain ranges and dip to the south. The normal faults are vertical or steeply dipping and often trend approximately north-south, truncating and offsetting the reverse (thrust) faults (Williams, 1960).

Late Cretaceous and Early Tertiary marine clastic sediments of the Sepur formation are preserved in the synclinal troughs and fault controlled valleys of the northern ranges (Bonis, 1967). This formation consists mainly of shales, calcarenites (calcium carbonate rich sandstone) and conglomerates. Pre-Mesozoic sedimentary rocks (sandstone, shale, conglomerate and phyllite of the Santa Rosa Group) are found in the Maya Mountains, and Sierra de los Cuchumatanes, as are Paleozoic metamorphic rocks and Jurassic-early Cretaceous continental and marine redbeds (mainly sandstone and conglomerate) of the Todos Santos formation (Bonis, 1967). Ultramafic rocks, predominantly serpentinites, also occur in the northern ranges, making up much of the Sierra de Santa Cruz (Dengo and Bohnenberger, 1969).

To the north the rocks are less intensely folded and thrust faulted, forming an intermediate zone between the main fold belt of the high sierras to the south and the nearly flat lying rocks of the Yucatan Platform to the north. This series of low ranges and intervening lowlands makes up the marginal folded belt (see Figure 2-2).

The southern group of mountain ranges consists primarily of pre-Pennsylvanian (before about 300 million years ago) schists, gneisses, amphibolites and marbles of the Chuacas Series (McBirney, 1963), granitic batholiths and ultramafic rocks, mainly serpentinites. Paleozoic and Mesozoic sedimentary rocks (carbonates, shales, sandstones, conglomerates, and phyllites) occur only locally within these ranges. Within the Motagua



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Figure 2-4. Generalized geologic cross section through Guatemala, showing the different surface and subsurface rock units and the major structures. (From Dengo, 1968)

Valley, and bounded by the Motagua fault system are Cretaceous to Eocene (135 to 40 million years old) and redbeds of the Subinal formation. Also within the Motagua Valley, and other intervening valleys, are small areas of Quaternary pumice deposits.

The faulting in the southern group of mountain ranges is less dominated by east-west trending, south dipping thrust faults, but rather by a few major left lateral strike-slip faults, namely the Cuilco-Chixoy-Polochic fault, the Motagua fault and the Jocotan-Chamelecon fault to the south.

#### Yucatan Platform:

The Yucatan Platform, or Peten Lowland (Bonis, 1967) extends across northern Guatemala from Tabasco, Mexico into northern Belize (see Figure 2-2). The southern limit of the Yucatan Platform is marked by the La Libertad Arch, (Vinson and Brineman, 1961). Nearly flat lying Paleozoic and Eocene marine sediments cover much of the northern portion of the area, overlying Cretaceous carbonate and evaporite rocks, which crop out extensively in the southern part of the Yucatan Platform.

An extensive karst topography has developed on the carbonate rocks, including underground drainages and large caverns. Collapse of these caverns has produced earthquakes which commonly have shallow epicenters and fairly local felt areas (Lomnitz, 1974).

#### II-3 Volcanic Activity:

Quaternary volcanic activity is restricted to the southern part of the country, occurring along the Pacific Volcanic Chain and in the Volcanic Ranges and Plateaus region, east of the main chain. The types of Quaternary deposits include andesitic and basaltic flows, and pumice deposits laid down by torrential floods which removed airborne pumice falls from the

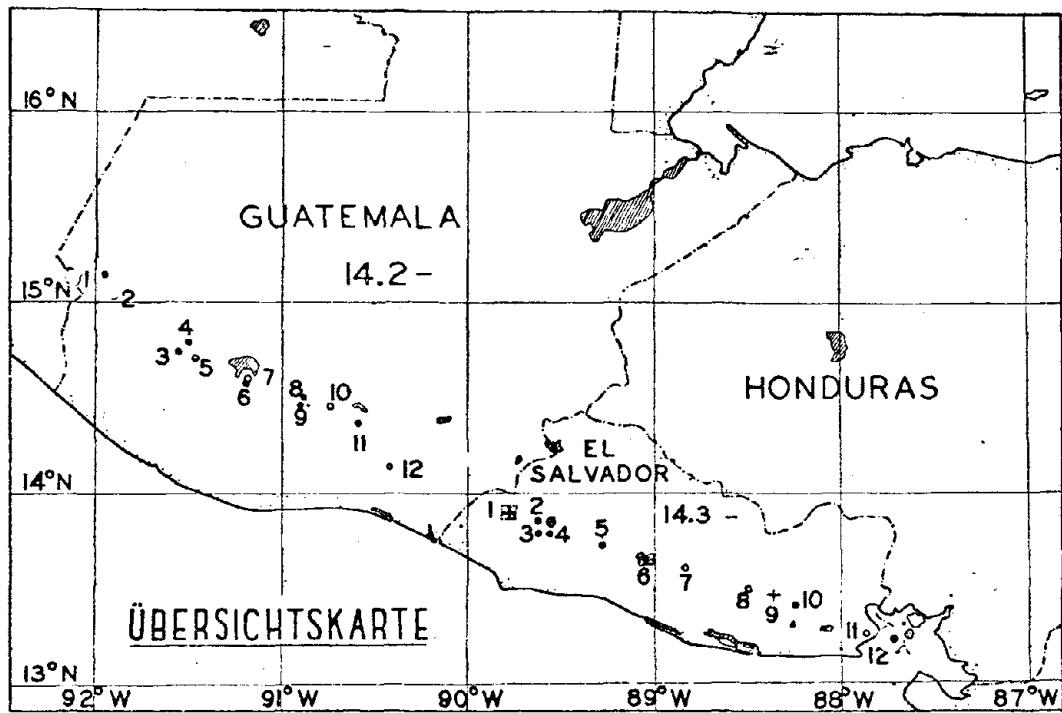
surrounding hills, and less commonly the product of glowing avalanches.

The principal group of volcanoes in Guatemala (see Figure 2-5) lie in a nearly straight line for 122 kilometers from Volcano Siete Orejas to Volcano Pacaya, at which point the line splits with one strand to the north linking the twin volcanoes of Lake Ayarza, the other strand, offset several kilometers to the south, links volcano Tecuamburo and Volcano Moyota. To the north, near the Guatemala-Mexico border, lie Volcan Tacana and Volcano Tajumulco, which are offset to the north of the main chain. Near the Guatemala-El Salvador border, the cones are more widely scattered and irregularly distributed.

Activity within the past few centuries has been recorded from several volcanoes. Cerro Quemado, south of Quetzaltenango, erupted most recently in 1785 with fumerolic activity noted by recent investigators. In 1853, Volcano Atitlan, south of Lake Atitlan, erupted. This volcano had had prior periods of explosive activity, however, no recent eruptions have been recorded. Volcano Pacaya, located south of Guatemala City has been intermittently active over the last several hundred years. The two most active volcanoes in Guatemala are Volcano Fuego, nearly 40 kilometers southwest of Guatemala City, and Santiaguito on the southern flank of Volcano Santa Maria, about eight kilometers south of Quetzaltenango. Both of these volcanoes have continued to show signs of activity up to the present. The basaltic cinder cones and related flows of eastern Guatemala are thought to be no more than a few thousand years old (Williams, 1960).

#### II-4 Faulting:

Guatemala can be subdivided into three major structural regions, each characterized by different types, orientations and intensities of faulting and folding. These regions generally coincide with some of the



Index map showing the sites of the volcanic centres of Guatemala and El Salvador.

- Figure 2-5.
- |                  |                 |
|------------------|-----------------|
| 1. Tacaná        | 7. Toliman      |
| 2. Tajumulco     | 8. Acatenango   |
| 3. Santa María   | 9. Fuego        |
| 4. Cerro Quemado | 10. Agua        |
| 5. Zuñil         | 11. Pacaya      |
| 6. Atitlán       | 12. Tecuamburro |

major morphotectonic subdivisions of Dengo and Bohnenberger (1969), and include: the south dipping thrust faults and normal faults of the northern group of mountain ranges and the Yucatan Platform; the east-west longitudinal strike-slip faults of the southern group of mountain ranges; and the normal and transcurrent fault zones of the Volcanic Ranges and Plateaus and Pacific Volcanic Chain. This transcurrent fault zone includes N30-45°E trending left lateral strike-slip faults, a conjugate set of N45-65°W trending normal and right lateral strike-slip faults, plus a set of normal (extensional) faults trending generally north-south. These various regions, and their fault zones, may be the result of different stress systems operating at different times, or one stress system of very large dimensions, portions of which have been active at different times (Dengo and Bohnenberg, 1969).

The northern system of thrust faults and subsequent normal faults are thought to be of late Cretaceous and Tertiary age. The thrust faulting began during late Cretaceous times and climaxed in early Eocene time. Uplift of the Yucatan Platform and adjacent regions took place during late Eocene and Oligocene times (50 to 25 million years ago). The normal faulting is thought to have occurred during this period of uplift. Present day earthquakes in this region are more commonly due to limestone cavern collapse, as previously mentioned, rather than of tectonic origin (Lomnitz, 1974).

The arcuate (convex southward) generally east-west trending longitudinal faults of the southern mountain group make up the second subdivision. This includes from north to south, the Cuilco-Chixoy-Polochic fault zone, the Motagua fault zone and the Jocotan-Chamelecon fault zone (see Figure 2-6). The Polochic fault zone extends from the Caribbean Sea to west of the Guatemala-Mexico border, stretching nearly 400

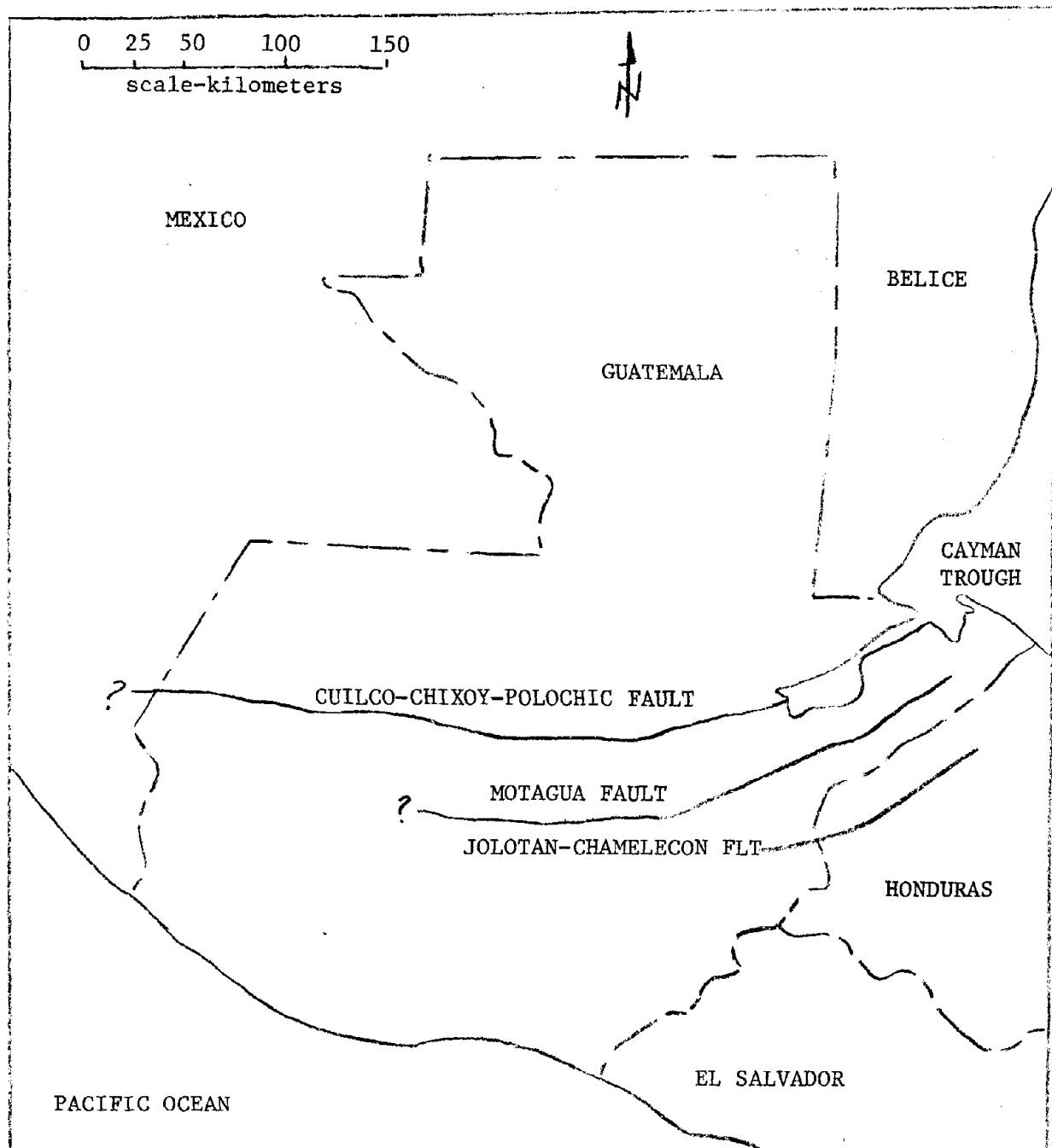


Figure 2-6. Map of Guatemala, showing the locations of the major longitudinal fault zones.  
 (Modified from Anderson and others, 1973)

kilometers, whereas the Motagua fault zone extends from near the Caribbean Sea to the west nearly 300 kilometers. The western extent of the Motagua fault is uncertain because Quaternary volcanic deposits of the Pacific volcanic belt conceal the fault trace (Anderson, 1973; Dengo and Bohnenberger, 1969). The Jocotan-Chamelecon fault zone trends generally northeast-southwest, subparallel to the eastern half of the Motagua fault zone, and extends across northern Honduras into eastern Guatemala, a distance of 125 kilometers. The Motagua and Polochic faults are the landward projection of the Cayman trough, a deep submarine trough which marks the northwest boundary of the Caribbean Plate, as mentioned previously (see Figure 2-1).

Until recently, movements along the Cuilco-Chixoy-Polochic, Motagua and Jocotan-Chamelecon fault zones were thought to be in the vertical direction, with only minor left lateral displacement, based on geologic field evidence in Guatemala. However, many investigators are now postulating large magnitude left lateral strike-slip faulting along these fault zones, suggested by the linearity and width of the fault zones, offset streams, geomorphic features such as beheaded valleys, shutter ridges and chains of scarples (Kupfer and Godoy, 1967) and a series of inverted open S-shaped folds in Late Tertiary sediments deposited in the Polochic fault zone, which show drag features close to the faults (Dengo and Bohnenberger, 1969). Left lateral transcurrent faulting is further supported by the modern plate tectonics model for this area, earthquake focal mechanism studies (Molnar and Sykes, 1969), and by the displacements observed along the Motagua fault zone following the 1976 Guatemala earthquake (Plafker & Bonilla, 1976).

Recent activity along these major faults is indicated by the abundance of fault scarps and offsets of recent geomorphic features, as well

as the occurrence of earthquakes with epicenters along the faults. A more complete discussion of the 1976 rupture along the Motagua fault is given in Appendix C.

Quaternary faulting in the third region includes three major fault trends; transcurrent N30-45°E faults, right lateral and normal N45-65°W faults, and generally north-south extensional faults (Carr, 1976). These faults are most readily observed in the Volcanic Ranges and Plateaus region of Figure 2.

The two major left lateral, northeast trending faults of this region, as described by Carr (1976), are the Palin Shear Zone and the Rio Paz Shear Zone, about 75 kilometers to the southeast of the Palin Shear Zone. Carr suggests a relationship between these left lateral faults and the segmentation of the volcanic chain to discontinuities in the inclined seismic zone, or Benioff Zone (Carr, 1976).

The northwest trending faults may be a conjugate system to the northeast trending faults. Carr (1976) suggests that they often have right lateral motion, however, many also show a component of normal displacement (vertical movement) (Williams, McBirney and Dengo, 1964). One example of these faults is the Median Trough in El Salvador. The Jalapatagua fault of southeastern Guatemala, southeast of Guatemala City, exhibits right lateral strike-slip motion. Northwest trending faults have also been noted to be associated with, and in close proximity to, the volcanic chain of the Pacific.

The third principal fault orientation of this region is in the north-south direction. These faults are primarily normal faults, producing north-south trending grabens (down-dropped blocks), such as the Guatemala City and Ipala grabens. These faults also tend to control the distribution of Quaternary volcanics. These north-south trending faults

become much more dominant in the northern and northeastern portion of the Volcanic Ranges and Plateaus unit, whereas to the south, all three fault trends occur together.

All three fault trends of this southern region show signs of recent activity, based on geomorphic evidence and the cross cutting relationships of the various faults. Recent activity along the north-south faults is demonstrated by the occurrence of north-south trending scarps offsetting Quaternary pumice deposits and fresh cinder cones (Dengo and Bohnenberger, 1969). The Mixco fault (see Figure 2-7), west of Guatemala City, exhibited dip-slip movement (Plafker & Bonilla, 1976) in association with the February 4, 1976 Guatemala earthquake, further supporting that the north-south trend is still active (Appendix C).

Seismic zoning, based on a thorough understanding of the nature of faulting, the tectonic history and present day regime, and other critical factors, is essential for safe growth in a seismically active region, such as Guatemala. A few important points relating to seismic zoning follow:

1. Numerous active and potentially active faults are observable in Guatemala, as described above. Recognition of these active faults is not as difficult in Guatemala as in other countries, because of the abundance of Quaternary volcanic deposits (for dating the faulting), and the existence and preservation of geomorphic features such as fault scarps, fault valleys, etc., as well as other factors. Not all of the active faults ruptured during the 1976 Guatemala earthquake. Therefore, in seismic zoning, all of the active faults should be considered capable of rupturing during future earthquakes, except where specific studies suggest otherwise.

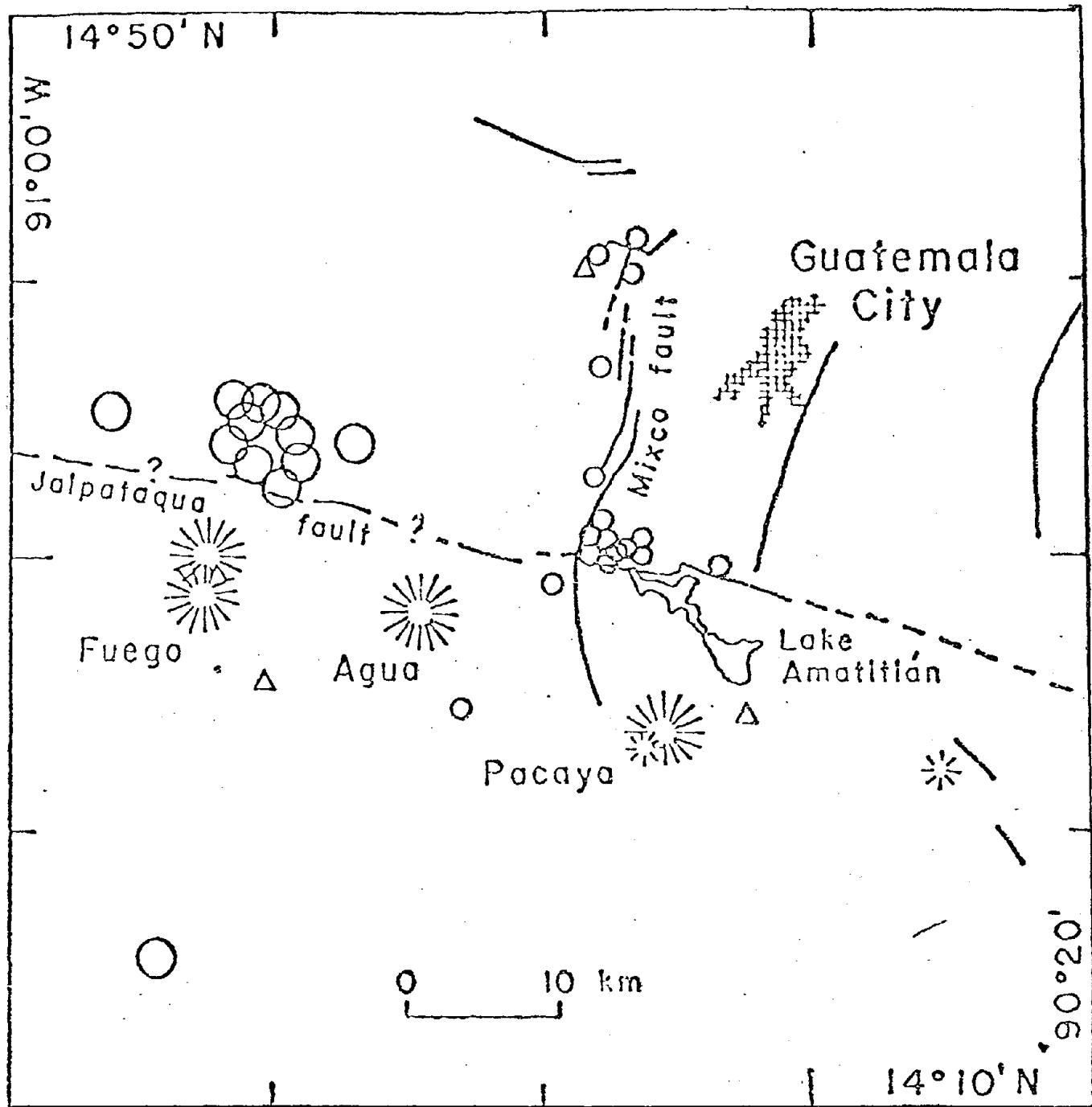


Figure 2-7. Location of Mixco Fault. Open circles are epicenter locations from 1973-1974. The size of the circles reflects the estimated error in the calculated location.

2. It should be noted, as was observed during the 1976 Guatemala earthquake, that several apparently unrelated faults can rupture and/or show other signs of activity during a single event. Therefore, recognition of more than the dominant orientation of active faulting is necessary for safe seismic zoning.

3. The definition of an "active" fault is not yet agreed upon. The number of years since the most recent movement along the fault, before it can be considered "inactive", is still unresolved. Many geologists use 40,000 years since the last movement as the criterion of "active". The valid argument has been made that even 10,000 years or less is economically impractical when considering structures with an anticipated lifetime of less than 100 years. A definition developed by Nichols and Buchanan-Banks (1974) is that "faults are regarded as active and of concern to land-use planning when there is evidence that they have moved during historic time or, through geologic evidence, there is a significant likelihood that they will move during the projected use of a particular structure or piece of land". An understanding of the recurrence interval and nature of faulting is necessary in selecting a minimum age since the most recent movement. However, it is our opinion that strategically important and large public structures should not be located over defined faults, unless the most conservative (40,000 years) definition of "active" is used.

4. The faults that are designated as "active" should be considered as zones, rather than a line of movement. It has been observed that certain faults (such as the San Andreas fault in California) tend to rupture repeatedly along nearly the same line (Nichols and Buchanan-Banks, 1974). However, in some regions many recent fault scarps are located

within wider fault zones; while in alluvium, the fault rupture could occur outward from the previously "recognized" break. The width of this "zone" depends on the nature of the faulting, the orientation of the fault plane, and the materials being faulted, along with several other factors. Once the fault zone can be reliably delineated, it is our opinion that at least the vital structures, such as hospitals, police and fire stations, etc., should be located at least 30 to 45 meters from these fault zones.

5. The type of material on which the structure rests, and the degree of saturation of the material are extremely important.

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## CHAPTER III

### SEISMIC DATA BASE

#### Scope

This chapter describes the available seismological data for the country. Its characteristics and limitations are discussed. Based on the fault locations, eighteen line sources are modeled, their seismicity determined and the confidence on the recurrence relationship presented.

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#### III-1 Introduction

A difficult but essential task for the current study is the acquisition of past earthquake data. In general it is necessary for the data to contain information on the severity of the earthquake, the epicentral location, the focal depth to the hypocenter, and the time of occurrence.

The most commonly used parameters representing the severity of the seismic events are the Modified Mercalli intensity, the Richter magnitude and the peak ground acceleration. The Modified Mercalli intensity is a subjective scale of the damage at a site. Forecasting of MM intensities can be useful in determining the future damage potential in a region, however they cannot be related to structural response characteristics for analysis and design purposes. Thus, in the present study MM intensity data will be used only when it can be translated into either a Richter magnitude or a peak ground acceleration.

The Richter magnitude is related to the overall energy release of a seismic event. It is important to note that the Richter magnitude conveys

information only at the epicenter (or hypocenter). The parameter popular among earthquake engineers which represents the ground motion at a site is the peak ground acceleration. It is frequently used for analysis and design of structures, consequently it will be employed in the present report as the parameter for determining the "seismic loads" at a site. Recently other load measuring parameters such as root-mean-square of ground motion, stress drop, and seismic moment have become popular among seismologists and engineers. However these parameters have been determined only for a few past earthquake events and thus cannot be used in the present study.

In addition to the above specified characteristics of the data base, it is desirable to have strong motion accelerograph records from major earthquakes and also information on how the energy (or the peak ground acceleration) attenuates from the source to the site. Only a few accelerograms of very low acceleration amplitudes are available for the country and hence are hardly sufficient for determining the attenuation characteristics in Guatemala.

Before carrying the discussion further into the analysis and use of the available data certain observations should be made regarding the type, the amount and the reliability of the data base used in the current work.

A shortcoming common to all the data sources is that the frequency of recorded earthquakes increases with time, (i.e. the number of seismic events recorded increases with each year). This nonhomogeneity in the data base is due not because there is necessarily an increase in the seismic activity in the region, but because of the improvement of instruments, measurements and measurement analysis techniques resulting in a bias towards recent years. This non-uniformity in the data can be observed to be true for all historical-time-dependent records and is a "fact of

life." Thus one cannot get away from it.

The other problems associated with the data base deal specifically with the variability of the epicentral location, the Richter magnitude and the focal depth. Variations in epicentral location are dependent on the seismological station reporting the earthquakes. For earlier seismic events only the general region of the earthquake occurrence is reported where on occasions it encircles an area of 100 km radius. Furthermore, for many of these events the focal depth is not determined due to lack of seismograph records close to the epicenter. In cases where a focal depth is reported, its reliability can also be questioned. Similarly, the value of Richter magnitude is not always recorded and when recorded it contains certain error. The variability of the data and its effect on the modeling has been extensively studied (Kiremidjian and Shah, 1975) previously. An assessment of the uncertainties of the data associated with the present study will be made at the end of this report.

### III-2 Data Analysis

Past earthquake data was collected for a region spanning from 12.5°N to 19.5°N and from 86.5°W to 93.5°W. These boundaries are chosen so that they are at least one degree (longitude or latitude) outside of the border of Guatemala. The two main sources of information that are considered for data acquisition are:

- The NEIC-NOAA data file containing events from January 1900 to August 1973 and referred to hereafter as Data File 1;
- The Preliminary Data Epicenters of the NEIC-USGS covering the period from September 1973 to February 1976, and referred to hereafter as Data File 2.

In addition, the events reported by the Bulletin of the Seismological Society of America and the Observatorio Meteorologica Nacional of

Guatemala were used to check the completeness of the data. Information on the major shock of the February 4, 1976 earthquake and its aftershocks was obtained from the United States Geological Survey (Page ed., 1976) and records of the Observatorio Meteorologico Nacional. In spite of the complementarity of the different data sources, a large number of events remain insufficiently documented in order to be used as such in the analysis. Thus in cases where basic information such as epicentral location or magnitude is missing, the event is disregarded. Records with a Richter magnitude smaller than 3.0 are also not considered.

Both Date Files 1 and 2 are very similar thus they will be discussed together. When complete, the information for a given seismic event contained in these files includes the time of occurrence, the epicentral location (degrees longitude and latitude), the depth to the hypocenter (km), and the magnitude. The magnitude is given in terms of one of the following:

- (1) CGS  $M_b$  average (body wave magnitude)
- (2) CGS  $M_s$  average (surface wave magnitude)
- (3) Richter magnitude  $M$ .

For the study at hand, it is necessary that all the data be expressed in terms of a single magnitude parameter. The Richter magnitude is chosen for that purpose and whenever only  $M_b$  values are reported these values are converted to Richter magnitude. It is known that for a given part of the world, the Richter magnitude and CGS  $M_b$  are linearly related such that

$$M = a + b M_b$$

3-1

In order to determine the coefficients  $a$  and  $b$ , a regression analysis was performed for all earthquakes of which  $M$  and  $M_b$  were known using the

total data of Central America. The Richter magnitude was then obtained by substituting the value of  $M_b$  in Equation 3-1.

The focal depth given in these files is expressed either in kilometers or by a letter symbol N (0 - 33 km).

From Data File 1, 979 events were reported and from Data File 2, 174 events were obtained, thus constituting a total sample of 1153 past earthquake records containing complete information. The epicenters and the magnitudes of these events are shown on Chart 1. Table 3-1 lists these records as a function of focal depths. A listing of all the data appears in Appendix D.

The general seismic pattern of Guatemala is studied from Charts 1 to 6. The country is divided into the following regions of tectonic activity:

(1) The Benioff Zone dipping northeast under the Guatemalan coast. This zone is marked by a numerous earthquakes covering the entire range of magnitudes. It extends several hundred kilometers beneath the earth's surface. Some of the deepest earthquakes recorded along the Benioff Zone go as far as 299 km projecting around the central portion of the country. The Benioff Zone in this part of the Central Americas has been observed (Carr, 1976) to start out with an extended shallow section and then dip at about  $40^\circ$  angle under the coast of the country. This observation is confirmed by looking at Charts 2 to 6. The range of shallow epicenters extends from the Middle American trench almost to the coastline forming a rather diffused zone.

(2) The Motagua and Polochic fault zones are associated with and extend into the Cayman (Bartlett) trough. In this region of the country primarily shallow earthquakes are observed (5 - 33 km) and the magnitudes are found up to 7.5 on the Richter scale. However, due to the lengths of these

TABLE 3-1

Earthquake Data Sorted According to Depth of Hypocenter

<u>Number of Earthquakes</u>	<u>Depth Range (km)</u>
81	0 - 9
37	10 - 19
34	20 - 29
298	30 - 39
53	40 - 49
65	50 - 59
116	60 - 69
87	70 - 79
72	80 - 89
60	90 - 99
63	100 - 109
39	110 - 119
27	120 - 129
18	130 - 139
13	140 - 149
18	150 - 159
11	160 - 169
8	170 - 179
6	180 - 189
10	190 - 199
20	200 - 219
7	220 - 239
5	240 - 259
4	260 - 279
1	280 - 299

fault zones a maximum magnitude of 8.5 is believed to be possible on either of them. This information is directly incorporated in the seismic modeling of the country (see Table 3-2). These faults are of particular importance because of their capability for large magnitude earthquakes, their shallowness and their proximity to highly populated areas. A smaller, but still important fault that can be linked with these zones is the Jocotan. Its characteristics were already discussed in Chapter II.

(3) The line of volcanoes from northwest to southeast (Cordillera de los Marrabios) represents sources of future seismic activities. Volcanic eruptions are seldom by themselves sources of seismic activity, and in the past various earthquakes have been recorded preceding volcanic eruptions. For this reason earthquakes "associated" with volcanic activity were treated in the model (Chapter II) as shallow tectonic earthquakes.

(4) Several shallow and short faults have been found in the vicinity of Guatemala City of which the Mixco has been observed to have the most recent movement. Their geologic characteristics differ from the major fault zones considered (see Chapter II). Only a few earthquakes have been associated with the Mixco fault, but their severity is of particular concern because of its nearness to Guatemala City.

(5) The Yucatan Platform is observed to have a very low seismicity. In this region no earthquakes of tectonic origin have been recorded.

### III-3 Source Location and Seismicity

Based on the above observations, the total number of events was divided into 18 seismic line sources. Table 3-2 identifies the sources, the number of events and the focal depth range for each source. Charts 2 to 6 show the epicenters of past earthquakes associated with each source. Most of the sources are composed of several line segments in

TABLE 3-2

Seismic Sources For Guatemala

SOURCE	NUMBER OF EVENTS	NAME OF SOURCE	DEPTH (km)
S1	56	Motagua	33
S2	16	Polochic	40
S3	31	Line of Volcanoes	30
S4	67	Pacific Coastline	30
S5	19	Pacific Coastline	34
S6	5	Near Mexico Border	30
S7	7	Near Mexico Border	35
S8	16	Mixco	30
S9	17	"Mexico"	60
S10	10	"Mexico"	75
S11	7	Jocotan	30
S12 - B1	244	Benioff 1	30
S13 - B2	87	Benioff 2	50
S14 - B3	192	Benioff 3	70
S15 - B4	196	Benioff 4	90
S16 - B5	111	Benioff 5	130
S17 - B6	35	Benioff 6	180
S18 - B7	37	Benioff 7	225

order that they fit the curvature of a given fault zone. Appendix D gives a listing of the earthquakes included in each source. The depth of each source was computed as an average hypocentral depth of all events included in the sources. Earthquakes with no or limited information were not included in this averaging process. However, they were considered in determining the location and the seismicity of the sources.

The recurrence relationship for each individual source was obtained by fitting a regression line through the data for that source and obtaining

$$\ln N(M) = \alpha + \beta M$$

3-2

where  $N(M)$  = Number of events above magnitude  $M$

$M$  = Richter magnitude

$\alpha$  and  $\beta$  are regression constants.

$\alpha$  is a measure of the number of events above magnitude 0 for a given source and  $\beta$  is a measure of the seismic severity for a given source. The larger the negative value of  $\beta$ , the smaller the seismic severity. For many sources, a single regression line gave erroneous results because the interpolation of the line beyond the range of data indicated unreasonably high magnitude occurrences. For such cases, two regression lines were fitted to the data, and a geologically consistent upper magnitude value was used for cutoff. (See Figures 3-1 through 3-18.) Table 3-3 gives a summary of  $\alpha'$  and  $\beta$  values for each source and the magnitude cutoff point for each source.

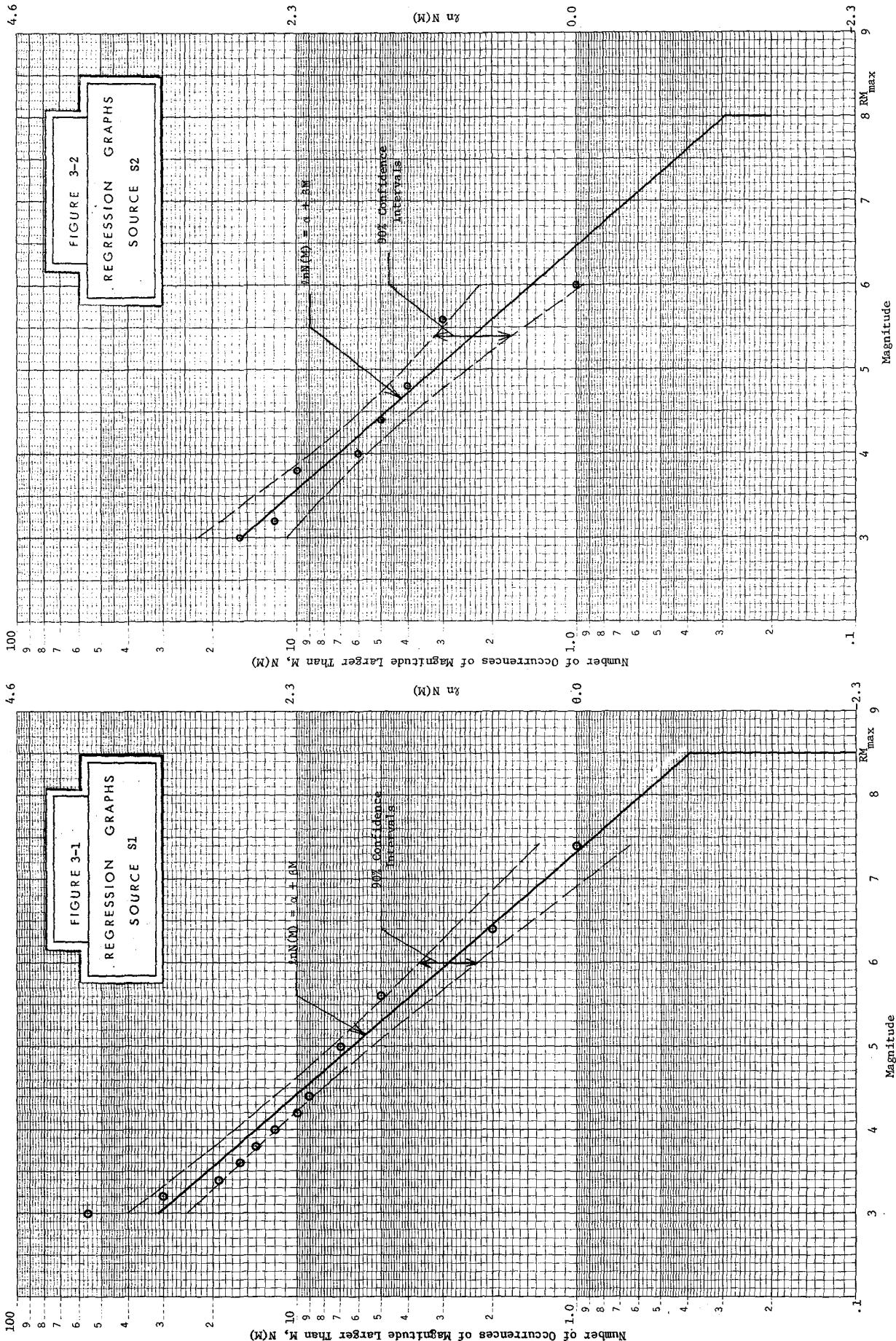
$$\text{Let } N'(M) = \frac{N(M)}{AT} \text{ for an area source}$$

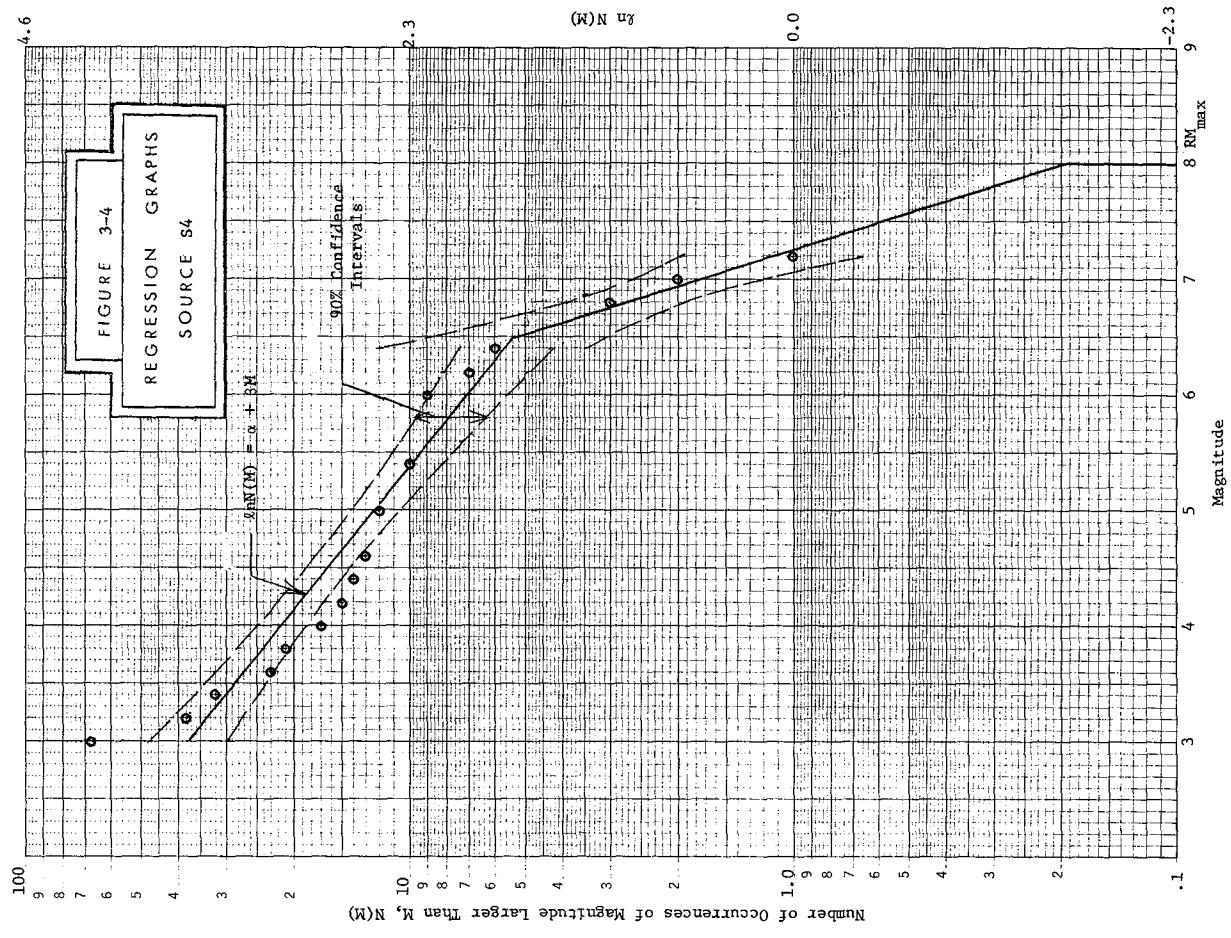
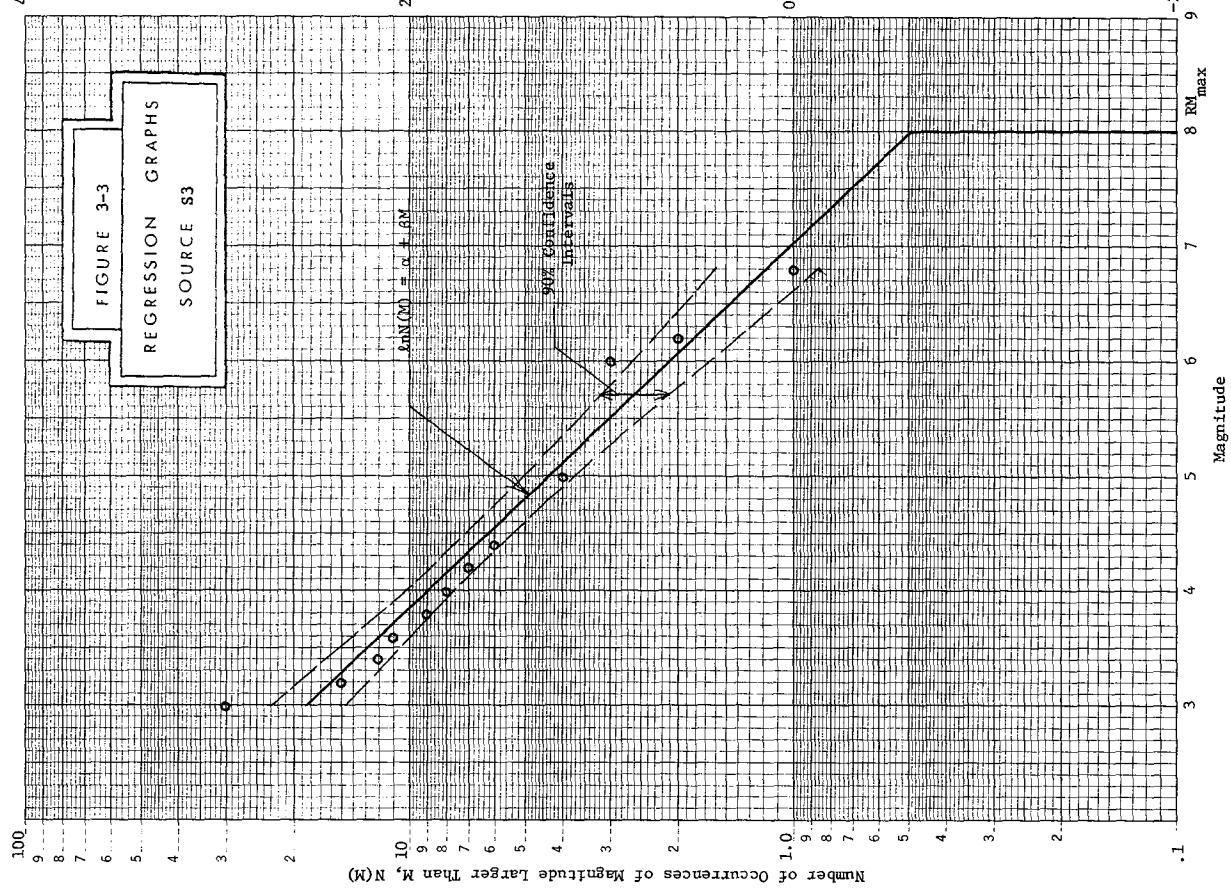
3-3a

$$= \frac{N(M)}{LT} \text{ for a line source}$$

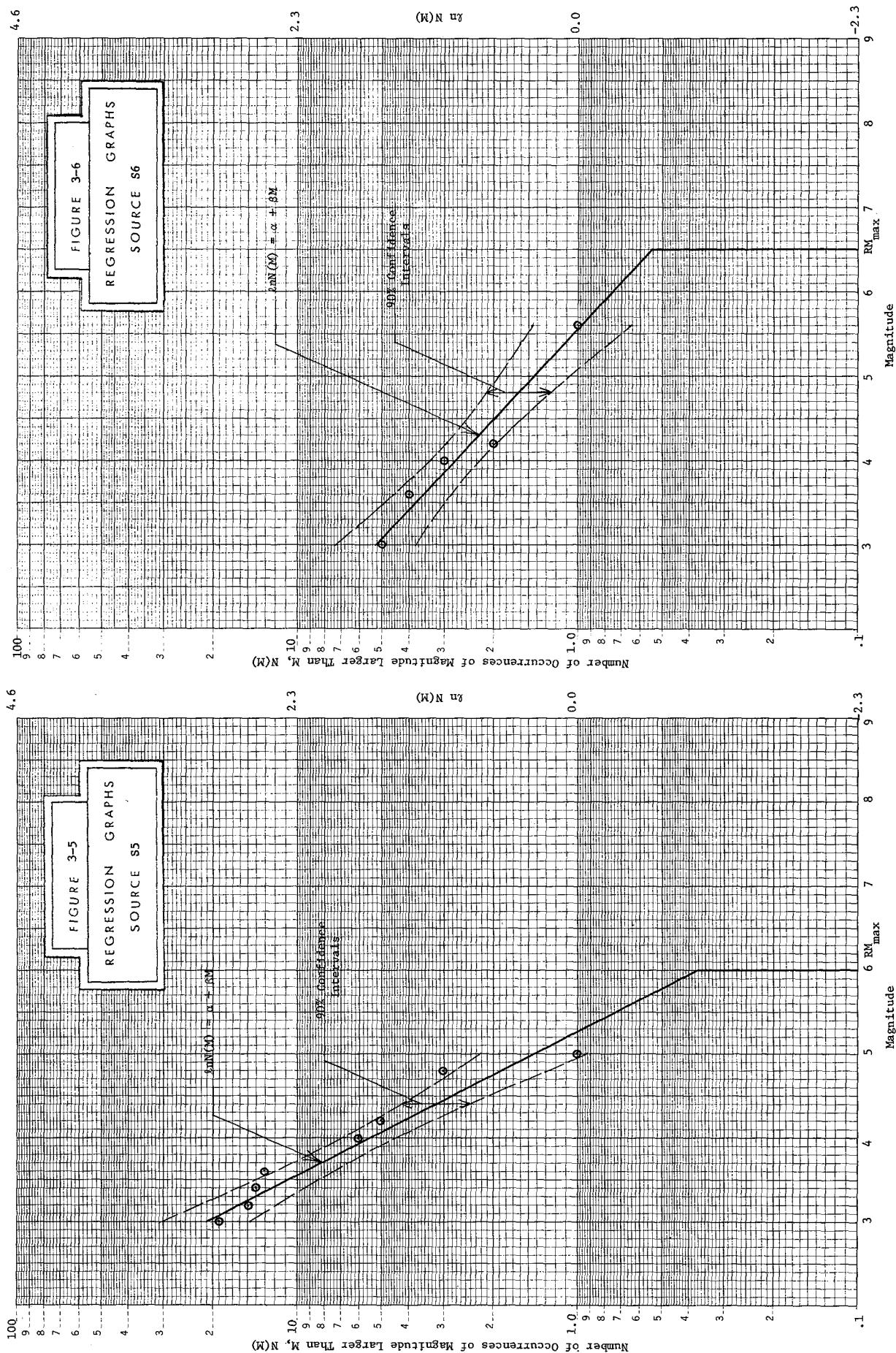
3-3b

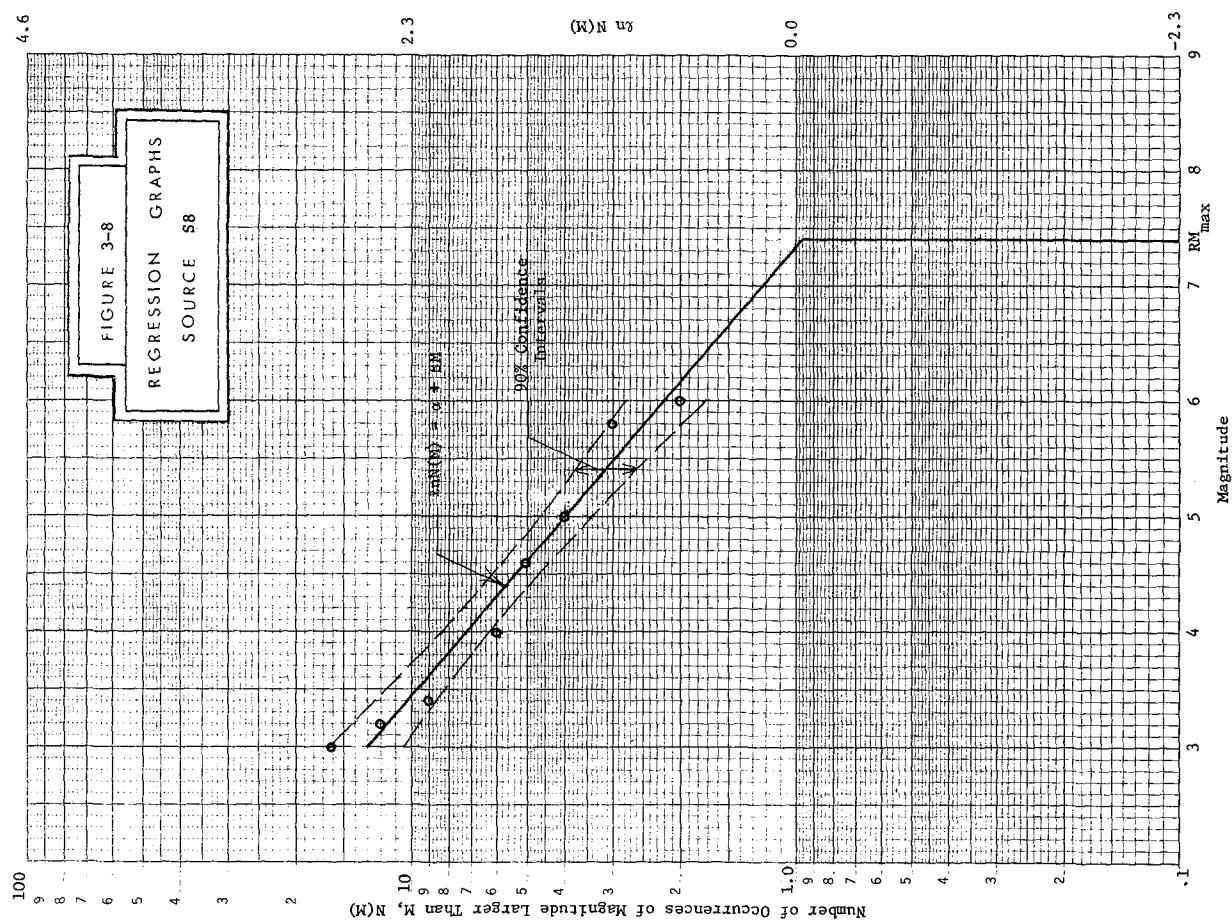
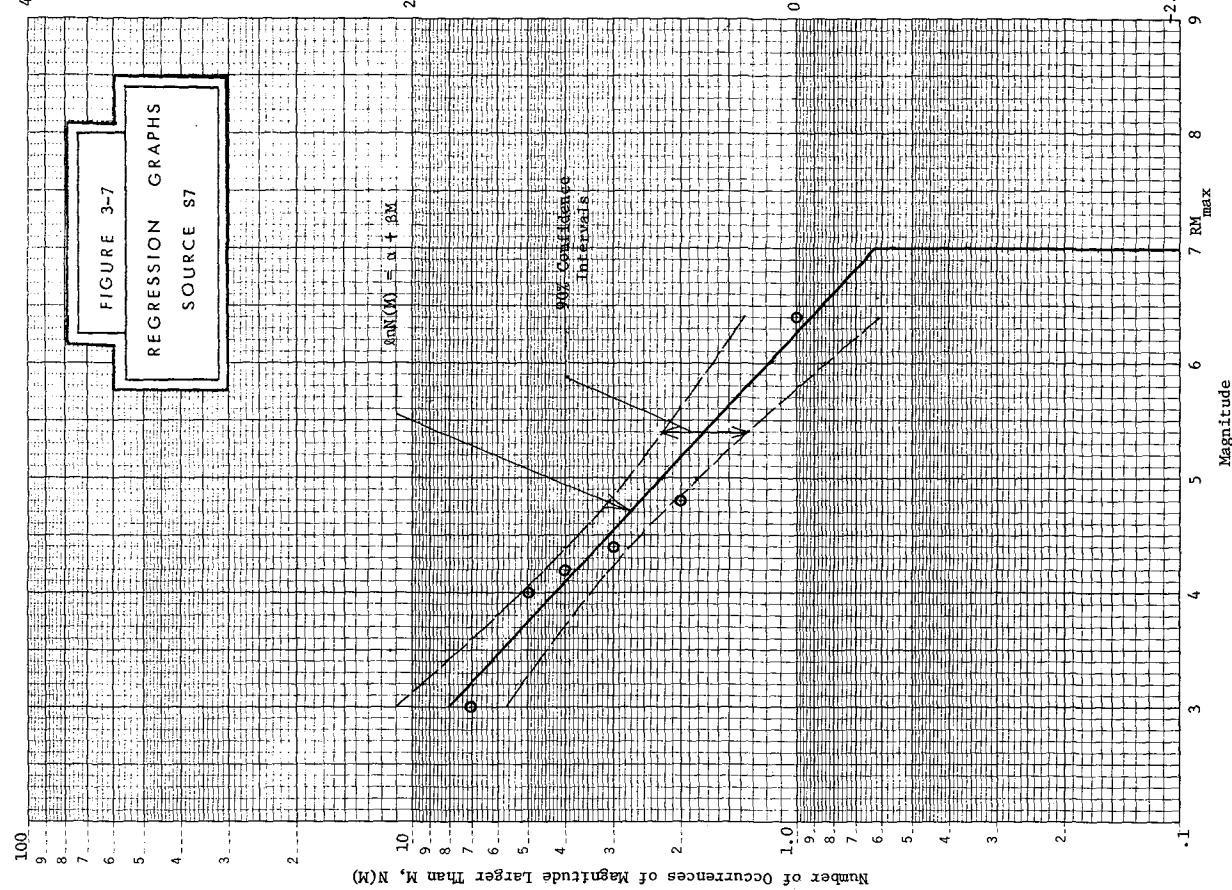
where  $L$  = Length of the line source

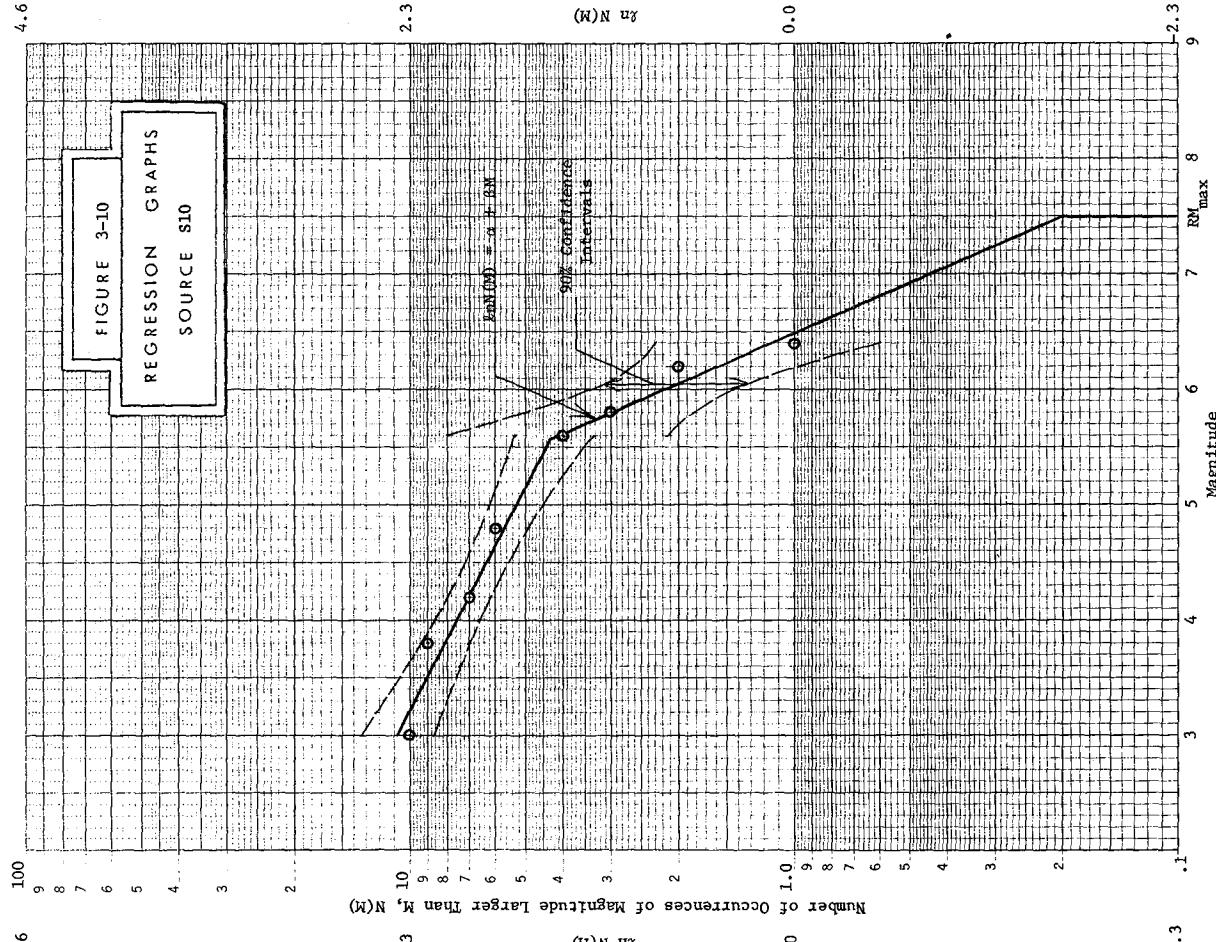
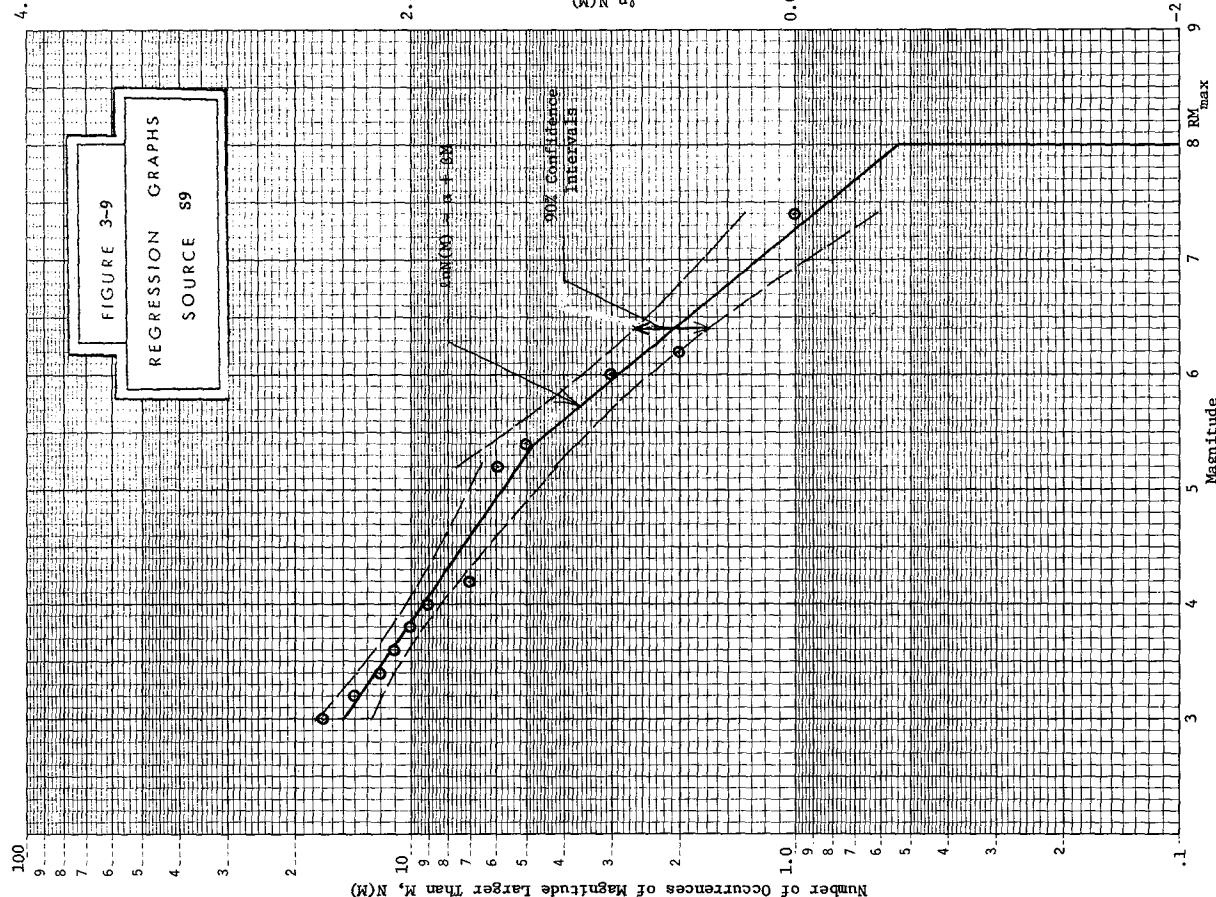


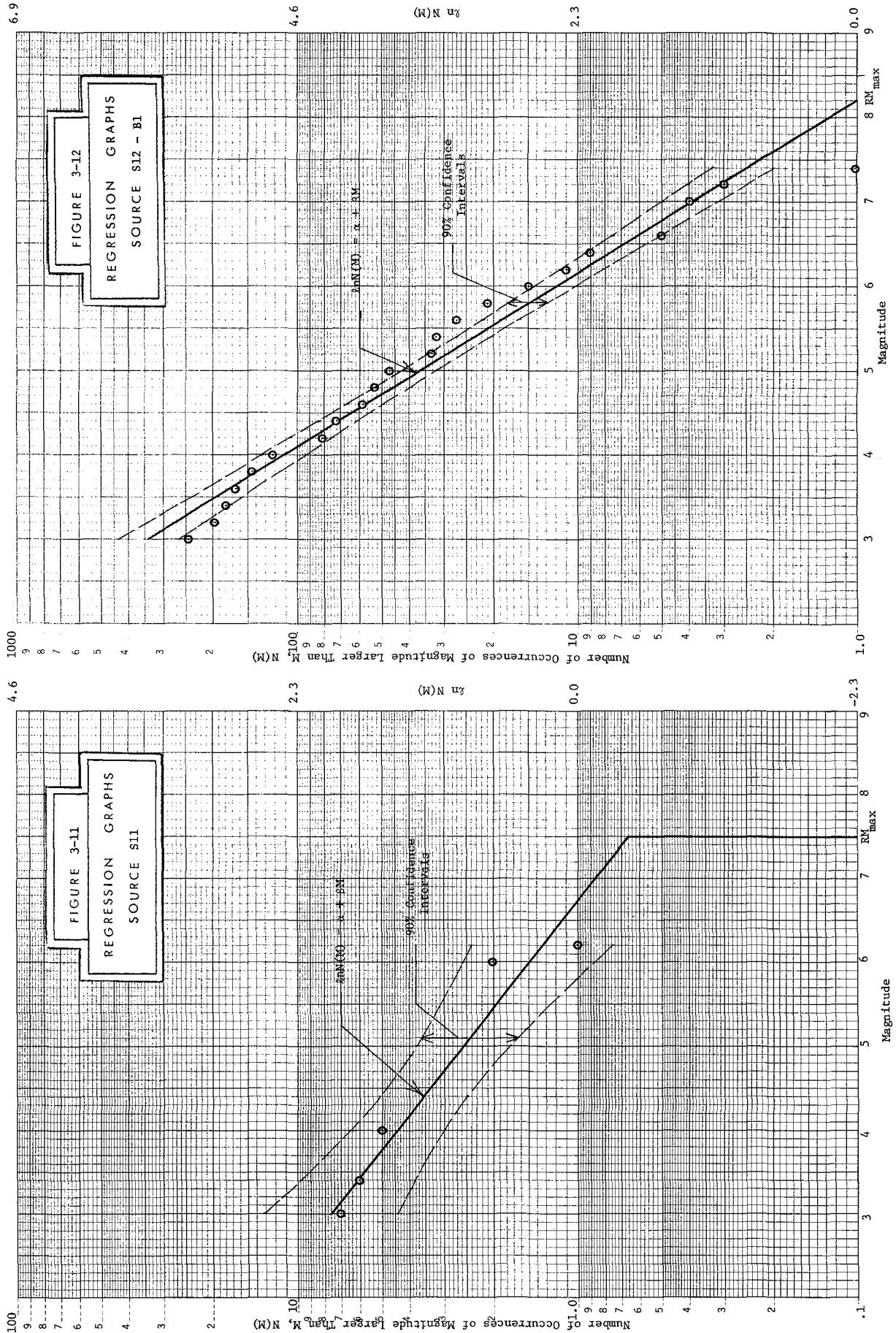


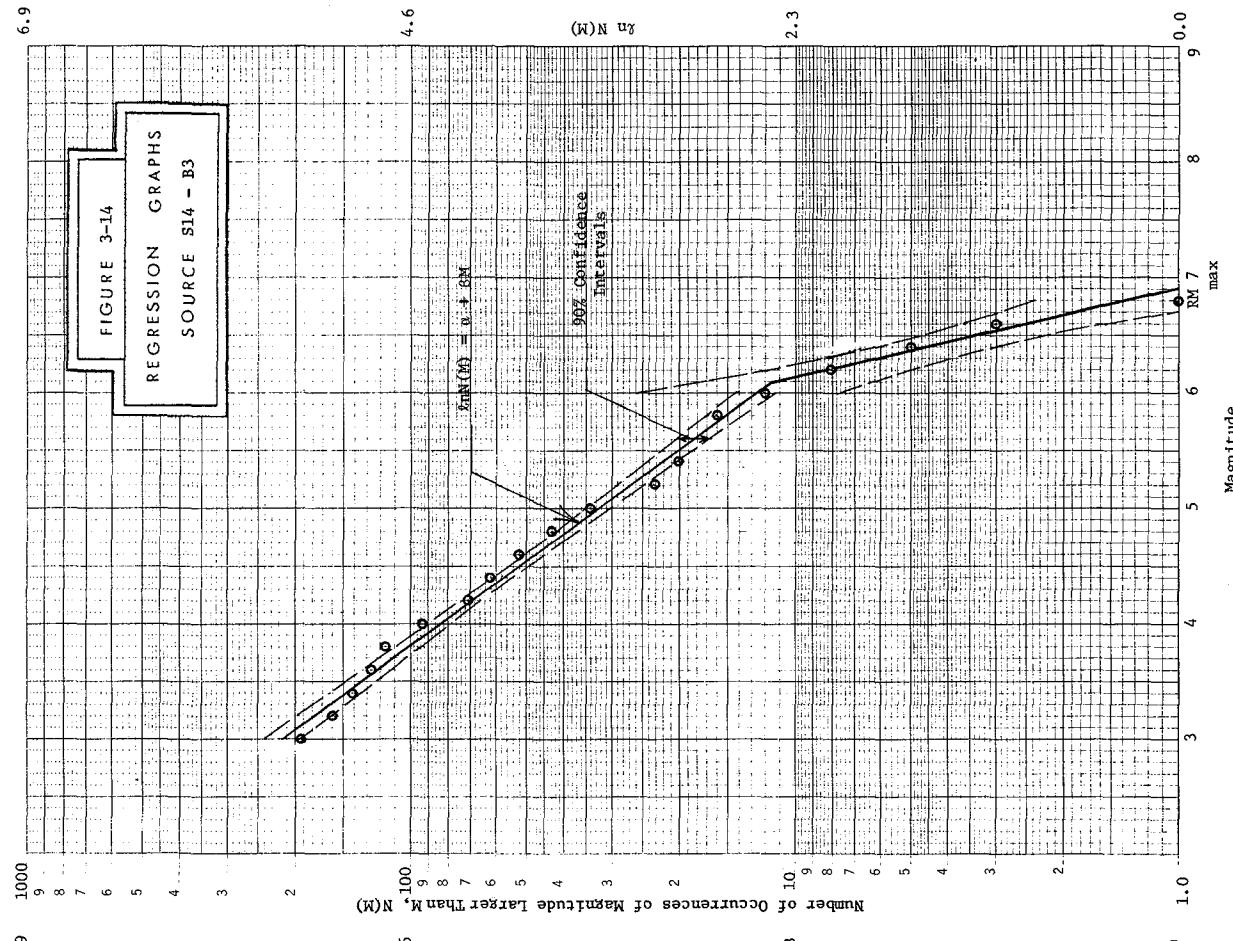
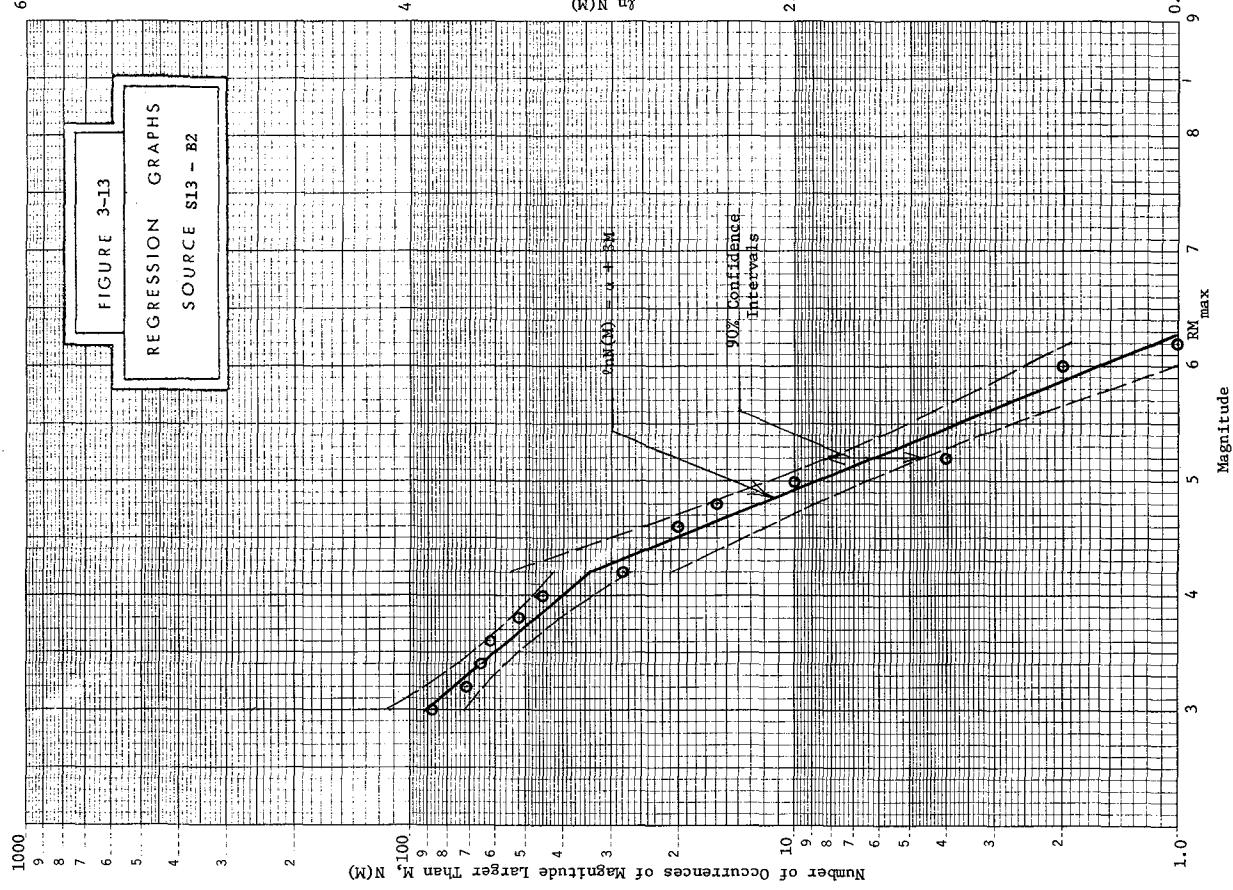
4.6

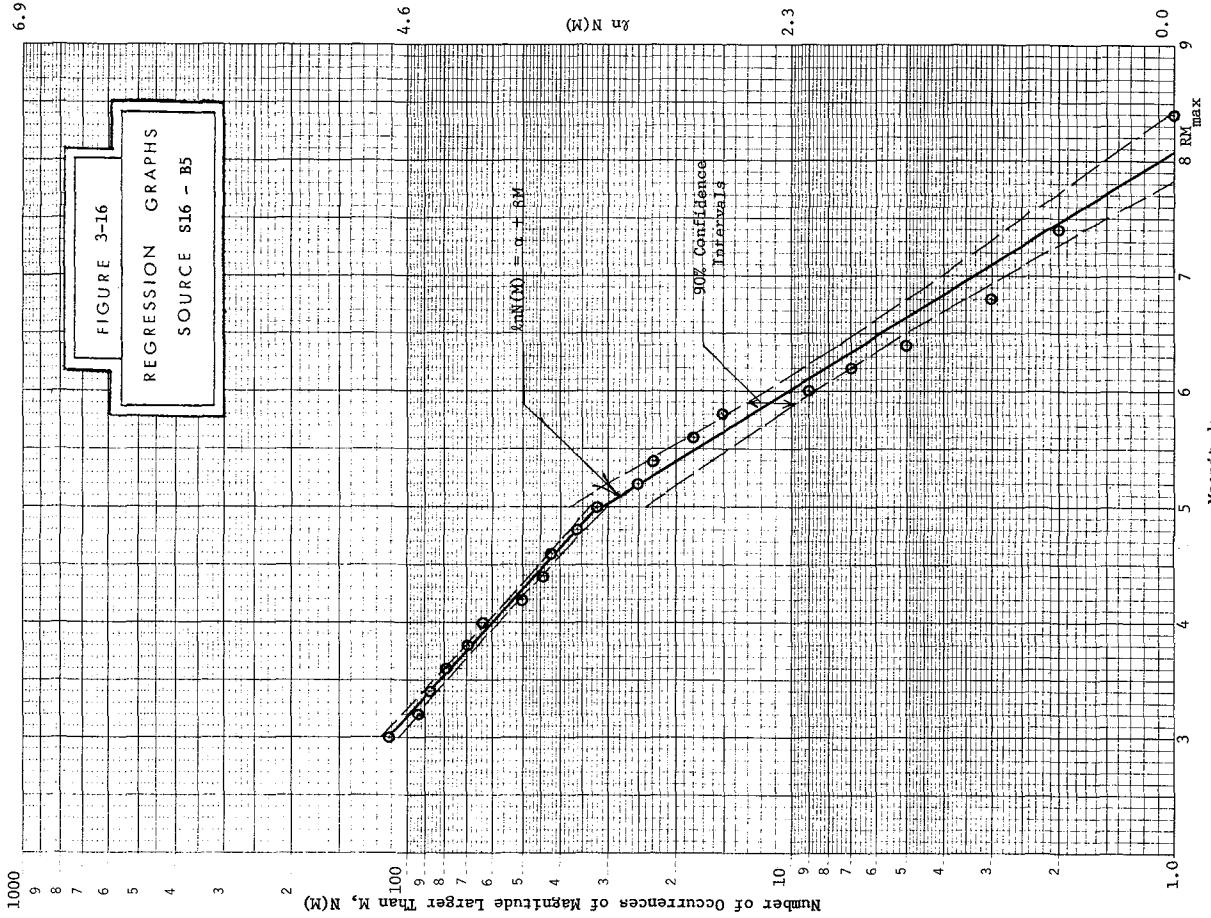
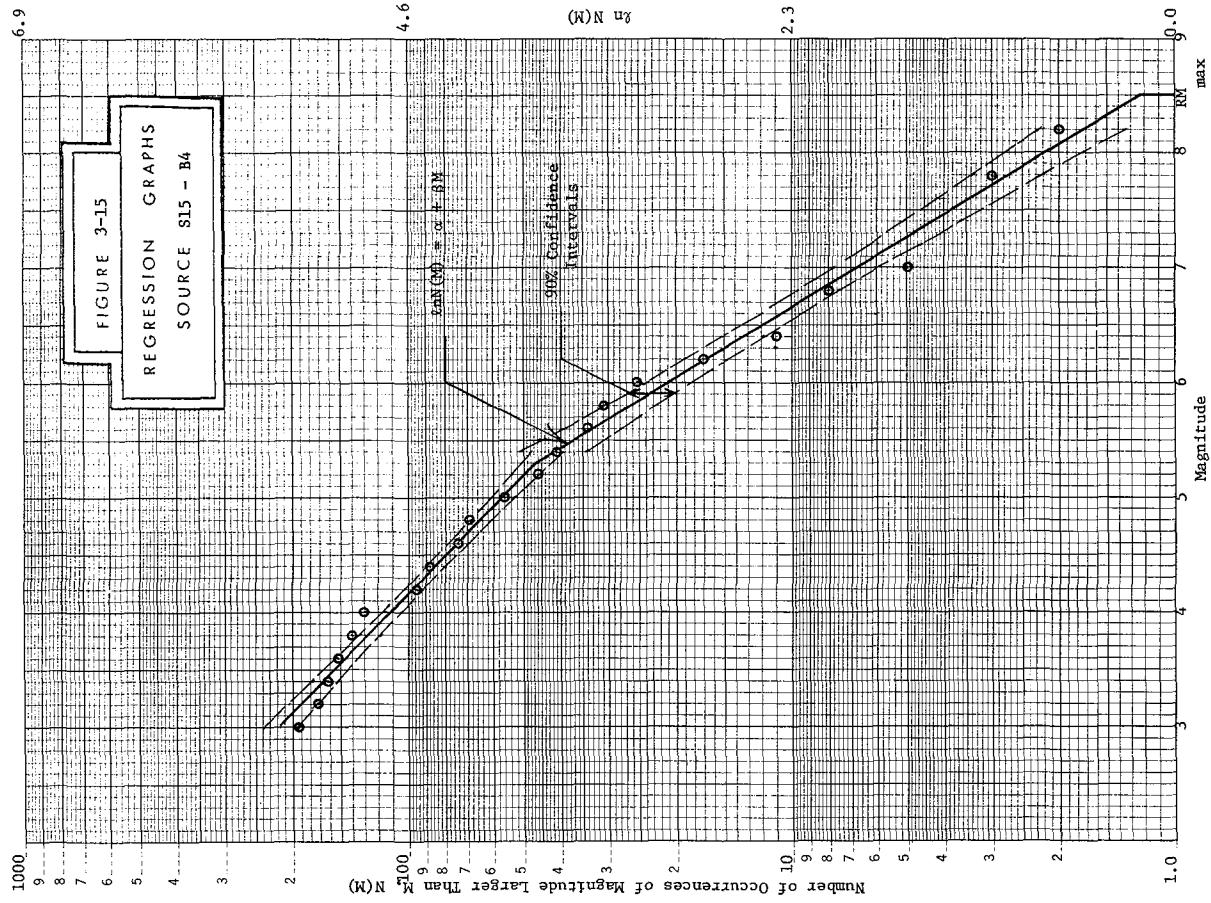












4.6

100

4.6

FIGURE 3-17

**REGRESSION GRAPHS**  
SOURCE S17 - B6

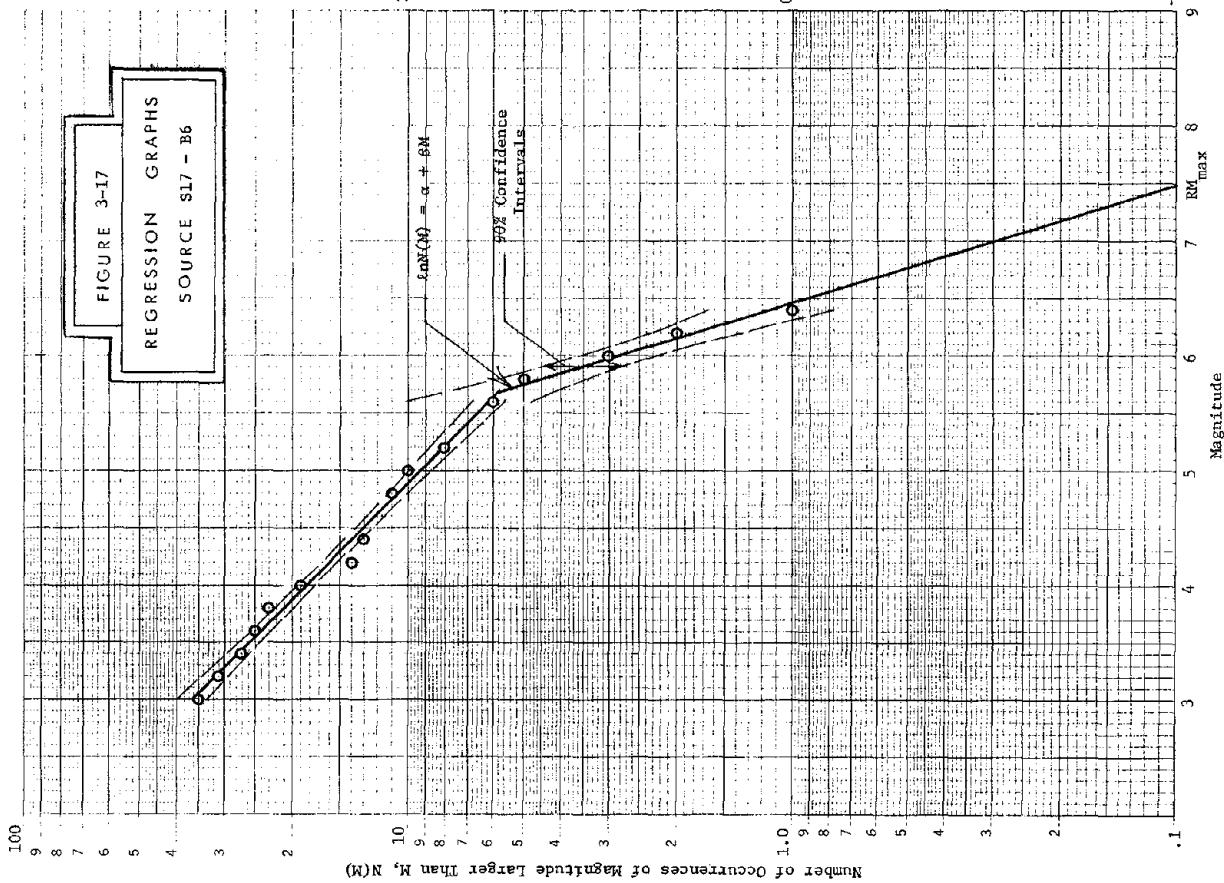


TABLE 3-3

## Line Source Information

Source	$\alpha'_1$	$\beta_1$	$\alpha'_2$	$\beta_2$	$RM_{max}$ (recorded)	$RM_{max}$ (cutoff)	Length (degrees)	RMBK
S1	-0.0632	-0.7955			7.5	8.5	4.84	
S2	-0.4323	-0.7961			6.0	8.0	3.52	
S3	-0.2486	-0.7211			6.8	8.0	2.72	
S4	-0.0023	-0.5579	10.4009	-2.1618	7.3	8.0	2.68	6.49
S5	2.1556	-1.3430			5.0	6.0	1.84	
S6	-0.4873	-0.6460			5.7	6.5	0.79	
S7	-0.0187	-0.6363			6.5	7.0	0.73	
S8	1.2284	-0.5930			6.1	7.4	0.30	
S9	-0.8131	-0.4753	1.0776	-0.8249	7.5	8.0	1.87	5.41
S10	-1.7146	-0.3591	5.1519	-1.5889	6.5	7.5	2.36	5.58
S11	-0.7026	-0.5397			6.3	7.5	1.12	
S12 - B1	2.9900	-1.1107			7.4	8.5	6.34	
S13 - B2	0.6477	-0.8098	4.3405	-1.6915	6.3	8.5	7.15	
S14 - B3	1.8929	-0.9447	14.1711	-2.9741	6.8	8.5	7.39	6.05
S15 - B4	1.0108	-0.6659	3.4927	-1.1340	8.3	8.6	7.72	5.30
S16 - B5	0.1986	-0.6222	2.5188	-1.0979	8.4	8.6	7.83	4.88
S17 - B6	-0.7732	-0.6795	8.1302	-2.2485	6.5	8.5	8.02	5.67
S18 - B7	-0.8487	-0.6290	3.0325	-1.2867	6.9	8.5	8.02	5.90

A = Area of the area source

T = Time for which the data was obtained

$N'(M)$  = Normalized mean number of events above magnitude M for unit-time (1 year) and unit-area or unit length.

Then

$$\ln N'(M) = \alpha' + \beta M \quad 3-4$$

where  $\alpha' = \alpha - \ln(AT)$  for area source

$= \alpha - \ln(LT)$  for line source.

Table 3-3 shows values of  $\alpha'$ ,  $\beta$  and the upper cutoff magnitude as described previously. The table gives the values of  $\alpha'$  and  $\beta$  normalized with respect to degrees of latitude and longitude and develop the forecasting model. RMBK is the Richter Magnitude where the slope of the recurrence lines change.

#### III-4 Confidence Levels

In addition to the  $\alpha'$  and  $\beta$  parameters, the 90% confidence intervals are computed for all regression lines. These intervals are measures of the accuracy of the regression fit to the data. The 90% confidence intervals indicate that there is 0.90 probability that the true value of  $\ln N(M)$  lies within these bounds (see Benjamin and Cornell, 1970, for further detail). For the cases of double regression lines, two separate confidence intervals are obtained for each segment. The discontinuity at the intersection of the two regression lines is to be expected since the least-squares fit for each segment is performed using its corresponding set of data independent of each other. The confidence lines for all the sources appear close to the regression line showing high confidence in the recurrence relationships. The confidence bounds are particularly good for sources S12 - B1 to S18 - B7, and are worse for S10 and S11. In general the intervals are narrow whenever there are many data points.

TABLE 3-4

## Statistics of Regression Lines

Source	$\sigma_1$	$\rho_1$	$V_1$	$\sigma_2$	$\rho_2$	$V_2$
S1	0.245	0.955	.109			
S2	0.253	0.930	.150			
S3	0.221	0.949	.120			
S4	0.235	0.887	.084	0.159	.177	
S5	0.276	0.927	.148			
S6	0.141	0.963	.147			
S7	0.163	0.956	.145			
S8	0.153	0.960	.087			
S9	0.111	0.912	.048	0.140	0.972	.135
S10	0.089	0.955	.046	0.193	0.932	.242
S11	0.254	0.930	.210			
S12 - B1	0.292	0.965	.085			
S13 - B2	0.128	0.900	.032	0.287	0.957	.149
S14 - B3	0.100	0.988	.025	0.244	0.952	.168
S15 - B4	0.081	0.978	.018	0.166	0.979	.067
S16 - B5	0.041	0.991	.010	0.213	0.968	.102
S17 - B6	0.073	0.985	.026	0.154	0.966	.148
S18 - B7	0.086	0.976	.031	0.078	0.987	.053

The constant standard deviation  $\pm \sigma$  and the coefficient of variation V of  $\ln N(M)$ , which are indicators of the variability of the estimated regression line, are also determined for each source. The values of  $\pm \sigma$  and V are given in Table 3-4. Most of the sources have low coefficients of variation V. It should be noted that in the model used for the probabilistic acceleration computations (Section III-3) a maximum of four sources have an affect on any given point in the worst cases. For such points the largest coefficient of variation due to the input data is found to be 14.5% assuming the contribution from all four sources are independent and the following relationship is true:

$$v_y^2 = v_{x1}^2 + v_{x2}^2 + v_{x3}^2 + \dots + v_{xn}^2 \quad 3-5$$

Most points will have a value for V much lower than 14.5%.

The correlation coefficients,  $\rho$ , shown in Table 3-4 express the degree of linear dependence of  $\ln N(M)$  to M. For almost all sources  $\rho$  is greater than 0.9, a value of 1.0 indicating perfect linear correlation, thus denoting relatively high correlation between the variables.

### III-5 Conclusion

Some of the limitations of the data were already discussed throughout the chapter. The variability of the regression equation was determined for each source and an upper bound uncertainty in the final probabilistic loads due to them were also obtained. Other sources of error in the modeling, such as fault locations, focal depths, and cutoff magnitudes, will be discussed through subsequent chapters.

It should be emphasized that as additional data becomes available to give more reliable information on epicentral locations, the methodology presented in this research project will be able to modify the results accordingly.

## CHAPTER IV

### PROBABILISTIC SEISMIC LOADING

#### Scope

In this chapter, the Poisson model used to analyse the available data is discussed briefly. The available data presented in Chapter III is used to obtain iso-acceleration maps of Guatemala. In addition, the cumulative probability distributions on peak ground acceleration for eight cities in Guatemala are also presented.

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#### IV-1 Introduction

In Chapter III, the data base was discussed and the limitations of the available information and the approximations made in using the seismic data for Guatemala were reviewed. The recurrence relationships associated with all the seismic sources for the region were also presented. These recurrence relationships give the mean number of events of magnitude greater than M due to a seismic source and a time period. The normalized recurrence relationships are obtained when the mean number of events above a specified magnitude M is normalized with respect to the time period of the historic data and the length of the source in the case of line sources or the area of the source in the cases of area sources.

To determine the seismic risk in Guatemala, it is necessary to forecast earthquake events in this region. The three statistical models for forecasting that have been used previously are:

- (1) Poisson model
- (2) Markov model
- (3) Bayesian model

In the present study, the Poisson model used by Shah et al. (1975) is adapted because of its simplicity, its widespread use in literature, and because the results it gives are very similar to results arising from more complex models such as the Markov Chain model. The development of the Poisson model is presented in Appendix A for completeness and further reference. In addition the Bayesian model of Mortgat (1976) is also used to develop seismic hazard maps for Guatemala for peak ground acceleration as well as for time duration of an earthquake. A review of the Bayesian model and the resulting hazard maps from its application to Guatemala are discussed in Appendix B.

#### IV-2 Iso-Acceleration Maps for Guatemala

Considering the Poisson model, the seismic sources and their corresponding frequency relationships were already discussed in the previous chapter. The maximum Richter magnitude was assigned to each source by considering the largest earthquake event associated with each source, the length of the source and the fault system to which it may belong. The values of the largest Richter magnitude recorded ( $M_{max}$  recorded) and the cutoff Richter magnitude ( $M_{max}$  cutoff) for each source is listed in Table 3-3. The attenuation relationship (see Equation A-9) used in the modeling is Esteva's (1973) equation given below:

$$a = \frac{5000 \exp (0.8M)}{(R_h + 40)^2} \quad 4-1$$

where  $a$  = Peak Ground Acceleration in  $\text{cm/sec}^2$

$M$  = Richter magnitude

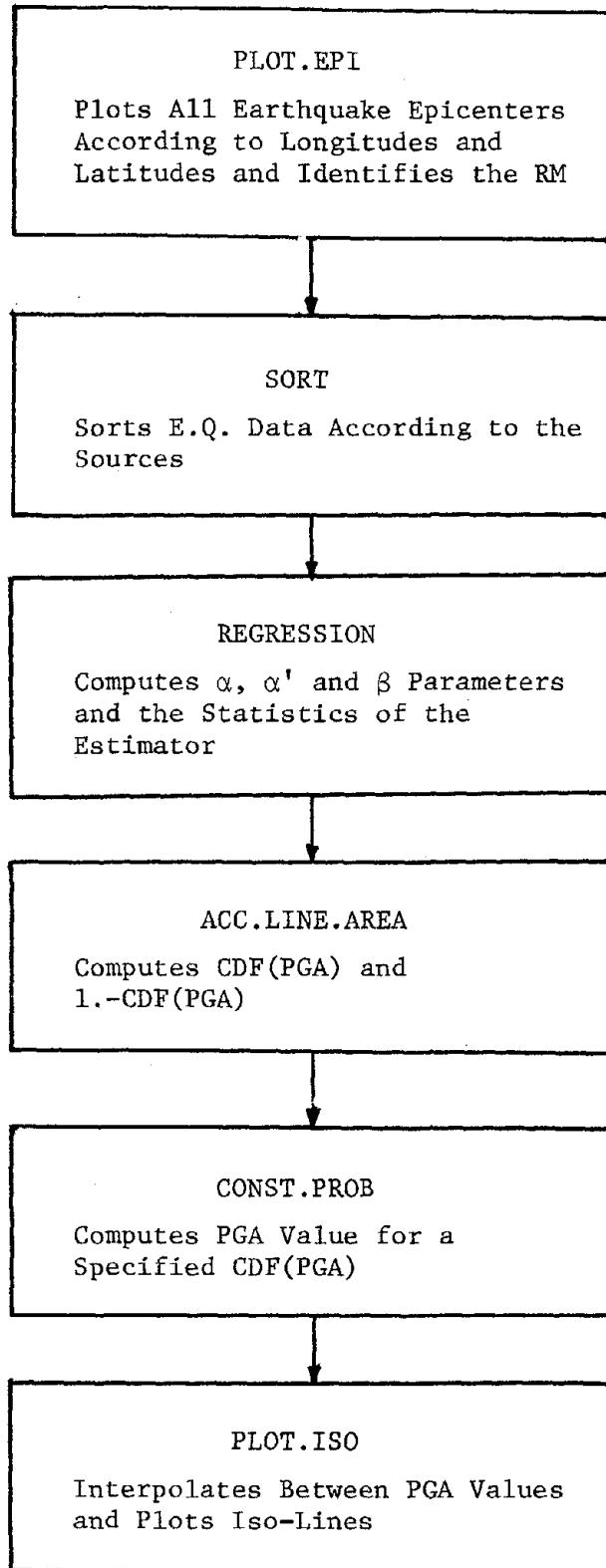
$R_h$  = Hypocentral distance in km.

Since no strong motion records from major earthquake events or high amplitude accelerograph records are available for the Guatemalan region, it was

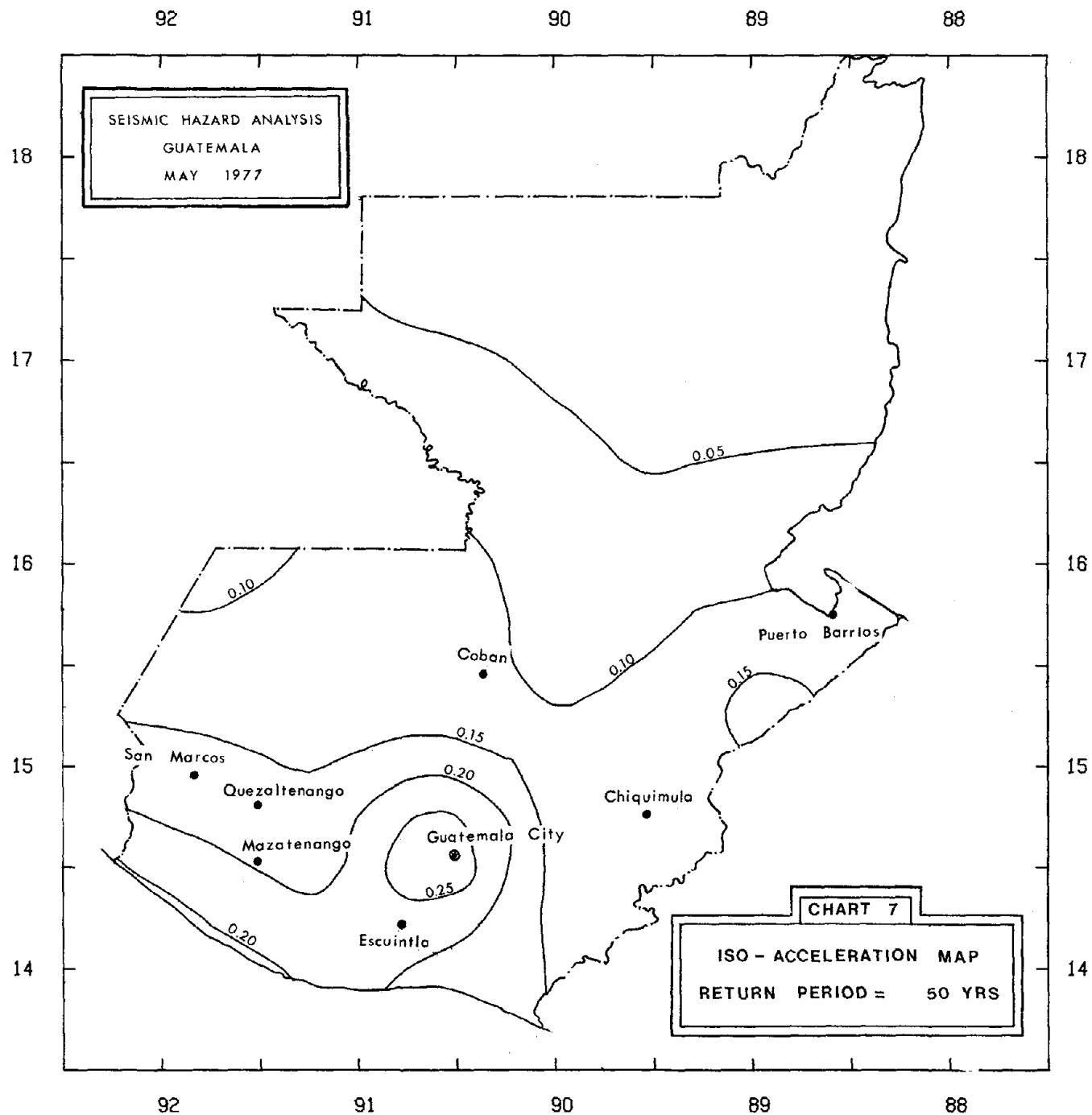
not possible to adjust Esteva's equation or to obtain an attenuation relationship specific to Guatemala. The general feeling among Guatemalan geologists and seismologists is that the ground motion attenuates at a slower rate than in California, for example, however the peak ground acceleration values corresponding to each Richter magnitude are presumed to be lower. It is hoped that with the better instrumentation and additional strong motion recording in the future, these hypotheses can be scientifically substantiated, and consequently can be used for developing a new attenuation relationship. Such changes, it should be noted, can be easily incorporated in the seismic hazard model.

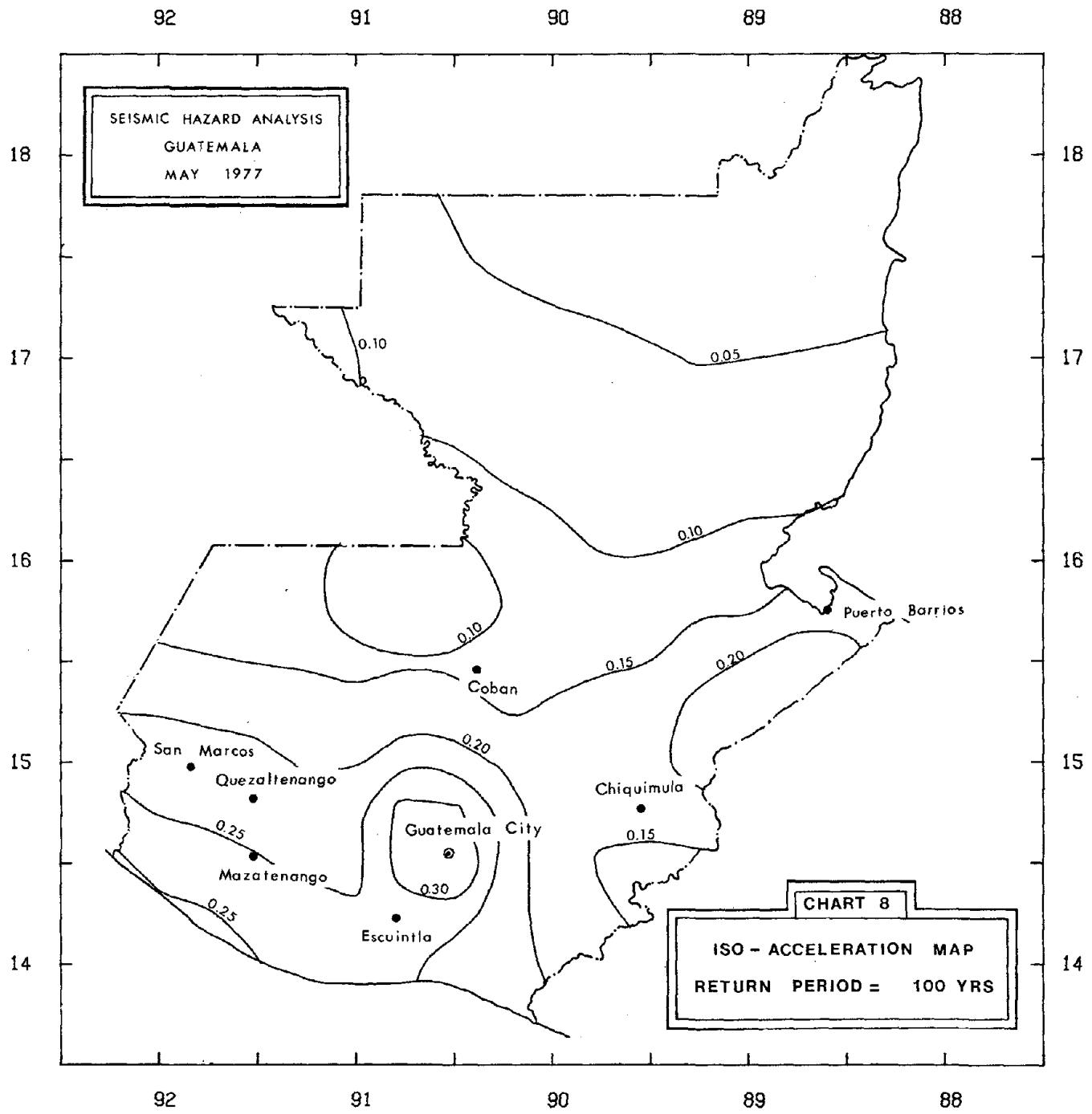
All of the above information along with the source location and focal depth, are put together in Equation A-34 (see Appendix A) to compute the cumulative probability distributions on peak ground acceleration. The entire country of Guatemala is divided into a grid of  $1/4^{\circ}$  longitude by  $1/4^{\circ}$  latitude spacings. The CDF of peak ground acceleration at each node on the grid is evaluated using the computer program ACC.LINE.AREA (see Appendix E). Then for a specified probability of exceedence, in other words for a specified hazard level, the peak ground acceleration values computed from the CDF's at each node are obtained using the computer procedure CONST.PROB. The iso-acceleration lines, which are lines of equal acceleration, are drawn by interpolation between nodal values of PGA. The program PLOT.ISO used in obtaining the hazard map contains both an interpolation routine and a graphical plotting routine. The flow chart below shows the major steps in the development of the map and the computer programs used at each step.

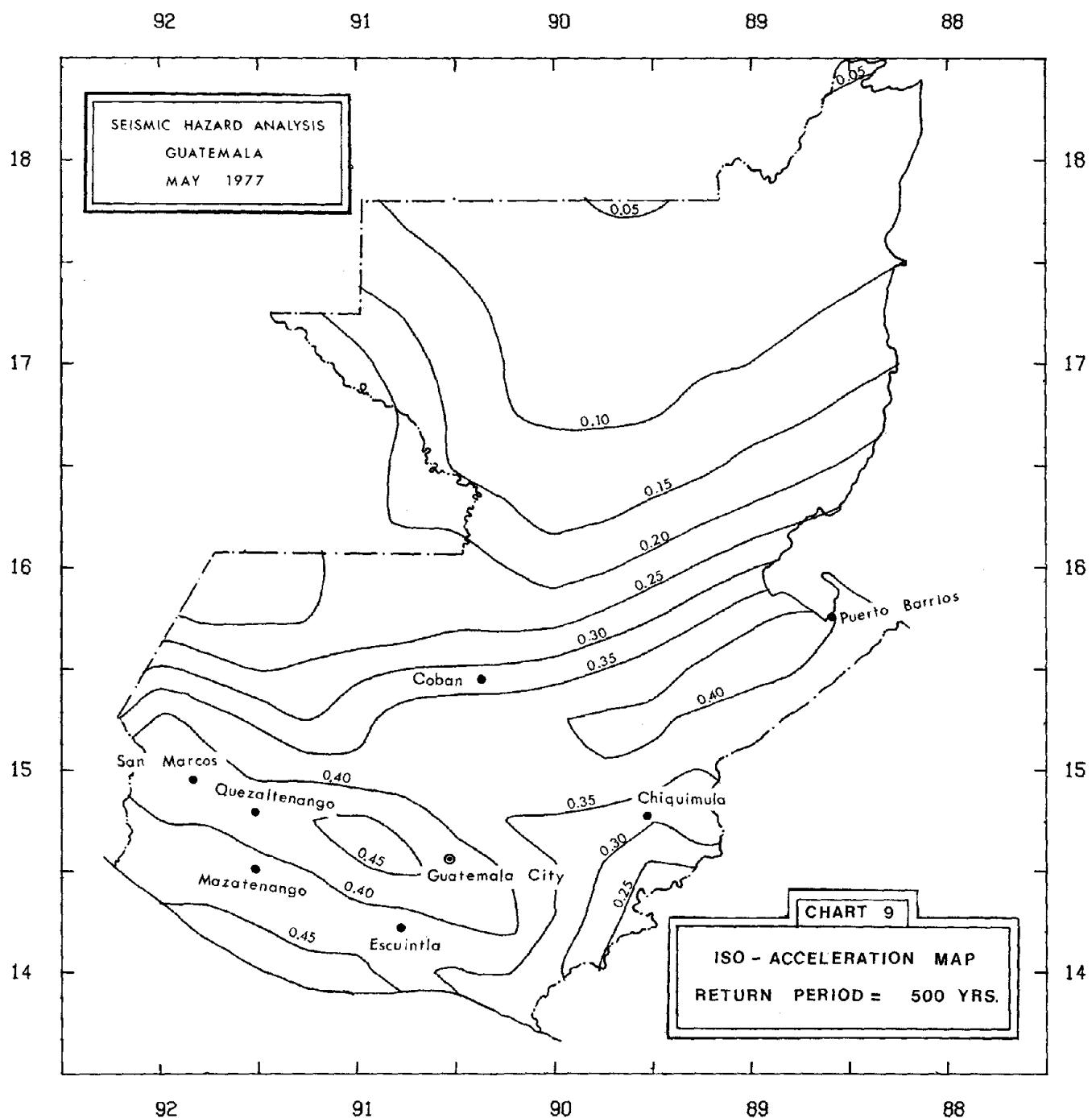
Iso-acceleration maps also referred to as seismic hazard maps are obtained for return period of events of 50, 100, 500, and 1000 years. Charts 7 to 10 show the iso-acceleration maps for these return periods.

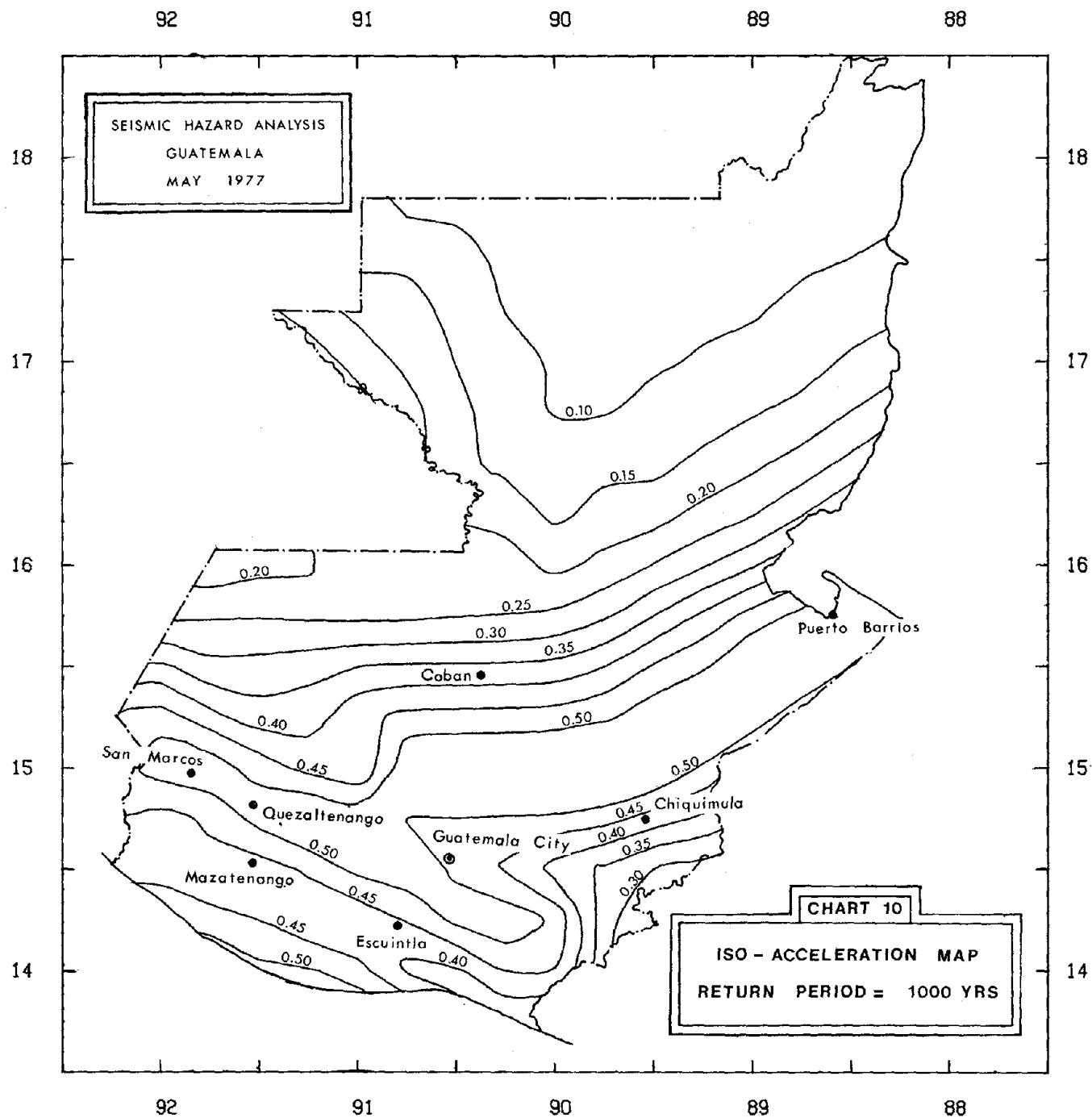


A listing of all programs is given in Appendix E.









The concept of return period and economic life in conjunction with exceedence probabilities will be discussed in Chapter V. Detailed methodology describing the use of these maps for structural design will be presented in the Part II report of the present study.

From these maps a low seismicity range is observed in the northern part of Guatemala. The high seismicity region in the south-southwestern part of Guatemala reaches a peak at the intersection of the Motagua fault, the Benioff zone and the Mixco fault. These high and low regions are in compliance with both the geologic as well as the seismologic data for the country.

In addition to the seismic hazard maps for the whole country, the following cities were studied in detail:

1. Guatemala City
2. Quezaltenango
3. Mazatenango
4. Chiquimula
5. Puerto Barrios
6. Escuintla
7. San Marcos
8. Copan

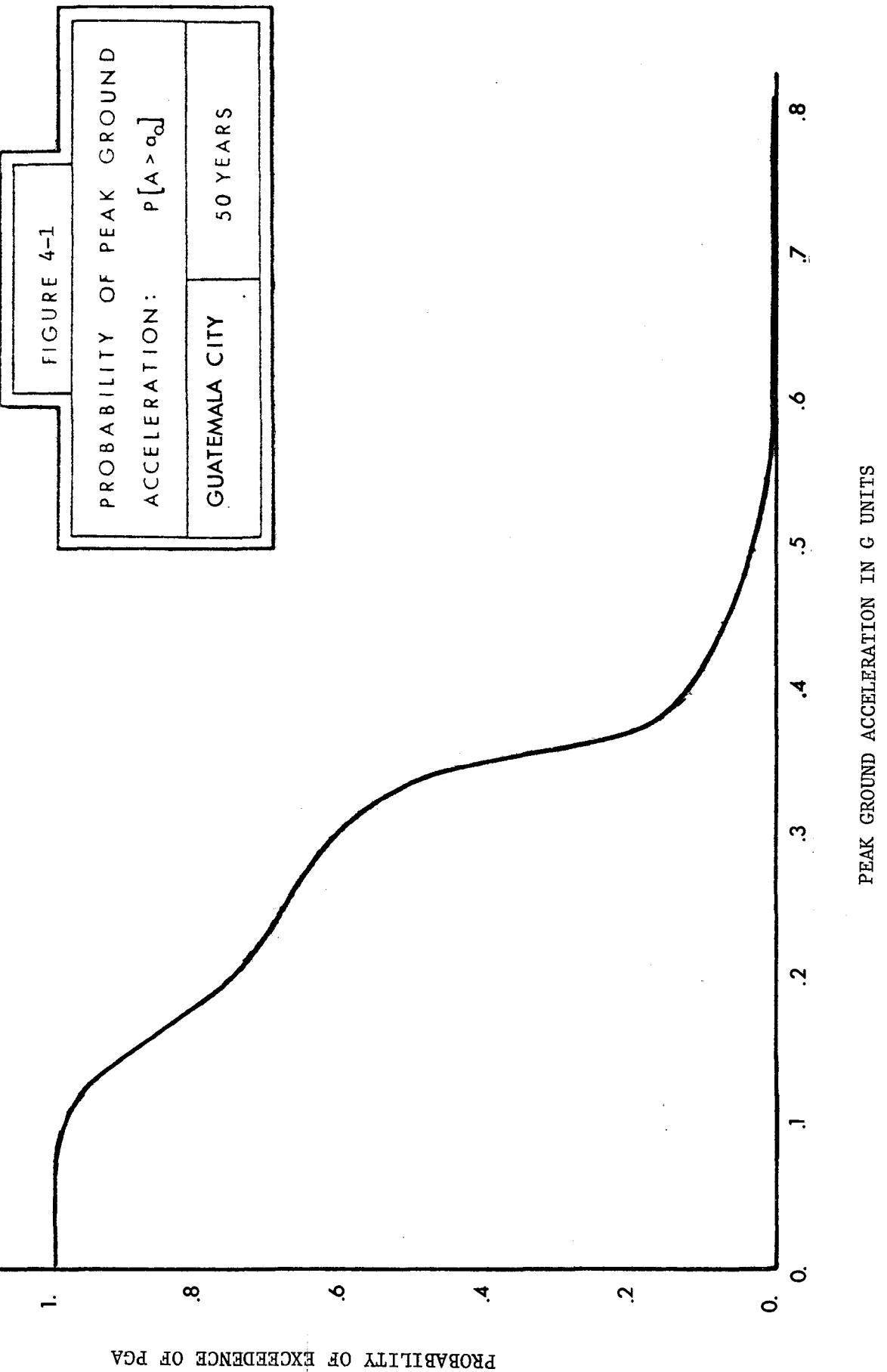
Figures 4-1 through 4-8 show the cumulative distribution function of peak ground acceleration for each of the cities. The results are presented for 50 years of future exposure time. The seismicity of Guatemala City appears to be the highest and for Coban it is the lowest.

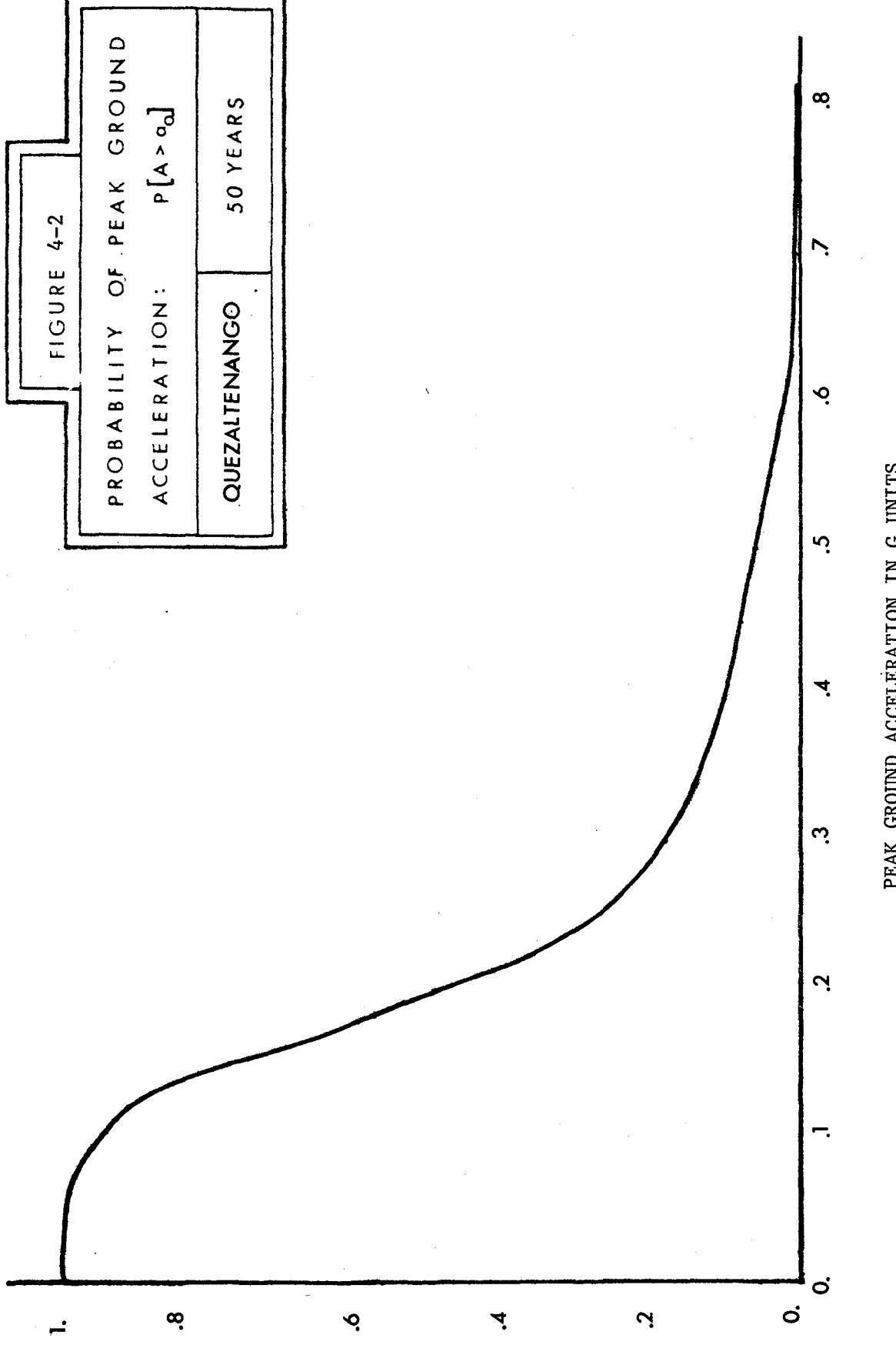
The implications of these probability values and the corresponding acceleration values will be discussed in Chapter V.

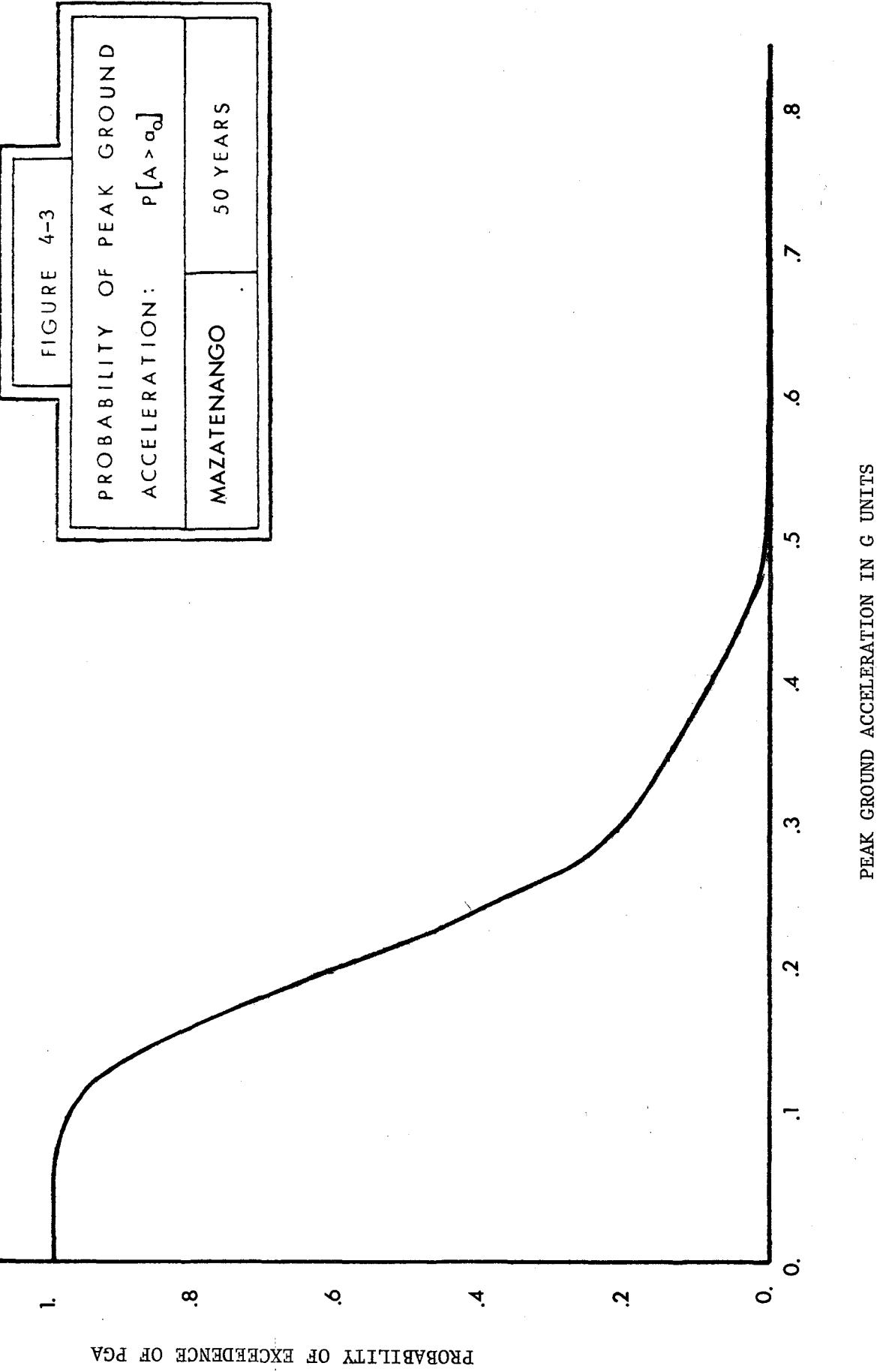
When the cumulative distribution plots for different cities are compared, one can see the relative seismicity in terms of peak ground acceleration for each city. In conclusion, it can be said that one method of

representing seismic risk is by means of cumulative distribution function plots of Figures 4-1 through 4-8.

The engineering interpretation of these results will be presented in the Part II report of the present study. It should also be pointed out that the iso-acceleration maps and any zoning based on such maps only represent macro characteristics. The macrozoning of the country should be modified with site-specific micro characteristics to microzone a given region. In that case, the local geotechnical and geological features (such as those discussed in Chapter II) should be incorporated together with the macro characteristics presented in this chapter.







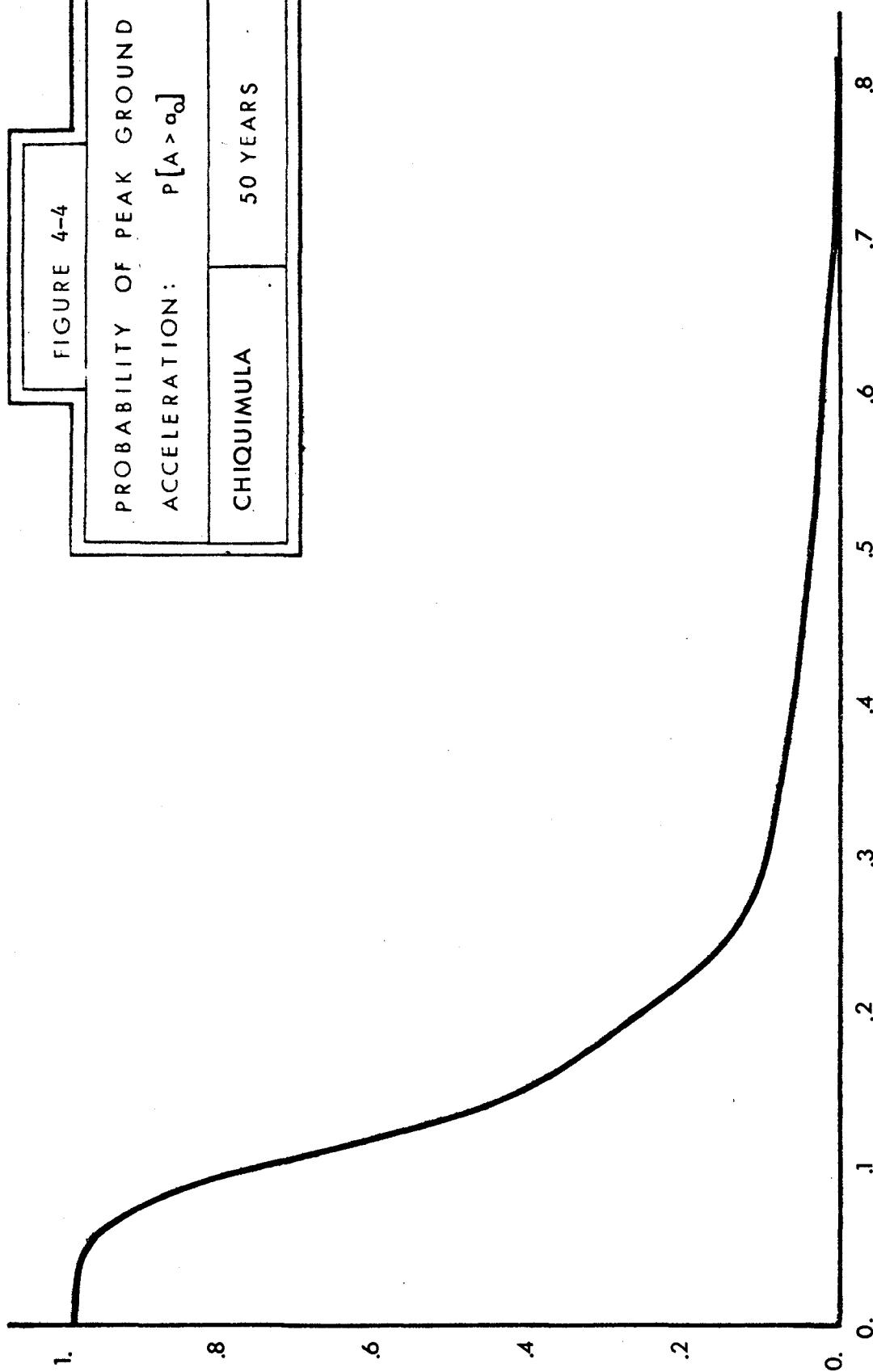
PROBABILITY OF EXCEEDENCE OF PGA

FIGURE 4-4

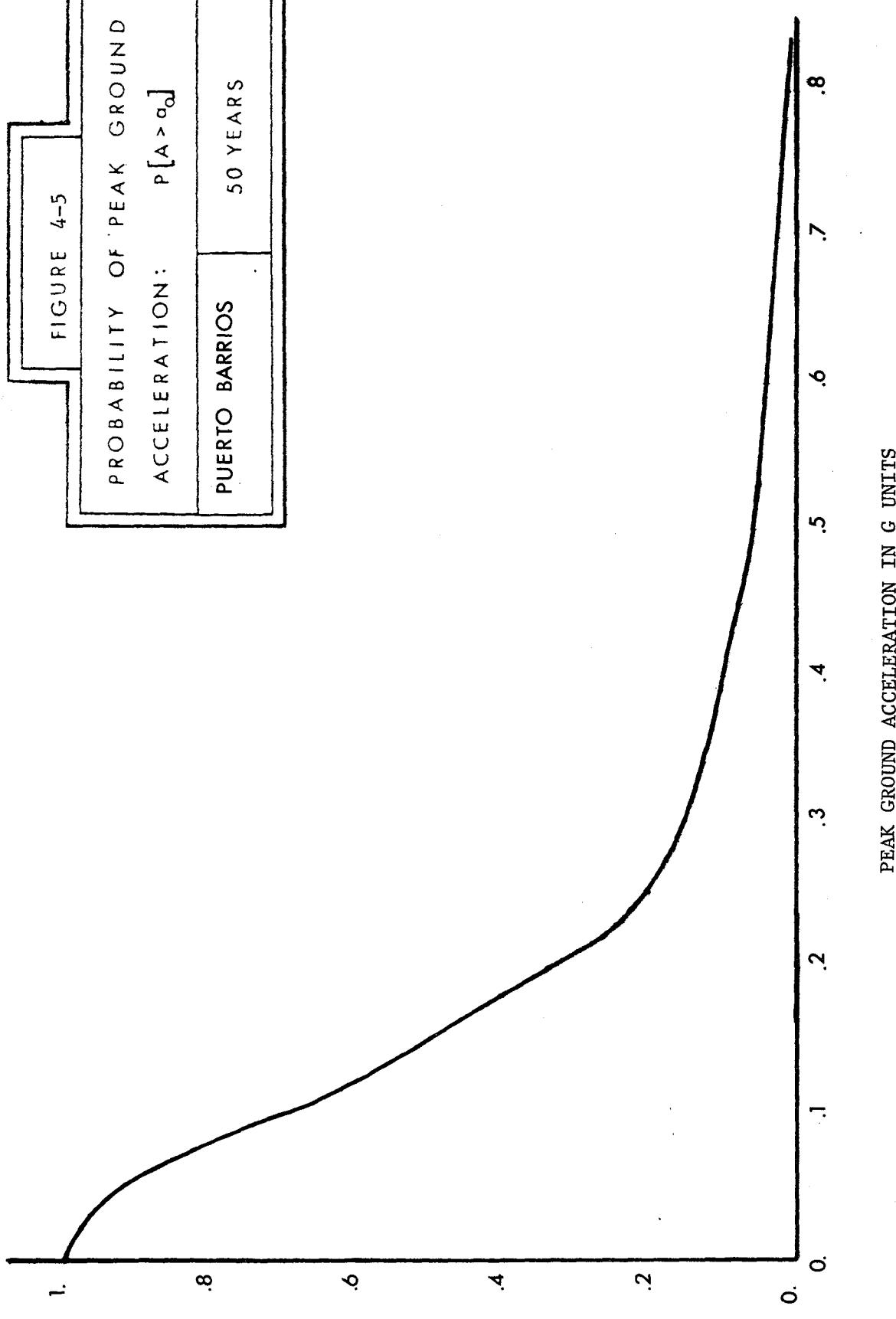
PROBABILITY OF PEAK GROUND  
ACCELERATION:  $P[A > a_0]$

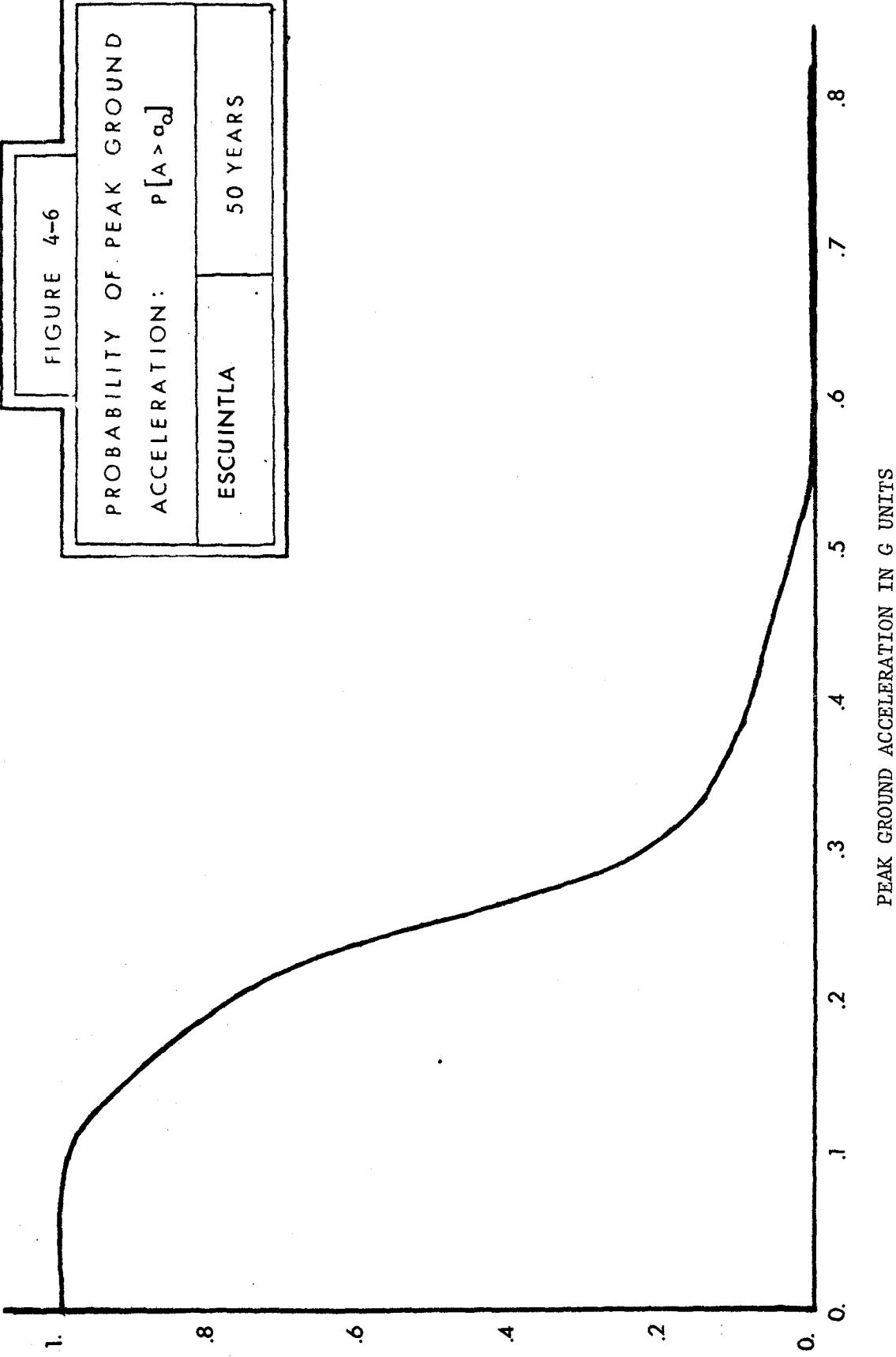
CHIQUIMULA

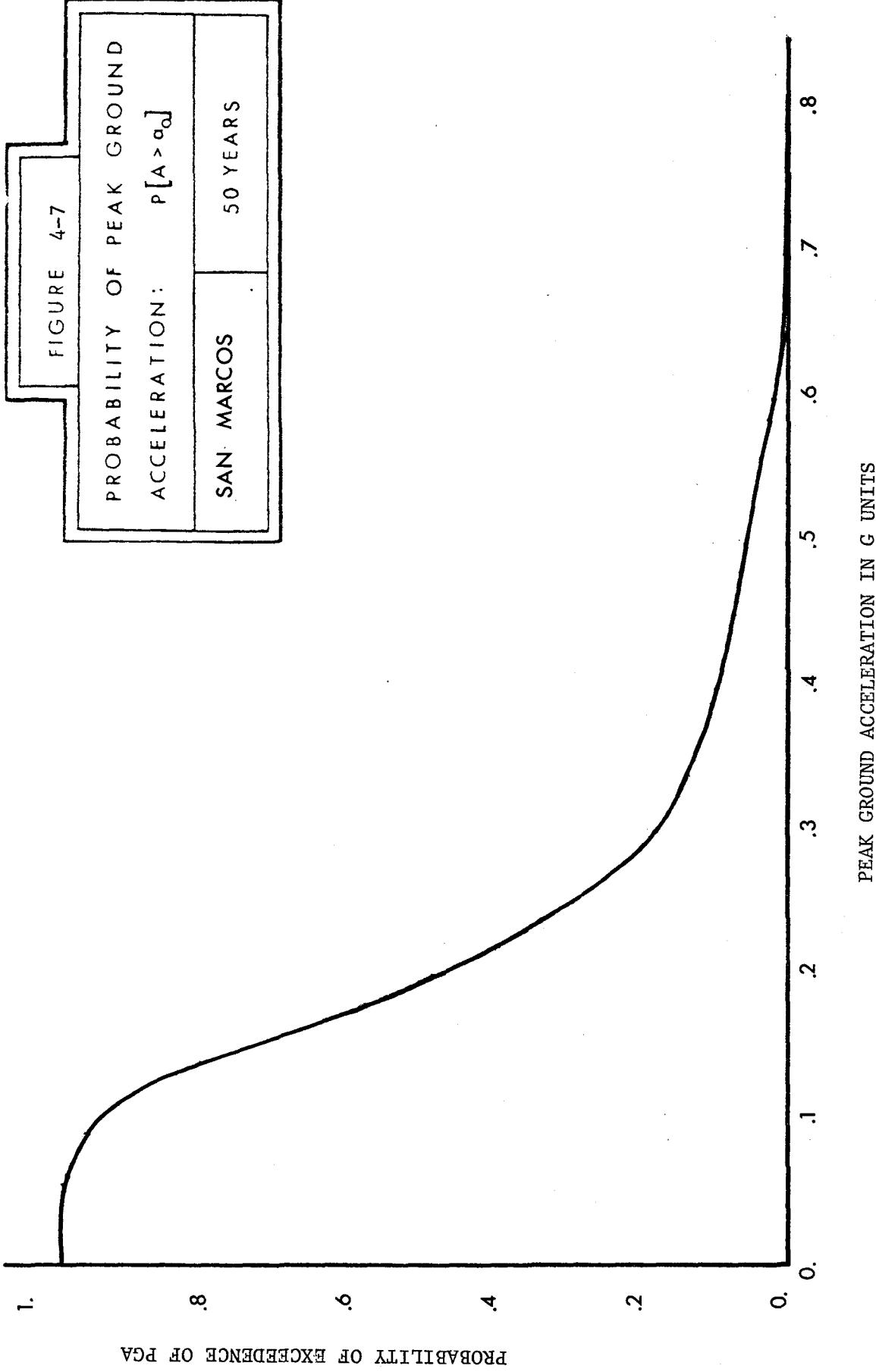
50 YEARS



PROBABILITY OF EXCEEDENCE OF PGA





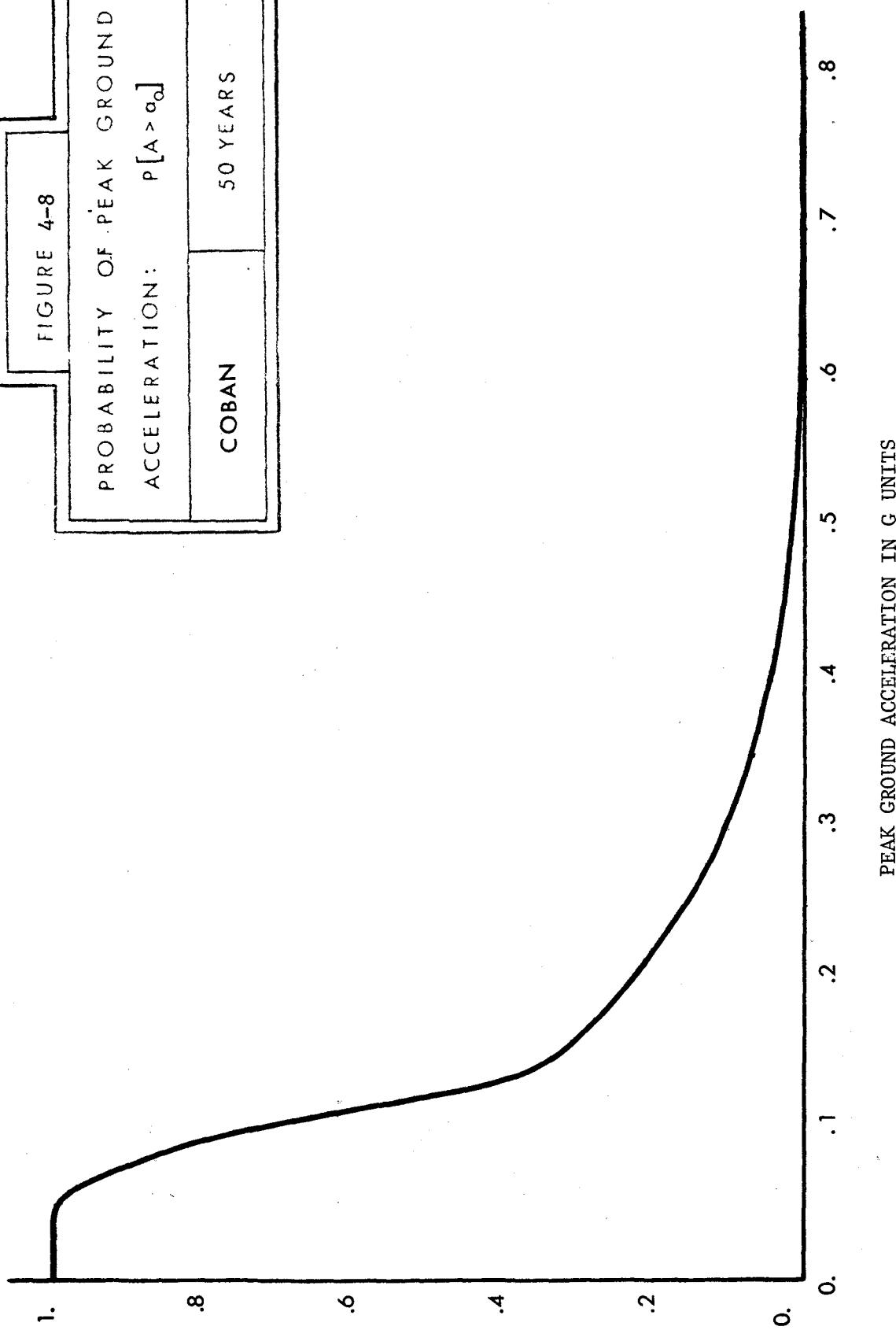


PROBABILITY OF EXCEEDENCE OF PGA

FIGURE 4-8

PROBABILITY OF PEAK GROUND  
ACCELERATION:  $P[A > a_0]$

COBAN 50 YEARS



PROBABILITY OF EXCEEDENCE OF PGA

## CHAPTER V

### SEISMIC RISK ZONING

#### Scope

In this chapter, the concept of return period and economic life is discussed in detail. The iso-acceleration maps are used to develop acceleration zone graphs for seven cities in Guatemala.

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#### V-1 Concept of Return Period and Acceleration Zone Graphs (AZG)

In deriving the probabilistic loading at a given site as a function of time, it is assumed that the forecasting process is Poisson. This process implies that the events are independent in time and space. Using this assumption and an appropriate attenuation relationship, the iso-acceleration maps for the country were developed. As mentioned in Chapter IV, the cumulative distribution function of the peak ground acceleration A, were obtained for eight cities. Consider the cumulative distribution function of peak ground acceleration in Guatemala City for an exposure time of 50 years. (See Figure 4-1.)

Then

$$P_{50} (A > 0.20g) = 0.76$$

5-1

Equation 5-1 can be interpreted in the following way: "For Guatemala City, there is a 76% chance that during the next 50 years, the peak ground acceleration of 0.20g will be exceeded at least once."

Thus, there is a 24% chance that for Guatemala City, 0.20g peak ground acceleration will not be exceeded a single time.

Hence,

$$P (\text{Zero exceedence of } 0.20g \text{ in 50 years}) = 0.24$$

From the Binomial Probability Law, it is known that for independent trials with probability of success  $p$  at each trial, the probability of  $r$  successes in  $n$  trials is given by

$$P_n(r) = \binom{n}{r} p^r (1-p)^{n-r} \quad 5-2$$

where

$$r = 0, 1, \dots, n; \quad n = r, r+1, r+2, \dots$$

and

$$\binom{n}{r} = \frac{n!}{r!(n-r)!}$$

Let each trial be a one-year duration for which the level of peak ground acceleration is under consideration. Define success as that event when the peak ground acceleration for a given trial (year) exceeds 0.2g. Thus, the probability of zero exceedence of level 0.2g in 50 years is the same as the probability of 0 successes in 50 trials. Hence, from Eq. 5-2:

$$P_{50}(0) = \binom{50}{0} p^0 (1-p)^{50}$$

$$P_{50}(0) = (1-p)^{50}$$

However,

$$P_{50}(0) = 0.24$$

$$(1-p)^{50} = 0.24$$

or  $p = 0.028.$

Thus, for Guatemala City, there is a 2.8% chance that in any given year, a peak ground acceleration of 0.20g will be exceeded.

However, the return period is defined as

$$\text{Return Period} = RP = \frac{1}{p} \quad 5-3$$

Thus, the return period RP in Guatemala City for a peak ground acceleration of 0.20g is  $\frac{1}{0.028} \approx 36$  years.

It should be pointed out that this return period of 36 years corresponding to 0.2g, obtained by using the cumulative distribution function (CDF) of PGA at Guatemala City for a 50 year exposure time does not change if we use the CDF with a different future exposure time period.

Thus, using the CDFs for all the cities in Guatemala considered in Chapter IV, a table can be developed for peak ground acceleration and return period. Table 5-1 is a general table giving this relationship for the cities considered. The following statements should be understood in using the concept of return period:

- (1) A return period is the mean (or average) waiting time for an event of interest. Thus, the average (waiting) time between 2 events producing 0.20g in Guatemala City is approximately 36 years.
- (2) The probability that an event corresponding to a return period RP will occur in any given year is given by  $p = \frac{1}{RP}$ . Thus, probability of exceeding 0.20g in Guatemala City in any given year is  $\frac{1}{36} \approx .028$ .
- (3) The probability that not a single event of the RP type will occur in RP years is given by  $\frac{1}{e}$  where  $e = 2.718$ , the Naperian base. Thus, probability that in 36 years, there will not be a single event producing a peak ground acceleration of 0.20g in Guatemala City is given by  $\frac{1}{e} \approx 0.36$ .

Hence, there is 64% chance that in RP years there will be at least one event of RP type. For Guatemala City, there is a 64% chance that in 36 years

TABLE 5-1

PGA in g units	Guatemala City (1)	Quezaltenango (2)	Mazatenango (3)	Chiquimula (4)	Puerto Barrios (5)	Escuintla (6)	San Marcos (7)	Coban (8)
0.05	1	1	1	10	20	1	1	9
0.10	13	8	13	37	41	11	17	37
0.15	23	37	26	82	72	21	38	124
0.20	36	76	53	158	120	32	76	192
0.25	44	153	103	302	225	63	141	287
0.30	52	248	205	441	297	178	250	432
0.35	86	337	325	571	385	381	361	696
0.40	393	448	540	731	494	571	468	1848
0.45	603	604	1254	933	645	807	610	
0.50	1620	818		1201	813	1302	820	
0.55	2169	1183		1583	990		1186	
0.60		2238		2198	1213		2248	
0.65				3472	1504			
0.70					1921			
0.75					2607			
0.80					4177			

0.20g peak ground acceleration will be exceeded. Consider again Table 5-1.

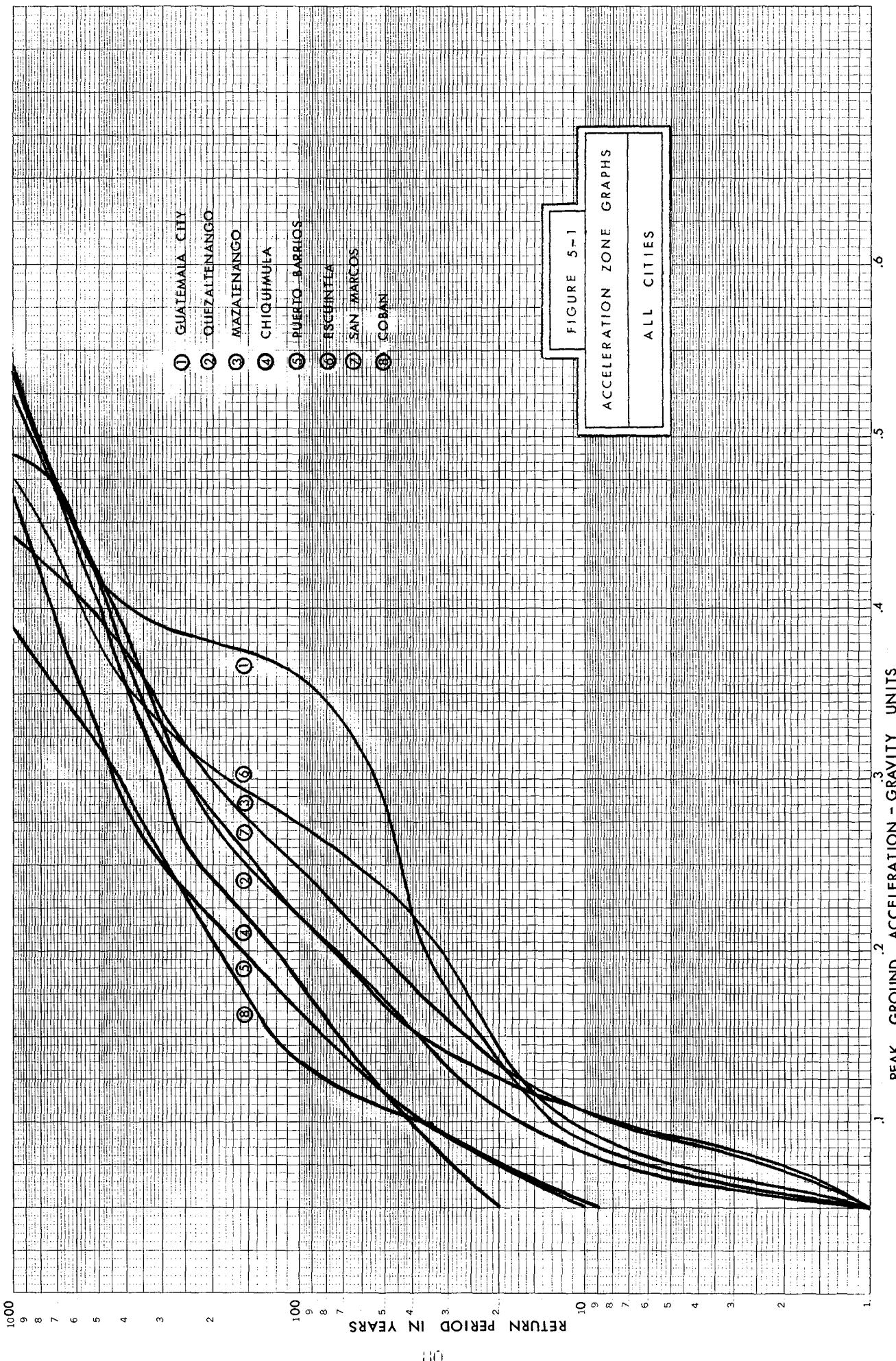
For seismic zoning purposes, the following statements can be made:

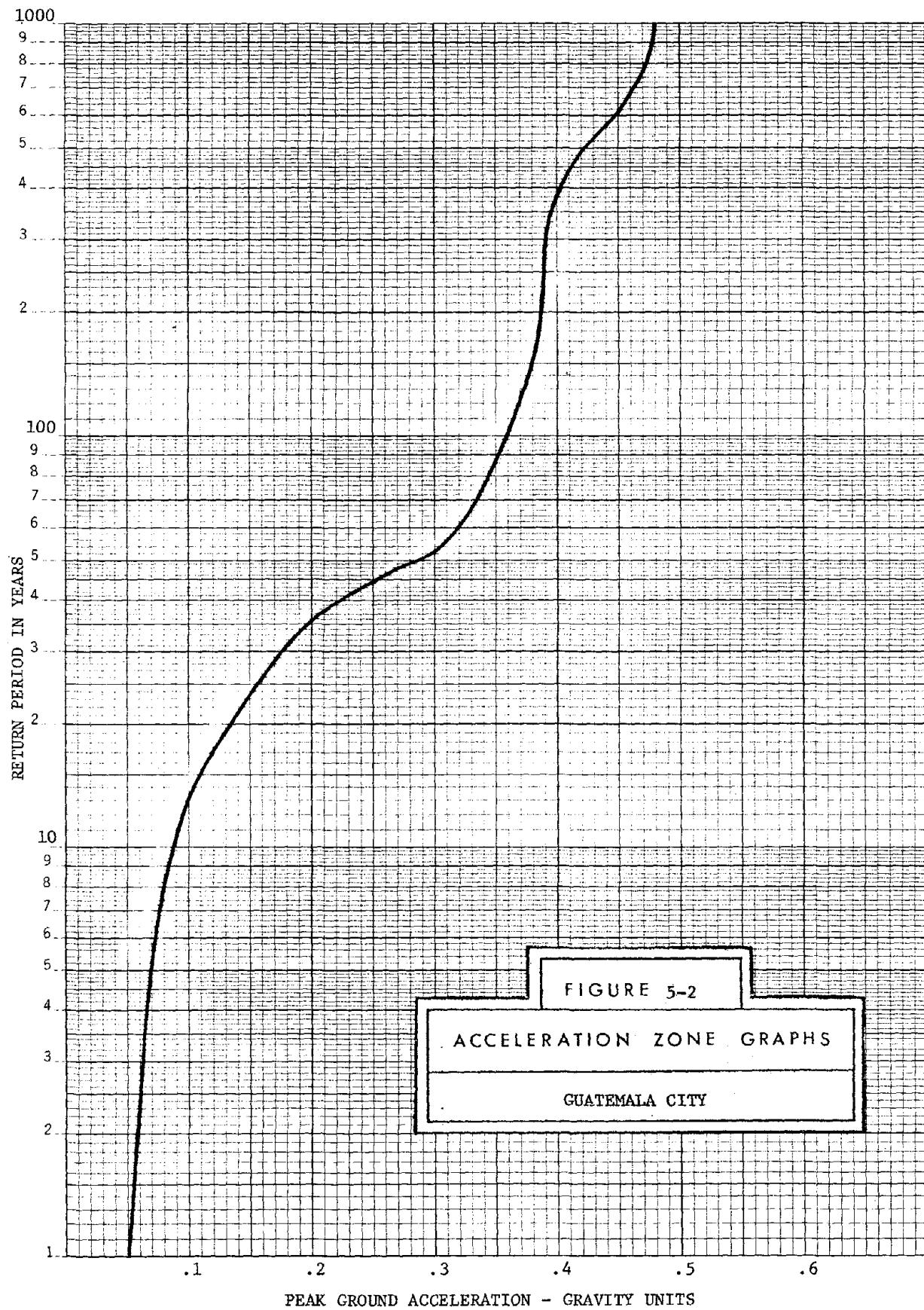
The return period corresponding to a peak ground acceleration of 0.20g in Guatemala City is 36 years, in Escuintla is 32 years, in Quezaltenango and San Marcos is 76 years, in Mazatenango is 53 years, in Puerto Barrios is 120 years and in Chiquimula is 158 years. Thus, for each city, a graph relating the peak ground acceleration and return period can be plotted.

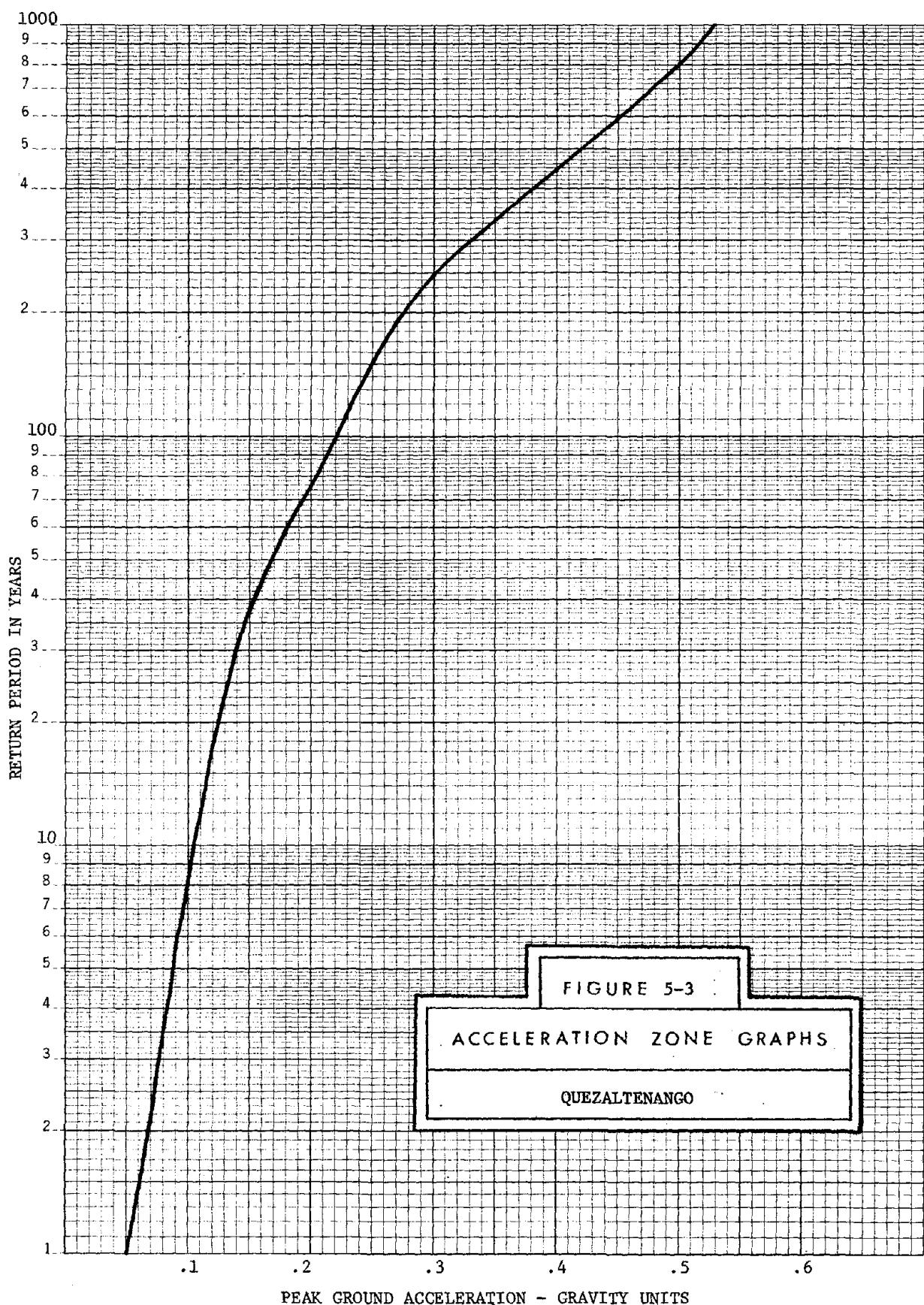
Figures 5-1 through 5-9 show these graphs. They are referred to hereafter as Acceleration Zones Graphs. Figure 5-1 shows return period vs. peak ground acceleration for all the cities. It can be seen that for a given return period event (say, 100 years), Chiquimula has the lowest value of peak ground acceleration ( $\approx .16g$ ) and Guatemala City has the highest value of peak ground acceleration (.36g). The values for other cities lies between these two limits. Qualitatively, it can be said that for a facility requiring a design loading corresponding to a 200 year return period, Chiquimula and Puerto Barrios have the lowest seismic zoning requirement; San Marcos, Quezaltenango, Mazatenango and Escuintla have similar zoning requirement; and finally, the highest level is for Guatemala City. This type of graph can help in macrozoning a country for a given class and use of a structure or facility.

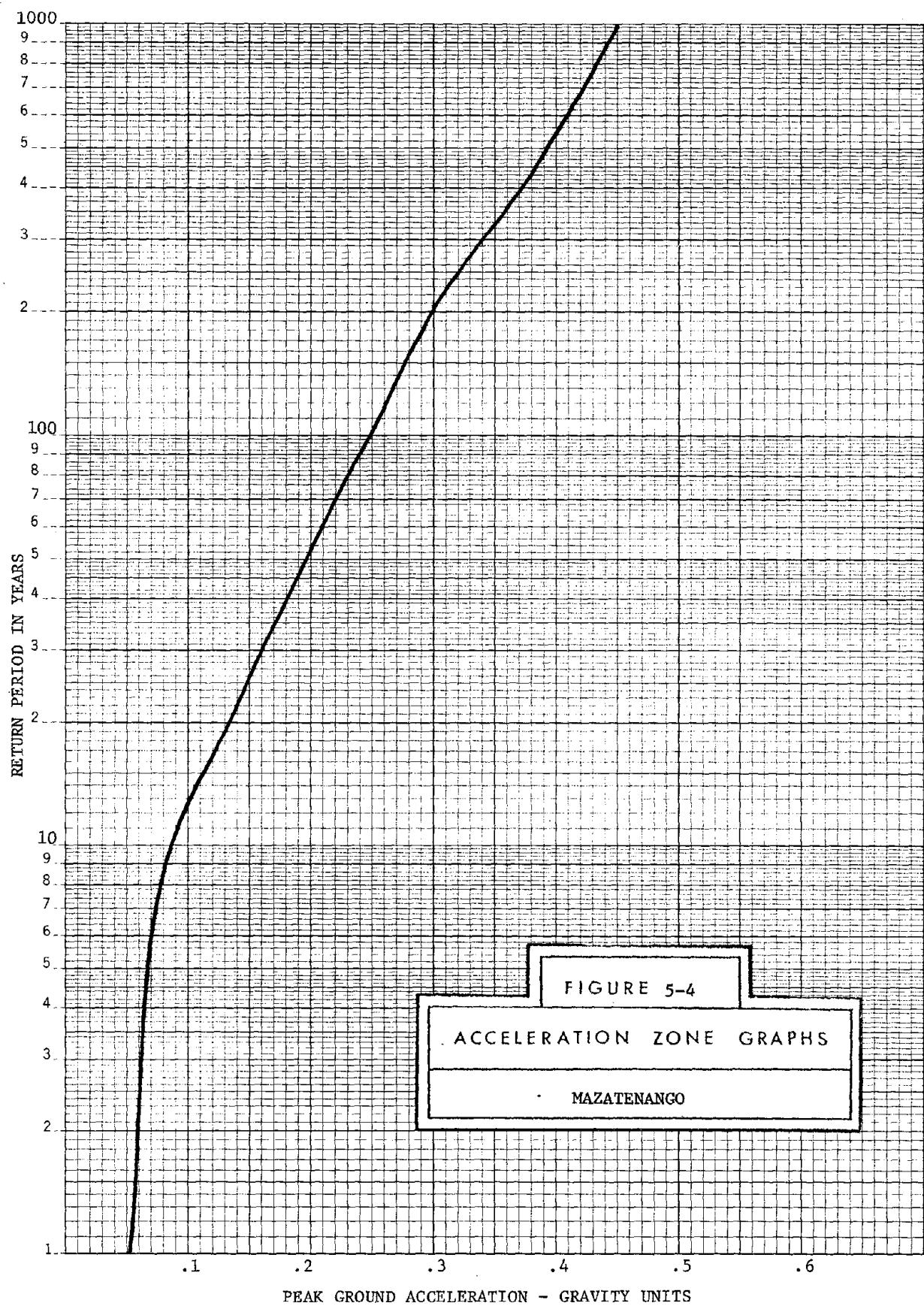
## V-2 Seismic Risk Zoning

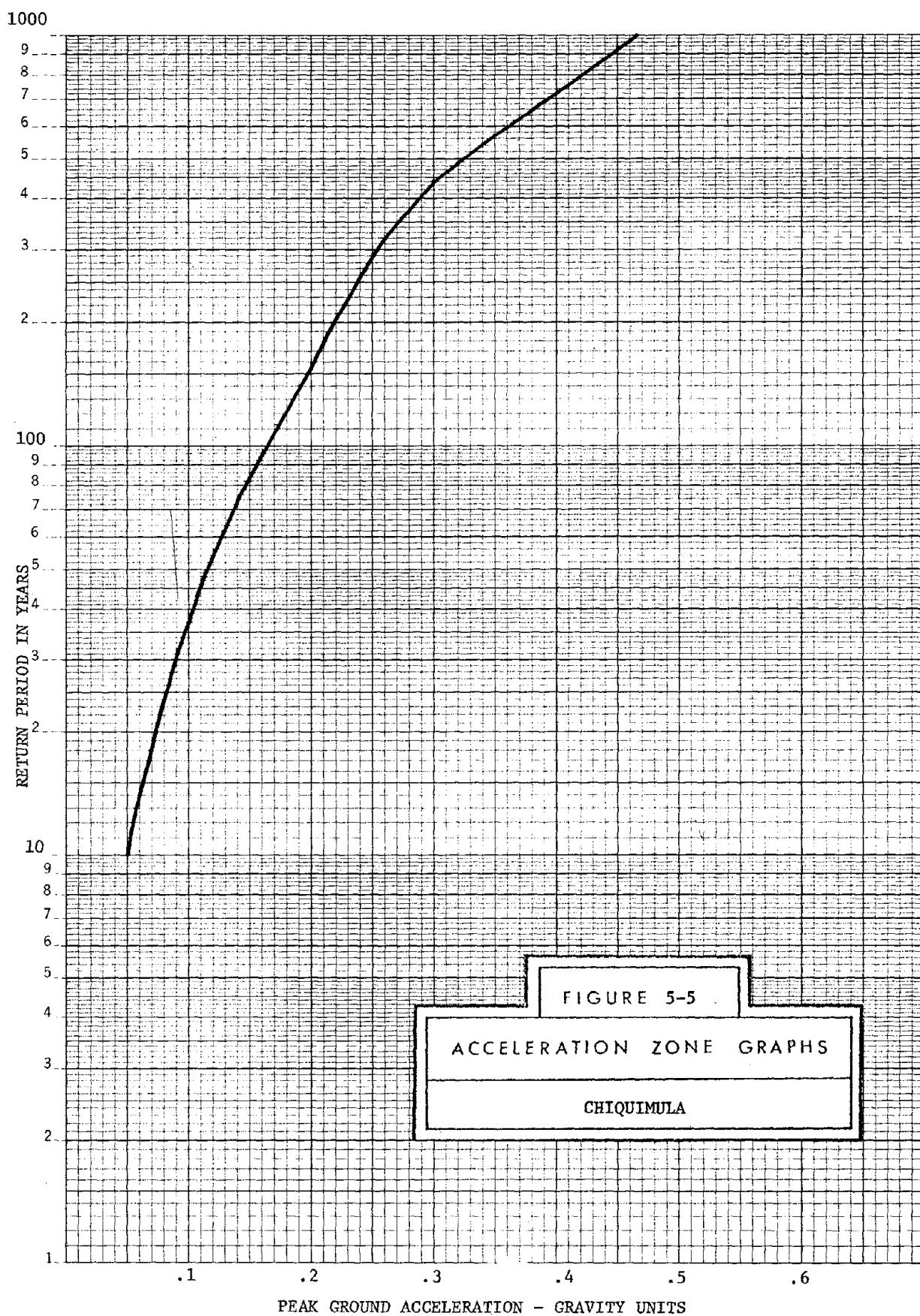
In the previous section, we have seen the relationship between the peak ground acceleration and the corresponding return period for different cities of Guatemala. However, these relationships by themselves do not help in selecting a return period for a given acceptable level of risk. The next step, in any seismic zoning procedure, is to obtain a relationship between the economic (or exposure) life of a structure, the level of risk one is willing to take, and the return period consistent with the

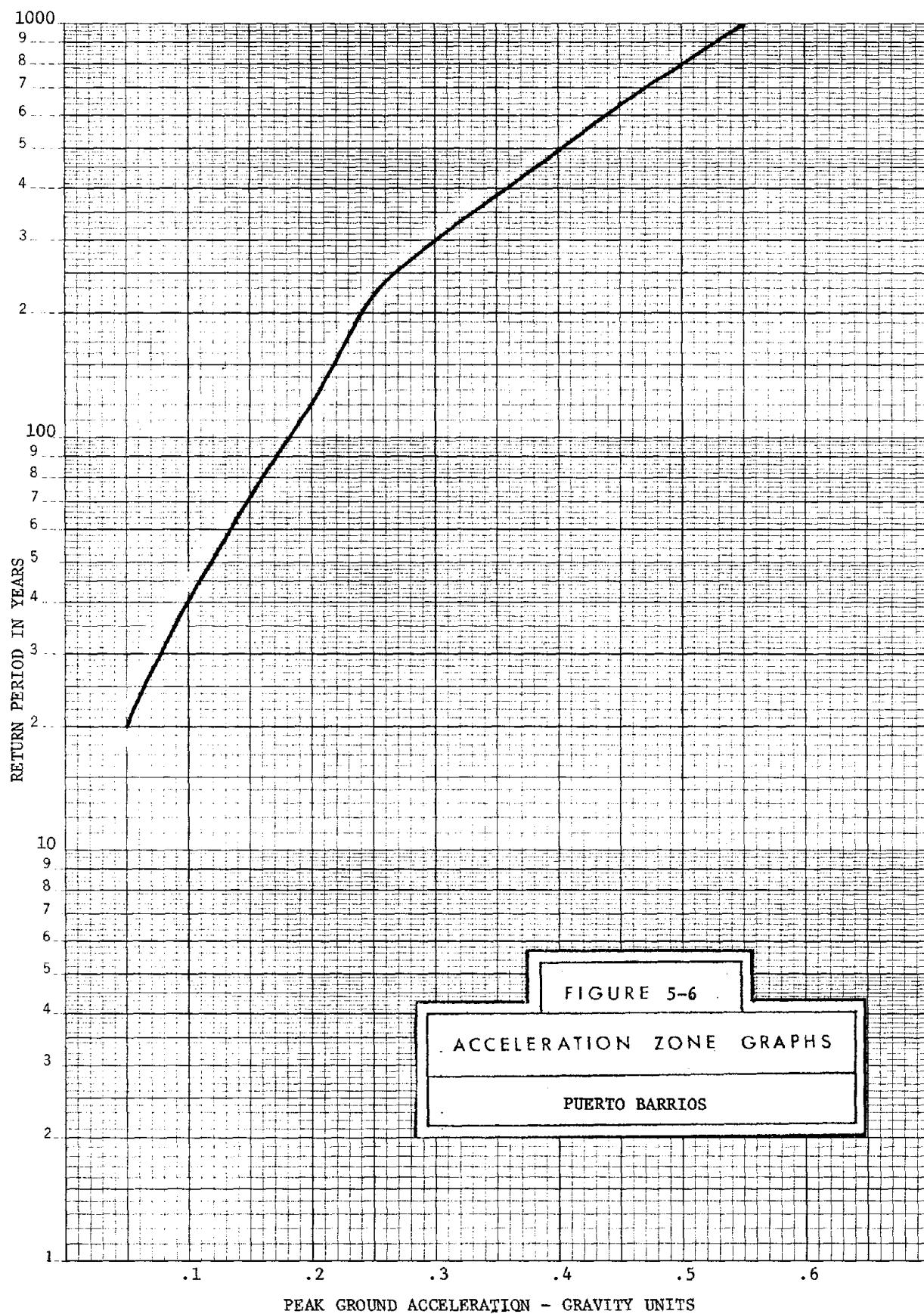


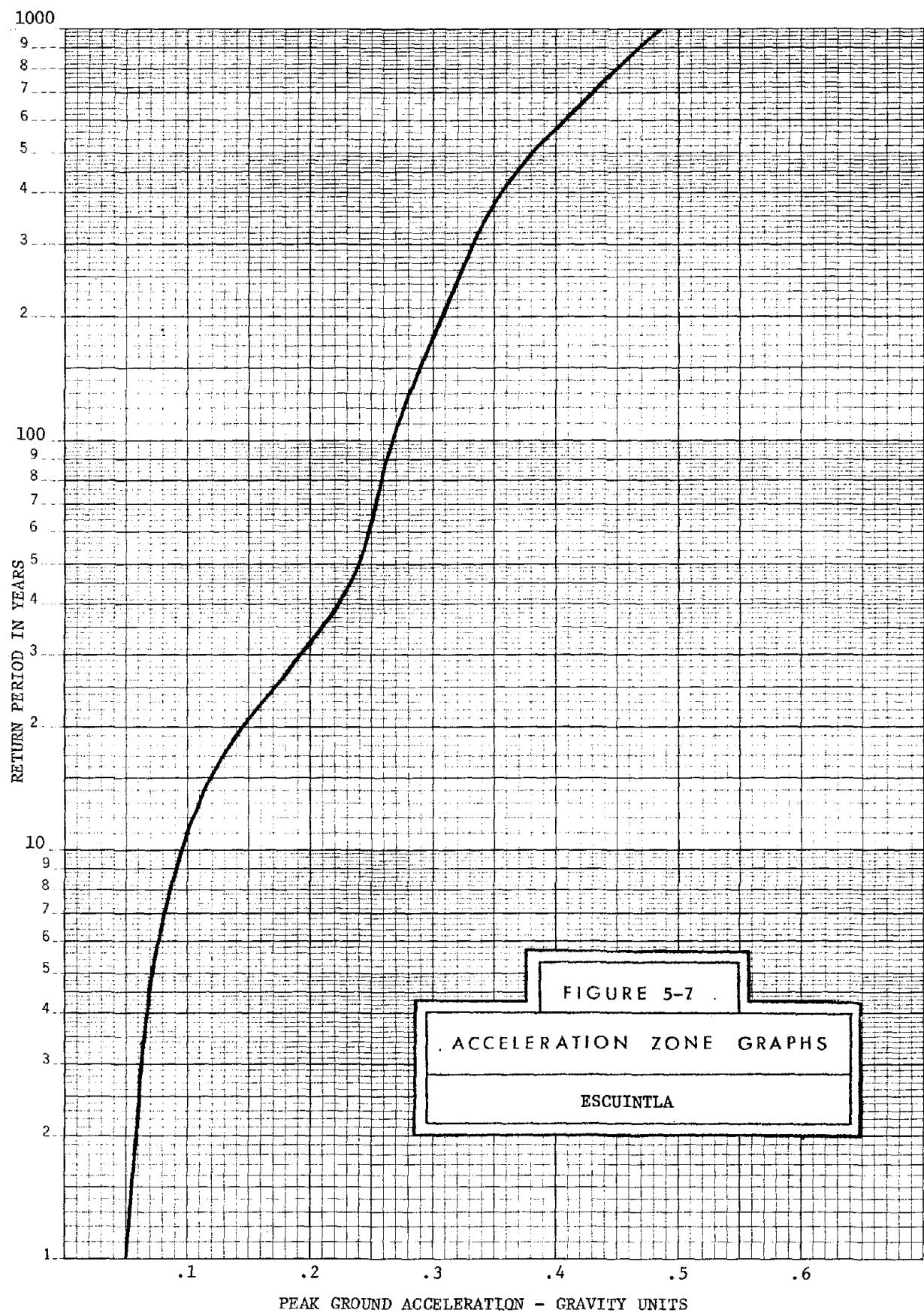


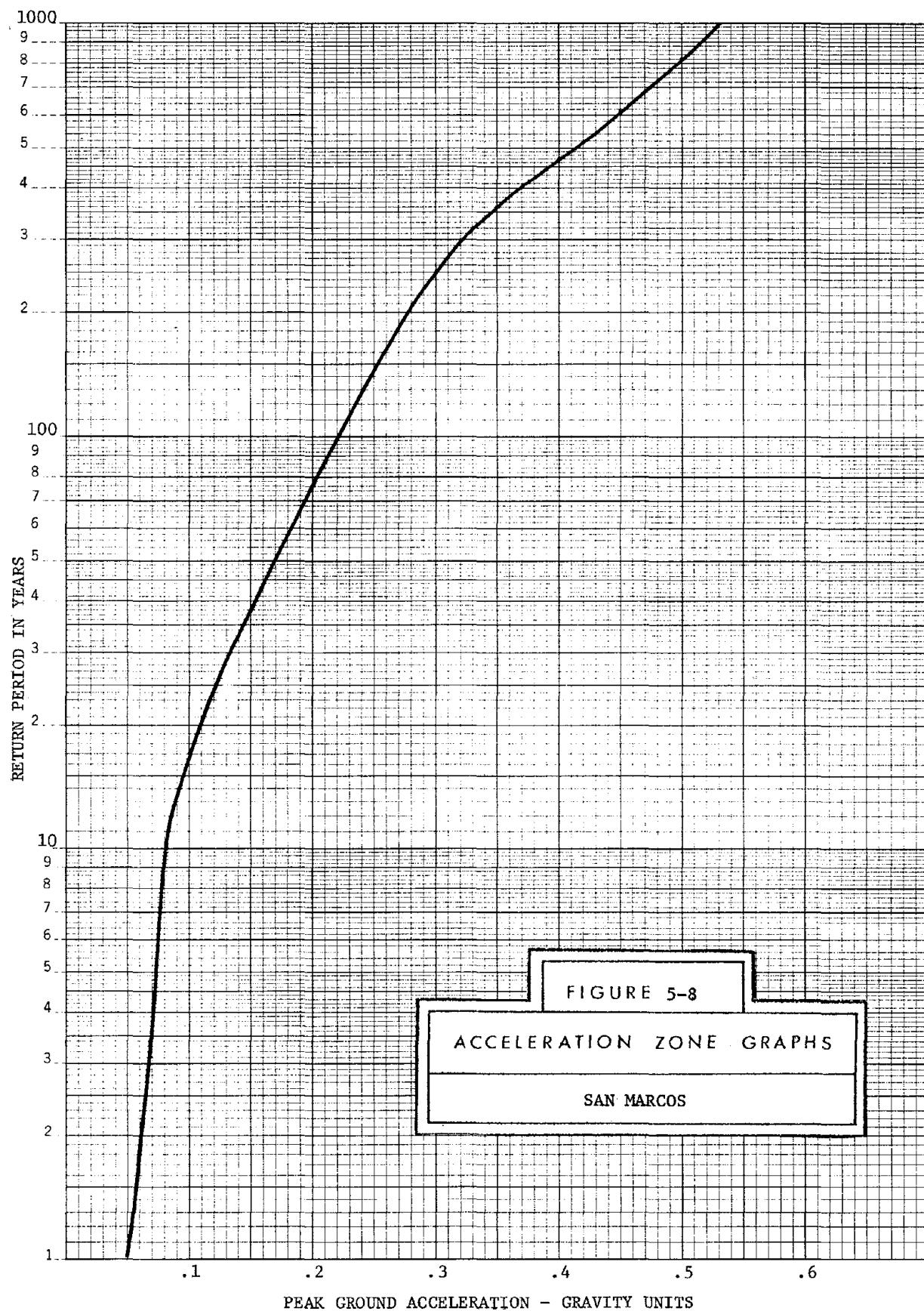


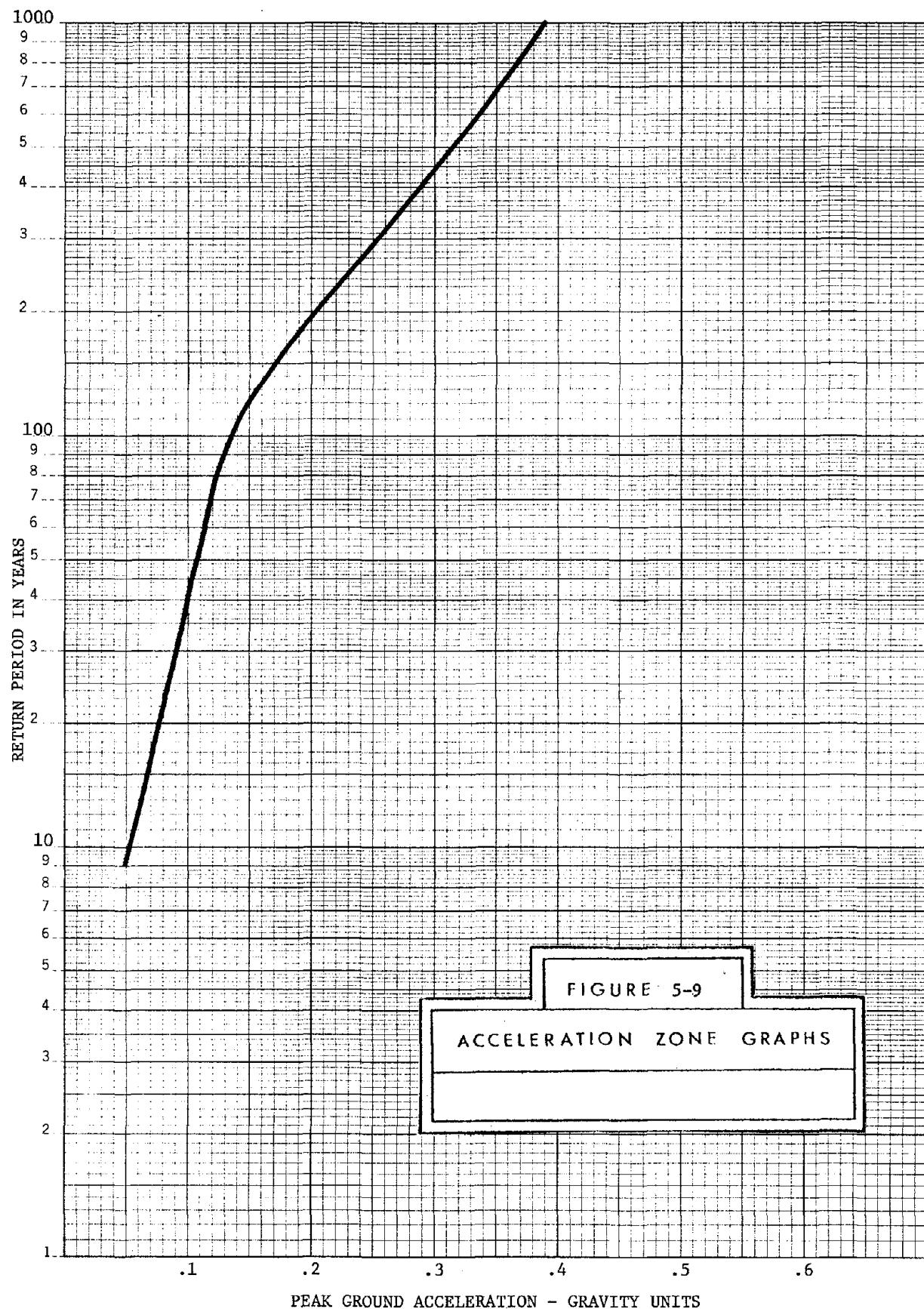












risk and economic life. Consider again the Binomial distribution. The probability of  $r$  successes in  $n$  independent Bernoulli trials, with probability  $p$  of success at each trial, is given by

$$p_n(r) = \binom{n}{r} p^r (1-p)^{n-r} \quad \text{Eq. 5-2 repeated}$$

Thus

$$\begin{aligned} p_{10}(0) &= \binom{10}{0} (p)^0 (1-p)^{10} \\ &= (1-p)^{10} = \text{probability of zero successes} \\ &\quad \text{in ten trials (years).} \end{aligned}$$

Let  $p(0) = (1-p)^{10}$  be equal to 0.90. Then the probability of no occurrence (or success) of a certain level of loading in ten years is given by 0.90.

or  $(1-p)^{10} = 0.90$

Hence  $p = .01048$

or return period RP = 95 years.

Thus, for a structure whose economic life is ten years, if the acceptable risk level is 10% of exceeding the specified loading level, then the structure should be designed for a return period of 95 years. Table 5-2 gives the relationship between acceptable risk level, economic life and return period. If, for example, the acceptable risk level is 20% for a structure whose economic life is 50 years, then the loading level should correspond to a return period of 225 years. If this structure is in Guatemala City, the corresponding peak ground acceleration level is approximately 0.39g. If the same facility for the same risk level is to be built in Puerto Barrios, the corresponding peak ground acceleration level should be approximately 0.25g. Thus, for a given class and use of structure,

having the same economic life (50 years) and same acceptable risk (20%), the two consistent values of peak ground accelerations in Guatemala City and Puerto Barrios are 0.39g and 0.25g. This is the concept of consistent risk design from one seismic region to another region of different seismicity. Figure 5-10 shows the graph relating the risk level, economic life and the return period. This particular graph is independent of any region and gives return periods only as functions of risk and economic life. Such graphs can easily be codified. Once the acceptable risk level for a given economic life is selected for a given class and use of a structure, the corresponding return period is immediately obtained from Figure 5-10. Then, based on the graph of return period vs. peak ground acceleration (similar to Figures 5-1 to 5-9), the loading at a site can be determined. Let us describe this concept of risk, economic life, return period and Acceleration Zone Graphs (AZG).

As an example, consider a design of a hospital facility. Assume that the exposure time or economic life of the system is 50 years. The peak ground acceleration level for which this facility should be designed for each of three different cities is to be determined. The cities are Guatemala City, Quezaltenango and Puerto Barrios. Assume that for the hospital, which is a critical facility that must remain functional after a seismic event, the acceptable level of risk corresponding to damage is 20%. Thus, whether the planned facility is in Guatemala City, Quezaltenango, or Puerto Barrios, the engineers are willing to accept a 20% chance of damage during the 50 years economic life of the structure. Then, from Figure 5-10, the return period corresponding to the 50 year economic life and 20% risk is 225 years.

Now refer to the AZG corresponding to Guatemala City (see Figure 5-2). The peak ground acceleration for a 225 year return period in Guatemala City

TABLE 5-2

Return Period as a Function of Economic Life and  
Probability of Non-exceedence

Probability of exceeding %	Economic Life Years					
	10	20	30	40	50	100
10	95	190	285	390	475	950
20	45	90	135	180	225	449
30	29	57	84	113	140	281
40	20	40	59	79	98	196
50	15	29	44	58	72	145
60	11	22	33	44	55	110
70	9	17	25	34	42	84
80	7	13	19	25	31	63
90	5	9	14	18	22	44
95	4	7	11	14	18	34
99	3	5	7	9	11	22
99.5	2	4	6	8	10	19

is 0.39g. Similarly, referring to the AZG for Quezaltenango and Puerto Barrios, the peak ground acceleration values corresponding to the 225 year return period are 0.29g and 0.25g, respectively. Thus, these three values of peak ground acceleration in the three different cities are consistent with the given acceptable risk.

As an alternate situation, consider two separate classes of structures to be built in Guatemala City. Let a school building with an economic life of 30 years have an acceptable risk level of 20%, and a warehouse with a ten year economic life have a 40% acceptable risk level. Referring to Figure 5-10, the return period for which the school should be designed is 135 years, and the return period for which the warehouse should be designed is 20 years. Again from the Guatemala City AZG (Figure 5-2), the corresponding peak ground acceleration values are 0.37g and 0.14g for the school and warehouse, respectively. If the same two facilities were to be located in Chiquimula, the corresponding peak ground acceleration values would be 0.19g and 0.07g. The major advantage of this method of zoning is that one can keep a consistent risk level from one region to another. Variations in the economic life and acceptable risk levels can be accounted for in arriving at a loading level through the return period transformation. Further application of the AZG to structural design will be presented in Part II of the total study.

It should also be pointed out that even though Puerto Barrios and Quezaltenango have smaller values of PGA corresponding to lower return periods (100 years and 200 years), the potential for major ground shaking -- above 0.5g -- does exist as can be seen from the AZG of these two cities. Thus, in terms of maximum credible ground shaking, Puerto Barrios and Quezaltenango are the two cities which should be kept in mind.

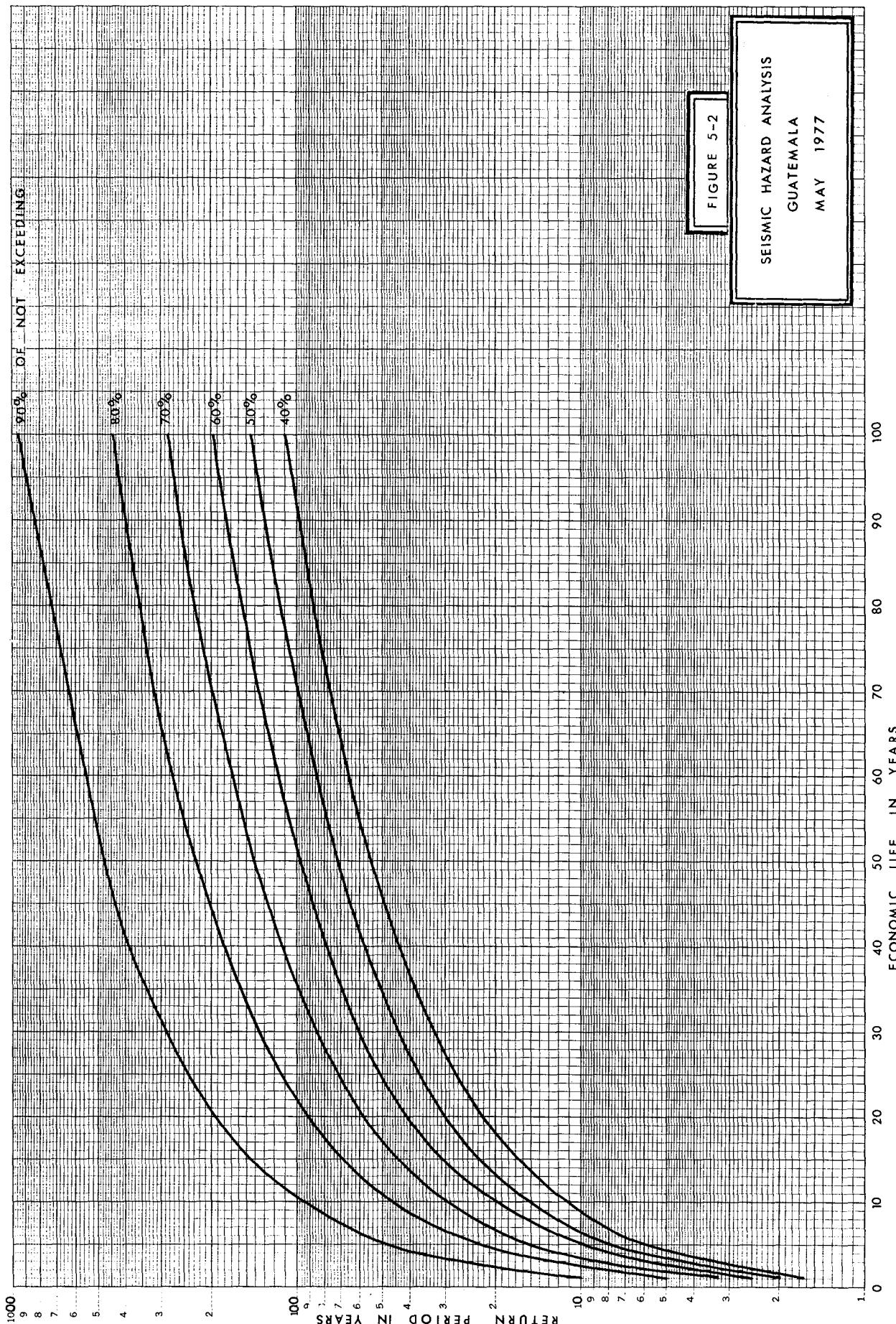


FIGURE 5-2

SEISMIC HAZARD ANALYSIS  
GUATEMALA  
MAY 1977

### V-3 Concluding Remarks on Seismic Hazard Maps

Unlike the older seismic zone maps (such as the 1973 Uniform Building Code "risk" map) the recommended hazard map takes into account the frequency of seismic events, the level of "risk" one is willing to take in selecting a specific peak ground acceleration value and the future time horizons for which one wishes to consider the economic or structural life of the facility being designed.

Various questions come up regarding the reliability and long range stability of such hazard maps. Some of the questions are:

1. How reliable are the maps that are developed based on only historical data?
2. How stable are such maps? In other words, will these hazard maps change dramatically with each new future seismic event?
3. Is the formulation such that any new information available in the future can be incorporated to update the hazard maps?
4. What is the effect of local site conditions on the values obtained from these maps?

These and many such questions were discussed in Shah et al. (1975). However, in summary, the following responses can be given to the four questions posed above.

With respect to the reliability of results based on historical data, it is felt that for engineering and planning purposes and for seismic code formulation, the results presented are sufficiently reliable. The usual economic life of any engineered facility is usually less than 100 years to 200 years. In terms of geological time spans, this is a short period. Hence, we can assume that the geological processes during this short period are at a steady state. Hence, any information available from historical data can be extrapolated into similar time spans in the future. This discussion does not mean to imply that there are no errors introduced.

This possibility always exists. However, to wait for a complete geological information before developing a "seismic load" criteria for a country is unrealistic and impractical.

Concerning the stability of the hazard map, it is felt that the results presented here are quite stable. As long as the future seismic events can be assigned to any one of the sources considered in this work, the shape of the maps as well as the level of PGA's suggested should not change substantially. The only time the maps should be updated and changed is when a major seismic event occurs in a region where no previously known seismic source existed. In that case, the formulation and the computer programs are such that the suggested maps can be readily updated with the new information incorporated. Thus, in reply to the third question, such maps could be updated very easily. As a general recommendation, it is felt that such maps should be updated every five to seven years.

Effect of local site conditions (micro-characteristics) is usually felt in the amplitude of vibrations and in the frequency content of the vibration. The hazard map developed here is based on "average" soil condition. Thus, no site specific information is included in their development. In Part II of this study the effect of soft soil will be introduced. by changing the shape of the response spectrum to include higher period components. However, it should be pointed out that for important facilities such as dams, power plants, hospitals, etc., a site specific study should be conducted. Such information can then be used to modify the values suggested by the hazard map of this chapter.

In conclusion, it can be said that the seismic ground shaking hazard information developed in this study represents "a state-of-the-art" engineering solution. It is not the total information but it is one of the best that can be developed with the available knowledge and resources.

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## CHAPTER VI

### SUMMARY, CONCLUSION AND FURTHER RESEARCH

#### Scope

This chapter summarizes the work done in this phase of the study regarding the seismic hazard mapping of Guatemala. The conclusions drawn from this work are presented and a summary look at the phase 2 of this study is introduced.

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#### VI-1 Summary of Work on Seismic Hazard Mapping

In evaluating the future seismic hazard for Guatemala, the general geologic setting of the country, the seismological environment and all the available historical data was reviewed in this report. Two types of models were used in developing the seismic hazard maps (or iso-acceleration maps) of the country. The first model was the Poisson occurrence model where the seismic recurrence for various sources was based on past earthquake data and on a hypothesized upper Richter magnitude. Chapter IV presents these maps for four separate return periods. In Part II of this study, the relevance of obtaining iso-acceleration maps for the four return periods (50, 100, 500 and 1000 years) will be discussed. The Bayesian model, where subjective and objective information on the number of occurrences and the magnitude level is used, is presented in the Appendix B. For this model the attenuation relationship is assumed to be probabilistic with Uniform distribution and the scatter of this distribution is represented by the coefficient of variation. Iso-acceleration maps developed using this Bayesian model are also given in Appendix B. For further details on this

model and its application see Mortgat (1976) and Mortgat et al. (1977).

Using the Bayesian model, iso-duration maps are also presented in the Appendix B. At this time, explicite use of duration in seismic design decisions is not available. However, it is felt that this information is an important engineering input and will be of great interest to planners and engineers.

Based on the information from iso-acceleration maps, the acceleration zone graphs (AZG) for the following eight major urban areas was developed.

1. Guatemala City
2. Quezaltenango
3. Mazatenango
4. Chiquimula
5. Puerto Barrios
6. Esquintla
7. San Marcos
8. Copan

These acceleration zone graphs can be used to evaluate the level of peak ground acceleration as a function of return period. In a future report on phase 2 of this study, the suggested levels of peak ground accelerations for various regions of the country and for various types and uses of structures will be presented.

#### VI-2 Conclusions from the Present Study

It is very interesting to note that the shape of the iso-acceleration maps obtained by using the Bayesian model (Appendix B) and the Poisson occurrence model are very similar. This is due to the following reasons:

- 1) The data base used for both models is the same.
- 2) The subjective data of the Bayesian model was developed from the regression line of the recurrence curve obtained in the Poisson occurrence model.

The value of the peak ground acceleration obtained using the Bayesian model will increase with the increase in attenuation uncertainty. The coefficient of variation used in the Appendix B maps was 0.3. Looking at this similarity and based on experience obtained in developing Nicaragua iso-acceleration maps (Shah et al., 1975) and Costa Rica iso-acceleration maps (Mortgat et al., 1977) it is reasonable to accept the iso-acceleration maps for various return periods presented in Chapter IV.

The iso-duration maps given in Appendix B can be used to evaluate damage potential and energy content of future hypothesized or forecasted seismic events.

It can be concluded from the results presented in this report that for engineering planning, sufficient data are available (together with geological information and subjective knowledge) to provide seismic "zoning" information for Guatemala. The methods presented in this report can use either objective or subjective seismological data. The method is simple and has the capabilities of being updated at regular intervals. It can be seen from the iso-acceleration maps of Chapter IV that the seismicity in the northern part of Guatemala is low. The high seismic region of the south-southwestern part of Guatemala reaches a peak near the intersection of the Motagua fault, the Benioff Zone and the Mixco fault. The probable seismic "loading" in Guatemala City, based on the available data and the resultant iso-acceleration maps, is high. In comparison to the Managua region of Nicaragua, the Guatemala City region is very similar. The seismic lateral load requirement for these various levels of peak ground accelerations will be developed in Part II of this study.

In Chapter V, the reliability and stability of the iso-acceleration maps was discussed. It should be noted once more here that the methodology available now can take into account not only the short historical data,

but also any geological or seismological information that may be available. It is felt by the authors of this report that such maps should be updated every five years so that any new information that will be available can be incorporated in the updated version.

In conclusion, it can be said that the seismic ground shaking hazard information in the form of iso-acceleration and iso-duration maps developed in this study represents the "state-of-the-art" methodology and engineering solution. It is consistent with the available information. Thus, its shortcomings are also consistent with the shortcomings of the available data. It is always possible that the results obtained by using the past knowledge can always be proven to be incorrect by the nature.

#### VI-3 Further Research

In order to implement and use the seismic hazard information presented in this report, the following topics and tasks will be accomplished in part II of this study:

- A thorough study of the 1973 UBC, the 1976 UBC and the ATC-3 work currently being completed. The above codes will be evaluated with the seismicity of Guatemala in mind. Such an evaluation will permit the engineers and planners in Guatemala to appreciate and understand the current code levels and their relationships with future seismic loading demands.
- A detailed discussion on the purpose and effects of earthquake codes will be presented. This will include the history of earthquake loading criteria, the relation of design loads and quality of structures, the objectives and qualities of workable seismic codes and the role of design detailing and design forces in providing a safe economic construction.

- Introduction to the Proposed Seismic Design Provisions.

This will take into account the current codes, their advantages, their shortcomings and the available solutions to eliminate or reduce the shortcomings. This part of the study will explain in detail the concept of acceptable risk and the associated loading levels from the hazard maps of Guatemala. The study will also develop the shape and levels of various design spectra. These spectra will include the site characteristics information.
- A detailed description of the type of structural systems, their effects on the design level and a step-by-step design procedure will be developed.
- Based on the spectral approach of seismic design, a simplified equivalent static load method, similar to the 1976 UBC method of design, will be developed. It should be emphasized here that a workable code should have the following four ingredients.
  - 1) Simplicity.
  - 2) Rationality.
  - 3) Freedom to use responsible ingenuity for special structures.
  - 4) Reward and encouragement for using dynamic analysis when merited by the complexity of a given structure.
- In developing the methodology in Part II of this study, the above four ingredients will be kept in mind.
- Finally, a detailed comparison with the proposed methodology and the 1976 UBC will be made. This will be done with the seismic environment of Guatemala (as obtained in this report) in mind.

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APPENDIX A

POISSON MODEL

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## APPENDIX A

### POISSON MODEL

#### A-1 Poisson Model of Seismic Occurrences

As mentioned in Chapter II, earthquake occurrences can be modeled using the Poisson probability law. For earthquake events to follow the Poisson Model, the following assumptions must be valid:

- (1) Earthquakes are spatially independent;
- (2) Earthquakes are temporally independent;
- (3) Probability that two seismic events will take place at the same place and at the same instant of time approaches zero.

These assumptions are necessary for the formulation of the Poisson Model.

The first assumption implies that occurrence or nonoccurrence of a seismic event at one site does not affect the occurrence or nonoccurrence of another seismic event at some other site. The second assumption implies that the seismic events do not have memory in time. A Markovian assumption of one-step memory in time may be a better assumption, but as mentioned previously, this assumption for large events does not introduce major errors (Gardner and Knopoff, 1974). The third assumption implies that for a small time interval,  $\Delta t$ , no more than one seismic event can occur. This assumption is considered to be realistic and to fit the physical phenomenon.

For events which satisfy the above assumptions, the Poisson law can be written as

$$P_n(t) = \frac{e^{-\lambda t} (\lambda t)^n}{n!}$$

A-1

where  $P_n(t)$  = Probability of having n events in time period t.

n = Number of events.

$\lambda$  = Mean rate of occurrence per unit of time.

In Chapter III, it was shown how the mean number of occurrences above magnitude M for a given source can be obtained using recurrence relationships. This relationship in its general form can be stated as

$$N(M) = \phi(M, A, T) \quad A-2$$

where  $N(M)$  = Number of occurrences above Richter magnitude M.

M = Richter magnitude.

A = Source characteristic (area for area source, length for line source).

T = Time period of data base.

As mentioned in Chapter III, a log-linear recurrence relationship is assumed for all sources. Also, for some sources, the relationship is bi-linear (two lines described by  $\alpha_1$ ,  $\beta_1$ , and  $\alpha_2$ ,  $\beta_2$ ). (See Table 3-3 of Chapter III.) Thus, for a given source, the two lines describing the recurrence relationship are given by:

$$\ln N'(M) = \alpha_1' + \beta_1 M \quad 0 \leq M \leq M_1 \quad A-3$$

$$\ln N'(M) = \alpha_2' + \beta_2 M \quad M_1 \leq M \leq M_2$$

where  $M_1$  is the magnitude at which the two recurrence lines intersect (see, for example, Fig. 3-4).

$M_2$  is the upper cutoff magnitude for a given source (see Table 3-3, Chapter III).

Thus, depending on the source and the value of M, the mean number of events above magnitude M for a unit area for an area source, a unit length for a

line source, and a unit time is given by:

$$N'(M) = \exp [\alpha_i' + \beta_i M] \quad A-4$$

From equation A-1 it follows that

$$P_n(t) = \frac{\exp [-\exp (\alpha_i' + \beta_i M)t] [\exp (\alpha_i' + \beta_i M)t]^n}{n!} \quad A-5$$

Note that in equation A-5 above,  $\lambda$  is replaced by  $N'(M)$ . Equation A-5 gives the probability of observing  $n$  events above magnitude  $M$  in time period  $t$ , based on the seismic history of a given source.

#### A-2 Source Mechanisms

Three different types of sources can be used to represent the seismicity of any region. They are point, line, and area sources. All three source mechanisms will be discussed for generality and completeness of the development, although only line sources were considered for the Guatemala region.

##### a. Point Source

For this type of source, all occurrences (past and future) take place at one point. The recurrence relationship can be normalized with respect to time  $T$  as follows:

$$N'(m) = \frac{N(m)}{T} \quad A-6$$

Substituting the value of  $N'(m)$  in the Poisson law, Equation A-1 becomes:

$$P_n[M > m, t] = \frac{\exp [-N'(m)t] [N'(m)t]^n}{n!} \quad A-7$$

where the notation

$P_n[M > m, t]$  gives the probability that there will be  $n$  events

of Richter magnitude greater than  $m$  in time period  $t$ .

For engineering purposes, it is of primary interest to determine the probability of at least one event greater than  $m$  in time period  $t$ . The probability is given by

$$\begin{aligned} P(\text{at least one event of magnitude } M > m \text{ in time } t) \\ = 1 - P(\text{no earthquake of magnitude } M < m \text{ in time } t) \end{aligned}$$

Hence, from Equation A-7

$$\begin{aligned} P(\text{at least one event of magnitude } M > m \text{ in time } t) \\ = 1 - \exp [-N'(m)t] \end{aligned}$$

b. Line Source

In the case of a line source, it is assumed that epicenters lie along a linear fault pattern. For a line source of length  $L$  (fault length  $L$ ) and a data base for a time period  $T$ , the recurrence relationship can be normalized to:

$$N'(m) = \frac{N(m)}{LT} \quad (\text{Equation 3-3b repeated})$$

This  $N'(m)$  can be substituted directly in Equation A-8 to obtain the probability of at least one event of magnitude  $M > m$  in time  $t$ .

c. Area Source

When the past earthquake epicenters do not lie on a line (i.e., along a given fault line) or when there is no information on fault locations, but earthquake events are scattered over a region, the seismic source should be con-

considered as an area source. The area source can be approximated by a full circle or any section of a circle in which the epicenters are scattered. In this case, the recurrence relationship is normalized with respect to the area A and the time of the data base T.

$$N'(m) = \frac{N(m)}{AT} \quad (\text{Equation 3-3a repeated})$$

Again, the probability of at least one event due to this area source above magnitude m in time period t is given by

$$P(\text{at least one } M > m \text{ in time } t) = 1 - \exp [-N'(m)t]$$

It should be noted that while Equation A-8 has the same form for a point source, line source and area source, in each case the normalized  $N'(m)$  has a different interpretation.

#### A-3 Peak Ground Acceleration at a Site

For design purposes, it is necessary to know the loading at a site. As mentioned in Chapter III, the most commonly used parameter to describe the seismic loading at a given site is the peak ground acceleration. In order to derive the probability distribution on peak ground acceleration at a site for a future time period t, the following information is required:

1. Probabilistic formulation on Richter magnitude for a source as a function of future time t.
2. Distance from the source to the site.
3. Attenuation of peak ground acceleration from source to site.

The first parameter has already been determined in the previous section.

Various attenuation formulae are available which give the relationship

between the Richter magnitude  $M$ , the epicentral distance or the hypocentral distance, and the peak ground acceleration. Donovan (1974) has summarized and compared ten attenuation relationships. These relations are repeated in Table A-1 and their graphs are shown in Figure A-1. Most of the relationships in Table A-1, can be written in the following general form:

$$a = \frac{b_1 \exp(b_2 m)}{(R_h + b_4)^{b_3}} \quad A-9$$

where  $a$  = Peak Ground Acceleration (PGA) in  $\text{cm/sec}^2$  or  $g$

$R_h$  = Hypocentral distance from source to site in km or miles

$m$  = Richter magnitude

$b_1$ ,  $b_2$ ,  $b_3$  and  $b_4$  are constants depending on the region

Using the general form of the attenuation relationship given by Equation A-9, the probability distribution on peak ground acceleration for a site due to the three types of seismic sources can be obtained.

#### Point Source

The probability of at least one event greater than  $m$  in time  $t$  from a point source such as the one shown on Figure A-2 is:

$$P[M > m, t] = 1 - \exp[-N'(m)t] \quad A-10$$

Substituting the recurrence relationships given by Equation 3-2, the probability distribution is derived as follows:

$$P[M > m, t] = 1 - \exp[-\exp(\alpha' + \beta m)t] \quad A-11$$

where  $\alpha'$  = normalized regression constant.

TABLE A-1  
Attenuation Equations

<u>Data Source</u>	<u>Equation</u>	<u>Reference</u>
1. San Fernando Earthquake February 9, 1971	$y = 186206 R^{-1.83}$	--
2. California Earthquakes	$y = \frac{981 y_o}{1 + \frac{R'}{h}}^2$ where $\log y_o = -(\bar{b}+3) + 0.81m - 0.027m^2$ $\bar{b}$ is a site factor	Blume (1965)
3. California Earthquakes	Graphical Presentation	Housner (1965)
4. California & Japanese	$y = \frac{5}{T_G} 10^{0.61m - P \log R + Q}$ where $P = 1.66 + \frac{3.60}{R}$ $Q = 0.167 - \frac{1.83}{R}$ $T_G$ = fundamental period of site	Kanai (1966)
5. Cloud (1963)	$y = \frac{6.77 e^{1.64m}}{1.1e^{1.1m} + R^2}$	Milne & Davenport (1969)
6. Cloud (1963) Housner (1962)	$y = 1230 e^{0.8m} (R+25)^{-2}$	Esteva (1970)
7. U.S.C. & G.S.	$\log_{10} y = 6.5-2 \log_{10} (R'+80)$	Cloud & Perez (1971)
8. 11 Selected Records	Graphical Presentation	Schnabel & Seed (1972)
9. 303 Instrumental Values	$y = 1300 e^{0.67m} (R+25)^{-1.6}$	--
10. Western U. S. Records	$y = 18.9 e^{0.8m} (R^2+400)^{-1}$ $y$ is cm/sec <sup>2</sup> $R$ is kilometers (distance to causative fault) $R'$ is miles (epicentral distance) $h$ is miles (focal depth) $m$ is magnitude	--

\* Taken from Donovan 1974

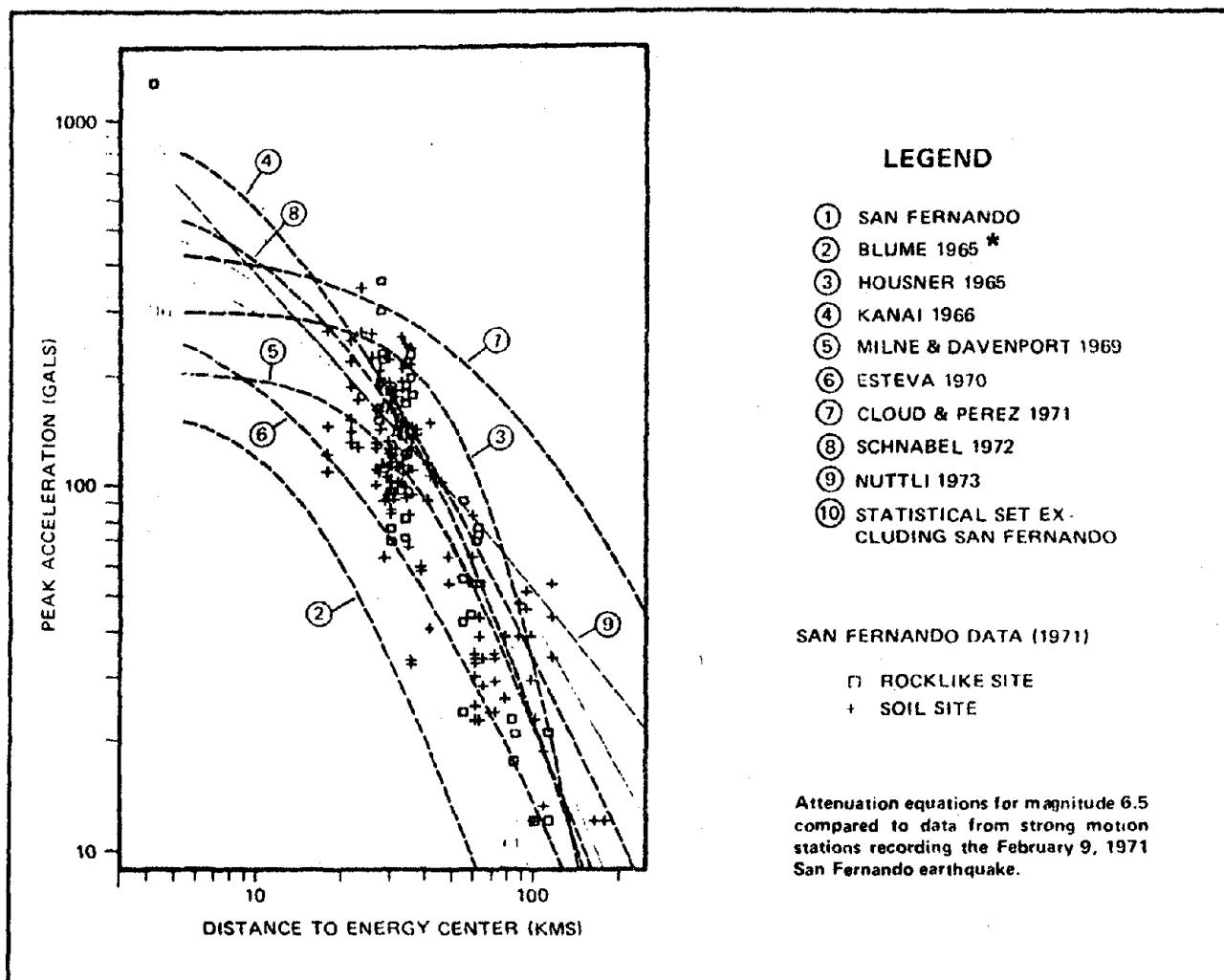


FIGURE A-1a Attenuation Relationships.  
(Taken from Shah et al., 1975).

\* This relation can provide results which are very close to mean data behavior if the soil characteristics for the region are recognized. The soil characteristics input was not used in the preparation of this figure from Shah et al., 1975.

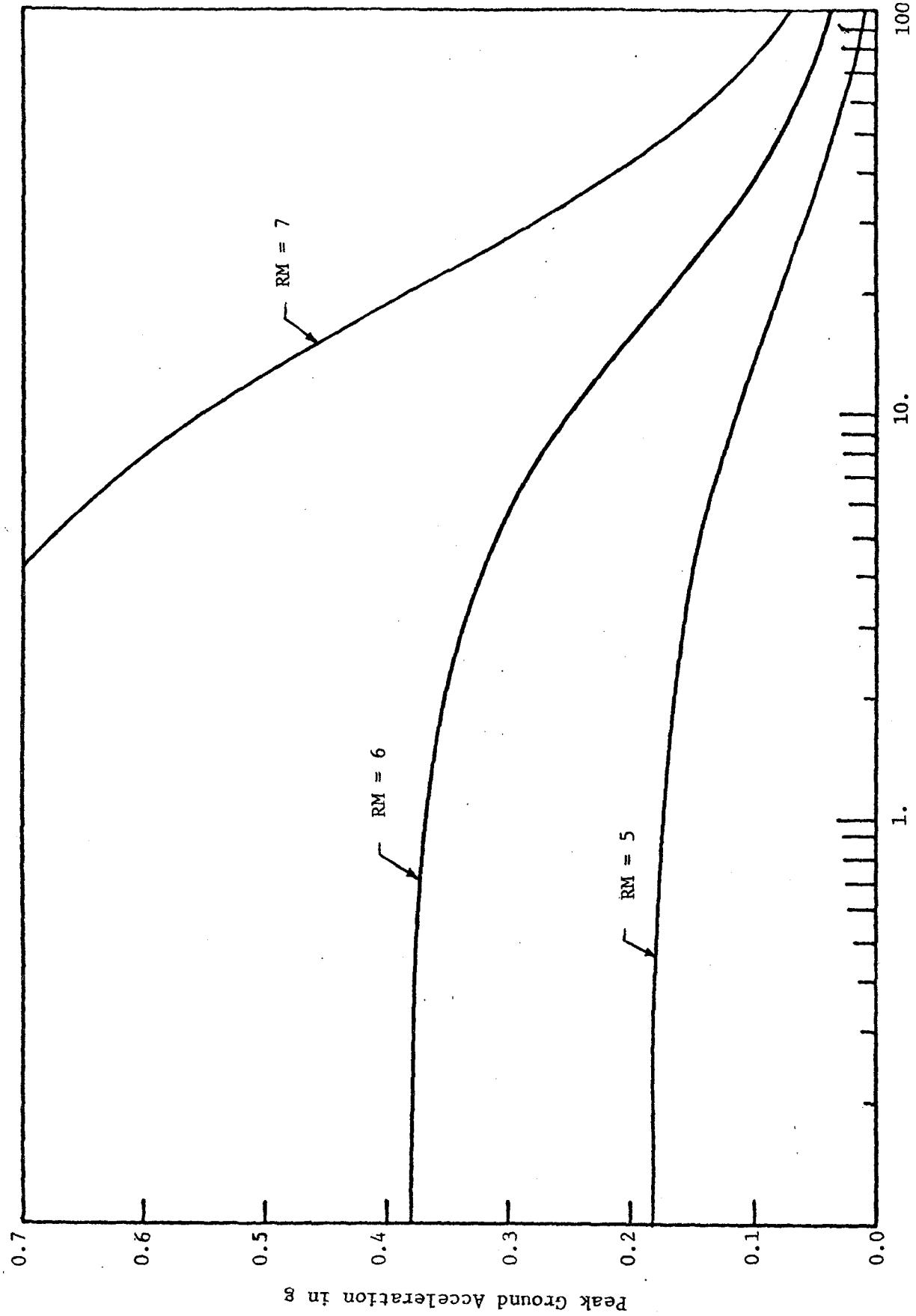


FIGURE A-1b Esteva's 1973 Attenuation Relationship  
Hypocentral Distance -  $R_h$  in Km.

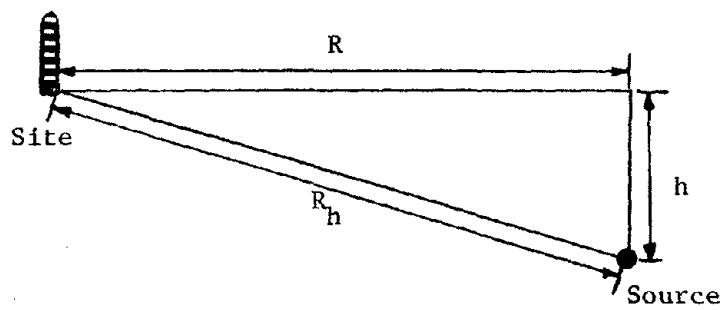
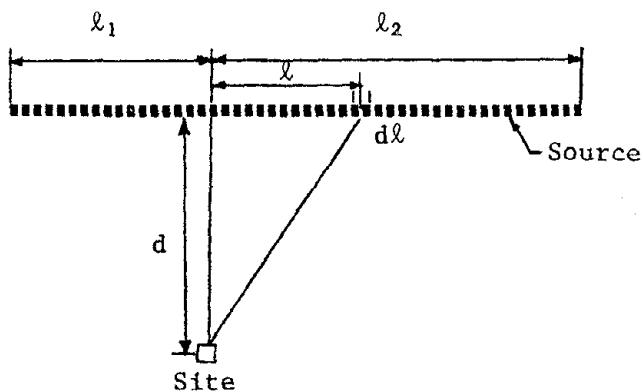


FIGURE A-2 Point Source: Top View



(a.)

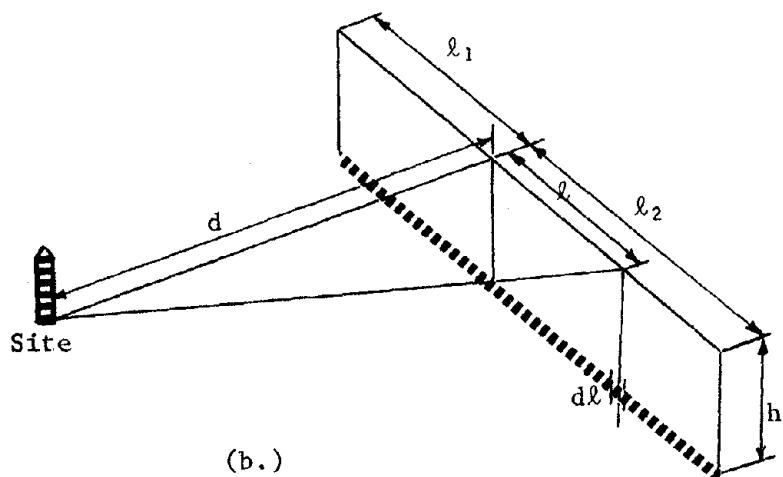


FIGURE A-3 Line Source: (a.) Top View; (b.) Isometric View

To determine the probability distribution on peak ground acceleration  $a$ , the attenuation relationship is used as shown below:

$$\begin{aligned} P[A > a, t] &= P\left[\frac{b_1 \exp(b_2 M)}{(R_h + b_4)^{b_3}} > a, t\right] \\ &= P\left[M > \ln\left[\frac{a}{b_1}\left[R_h + b_4\right]^{b_3}\right]^{\frac{1}{b_2}}, t\right] \end{aligned} \quad A-12$$

The last expression in Equation A-12 can be recognized as the distribution on magnitude. Thus, Equation A-12 becomes

$$P[A > a, t] = 1 - \exp\left[-e^{\alpha'}\left[\frac{a}{b_1}\right]^{\frac{\beta}{b_2}}\left[R_h + b_4\right]^{\frac{\beta b_3}{b_2}}t\right] \quad A-13$$

By denoting

$$\gamma = e^{\alpha'} \quad A-14$$

$$\delta = \frac{\beta}{b_2} \quad A-15$$

$$\rho = \frac{\beta b_3}{b_2} = \delta b_3 \quad A-16$$

one finally obtains

$$P[A > a, t] = 1 - \exp\left[-\gamma\left[\frac{a}{b_1}\right]^{\delta}\left[R_h + b_4\right]^{\rho}t\right] \quad A-17$$

#### Line Source

Most of the earthquake epicenters around the world are generally located along the major fault systems. Thus, the usual case of epicenters falling along a line gives rise to the so-called line source. The line source can be divided into  $\kappa$  small segments of length  $\Delta l_i$ ,  $1 \leq i \leq \kappa$ .

Let  $E_{\lambda_i}$  be the event that no earthquake with a Richter magnitude greater than  $m$  occurs on the element  $\Delta\ell_i$  in time  $t$ . Then, from the Poisson condition on spatial independence the following is true:

$$P \left[ \sum_{i=1}^K E_{\lambda_i} \right] = \sum_{i=1}^K P [E_{\lambda_i}] \quad A-18$$

The probability of having an earthquake of Richter magnitude smaller than or equal to  $m$  due to the entire line source is:

$$\begin{aligned} P[M \leq m, t] &= \lim_{\Delta\ell \rightarrow 0} \sum_{i=1}^K P [E_{\lambda_i}] = \lim_{\Delta\ell \rightarrow 0} \exp \left[ - \sum_{i=1}^K N'(m) \Delta\ell_i \cdot t \right] = \\ &= \exp \left[ - \int_{\ell_1}^{\ell_2} N'(m) d\ell \cdot t \right] \quad A-19 \end{aligned}$$

where  $\Delta\ell = \max_{1 \leq i \leq K} \Delta\ell_i$

The log-linear recurrence relationship is recalled to be

$$\ln N'(m) = \alpha' + \beta M$$

where  $N'(m)$  and  $\alpha'$  are normalized with respect to the length of the line source and the time period of the data. Substituting for  $N'(m)$  into Equation A-19 one gets:

$$P[M \leq m, t] = \exp \left[ - \int_{\ell_1}^{\ell_2} \exp [\alpha' + \beta m] d\ell \cdot t \right] \quad A-20$$

To obtain the probability of having a maximum peak ground acceleration  $a$ , the attenuation relationship (Equation A-9) is used as follows:

$$\begin{aligned}
P[A \leq a, t] &= P\left[\frac{b_1 \exp(b_2 M)}{[R_h + b_4]^{b_3}} \leq a, t\right] = P\left[M \leq \ln\left[\frac{a}{b_1}\right]^{\frac{1}{b_2}} \left[R_h + b_4\right]^{\frac{b_3}{b_2}}, t\right] \\
&= \exp\left[-e^{\alpha'}\left[\frac{a}{b_1}\right]^{\frac{\beta}{b_1}} \int_{\ell_1}^{\ell_2} [R_h + b_4]^{\frac{\beta b_3}{b_2}} d\ell + t\right]
\end{aligned} \tag{A-21}$$

Letting  $e^{\alpha'}$ ,  $\frac{\beta}{b_1}$  and  $\frac{\beta b_3}{b_2}$  be denoted by  $\gamma$ ,  $\delta$  and  $\rho$ , respectively, and

noting from the geometry of the line source (Figure A-3) that

$$R_h = [d^2 + \ell^2 + h^2]^{1/2} \tag{A-22}$$

one can write:

$$P[A \leq a, t] = \exp\left[-\gamma\left[\frac{a}{b_1}\right]^{\delta} t \int_{\ell_1}^{\ell_2} \left[\left[d^2 + \ell^2 + h^2\right]^{1/2} + b_4\right]^{\delta} d\ell\right] \tag{A-24}$$

Alternatively,

$$P[A > a, t] = 1 - \exp\left[-\gamma\left[\frac{a}{b_1}\right]^{\delta} t \int_{\ell_1}^{\ell_2} \left[\left[d^2 + \ell^2 + h^2\right]^{1/2} + b_4\right]^{\delta} d\ell\right] \tag{A-25}$$

### Area Source

The peak ground acceleration cumulative distribution function due to an area source at a site can be obtained in a manner similar to that for the line source. Figure A-4 shows schematically the geometry of the area source. Consider an elemental area  $\Delta A_i = R_i \Delta R_i \Delta \theta_i$ . Let  $E_{A_i}$  be the event that no earthquake occurring on the element  $\Delta A_i$  will have a Richter magnitude larger than  $m$  in time  $t$ . Then, again from the Poisson condition on spatial independence it follows:

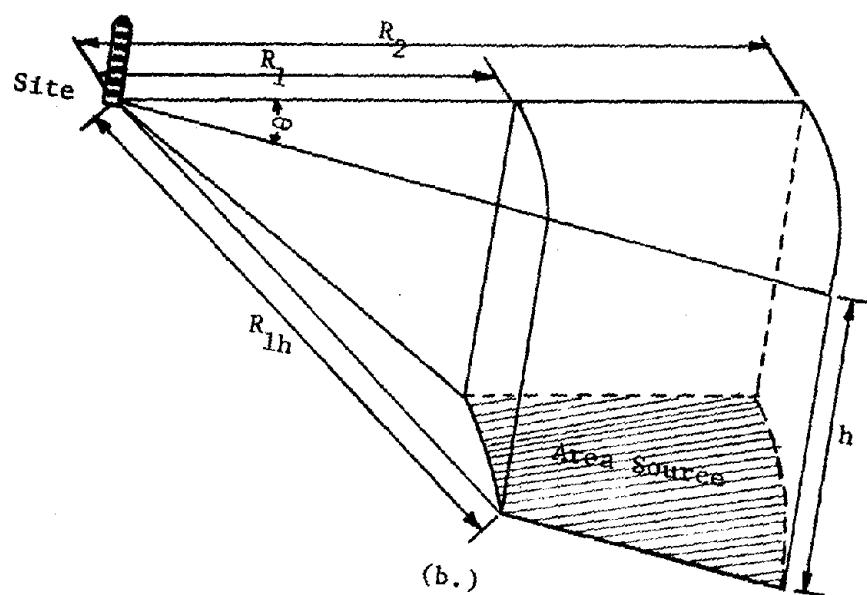
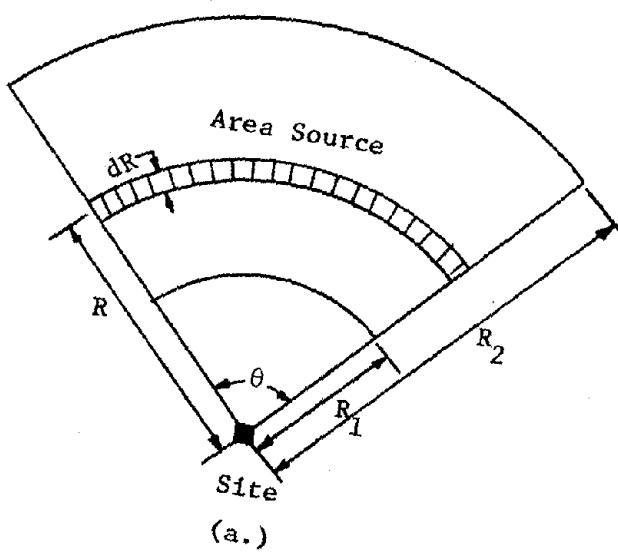


FIGURE A-4 Area Source: (a.) Top View; (b.) Isometric View

$$P\left[\sum_{i=1}^K E_{A_i}\right] = \sum_{i=1}^K P[E_{A_i}] \quad A-26$$

As  $\Delta A \rightarrow 0$ ,  $\Delta R$  and  $\Delta \theta$  both approach 0. The probability of having an earthquake of Richter magnitude smaller than or equal to  $m$  due to the entire area source is then:

$$\begin{aligned} P[M \leq m, t] &= \lim_{\Delta A \rightarrow 0} \sum_{i=1}^K P[E_{A_i}] = \lim_{\Delta A \rightarrow 0} \exp\left[-\sum_{i=1}^K N'(m) \Delta A_i t\right] \\ &= \exp\left[-\int_0^\theta \int_{R_1}^{R_2} N'(m) R dR d\theta \cdot t\right] \end{aligned} \quad A-27$$

Where  $N'(m)$  is the mean rate of occurrence for the area source as defined by Equation 3-3a.

Substituting for  $n'(m)$  in Equation A-27 above

$$P[M \leq m, t] = \exp\left[-\int_0^\theta \int_{R_1}^{R_2} \exp(\alpha' + \beta m) R dR d\theta \cdot t\right] \quad A-28$$

Using the attenuation relationship of Equation A-9

$$\begin{aligned} P[A \leq a, t] &= P\left[\frac{b_1 \exp(b_2 M)}{(R_h + b_4)^{b_3}} \leq a, t\right] \\ &= P\left[M \leq \ln \left[\frac{a}{b_1}\right]^{\frac{1}{b_2}} \left[R_h + b_4\right]^{\frac{b_3}{b_2}}, t\right] \end{aligned} \quad A-29$$

This equation is of the same form as the equation for the distribution on Richter magnitude (see Equation A-8) and thus can be written as:

$$P[A \leq a, t] = \exp \left[ -e^{\alpha'} \left[ \frac{a}{b_1} \right]^{\frac{\beta}{b_2}} t \int_0^\theta d\theta \int_{R_1}^{R_2} [R^2 + h^2 + b_4]^{\frac{\beta b_3}{b_2}} R dR \right]$$

$$= \exp \left[ -e^{\alpha'} \left[ \frac{a}{b_1} \right]^{\frac{\beta}{b_2}} t \theta \int_{R_1}^{R_2} [R^2 + h^2 + b_4]^{\frac{\beta b_3}{b_2}} R dR \right] \quad A-30$$

Let  $\gamma = e^{\alpha'}$

$$\delta = \beta/b_2$$

$$\rho = \beta b_3/b_2 \text{ as before}$$

and

$$R_h = \sqrt{R^2 + h^2} \quad A-31$$

Then

$$P[A \leq a, t] = \exp \left[ -\gamma \left[ \frac{a}{b_1} \right]^\delta t \theta \int_{R_1}^{R_2} (R_h + b_4)^\rho R dR \right] \quad A-32$$

and

$$P[A > a, t] = 1 - \exp \left[ -\gamma \left[ \frac{a}{b_1} \right]^\delta t \theta \int_{R_1}^{R_2} (R_h + b_4)^\rho R dR \right] \quad A-33$$

In general, a site may be surrounded by any or all three types of sources discussed in this section. If there are

NP point sources

NL line sources

NA area sources

the cumulative distribution function of peak ground acceleration at a site is then given by:

$$\begin{aligned}
 P[\Lambda > a, t] = 1 - \exp \left[ - \sum_{i=1}^{NP} \gamma_i \left[ \frac{a}{b_1} \right]^{\delta_i} t [R_{h_i} + b_4]^{\rho_i} \right. \\
 - \sum_{j=1}^{NL} \gamma_j \left[ \frac{a}{b_1} \right]^{\delta_j} t \int_{\ell_{1j}}^{\ell_{2j}} \left[ [d_j^2 + \ell^2 + h_j^2]^{1/2} + b_4 \right]^{\rho_j} d\ell \\
 \left. - \sum_{k=1}^{NA} \gamma_k \left[ \frac{a}{b_1} \right]^{\delta_k} t^{\theta_k} \int_{R_{1k}}^{R_{2k}} [R_{h_k} + b_4]^{\rho_k} R dR \right] \quad A-34
 \end{aligned}$$

In Equation A-34, the summation over  $i$  is for all point sources, that over  $j$  is for all line sources, and over  $k$  is for all area sources.

As it was shown in Chapter III, eighteen line sources were formulated for the Guatemalan region, based on past data. Any part of the country is affected by these sources, depending upon the proximity of the site to the source location.

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APPENDIX B

BAYESIAN HAZARD MODEL

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## B-1 Introduction

Bayesian probabilistic approach has the distinct advantage that it can include subjective information acquired through experience together with the quantitative data. As an alternative to the Poisson modeling presented in Appendix A, a Bayesian model will be used to develop hazard maps in terms of peak ground acceleration and duration of future earthquakes. The method for obtaining these maps will be only reviewed in the present appendix. Its detailed development is given in Mortgat (1976).

## B-2 Procedure

The procedure for estimating seismic risk for peak ground accelerations in essence consists of the following steps:

### STEP 1. Identification of Earthquake Sources

Based on the geology and historic seismicity of the area sources are identified as point sources, line sources (faults) or area sources. The maximum earthquakes associated with each source are established from historic seismicity and geology (in terms of magnitude or intensity). Typical examples of this approach are Cornell and Vanmarcke (1969), Kiremidjian (1975), Wiggins (1975), Shah et al. (1975), Algermissen (1969).

### STEP 2. Selection of Attenuation Relationship

Using one of the numerous empirical attenuation relationships, the peak accelerations at a given site due to earthquakes of various sizes occurring at different locations are estimated.

The attenuation relationship is based on data of non-uniform

quality. Most procedures utilize only the mean curves determined from a regression analysis.

#### STEP 3. Recurrence of Earthquakes

The recurrence of earthquakes of various sizes is estimated based primarily on the historic seismicity. A straight line or a set of straight lines is fitted on the data using regression analysis. This method usually results in unacceptable uncertainties for large magnitudes where the data is scarce. Some variations have been proposed. For example Wiggins (1975) uses a Bayesian procedure at the level of the results once the analysis is complete.

#### STEP 4. Results

Utilizing the computations in Steps (1), (2) and (3) the probability that a certain acceleration will not be exceeded within a given time period  $t$  is determined. The results of the evaluation are presented in terms of iso-acceleration or iso-intensity curves for selected levels of probability and time periods.

### B-3 Seismic Mapping Model

The following paragraphs present the general model used for seismic mapping in the current study.

#### Source Location

The location of the sources is made using the recorded hypocentral position of past earthquakes for the period of the records. The spatial distribution of hypocenters is then divided in different sources as a function of their shapes and seismicity (defined as the number of occurrences per unit area or unit length).

### Seismic Model

In the data presently available, the most commonly used measure of energy release is the Richter magnitude. In this model, the seismicity of the sources is described by the distribution on the number of occurrences for each magnitude. This is different from the current practice where the source seismicity is described by the probability of generating an event larger than a given magnitude and not by a distribution on all the events.

Occurrences: Assuming that earthquake occurrences form a Poisson process with mean rate of occurrence independent of magnitude, a distribution is obtained on the number of occurrences for a given period of time for a given source. The assumption of spatial and temporal independence is fairly well verified by data and is a common accepted practice in seismology. Moreover the amount of dependence due to the dual mechanism of stress accumulation and release has not been determined as yet.

Magnitude: Given that an event has occurred, a distribution on the magnitude is determined from past data. The magnitudes are discretized every 1/4 point as it is commonly done in data recording. This representation has the advantage of getting away from the data fitting. This method is specially valuable for regions where little data is available. The probability corresponding to each magnitude can be used in a Bernoulli trial where one outcome will be an event of the magnitude considered (success) and the other an event of any other magnitude (failure). The following question can then be answered: "Given that N earthquakes will occur, what is the probability that there will be 0, 1, 2, . . . N events of any given magnitude?" Combining those Binomial conditional distributions with the Poisson distribution on occurrences, the distribution on the number of occurrences for each magnitude is obtained.

### Bayesian Statistics

Bayesian statistics is applied to the Poisson and Binomial laws to eliminate some of the incompleteness of the data. For example, considering the fault length and the type of fault, geologists can determine the maximum magnitude earthquake the source can generate. This information has to be taken into consideration even if no such earthquake has been recorded in the data. This is done by assuming the mean rate of occurrences of the Poisson law to be a Gamma probability distribution (Benjamin and Cornell, 1970) and the probability of success of the Binomial law to be a Beta probability distribution (Benjamin and Cornell, 1970). This method has the advantage of including personal experience together with the data as well as updating the distribution as new data is made available.

### Mapping Parameters

Two mapping parameters are used, namely the PGA and the duration of the ground motion. PGA was used since no other attenuation relationship is readily available in the literature. The methodology allows for the use of a more stable parameter such as rms that would certainly improve the reliability of the model.

### Attenuation Relationship

The Esteva (1973) relationship is used to relate PGA to magnitude as a function of distance. Attenuation on duration is obtained using the relation developed by Bolt (1973).

Both relationships are treated probabilistically to take into consideration uncertainty in the attenuation decay.

Combining the distributions that describe the seismicity at the sources with the two transfer functions, the probability of exceedance of any PGA or duration is obtained at the site.

TABLE B-1

Duration Attenuation

M distance	5.5	6.0	6.5	7.0	7.5	8.0	8.5
10	8	12	19	26	31	34	35
25	4	9	15	24	28	30	32
50	2	3	10	22	26	28	29
75	1	1	5	10	14	16	17
100	0	0	1	4	5	6	7
125	0	0	1	2	2	3	3
150	0	0	0	1	2	2	3
175	0	0	0	0	1	2	2
200	0	0	0	0	0	1	2

B-4 Guatemala Bayesian Acceleration and Duration Maps

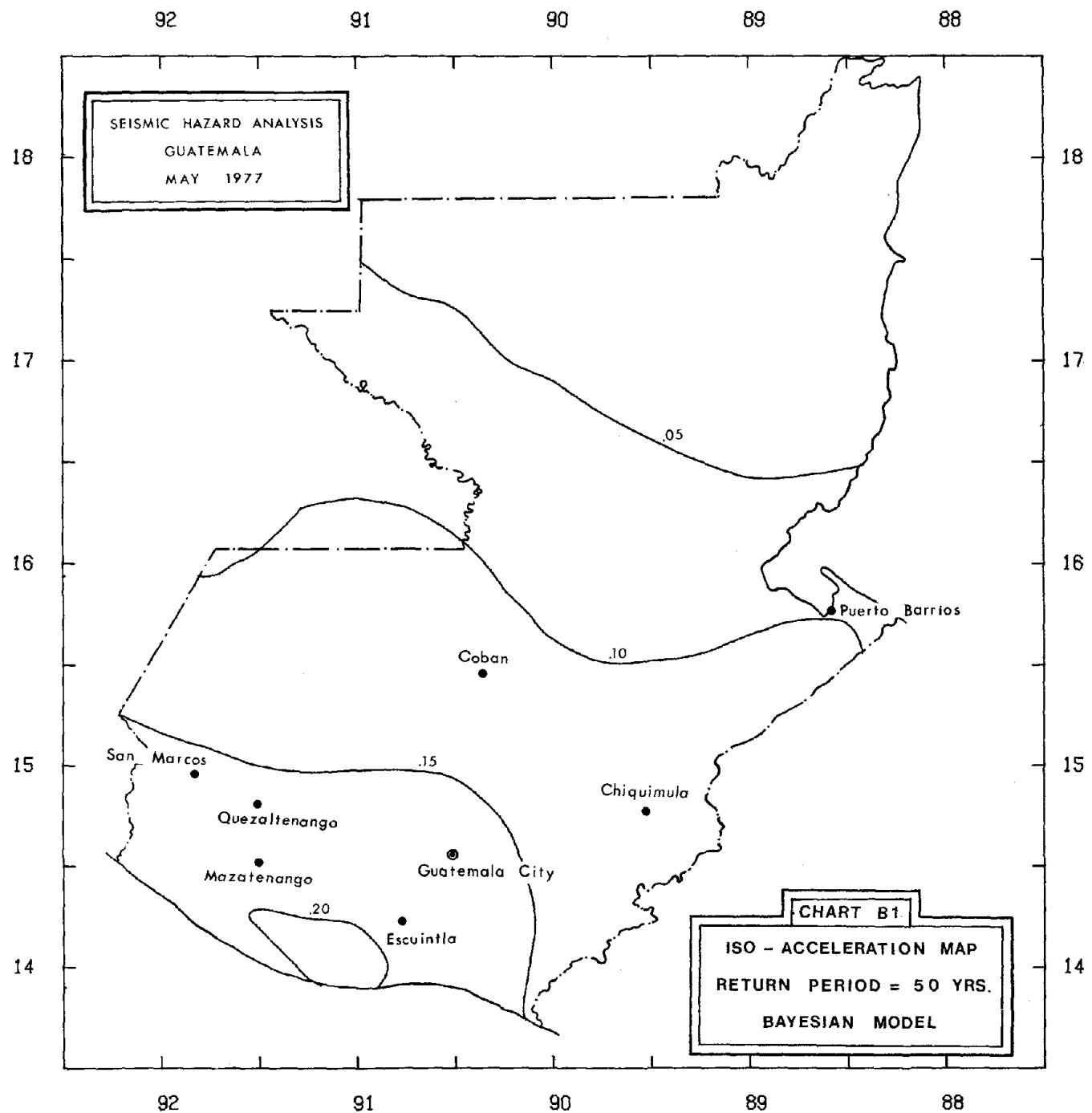
The Bayesian forecast model discussed in the previous section is applied to Guatemala. The data base and the eighteen seismic sources described in Chapter III are used directly in this model. For each source the following information is gathered:

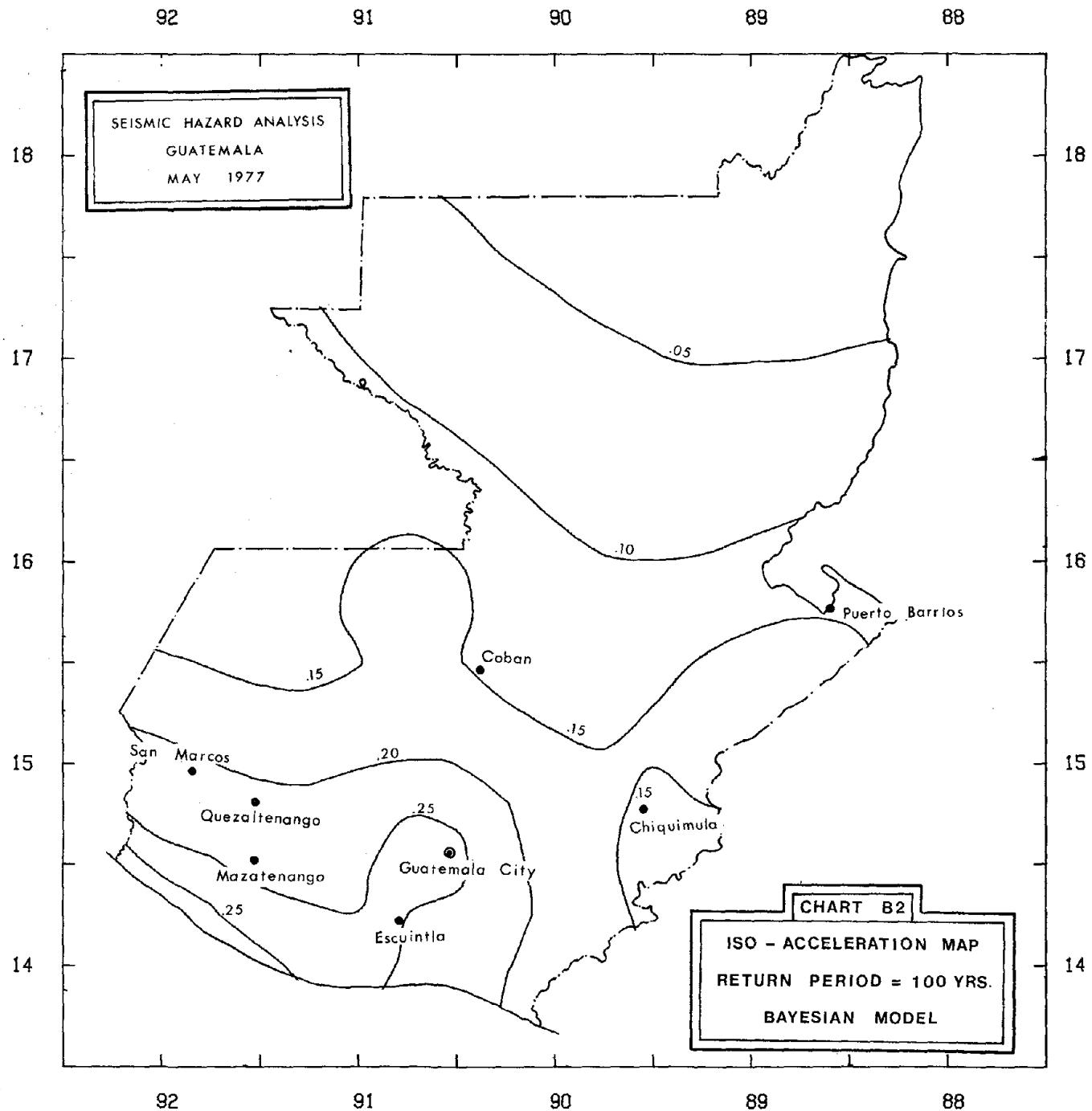
- (1) The number of recorded earthquakes N for discretized intervals of Richter magnitude M. An increment of 0.25M is used for the discretization.
- (2) The time period of the data T.
- (3) The value of the parameters  $\lambda'$  and  $\nu'$  of the Gamma distribution (Benjamin and Cornell, 1970) determined from subjective input.
- (4) The values of the parameters  $n'_{M_i}$  and  $\xi'_{M_i}$  of the Beta distribution (Benjamin and Cornell, 1970) which also come from subjective input.

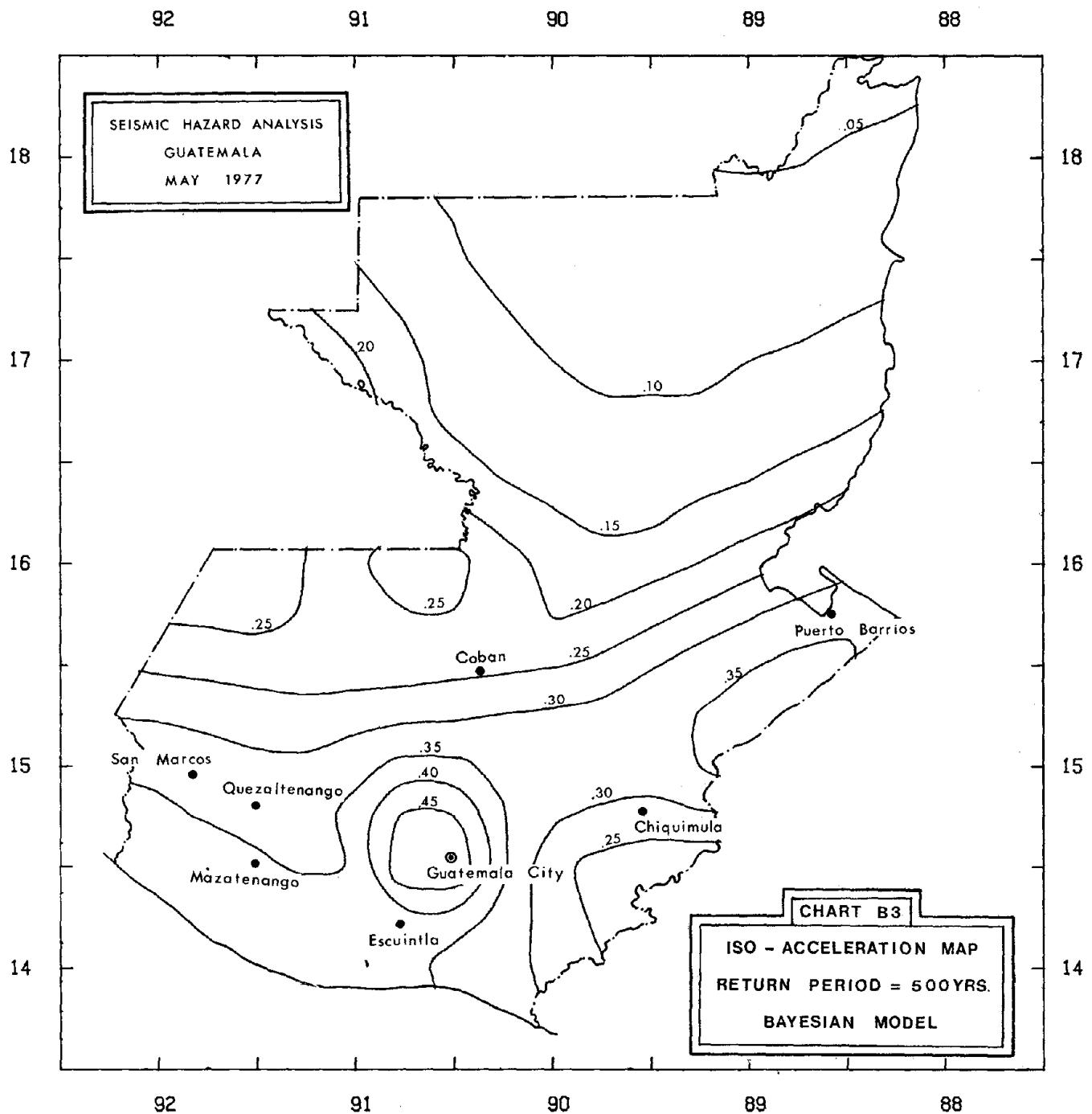
The values of all of these parameters are determined for each source and are listed in Tables B2 - B19. This procedure developed by Mortgat (1976) has a uniform distribution associated with the peak ground acceleration attenuation relationship (Equation 4-1). The coefficient of variation for this distribution is assumed to be 0.30.

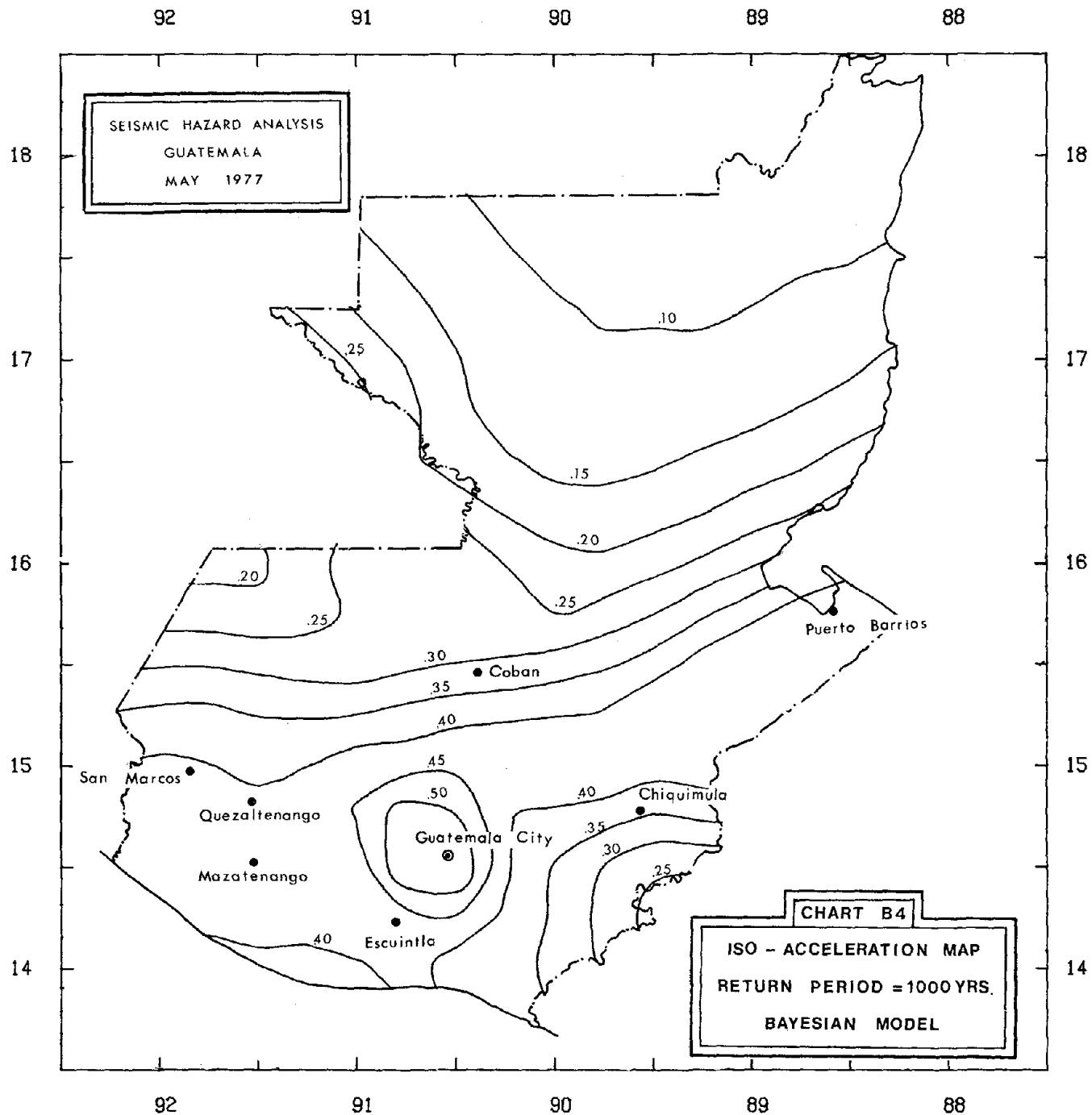
For the duration maps, Table B1 lists the relationship between the Richter magnitude, distance from the source to the site and the duration (from Bolt, 1973). A coefficient of variation of 0.30 is also applied to these duration values.

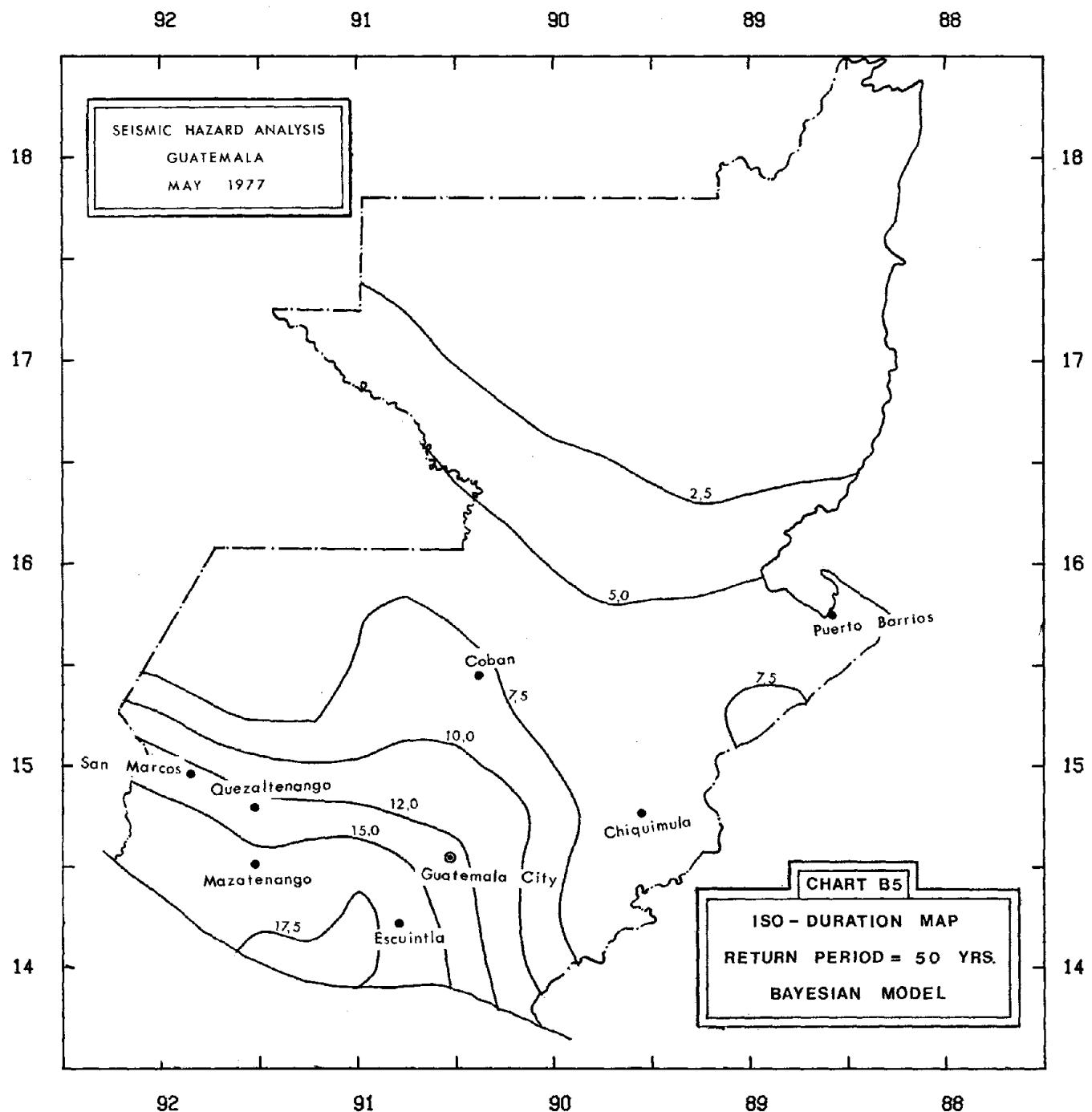
Charts B1 to B4 show the resulting iso-acceleration maps for return periods of 50, 100, 500 and 1000 years. The iso-duration maps for the same return periods are represented in Charts B5 to B8. A low seismicity region in the northern part of Guatemala can be observed in all maps both for duration and acceleration. The high regions in the south-south-west portion and along the Cayman trough is also common to all maps.

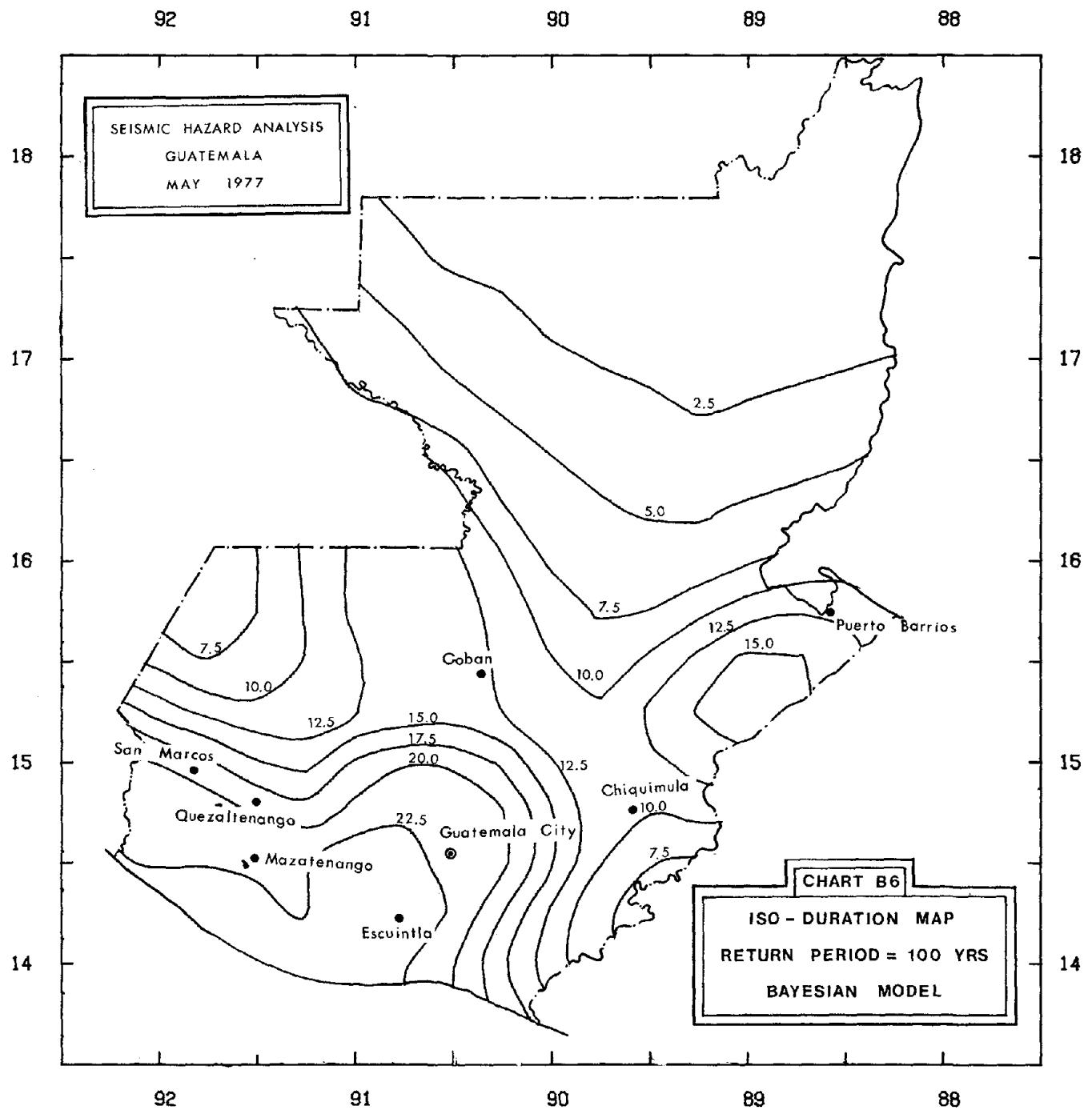


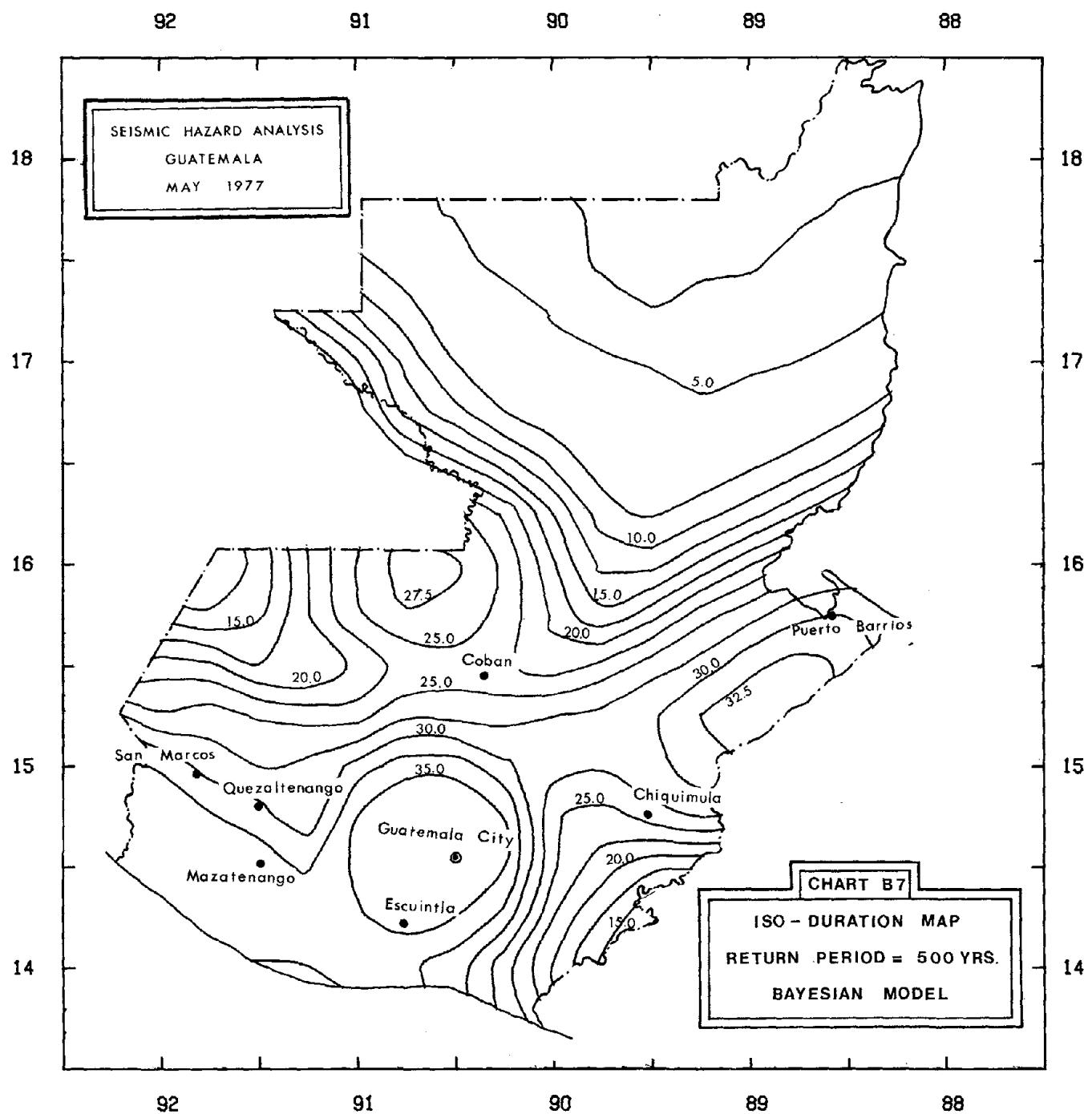


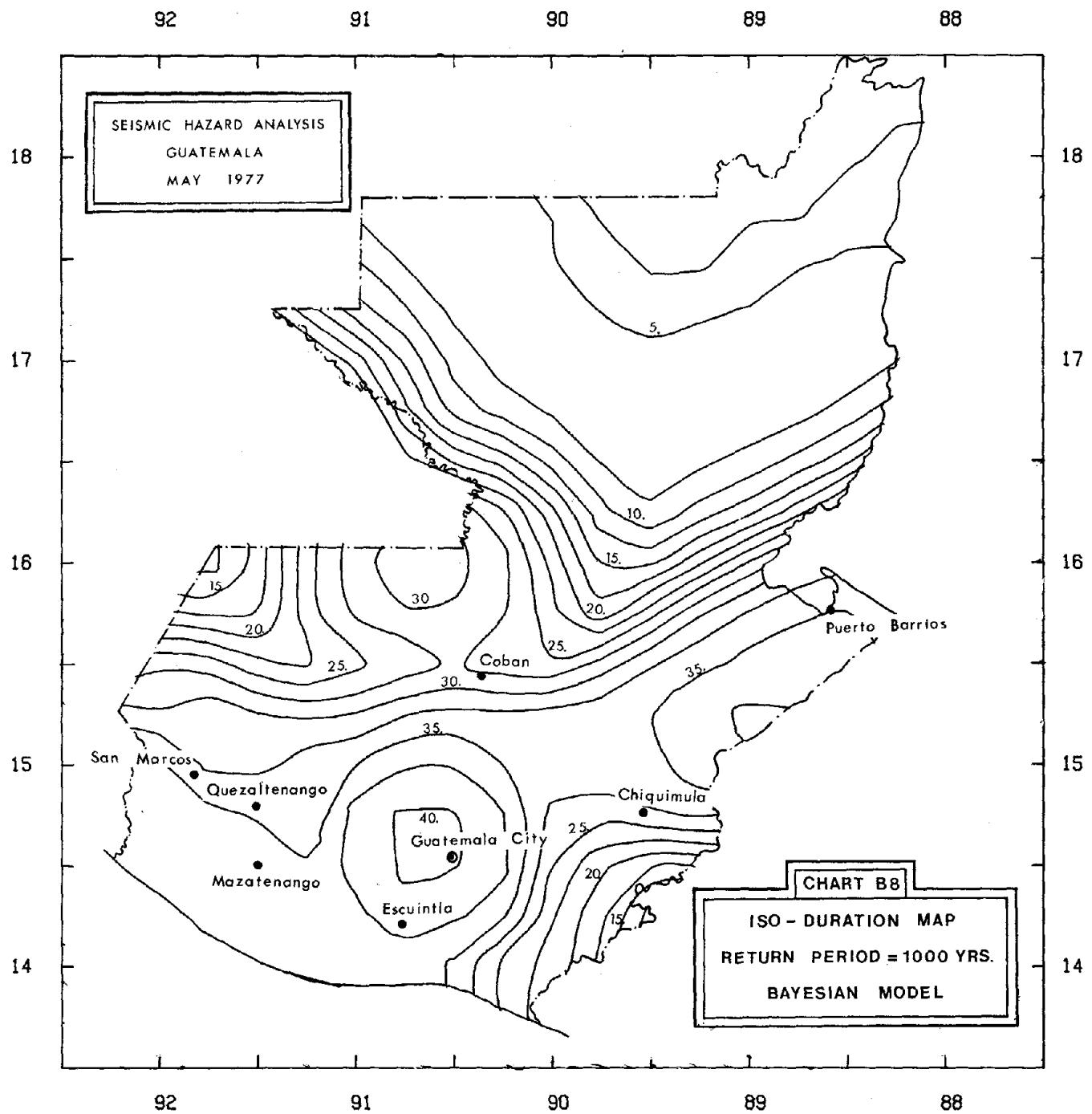












Comparing the iso-acceleration maps from the Bayesian model to the iso-acceleration maps from the original Poisson model (Charts 7 to 10), the acceleration values are higher for the maps in Charts B1 to B4. Also, in Charts 7 to 10 the effect of some of the faults (e.g., Mixco, Motagua, Polochic) has been accentuated while the effect of some others (e.g., Jocotan) has been reduced. Overall however, the maps appear quite similar. It should be pointed out that the advantages of the Bayesian approach are:

- (1) subjective information can be included
- (2) it is easier to incorporate additional earthquake data.

In a manner similar to Charts 7 to 10, the iso-acceleration and iso-duration maps resulting from the Bayesian approach can be used for zoning purposes. Again, their application for design and analysis of structures in Guatemala will be discussed at length in the Part II report of the present study.

TABLE B2  
Bayesian Data Information

Source 1				
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $\xi'_{M_i}$ )	$\xi'_{M_i} + R_{M_i}$ ( $\xi''_{M_i}$ )
4.00	2	14.14	2.55	4.55
4.25	1	11.59	2.09	3.09
4.50	2	9.50	1.71	3.71
4.75	0	7.79	1.41	1.41
5.00	2	6.38	1.15	3.15
5.25	0	5.23	0.94	0.94
5.50	2	4.29	0.78	2.78
5.75	1	3.51	0.63	1.63
6.00	0	2.88	0.52	0.52
6.25	0	2.36	0.42	0.42
6.50	1	1.94	0.35	1.35
6.75	0	1.59	0.29	0.29
7.00	0	1.30	0.24	0.24
7.25	0	1.06	0.19	0.19
7.50	1	0.87	0.15	1.15
7.75	0	0.72	0.13	0.13
8.00	0	0.59	0.11	0.11
8.25	0	0.48	0.09	0.09
8.50	0	0.39	0.39	0.39

TABLE B3  
Bayesian Data Information

Source 2				
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $\xi'_{M_i}$ )	$\xi'_{M_i} + R_{M_i}$ ( $\xi''_{M_i}$ )
4.00	1	7.09	1.28	2.28
4.25	0	5.81	1.05	1.05
4.50	1	4.76	0.86	1.86
4.75	0	3.90	0.70	0.70
5.00	1	3.20	0.58	1.58
5.25	0	2.62	0.47	0.47
5.50	2	2.15	0.39	2.39
5.75	0	1.76	0.32	0.32
6.00	1	1.44	0.26	1.26
6.25	0	1.18	0.21	0.21
6.50	0	0.97	0.18	0.18
6.75	0	0.79	0.14	0.14
7.00	0	0.65	0.12	0.12
7.25	0	0.53	0.09	0.09
7.50	0	0.44	0.08	0.08
7.75	0	0.36	0.07	0.07
8.00	0	0.29	0.29	0.29

TABLE B4  
Bayesian Data Information

Source 3				
	Time Data Base (T) : 75 years Number of Recorded Events (N) : 9 $v'$ from log-linear fit : 8.89 $\lambda'' = \lambda' + T = 75 + 75 = 150$ $v'' = v' + N = 8.89 + 9 = 17.89$ $n_{M_i}'' = n_{M_i}' + N = 8.89 + 9 = 17.89$			
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $\xi_{M_i}'$ )	$\xi_{M_i}' + R_{M_i}$ ( $\xi_{M_i}''$ )
4.00	1	8.89	1.47	2.47
4.25	1	7.42	1.22	2.22
4.50	2	6.20	1.02	3.02
4.75	0	5.18	0.86	0.86
5.00	2	4.32	0.71	2.71
5.25	0	3.61	0.60	0.60
5.50	0	3.01	0.55	0.55
5.75	0	2.52	0.42	0.42
6.00	1	2.10	0.34	1.34
6.25	1	1.76	0.29	1.29
6.50	0	1.47	0.22	0.22
6.75	1	1.22	0.20	1.20
7.00	0	1.02	0.17	0.17
7.25	0	0.85	0.14	0.14
7.50	0	0.71	0.11	0.11
7.75	0	0.60	0.10	0.10
8.00	0	0.50	0.50	0.50

TABLE B5  
Bayesian Data Information

Source 4				
	Time Data Base (T) : 75 years Number of Recorded Events (N) : 17 $v'$ from log-linear fit : 21.53 $\lambda'' = \lambda' + T = 75 + 75 = 150$ $v'' = v' + N = 21.53 + 17 = 38.53$ $n_{M_i}'' = n_{M_i}' + N = 21.53 + 17 = 38.53$			
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $\xi_{M_i}'$ )	$\xi_{M_i}' + R_{M_i}$ ( $\xi_{M_i}''$ )
4.00	2	21.53	2.80	4.80
4.25	1	18.73	2.44	3.44
4.50	2	16.29	2.12	5.12
4.75	0	14.17	1.85	1.85
5.00	2	12.32	1.50	3.50
5.25	0	10.82	1.50	1.50
5.50	1	9.32	1.21	2.21
5.75	0	8.11	1.06	1.06
6.00	2	7.05	0.91	2.91
6.25	1	6.14	0.92	1.92
6.50	3	5.22	2.18	5.18
6.75	1	3.04	1.27	2.27
7.00	1	1.77	0.74	1.74
7.25	1	1.03	0.43	1.43
7.50	0	0.60	0.25	0.25
7.75	0	0.35	0.15	0.15
8.00	0	0.20	0.20	0.20

TABLE B6  
Bayesian Data Information

Source 5				
	Time Data Base (T) : 75 years Number of Recorded Events (N) : 5 $v'$ from log-linear fit : 5.52 $\lambda'' = \lambda' + T = 75 + 75 = 150$ $v'' = v' + N = 5.52 + 5 = 10.53$ $n''_{M_i} = n'_{M_i} + N = 5.52 + 5 = 10.53$			
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $E'_{M_i}$ )	$E'_{M_i} + R_{M_i}$ ( $E''_{M_i}$ )
4.00	1	5.53	1.57	2.57
4.25	1	3.96	1.14	2.14
4.50	0	2.82	0.80	0.80
4.75	2	2.02	0.58	2.58
5.00	1	1.44	0.41	1.41
5.25	0	1.03	0.29	0.29
5.50	0	0.74	0.21	0.21
5.75	0	0.53	0.15	0.15
6.00	0	0.38	0.38	0.38

TABLE B7  
Bayesian Data Information

Source 6				
	Time Data Base (T) : 75 years Number of Recorded Events (N) : 3 $v'$ from log-linear fit : 2.74 $\lambda'' = \lambda' + T = 75 + 75 = 150$ $v'' = v' + N = 2.74 + 3 = 5.74$ $n''_{M_i} = n'_{M_i} + N = 2.74 + 3 = 5.75$			
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $E'_{M_i}$ )	$E'_{M_i} + R_{M_i}$ ( $E''_{M_i}$ )
4.00	1	2.74	0.41	1.41
4.25	1	2.33	0.34	1.34
4.50	0	1.99	0.30	0.30
4.75	0	1.69	0.25	0.25
5.00	0	1.44	0.22	0.22
5.25	0	1.22	0.18	0.18
5.50	0	1.04	0.15	0.15
5.75	1	0.89	0.14	1.14
6.00	0	0.75	0.11	0.11
6.25	0	0.64	0.10	0.10
6.50	0	0.54	0.54	0.54

TABLE B8  
Bayesian Data Information

Source 7				
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $\xi'_{M_i}$ )	$\xi'_{M_i} + R_{M_i}$ ( $\xi''_{M_i}$ )
4.00	1	4.22	0.62	1.62
4.25	1	3.60	0.53	1.53
4.50	1	3.07	0.45	1.45
4.75	0	2.62	0.39	0.39
5.00	1	2.23	0.32	1.32
5.25	0	1.91	0.28	0.28
5.50	0	1.63	0.24	0.24
5.75	0	1.39	0.21	0.21
6.00	0	1.18	0.17	0.17
6.25	0	1.01	0.15	0.15
6.50	1	0.86	0.13	0.13
6.75	0	0.73	0.10	1.10
7.00	0	0.63	0.63	0.63

TABLE B9  
Bayesian Data Information

Source 8				
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $\xi'_{M_i}$ )	$\xi'_{M_i} + R_{M_i}$ ( $\xi''_{M_i}$ )
4.00	1	7.22	1.00	2.00
4.25	0	6.22	0.85	0.85
4.50	1	5.37	0.75	1.75
4.75	0	4.62	0.63	0.63
5.00	1	3.99	0.55	1.55
5.25	0	3.44	0.48	0.48
5.50	0	2.96	0.41	0.41
5.75	1	2.55	0.35	1.35
6.00	2	2.20	0.30	2.30
6.25	0	1.90	0.26	0.26
6.50	0	1.64	0.23	0.23
6.75	0	1.41	0.19	0.19
7.00	0	1.22	0.17	0.17
7.25	0	1.05	0.15	0.15
7.50	0	0.90	0.90	0.9

TABLE B10  
Bayesian Data Information

Source 9				
	Time Data Base (T) : 75 years Number of Recorded Events (N) : 9 $v' \text{ from log-linear fit} : 9.29$ $\lambda'' = \lambda' + T = 75 + 75 = 150$ $v'' = v' + N = 9.29 + 9 = 18.29$ $\eta''_{M_i} = \eta'_{M_i} + N = 9.29 + 9 = 18.29$			
Richter Magnitude (M <sub>i</sub> )	Nb of Recorded Occurrences in M <sub>i</sub> bands (R <sub>M<sub>i</sub></sub> )	Cumulative Nb of Occurrences (log-linear fit Fig. ) (N <sub>c</sub> )	Nb of Occurrences in M <sub>i</sub> bands (log-linear fit) (E <sub>M<sub>i</sub></sub> )	E <sub>M<sub>i</sub></sub> + R <sub>M<sub>i</sub></sub> (E <sub>M<sub>i</sub></sub> )
4.00	2	9.29	1.04	3.04
4.25	1	8.25	0.93	1.93
4.50	0	7.32	0.82	0.82
4.75	0	6.50	0.72	0.72
5.00	0	5.78	0.65	0.65
5.25	1	5.13	0.72	1.72
5.50	2	4.41	0.82	2.82
5.75	0	3.59	0.67	0.67
6.00	1	2.92	0.55	1.55
6.25	1	2.38	0.45	1.45
6.50	0	1.93	0.35	0.35
6.75	0	1.57	0.29	0.29
7.00	0	1.28	0.24	0.24
7.25	0	1.04	0.19	0.19
7.50	1	0.85	0.16	1.16
7.75	0	0.69	0.13	0.13
8.00	0	0.56	0.56	0.56

TABLE B11  
Bayesian Data Information

Source 10				
	Time Data Base (T) : 75 years Number of Recorded Events (N) : 7 $v' \text{ from log-linear fit} : 7.58$ $\lambda'' = \lambda' + T = 75 + 75 = 150$ $v'' = v' + N = 7.58 + 7 = 14.58$ $\eta''_{M_i} = \eta'_{M_i} + N = 7.58 + 7 = 14.58$			
Richter Magnitude (M <sub>i</sub> )	Nb of Recorded Occurrences in M <sub>i</sub> bands (R <sub>M<sub>i</sub></sub> )	Cumulative Nb of Occurrences (log-linear fit Fig. ) (N <sub>c</sub> )	Nb of Occurrences in M <sub>i</sub> bands (log-linear fit) (E <sub>M<sub>i</sub></sub> )	E <sub>M<sub>i</sub></sub> + R <sub>M<sub>i</sub></sub> (E <sub>M<sub>i</sub></sub> )
4.00	0	7.58	0.65	0.65
4.25	1	6.93	0.60	1.60
4.50	0	6.33	0.54	0.54
4.75	1	5.79	0.50	1.50
5.00	1	5.29	0.45	1.45
5.25	0	4.84	0.82	0.42
5.50	0	4.42	1.13	1.13
5.75	2	3.29	1.08	3.08
6.00	0	2.21	0.72	0.72
6.25	1	1.49	0.49	1.49
6.50	1	1.00	0.33	1.33
6.75	0	0.67	0.22	0.22
7.00	0	0.45	0.15	0.15
7.25	0	0.30	0.10	0.10
7.50	0	0.20	0.20	0.20

TABLE B12  
Bayesian Data Information

Source 11				
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $\xi'_{M_i}$ )	$\xi'_{M_i} + R_{M_i}$ ( $\xi''_{M_i}$ )
4.00	3	4.43	0.56	3.56
4.25	0	3.87	0.49	0.49
4.50	0	3.38	0.42	0.42
4.75	0	2.96	0.38	0.38
5.00	0	2.58	0.32	0.32
5.25	0	2.26	0.29	0.29
5.50	0	1.97	0.25	0.25
5.75	0	1.72	0.21	0.21
6.00	1	1.51	0.19	1.19
6.25	1	1.32	0.17	1.17
6.50	0	1.15	0.15	0.15
6.75	0	1.00	0.12	0.12
7.00	0	0.88	0.11	0.11
7.25	0	0.77	0.10	0.10
7.50	0	0.67	0.67	0.67

TABLE B13  
Bayesian Data Information

Source 12 - Benioff 1				
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $\xi'_{M_i}$ )	$\xi'_{M_i} + R_{M_i}$ ( $\xi''_{M_i}$ )
4.00	41	111.21	26.96	67.96
4.25	9	84.25	20.43	29.43
4.50	19	63.82	15.47	34.47
4.75	5	48.35	11.73	16.73
5.00	15	36.62	8.87	23.87
5.25	1	27.75	6.73	7.73
5.50	10	21.02	5.10	15.10
5.75	5	15.92	3.86	8.86
6.00	6	12.06	2.92	8.92
6.25	2	9.14	2.22	4.22
6.50	4	6.92	1.68	5.68
6.75	1	5.24	1.27	2.27
7.00	1	3.97	0.96	1.96
7.25	3	3.01	0.73	3.73
7.50	0	2.28	0.55	0.55
7.75	0	1.73	0.42	0.42
8.00	0	1.31	0.32	0.32
8.25	0	0.99	0.24	0.24
8.50	0	0.75	0.75	0.75

TABLE B14  
Bayesian Data Information

Source 13 - Benioff 2				
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $E'_{M_i}$ )	$E'_{M_i} + R_{M_i}$ ( $E''_{M_i}$ )
4.00	17	40.17	9.10	26.10
4.25	8	31.07	10.71	18.71
4.50	4	20.36	7.02	11.02
4.75	5	13.34	4.60	9.60
5.00	6	8.74	3.02	9.02
5.25	3	5.72	1.97	4.97
5.50	0	3.75	1.29	1.29
5.75	0	2.46	0.85	0.85
6.00	1	1.61	0.56	1.56
6.25	1	1.05	0.36	1.36
6.50	0	0.69	0.24	0.24
6.75	0	0.45	0.15	0.15
7.00	0	0.30	0.11	0.11
7.25	0	0.19	0.06	0.06
7.50	0	0.13	0.05	0.05
7.75	0	0.08	0.03	0.03
8.00	0	0.05	0.02	0.02
8.25	0	0.03	0.01	0.01
8.50	0	0.02	0.02	0.02

TABLE B15  
Bayesian Data Information

Source 14 - Benioff 3				
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $E'_{M_i}$ )	$E'_{M_i} + R_{M_i}$ ( $E''_{M_i}$ )
4.00	22	84.08	17.68	39.68
4.25	9	66.40	13.97	22.97
4.50	18	52.43	11.03	29.03
4.75	6	41.40	8.71	14.71
5.00	14	32.69	6.88	20.88
5.25	4	25.81	5.43	9.43
5.50	4	20.38	4.28	8.28
5.75	3	16.10	3.39	6.39
6.00	4	12.71	6.02	10.02
6.25	4	6.69	3.51	7.51
6.50	2	3.18	1.67	3.67
6.75	3	1.51	0.79	3.79
7.00	0	0.72	0.38	0.38
7.25	0	0.34	0.18	0.18
7.50	0	0.16	0.08	0.08
7.75	0	0.08	0.04	0.04
8.00	0	0.04	0.02	0.02
8.25	0	0.02	0.01	0.01
8.50	0	0.01	0.01	0.01

TABLE B16  
Bayesian Data Information

Source 15 - Benioff 4				
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $\xi'_{M_i}$ )	$\xi'_{M_i} + R_{M_i}$ ( $\xi''_{M_i}$ )
4.00	35	110.86	17.00	52.00
4.25	7	93.86	14.39	21.39
4.50	19	79.47	12.19	31.19
4.75	12	67.28	10.32	22.32
5.00	11	56.96	8.74	19.74
5.25	5	48.22	10.99	15.99
5.50	9	37.23	8.19	17.19
5.75	7	28.04	6.92	13.92
6.00	8	21.12	5.22	13.22
6.25	6	15.90	3.92	9.92
6.50	3	11.98	2.86	5.86
6.75	3	9.02	2.23	5.23
7.00	2	6.79	1.67	3.67
7.25	0	5.12	1.27	1.27
7.50	0	3.85	0.95	0.95
7.75	0	2.90	0.71	0.71
8.00	1	2.19	0.54	1.54
8.25	2	1.65	0.41	2.41
8.50	0	1.24	1.24	1.24

TABLE B17  
Bayesian Data Information

Source 16 - Benioff 5				
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $\xi'_{M_i}$ )	$\xi'_{M_i} + R_{M_i}$ ( $\xi''_{M_i}$ )
4.00	13	59.46	8.57	21.57
4.25	6	50.89	7.33	13.33
4.50	8	43.56	6.28	14.28
4.75	4	37.28	7.17	11.17
5.00	6	30.11	7.23	13.23
5.25	3	22.88	5.49	8.49
5.50	8	17.39	4.17	12.17
5.75	3	13.22	3.17	6.17
6.00	5	10.05	2.42	7.42
6.25	2	7.63	1.83	3.83
6.50	2	5.80	1.39	3.39
6.75	1	4.41	1.06	2.06
7.00	0	3.35	0.80	0.80
7.25	0	2.55	0.62	0.62
7.50	1	1.93	0.46	1.46
7.75	0	1.47	0.35	0.35
8.00	0	1.12	0.27	0.27
8.25	0	0.85	0.20	0.20
8.50	1	0.65	0.65	1.65

TABLE B18  
Bayesian Data Information

Source 17 - Benioff 6				
	Time Data Base (T) : 75 years Number of Recorded Events (N) : 19 $v'$ from log-linear fit : 18.32 $\lambda'' = \lambda' + T = 75 + 75 = 150$ $v'' = v' + N = 18.32 + 19 = 37.32$ $\eta_{M_i}'' = \eta_{M_i}' + N = 18.32 + 19 = 37.32$			
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $\xi_{M_i}'$ )	$\xi_{M_i}' + R_{M_i}$ ( $\xi_{M_i}''$ )
4.00	5	18.32	2.86	7.86
4.25	1	15.46	2.42	3.42
4.50	2	13.04	2.03	4.03
4.75	1	11.01	1.72	2.72
5.00	2	9.29	1.46	3.46
5.25	2	7.83	1.22	3.22
5.50	0	6.61	1.65	1.65
5.75	3	4.96	2.14	5.14
6.00	1	2.82	1.21	2.21
6.25	1	1.61	0.69	1.69
6.50	1	0.92	0.40	1.40
6.75	0	0.52	0.22	0.22
7.00	0	0.30	0.13	0.13
7.25	0	0.17	0.07	0.07
7.50	0	0.10	0.05	0.05
7.75	0	0.05	0.02	0.02
8.00	0	0.03	0.01	0.01
8.25	0	0.02	0.01	0.01
8.50	0	0.01	0.01	0.01

TABLE B19  
Bayesian Data Information

Source 18 - Benioff 7				
	Time Data Base (T) : 75 years Number of Recorded Events (N) : 24 $v'$ from log-linear fit : 20.80 $\lambda'' = \lambda' + T = 75 + 75 = 150$ $v'' = v' + N = 20.80 + 24 = 44.80$ $\eta_{M_i}'' = \eta_{M_i}' + N = 20.80 + 24 = 44.80$			
Richter Magnitude ( $M_i$ )	Nb of Recorded Occurrences in $M_i$ bands ( $R_{M_i}$ )	Cumulative Nb of Occurrences (log-linear fit Fig. ) ( $N_c$ )	Nb of Occurrences in $M_i$ bands (log-linear fit) ( $\xi_{M_i}'$ )	$\xi_{M_i}' + R_{M_i}$ ( $\xi_{M_i}''$ )
4.00	7	20.80	3.03	10.03
4.25	3	17.77	2.59	5.59
4.50	2	15.18	2.21	4.21
4.75	1	12.97	1.88	2.88
5.00	1	11.09	1.62	2.62
5.25	1	9.47	1.38	2.38
5.50	2	8.09	1.17	3.17
5.75	1	6.92	1.38	2.38
6.00	2	5.54	1.53	3.53
6.25	2	4.01	1.10	3.10
6.50	0	2.91	0.80	0.80
6.75	2	2.11	0.58	2.58
7.00	0	1.53	0.42	0.42
7.25	0	1.11	0.31	0.31
7.50	0	0.80	0.22	0.22
7.75	0	0.58	0.16	0.16
8.00	0	0.42	0.11	0.11
8.25	0	0.31	0.09	0.09
8.50	0	0.22	0.22	0.22

APPENDIX C

THE FEBRUARY 4, 1976 GUATEMALAN EARTHQUAKE

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## THE FEBRUARY 4, 1976 GUATEMALAN EARTHQUAKE

### C-1 Introduction:

On February 4, 1976 at 3:01 a.m. (local Guatemalan time), central Guatemala experienced a major earthquake. The surface-wave magnitude (Ms) of this earthquake was 7.5, and the body-wave magnitude (Mb), 5.8. The hypocentral location was 15.32°N latitude and 89.08°W longitude, with a focal depth of about five kilometers. Numerous aftershocks followed the main earthquake, the largest of which occurred on February 6, 1976, causing additional damage to and around Guatemala City. The Mb of the February 6 earthquake was about 5.8. Damage due to the earthquake resulted from 1) direct seismic shaking of structures, particularly those of poor construction such as adobe buildings, 2) fault rupture, 3) earthquake induced lateral spreading and cracking of unconsolidated deposits and 4) a variety of earthquake triggered landslides.

### C-2 Intensity:

The maximum intensity of shaking within Guatemala City and the Mixco area, based on the nature and amount of damage to man-made structures whose foundations did not fail, was IX on the Modified Mercalli Scale, with small pockets of higher intensity occurring in different zones throughout the city. MM Intensity IX is defined as follows: "severe damage to well built and ordinary masonry structures, collapse of unreinforced masonry (adobe) structures, some damage to earthquake resistant structures" (see Section C-3).

Outside of Guatemala City, Modified Mercalli Intensity VI was reached over an area of 33,000 square kilometers, commonly in close proximity to the rupture zone (see Figure C-1). MM Intensity VI is defined as "objects fall from walls and shelves, some damage to poorly constructed masonry (adobe) structures".

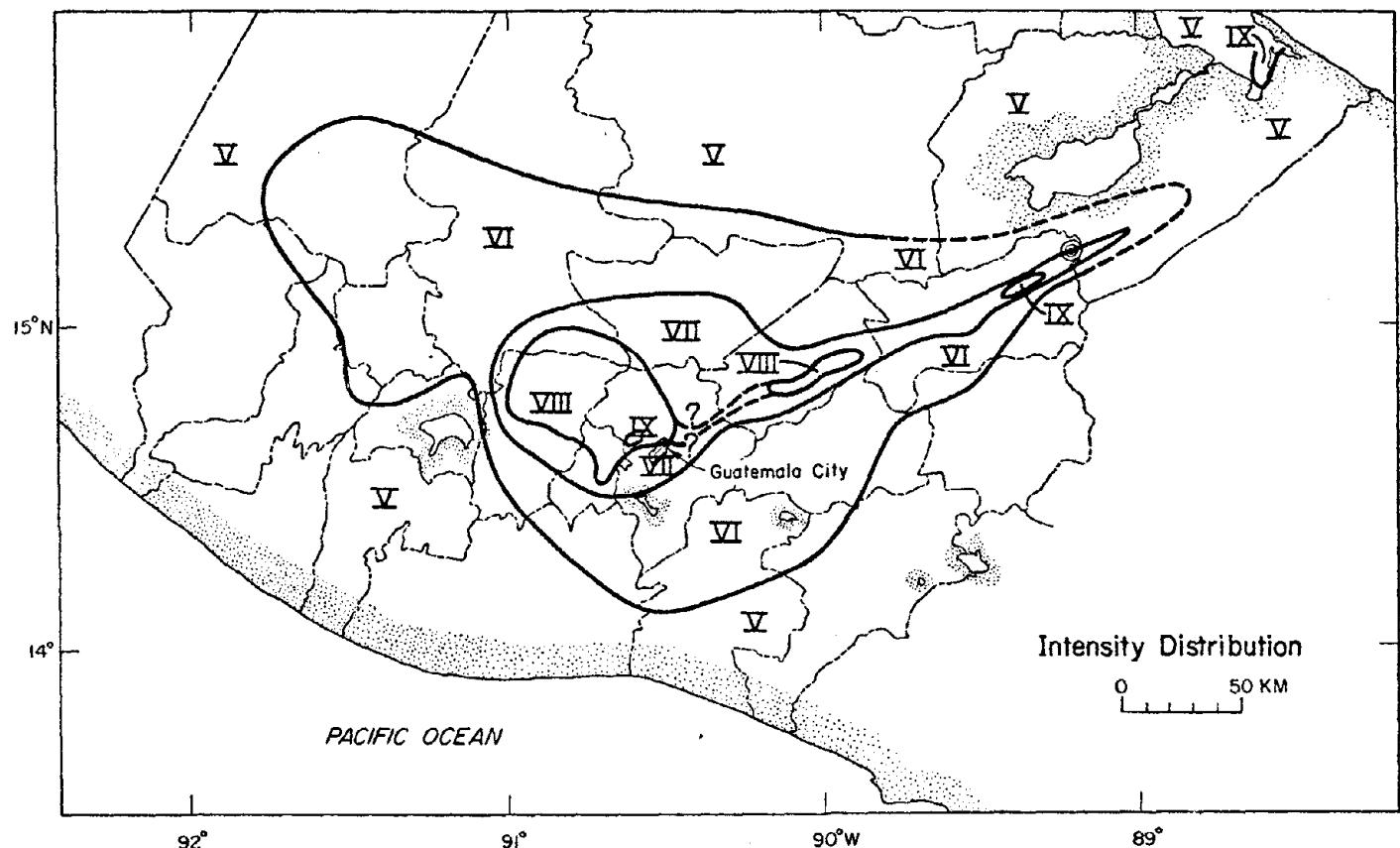


Figure C-1. Isoseismal Map showing Modified Mercalli intensity distribution in Guatemala from the February 4, 1976 earthquake.

indicates epicenter location.  
(From USGS PP1002)

C-3     Damage:

The zone of significant damage due to the earthquake and aftershocks extends about 300 kilometers in the east-west direction and 70

kilometers in the north-south direction, coinciding approximately with the region of fault rupture (see Figure C-2). The felt area for this earthquake was at least 100,000 square kilometers.

Severe damage occurred in the areas around and including Guatemala City, Joyabaj and Chimaltenango, and from El Progreso west to Gualan. A few towns up to 200 kilometers away from the instrumental epicenter were completely destroyed.

Foundation failures involving downslope slumping and/or sliding, particularly of loosely consolidated pumiceous pyroclastic rocks of the Guatemalan Highlands, lateral spreading, liquefaction and differential compaction of unconsolidated or poorly consolidated deposits damaged numerous buildings and roads throughout central Guatemala. (See Figure C-3).

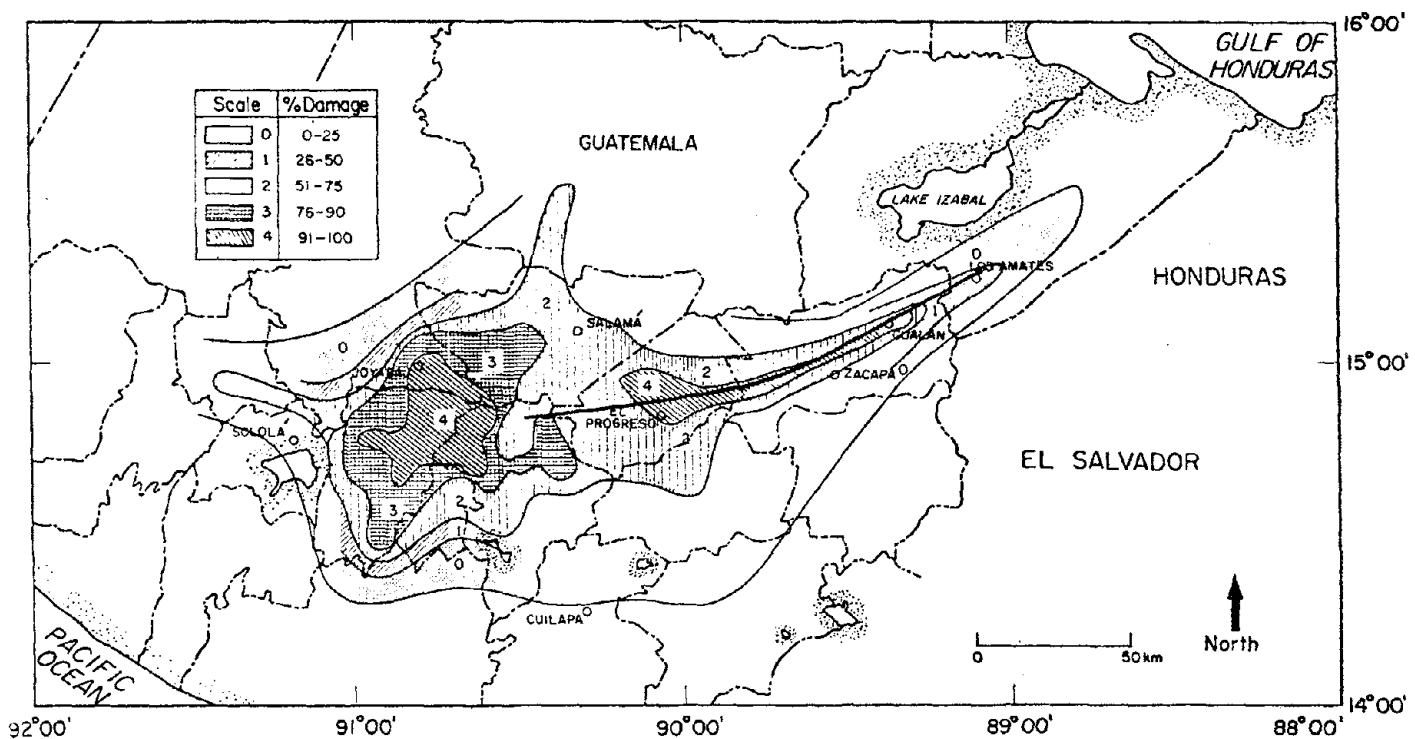


Figure C-2. Contour map showing damage to adobe-type structures in Guatemala due to the February 4, 1976 earthquake.  
(From USGS PP1002)

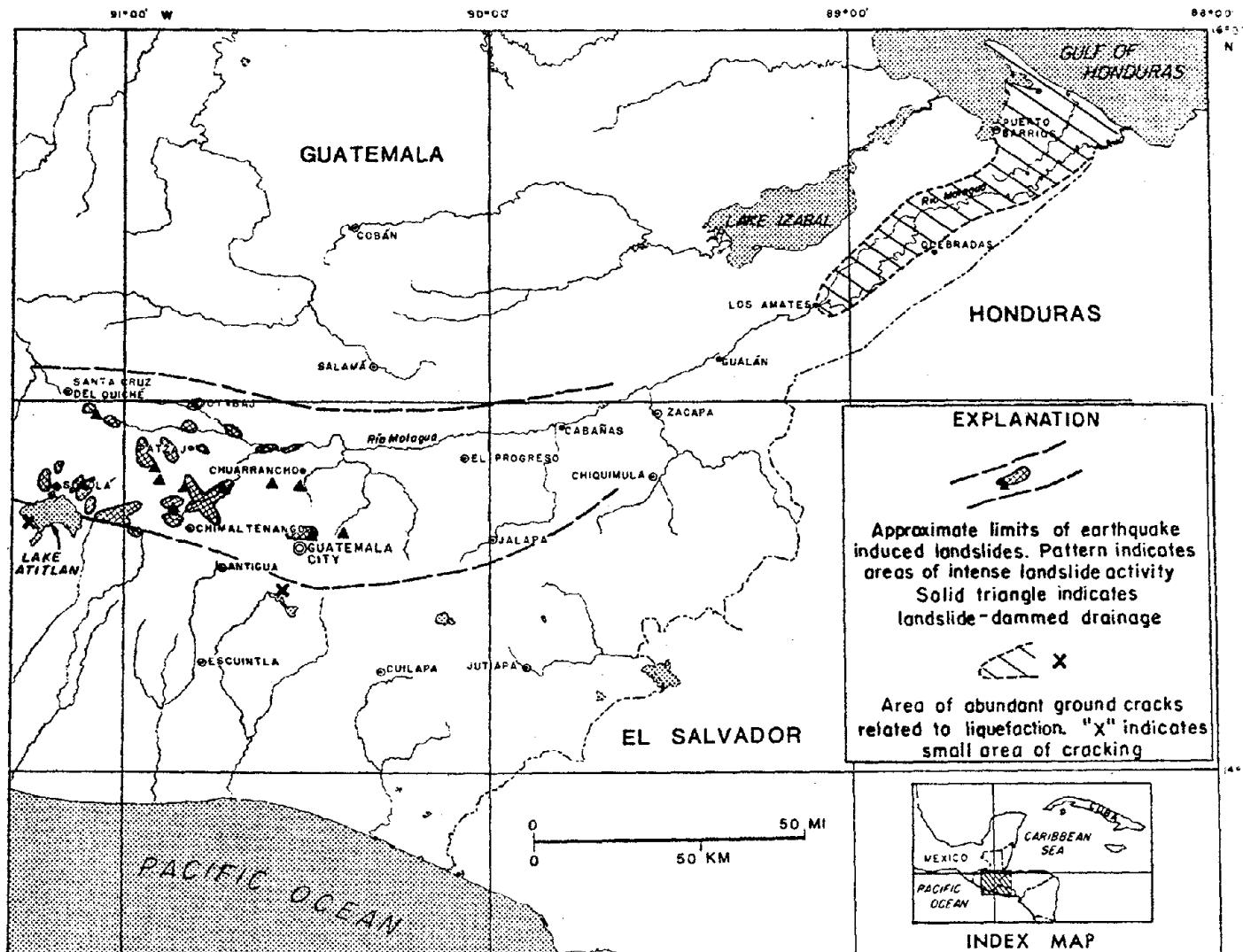


Figure C-3. Areas of earthquake-induced landsliding and of ground cracks probably related to liquefaction of unconsolidated deposits.  
(From USGS PP1002)

#### C-4 Hypocenter Location:

The hypocenter location of the main shock was determined by the National Earthquake Information Service (NEIS). The epicenter of this event was located near Los Amates, within the Motagua Valley, about 157 kilometers northeast of Guatemala City. The wide distribution of the 90 recording stations used in locating the epicenter gives reasonable confidence in the epicenter solution. However, a location bias of tens

of kilometers is possible, due to undetected seismic wave travel-time anomalies.

The depth of the hypocenter could not be reliably determined instrumentally because of the location of the seismic recording stations and the nature of the seismic waves recorded. A shallow depth of about five kilometers was assigned because of the surface faulting observed accompanying the earthquake and because of the shallow (0-12 km) depth of the aftershocks.

C-5      Nature and Amount of Fault Movement:

Movement along the Motagua fault was predominantly left lateral strike-slip, although minor vertical displacement was also observed. The length of surface faulting observed during field investigations by Plafker and others was 230 kilometers in the Motagua Valley and mountainous area west of the valley (see Figure C-4). The length of the main break, based on the occurrence of aftershocks is about 270 kilometers.

The main trace of the surface fault consists of right stepping en echelon fractures, forming a zone one to three meters wide. These fractures have a more northerly orientation than the overall trend of the fault zone.

The maximum horizontal displacement observed along the main trace of the fault was 325 centimeters, in the area between El Progreso and Chuarrancho (Espinosa, 1976). The average horizontal displacement was about 100 centimeters.

Subsidiary faults and splays of the Motagua fault appeared to be scarce. Secondary faulting (faults which underwent surface displacement approximately concurrent with that on the main fault but which at the surface do not join the main fault), however, occurred associated with

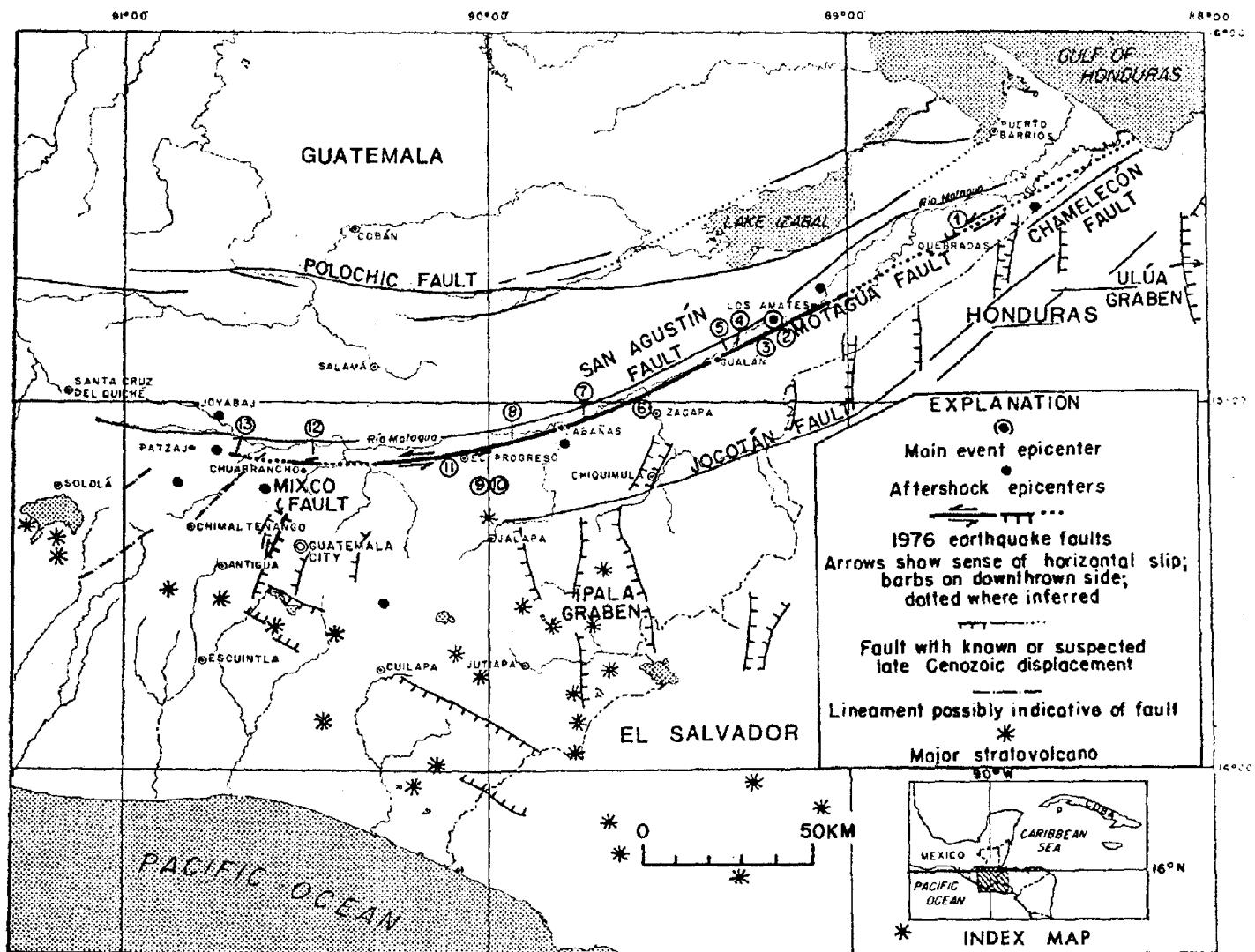


Figure C-4. Fault map of central Guatemala showing faults active during February 1976 earthquakes (heavy line) and other faults in the area (light lines). Epicenter of main event and major after shocks also shown (circles). (From USGS PP1002)

the earthquake. This faulting ruptured the ground surface in the Mixco area, west of Guatemala City, as much as 30 kilometers from the main fault trace. Ground rupture occurred along three generally north to north-north-east trending zones with predominantly vertical displacement on the order of 5 to 12 centimeters.

C-6     Cause of the Earthquake:

The February 4, 1976 earthquake occurred due to sinistral (left lateral) motions along the active plate boundary between the North American and Caribbean Plates. The North American Plate is moving westward relative to the Caribbean Plate, producing a strain accumulation along the margin of the two plates. When this situation reaches a critical point, the crust yields, producing an earthquake, as occurred on February 4, 1976. The faulting and earthquakes associated with the Mixco fault are likely due to extension produced by the plate motions and complexities related to the Cocos-Caribbean-North American triple junction.

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**APPENDIX D**

**EARTHQUAKE DATA**

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Data File Indices

CGS: Coast and Geodetic Survey  
CGS-B: Coast and Geodetic Survey  
Seismological Bulletin  
CGSPDE: Coast and Geodetic Survey  
Preliminary Determination of Epicenters  
ERL: Environmental Research Laboratories (NOAA)  
NOS: National Ocean Survey (NOAA)  
ISS: World Tape of Epicenters  
GUTE: Gutenberg

























CGS-B	03	02	67	09	05	33.5	1.4	*34N	93.46W	6	440 MB	CGSPDE	14	02	76	20	36	28.2	4.4	*15N	90.583W	1.1		
CGS-B	20	01	66	18	28	40.6	1.2	*34N	91.80W	6	440 MB	CGSPDE	15	02	76	20	36	28.2	4.4	*15N	90.583W	1.1		
CGS-B	03	07	67	08	22	42.0	0.6	*34N	90.80W	8	370 MB	CGSPDE	16	02	76	07	58	0.6	*4	*8N	90.67W	1.2		
CGS-B	03	07	64	02	32	50.0	1.2	*34N	90.90W	8	370 MB	CGSPDE	17	02	76	07	58	0.6	*4	*8N	90.67W	1.2		
CGS-B	32	04	70	21	12	56.0	0.1	*34N	93.00W	19	450 MB	CGSPDE	18	02	76	16	19	20.7	4.4	*27N	90.500W	3.3		
CGS-B	26	07	68	06	32	59.9	1.4	*34N	93.00W	19	450 MB	CGSPDE	19	02	76	16	19	20.7	4.4	*27N	90.500W	3.3		
CGS-B	31	30	04	70	08	32	59.1	1.4	*34N	93.00W	19	450 MB	CGSPDE	20	02	76	16	19	20.7	4.4	*27N	90.500W	3.3	
CGS-B	07	03	66	09	10	59.3	1.4	*34N	93.10W	17	500 MB	CGSPDE	21	02	76	16	19	20.7	4.4	*27N	90.500W	3.3		
CGS-B	17	02	66	01	24	57.3	1.4	*34N	92.80W	19	370 MB	CGSPDE	22	02	76	16	19	20.7	4.4	*27N	90.500W	3.3		
CGS-B	16	04	65	05	37	55.4	1.4	*34N	91.80W	17	450 MB	CGSPDE	23	02	76	16	19	20.7	4.4	*27N	90.500W	3.3		
CGS-B	23	06	64	04	13	47.3	1.4	*34N	92.80W	14	450 MB	CGSPDE	24	02	76	16	19	20.7	4.4	*27N	90.500W	3.3		
CGS-B	28	08	70	04	21	49.0	0.0	*7	1.4	35N	93.30W	33	510 MB	CGSPDE	25	02	76	16	19	20.7	4.4	*27N	90.500W	3.3
CGS-B	28	10	66	06	20	45.7	1.4	*34N	93.20W	23	450 MB	CGSPDE	19	05	66	05	55	51.4	4.4	*6JUN	91.100W	1.8		
CGS-B	27	03	66	21	07	30.4	1.4	*34N	93.20W	33	370 MB	CGSPDE	19	05	66	16	64	04	5.5	1.4	*6JUN	91.100W	1.8	
CGS-B	32	01	67	11	02	25.4	1.4	*34N	93.31W	33	450 MB	CGSPDE	03	08	66	16	64	04	22.9	1.4	*6JUN	91.100W	1.8	
CGS-B	32	04	66	05	23	45.4	1.4	*34N	93.31W	37	500 MB	CGSPDE	20	01	66	14	40	30.2	4.4	*27N	91.200W	3.3		
CGS-B	31	14	64	05	35	10.0	1.4	*34N	93.20W	33	350 MB	CGSPDE	10	01	37	17	18	9.7	4.4	*27N	91.095W	3.3		
CGS-B	14	04	66	05	35	15.0	1.4	*34N	93.30W	33	450 MB	CGSPDE	21	04	69	05	4.4	4.4	4.4	2.4	*27N	91.325W	0.0	
CGS-B	30	04	66	06	36	0.4	0.8	59	37.0	33	520 MB	CGSPDE	21	01	68	23	45	17.4	4.4	*27N	92.300W	7.7		
CGS-B	10	04	66	06	67	20	41	54.0	37.0	23	420 MB	CGSPDE	03	05	63	19	18	1.5	14.5	1.4	*27N	91.800W	3.3	
CGS-B	12	03	67	06	67	16	54.6	1.4	*48N	93.35W	35	390 MB	CGSPDE	05	02	54	15	17	5.9	0.1	1.4	*27N	92.500W	1.0
CGS-B	14	04	68	08	19	16.0	1.4	*49N	93.40W	33	400 MB	CGSPDE	01	04	54	23	11	27.0	1.4	*27N	92.600W	1.28		
CGS-B	14	06	67	02	03	56	28.0	1.4	*48N	93.25W	33	500 MB	CGSPDE	30	03	14	00	41	18.6	0.1	1.4	*27N	92.700W	0.0
NDS	85	15	12	70	10	53	1.9	3	426N	93.05W	33	510 MB	CGSPDE	30	11	67	07	00	40.1	1.4	*27N	91.900W	1.50	
NDS	85	15	12	70	10	53	1.9	3	426N	93.05W	33	480 MB	CGSPDE	11	05	00	00	00	00	0.0	0.0	0.0	0.0	
CGS-B	32	03	70	07	39	24.2	1.4	*34N	93.35W	33	450 MB	CGSPDE	21	01	68	23	45	17.4	4.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	27.5	1.4	*34N	93.35W	33	450 MB	CGSPDE	03	11	67	17	04	1.4	1.4	1.4	*27N	92.300W	7.7	
CGS-B	32	03	70	07	39	31.8	1.4	*34N	93.35W	28	520 MB	CGSPDE	15	16	75	01	03	5.9	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	35.5	1.4	*34N	93.35W	33	450 MB	CGSPDE	03	11	67	17	04	1.4	1.4	1.4	*27N	92.300W	7.7	
CGS-B	32	03	70	07	39	39.2	1.4	*34N	93.35W	33	510 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	42.9	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	46.6	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	50.3	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	54.0	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	57.7	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	61.4	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	65.1	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	68.8	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	72.5	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	76.2	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	79.9	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	83.6	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	87.3	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	91.0	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	94.7	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	98.4	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	102.1	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	105.8	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	109.5	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	113.2	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	116.9	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	120.6	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	124.3	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	128.0	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	131.7	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	135.4	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	139.1	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03	70	07	39	142.8	1.4	*34N	93.35W	33	450 MB	CGSPDE	06	15	75	06	54	2.6	1.4	*27N	92.300W	7.7		
CGS-B	32	03																						







NOS	23	26	72	71	C2	00	10.0	1.60	0.325N	89.	925W	71	4.00 MB	0	
ERL	66	19	11	72	C3	35	06.3	1.20	1.273N	88.	524W	73	5.00 MB	0	
CGS	33	03	05	69	C3	22	0.3	1.20	1.297N	88.	494W	77	4.50 MB	0	
CGS	25	01	04	69	C1	19	53.5	1.45	0.977N	88.	233W	71	4.50 MB	0	
ERL	32	32	30	04	C1	19	41.1	1.45	0.970N	88.	304W	76	4.50 MB	0	
CGS	34	21	02	62	C6	09	10.0	1.65	1.193N	88.	684W	77	4.40 MB	0	
CGS	34	21	02	69	C6	09	20.4	1.20	1.193N	88.	590W	75	4.40 MB	0	
CGS	34	21	02	67	C6	09	05.7	1.20	1.233N	87.	803W	70	4.40 MB	0	
CGS	34	22	02	67	C9	15	29.4	1.20	1.253N	87.	813W	72	5.00 MB	0	
CGS	34	22	02	67	C9	15	30.4	1.20	1.253N	87.	303W	75	4.90 MB	0	
CGSDE	28	11	65	68	C8	47	17.1	20.0	1.45	0.924N	87.	500W	79	5.10 MB	0
CGSDE	24	03	68	68	C8	47	17.1	17.3	20.0	1.45	0.924N	86.	500W	0	
CGSDE	21	08	73	12	C2	42	32.0	45.0	1.45	0.924N	86.	500W	70	3.00 MB	0
CGSDE	02	02	73	12	C7	07	7.4	14.2	1.45	0.924N	86.	500W	75	3.00 MB	0
CGSDE	04	03	74	14	C4	53	52.1	1.20	1.233N	86.	600W	75	5.00 MB	0	
CGSDE	02	02	74	14	C4	53	52.1	1.20	1.233N	86.	600W	79	5.00 MB	0	
CGSDE	21	02	74	14	C8	28	35.3	1.40	1.233N	92.	100W	70	5.30 MB	0	
CGSDE	31	12	74	20	C1	15	32.7	1.40	1.233N	91.	900W	75	5.40 MB	0	
CGSDE	31	12	74	21	C1	15	10.0	1.40	1.233N	91.	900W	75	4.90 MB	0	
CGSDE	26	06	75	06	C5	45	17.2	1.45	0.924N	91.	900W	76	4.80 MB	0	
CGSDE	15	12	75	14	C7	27	1.45	1.20	1.233N	90.	800W	75	5.00 MB	0	
CGSDE	15	12	75	14	C7	27	1.45	1.20	1.233N	90.	800W	75	5.00 MB	0	
CGS	28	04	59	11	C9	03	30.3	1.30	1.233N	93.	00W	0	4.50 MB	0	
CGS	28	04	59	11	C9	03	15.0	1.20	1.233N	93.	00W	49	3.50 MB	0	
CGS	28	04	59	11	C9	03	15.0	1.20	1.233N	93.	30W	46	3.50 MB	0	
ERL	38	01	07	72	C0	12	2.0	2.0	1.40	1.233N	91.	43W	47	4.40 MB	0
CGS	3	21	10	66	C0	12	3.0	3.2	1.40	1.233N	91.	43W	56	4.70 MB	0
CGS	3	16	12	63	C0	10	4.9	5.2	1.40	1.233N	90.	200W	59	4.30 MB	0
663PAS															
S15 - B4	03	12	66	66	B4	03	41.7	1.19	1.29U	93.	300W	89	3.80 MB	0	
CGSB	23	01	68	68	B4	03	43.1	1.20	1.29U	93.	381W	87	4.60 MB	0	
CGS	30	09	68	68	B4	03	50.5	1.20	1.29U	93.	646W	85	4.20 MB	0	
CGS	52	24	06	68	B4	02	51.0	1.20	1.29U	92.	790W	80	5.10 MB	0	
CGS	85	11	68	68	B4	03	1.1	1.1	1.29U	92.	790W	89	4.60 MB	0	
ERL	39	08	68	73	B3	10	3.2	0.4	1.40	1.272N	92.	052W	84	4.40 MB	0
CGS	20	04	65	65	B3	12	4.6	4.4	1.29U	92.	00W	84	4.40 MB	0	
CGS	20	02	67	67	B3	11	1.58	3.2	1.40	1.29U	92.	35W	89	4.50 MB	0
CGS	20	02	67	67	B3	11	33.7	1.40	1.29U	92.	00W	85	4.50 MB	0	
CGS	18	08	66	66	B3	10	3.1	32.0	1.40	1.29U	91.	30W	84	4.50 MB	0
CGS	1	03	08	69	B3	11	1.9	1.3	1.20	1.29U	91.	43W	82	5.00 MB	0
CGS	3	15	04	63	B3	22	3.5	5.8	1.20	1.29U	91.	35W	88	4.60 MB	0
CGS	3	19	06	64	B3	13	4.5	1.3	1.20	1.29U	91.	00W	85	3.90 MB	0
CGS	3	19	06	64	B3	13	1.2	1.2	1.20	1.29U	91.	00W	89	5.00 MB	0
GUT	04	33	03	71	C9	03	30.0	1.30	1.29U	93.	00W	89	5.00 MB	0	
CGS	03	12	66	66	C4	03	41.7	1.19	1.29U	93.	300W	87	4.60 MB	0	
CGS	03	12	66	66	C4	03	43.1	1.20	1.29U	93.	381W	87	4.60 MB	0	
CGS	30	09	68	68	C4	02	50.5	1.20	1.29U	93.	646W	85	4.20 MB	0	
CGS	52	24	06	68	C4	03	51.0	1.20	1.29U	92.	790W	80	5.10 MB	0	
CGS	85	11	68	68	C4	03	1.1	1.1	1.29U	92.	790W	89	4.60 MB	0	
ERL	39	08	68	73	C5	10	3.2	0.4	1.40	1.272N	92.	052W	84	4.40 MB	0
CGS	20	04	65	65	C5	12	4.6	4.4	1.29U	92.	00W	84	4.40 MB	0	
CGS	20	02	67	67	C5	11	1.58	3.2	1.40	1.29U	92.	35W	89	4.50 MB	0
CGS	18	08	66	66	C5	10	3.1	33.7	1.40	1.29U	91.	00W	85	4.50 MB	0
CGS	1	03	08	69	C5	11	1.9	1.3	1.20	1.29U	91.	30W	82	5.00 MB	0
CGS	3	15	04	63	C5	22	3.5	5.8	1.20	1.29U	91.	35W	88	4.60 MB	0
CGS	3	19	06	64	C5	13	4.5	1.3	1.20	1.29U	91.	00W	85	3.90 MB	0
GUT	04	33	03	71	C9	03	30.0	1.30	1.29U	93.	00W	89	5.00 MB	0	
CGS	03	12	66	66	C4	03	41.7	1.19	1.29U	93.	300W	87	4.60 MB	0	
CGS	03	12	66	66	C4	03	43.1	1.20	1.29U	93.	381W	87	4.60 MB	0	
CGS	30	09	68	68	C4	02	50.5	1.20	1.29U	93.	646W	85	4.20 MB	0	
CGS	52	24	06	68	C4	03	51.0	1.20	1.29U	92.	790W	80	5.10 MB	0	
CGS	85	11	68	68	C4	03	1.1	1.1	1.29U	92.	790W	89	4.60 MB	0	
ERL	39	08	68	73	C5	10	3.2	0.4	1.40	1.272N	92.	052W	84	4.40 MB	0
CGS	20	04	65	65	C5	12	4.6	4.4	1.29U	92.	00W	84	4.40 MB	0	
CGS	20	02	67	67	C5	11	1.58	3.2	1.40	1.29U	92.	35W	89	4.50 MB	0
CGS	18	08	66	66	C5	10	3.1	33.7	1.40	1.29U	91.	00W	85	4.50 MB	0
CGS	1	03	08	69	C5	11	1.9	1.3	1.20	1.29U	91.	30W	82	5.00 MB	0
CGS	3	15	04	63	C5	22	3.5	5.8	1.20	1.29U	91.	35W	88	4.60 MB	0
CGS	3	19	06	64	C5	13	4.5	1.3	1.20	1.29U	91.	00W	85	3.90 MB	0
GUT	04	33	03	71	C9	03	30.0	1.30	1.29U	93.	00W	89	5.00 MB	0	
CGS	03	12	66	66	C4	03	41.7	1.19	1.29U	93.	300W	87	4.60 MB	0	
CGS	03	12	66	66	C4	03	43.1	1.20	1.29U	93.	381W	87	4.60 MB	0	
CGS	30	09	68	68	C4	02	50.5	1.20	1.29U	93.	646W	85	4.20 MB	0	
CGS	52	24	06	68	C4	03	51.0	1.20	1.29U	92.	790W	80	5.10 MB	0	
CGS	85	11	68	68	C4	03	1.1	1.1	1.29U	92.	790W	89	4.60 MB	0	
ERL	39	08	68	73	C5	10	3.2	0.4	1.40	1.272N	92.	052W	84	4.40 MB	0
CGS	20	04	65	65	C5	12	4.6	4.4	1.29U	92.	00W	84	4.40 MB	0	
CGS	20	02	67	67	C5	11	1.58	3.2	1.40	1.29U	92.	35W	89	4.50 MB	0
CGS	18	08	66	66	C5	10	3.1	33.7	1.40	1.29U	91.	00W	85	4.50 MB	0
CGS	1	03	08	69	C5	11	1.9	1.3	1.20	1.29U	91.	30W	82	5.00 MB	0
CGS	3	15	04	63	C5	22	3.5	5.8	1.20	1.29U	91.	35W	88	4.60 MB	0
CGS	3	19	06	64	C5	13	4.5	1.3	1.20	1.29U	91.	00W	85	3.90 MB	0
GUT	04	33	03	71	C9	03	30.0	1.30	1.29U	93.	00W	89	5.00 MB	0	
CGS	03	12	66	66	C4	03	41.7	1.19	1.29U	93.	300W	87	4.60 MB	0	
CGS	03	12	66	66	C4	03	43.1	1.20	1.29U	93.	381W	87	4.60 MB	0	
CGS	30	09	68	68	C4	02	50.5	1.20	1.29U	93.	646W	85	4.20 MB	0	
CGS	52	24	06	68	C4	03	51.0	1.20	1.29U	92.	790W	80	5.10 MB	0	
CGS	85	11	68	68	C4	03	1.1	1.1	1.29U	92.	790W	89	4.60 MB	0	
ERL	39	08	68	73	C5	10	3.2	0.4	1.40	1.272N	92.	052W	84	4.40 MB	0
CGS	20	04	65	65	C5	12	4.6	4.4	1.29U	92.	00W	84			



CGS 4.5	23	06	69	02	38	57.3	4.2	77N	92.3	369W	11.2	400MB	0	3.12	C	*3.62
CGSPDE	28	03	68	01	07	37.0	4.2	30N	91.5	500W	11.1	520MB	625PAS	5.11	CFS-B	4.28
GUTE	28	09	69	14	58	27.0	4.2	30N	91.5	500W	11.0	0	625PAS	6.25	CFS-B	4.28
GUTE	23	15	60	15	00	44.0	4.2	30N	91.5	500W	11.0	0	675PAS	6.75	CFS-B	4.12
GUTE	17	03	69	21	33	18.0	4.2	30N	92.0	000W	11.0	0	575PAS	5.75	CFS-B	4.45
CGS 95	23	11	68	11	41	06.2	4.4	33N	92.2	620W	11.5	420MB	*3.45	CFS-B	3.62	
CGS 5	14	11	69	19	59	35.2	4.4	33N	92.2	600W	11.4	400MB	51.0	CFS-B	3.79	
CGS-B	11	08	65	02	63	14.7	4.4	33N	92.2	900W	11.1	400MB	51.3	CFS-B	4.95	
CGS-B	06	07	64	05	33	43.0	4.4	33N	91.3	300W	11.9	370MB	5.78	CFS-B	3.00	
CGS-B	05	04	64	05	30	12.0	4.4	33N	91.6	600W	11.1	400MB	3.00	CFS-B	3.95	
CGS 5	20	08	60	01	19	35.6	4.4	29N	91.5	500W	11.5	0	600PAS	6.00	CFS-B	4.25
CGS-B	14	05	65	12	44	43.0	4.4	29N	91.5	500W	11.3	400MB	*3.29	CFS-B	4.45	
CGS-B	05	05	69	26	08	09.3	4.4	29N	91.5	300W	11.2	420MB	3.45	CFS-B	4.45	
CGS-B	05	05	69	11	11	54.9	4.4	29N	91.5	900W	11.9	440MB	*3.70	CFS-B	3.79	
ERL	57	30	09	72	11	15.1	2.3	0	94.5	369N	91.3	356W	0	4.65	ERL-B	4.95
CGS-B	33	12	64	12	69	25.9	4.2	30N	91.1	700W	11.8	540MB	5.45	CFS-B	3.62	
CGS 66	29	08	69	13	53	6.8	4.2	30N	91.9	698W	11.1	430MB	3.62	CFS-B	3.62	
ERL	15	09	03	73	09	58.5	1.9	44.0	94.0	200W	11.4	470MB	4.26	GUTE	5.75	
CGS-B	21	03	67	03	67	09.5	1.9	44.0	94.0	200W	11.2	400MB	3.12	CFS-B	3.00	
ERL	50	21	08	72	23	3.2	29.1	4.2	29N	90.8	616W	11.9	430MB	6.00	CFS-B	5.60
ERL	84	16	71	00	71	51.2	4.2	29N	90.8	594W	11.6	410MB	*3.45	CFS-B	4.11	
CGS 73	12	11	69	03	48	28.6	4.2	29N	90.8	588W	11.5	410MB	*4.12	CFS-B	4.44	
CGS-B	24	12	65	22	57	16.3	4.2	29N	90.8	590W	11.8	450MB	3.95	CFS-B	5.43	
CGS-B	28	08	65	03	51.3	4.4	30N	90.8	200W	11.5	420MB	3.45	CFS-B	3.29		
CGS-B	30	04	67	03	67	01.0	4.4	30N	90.8	200W	11.5	420MB	3.95	CFS-B	3.78	
CGSPDE	02	11	73	03	62	3.2	29.1	4.2	29N	90.8	200W	11.8	410MB	4.28	CFS-B	4.11
ERL	84	16	71	00	71	51.2	4.2	29N	90.8	16W	11.6	410MB	4.28	CFS-B	3.62	
CGS 96	08	12	68	03	72	3.2	29.1	4.2	29N	90.8	16W	11.6	410MB	4.28	CFS-B	4.11
ERL	84	16	71	00	71	51.2	4.2	29N	90.8	16W	11.5	410MB	*4.12	CFS-B	4.44	
CGS 73	12	11	69	03	48	28.6	4.2	29N	90.8	16W	11.1	400MB	4.65	CFS-B	4.44	
CGS-B	24	12	65	22	57	16.3	4.2	29N	90.8	16W	11.8	450MB	3.95	CFS-B	4.77	
CGS-B	28	08	65	03	51.3	4.4	30N	90.8	16W	11.5	420MB	3.45	CFS-B	5.00		
NDS 81	29	11	70	20	00	56.4	4.4	33N	92.7	699W	12.4	510MB	0	700PAS	7.00	
CGS-B	27	08	65	07	65	07.1	4.4	33N	92.7	700W	12.6	410MB	3.95	CFS-B	6.25	
CGSPDE	02	11	73	03	62	0.6	4.4	33N	92.7	700W	12.6	410MB	4.28	CFS-B	6.25	
CGSPDE	05	07	65	00	56.0	56.0	4.4	33N	91.4	400W	11.1	470MB	4.61	CFS-B	6.25	
CGSPDE	04	05	74	17	49	28.6	4.2	30N	91.5	800W	11.5	460MB	3.95	CFS-B	6.25	
CGSPDE	01	09	74	19	38.0	4.2	30N	91.5	800W	11.1	400MB	4.61	CFS-B	6.25		
ERL	28	27	04	73	04	58.5	0.6	3	91.5	200W	12.6	470MB	2.8	GUTE	7.00	
NDS 81	29	11	70	20	00	56.4	4.4	33N	92.7	699W	12.4	510MB	0	625PAS	6.25	
CGS-B	27	08	65	07	65	07.1	4.4	33N	92.7	700W	12.6	410MB	3.95	CFS-B	6.25	
CGS-B	13	02	65	00	56.0	56.0	4.4	33N	92.7	700W	12.6	410MB	4.28	CFS-B	6.25	
GUTE	04	02	21	08	12	34.9	4.2	30N	91.5	300W	11.2	400MB	7.50	GUTE	6.25	
CGS 96	16	10	60	22	15	32.0	4.2	30N	91.5	200W	12.8	400MB	588PAL	7.50	GUTE	6.25
ERL	39	13	70	21	19	44.3	35.3	4.2	30N	92.2	820W	11.7	440MB	5.88	GUTE	8.30
NDS 81	06	01	60	27	02	18	5.6	30.0	91.5	967W	12.8	430MB	0	830PAS A	3.00	
CGS-B	27	02	65	00	53.9	35.8	4.2	30N	91.5	967W	12.8	430MB	*3.62	GUTE	8.30	
GUTE	19	05	34	10	47	37.4	4.2	30N	91.5	250W	12.0	400MB	6.25PAS	GUTE	3.00	
NDS 36	36	02	65	01	56.0	56.0	4.2	30N	91.5	300W	12.3	500MB	3.95	GUTE	3.00	
CGSPDE	26	09	67	12	21	51.2	4.2	29N	90.5	300W	11.0	400MB	7.50	GUTE	3.12	
CGSPDE	01	09	74	19	38.0	4.2	29N	90.5	300W	11.1	400MB	7.50	GUTE	3.12		
ERL	39	13	70	21	19	44.3	35.3	4.2	30N	92.2	820W	11.7	440MB	5.88	GUTE	3.12
NDS 81	06	01	60	27	02	18	5.6	30.0	91.5	967W	12.8	430MB	0	830PAS B	3.00	
CGS-B	27	02	65	00	53.9	35.8	4.2	30N	91.5	967W	12.8	430MB	3.95	GUTE	3.00	
GUTE	19	05	34	10	47	37.4	4.2	30N	91.5	250W	12.0	400MB	6.25PAS	GUTE	3.00	
NDS 36	36	02	65	01	56.0	56.0	4.2	30N	91.5	300W	12.3	500MB	3.95	GUTE	3.12	
CGSPDE	01	09	74	19	38.0	4.2	29N	91.5	300W	11.1	400MB	7.50	GUTE	3.12		
ERL	39	13	70	21	19	44.3	35.3	4.2	30N	92.2	820W	11.7	440MB	5.88	GUTE	3.12
NDS 81	06	01	60	27	02	18	5.6	30.0	91.5	967W	12.8	430MB	0	830PAS C	3.00	
CGS-B	27	02	65	00	53.9	35.8	4.2	30N	91.5	967W	12.8	430MB	3.95	GUTE	3.12	
GUTE	19	05	34	10	47	37.4	4.2	30N	91.5	250W	12.0	400MB	6.25PAS	GUTE	3.12	
NDS 36	36	02	65	01	56.0	56.0	4.2	30N	91.5	300W	12.3	500MB	3.95	GUTE	3.12	
CGSPDE	01	09	74	19	38.0	4.2	29N	91.5	300W	11.1	400MB	7.50	GUTE	3.12		
ERL	39	13	70	21	19	44.3	35.3	4.2	30N	92.2	820W	11.7	440MB	5.88	GUTE	3.12
NDS 81	06	01	60	27	02	18	5.6	30.0	91.5	967W	12.8	430MB	0	830PAS D	3.00	
CGS-B	27	02	65	00	53.9	35.8	4.2	30N	91.5	967W	12.8	430MB	3.95	GUTE	3.12	
GUTE	19	05	34	10	47	37.4	4.2	30N	91.5	250W	12.0	400MB	6.25PAS	GUTE	3.12	
NDS 36	36	02	65	01	56.0	56.0	4.2	30N	91.5	300W	12.3	500MB	3.95	GUTE	3.12	
CGSPDE	01	09	74	19	38.0	4.2	29N	91.5	300W	11.1	400MB	7.50	GUTE	3.12		
ERL	39	13	70	21	19	44.3	35.3	4.2	30N	92.2	820W	11.7	440MB	5.88	GUTE	3.12
NDS 81	06	01	60	27	02	18	5.6	30.0	91.5	967W	12.8	430MB	0	830PAS E	3.00	
CGS-B	27	02	65	00	53.9	35.8	4.2	30N	91.5	967W	12.8	430MB	3.95	GUTE	3.12	
GUTE	19	05	34	10	47	37.4	4.2	30N	91.5	250W	12.0	400MB	6.25PAS	GUTE	3.12	
NDS 36	36	02	65	01	56.0	56.0	4.2	30N	91.5	300W	12.3	500MB	3.95	GUTE	3.12	
CGSPDE	01	09	74	19	38.0	4.2	29N	91.5	300W	11.1	400MB	7.50	GUTE	3.12		
ERL	39	13	70	21	19	44.3	35.3	4.2	30N	92.2	820W	11.7	440MB	5.88	GUTE	3.12
NDS 81	06	01	60	27	02	18	5.6	30.0	91.5	967W	12.8	430MB	0	830PAS F	3.00	
CGS-B	27	02	65	00	53.9	35.8	4.2	30N	91.5	967W	12.8	430MB	3.95	GUTE	3.12	
GUTE	19	05	34	10	47	37.4	4.2	30N	91.5	250W	12.0	400MB	6.25PAS	GUTE	3.12	
NDS 36	36	02	65	01	56.0	56.0	4.2	30N	91.5	300W	12.3	500MB	3.95	GUTE	3.12	
CGSPDE	01	09	74	19	38.0	4.2	29N	91.5	300W	11.1	400MB	7.50	GUTE	3.12		
ERL	39	13	70	21	19	44.3	3									

CGS-B	26	10	68	14	31	0	2.0	4	200+9IN	92+000W	19.9	410 MB	0	3.29	
CGSPDE	12	10	67	03	17	27.9	4+	400+430N	90+000W	19.0	390 MB	0	3.00		
CGSPDE	23	11	71	10	24	31.9	4+	400+42N	90+000W	17.1	460 MB	0	*4.12		
CGSPDE	25	24	45.9	04	45.9	44+43N	90+000W	31.4W	19.7	420 MB	0	3.45			
CGSPDE	11	01	68	20	48	59.4	4+	400+30N	89+000W	1.9	460 MB	0	*3.79		
CGSPDE	06	16	18	54.9	05	52.3	33.3	4+	390+41N	88+000W	1.93	530 MB	0	*4.12	
CGSPDE	11	06	68	05	52	52.3	33.3	4+	390+41N	88+000W	1.93	530 MB	0	3.00	
ERL	4.9	27	07	73	08	43	52.9	4+	36.0+3N	86+000W	1.91	430 MB	0	3.62	
CGSPDE	27	11	75	08	42	52.9	4+	36.0+3N	87+000W	1.91	368 MB	0	*3.62		
CGSPDE	06	07	64	11	17	57.1	19.1	4+	7.7+00N	93+000W	2.45	370 MB	0	3.00	
GUTE	08	06	37	22	29	39.4	4+	6.0+22N	93+000W	2.00	625 PAS	0	3.25		
TAC	25	06	59	07	01	56.1	01	2.4+0	4.0+250N	93+000W	2.00	530 PAS	0	3.00	
CGS-B	09	01	56	11	12	65.1	11	2.6+0	4.0+250N	92+000W	2.43	400 MB	0	3.00	
CGS-B	09	10	65	09	37	07	3.7	0.7+0	4.0+250N	91+000W	2.14	450 MB	0	3.95	
CGS-B	13	09	64	11	52	18.7	52	2.0+0	4.0+250N	91+000W	2.34	380 MB	0	3.00	
CGS-B	26	09	55	08	28	20.3	52	2.0+0	4.0+250N	91+000W	2.00	687 PAS	0	6.87	
CGS-B	24	02	65	21	69	12.4	52	2.0+0	4.0+250N	91+000W	2.62	420 MB	0	3.00	
CGS-B	21	06	63	02	02	53.5	02	2.1+0	4.0+250N	91+000W	2.14	440 MB	0	3.45	
ERL	6.6	17	11	72	14	55.4	45.4	2.0+0	4.0+250N	91+000W	2.22	520 MB	0	3.00	
GUTE	12	12	39	02	50	1.2+0	2.0+0	2.0+0	4.0+250N	91+000W	2.40	0	550 PAS	0	5.11
GUTE	27	10	45	11	24	41.1	24	4.1+0	4.0+250N	91+000W	2.01	0	675 PAS	0	5.50
ERL	9.2	05	01	46	01	15	1.0+0	2.0+0	4.0+250N	91+000W	2.10	0	600 PAS	0	6.00
CGS-B	17	04	67	07	04	27.7	48.2	4+	2.4+250N	90+000W	2.05	450 MB	0	3.95	
NOS	8.1	16	01	70	06	3.9	35	3.8+2	4+	2.4+250N	91+000W	2.05	390 MB	0	3.00
CGS-B	38	29	05	69	02	17.5	54.1	4+	4.4+27N	90+000W	2.04	460 MB	0	4.12	
CGS-B	16	04	64	11	25	56.2	24	4.4+0	4.4+22N	90+000W	2.13	480 MB	0	4.45	
CGS-B	28	07	65	13	21	25.2	44.3	4+	3.4+250N	90+000W	2.09	370 MB	0	3.00	
CGS-B	14	12	65	14	21	0.5+0	4.4+30C	0	2.0+0	4.0+30C	2.20	430 MB	0	3.62	
CGS-B	01	07	66	20	17	4.9+0	4.4+370N	0	2.0+0	4.0+30W	2.01	460 MB	0	4.62	
CGS-B	09	12	64	14	22	0.3+0	4.4+320N	0	2.0+0	390 MB	0	3.00			
ERL	4.5	28	07	72	13	53.4	02	4.2+0	5.0+250N	90+000W	2.09	440 MB	0	*3.79	
CGS-B	25	12	08	71	05	59	0.0+1	4.2+0	5.0+250N	90+000W	2.10	400 MB	0	3.12	
ERL	6.0	19	01	67	02	1.9+0	2.0+0	2.0+0	4.2+0	5.0+250N	90+000W	2.01	470 MB	0	4.28
GUTE	24	02	34	05	33	30.3	4.2+0	4.2+0	4.2+0	5.0+250N	90+000W	2.00	600 PAS	0	6.95
CGPDE	10	11	73	19	4.6	26.9	4+	4.4+250N	90+000W	2.00	240 MB	4.11	0	4.12	
CGPDE	28	11	73	06	45	0.6+3	4.4+250N	90+000W	2.30	500 MB	2.31	460 MB	0	3.00	
CGPDE	23	10	74	13	51.0	36	51.0	4+	4.7+70N	90+000W	2.00	400 MB	0	3.45	
CGPDE	02	01	75	09	36	44.9	44.9	4+	5.0+250N	90+000W	2.37	430 MB	0	3.62	
CGPDE	03	02	75	01	03	25.4	44.9	4+	5.0+250N	90+000W	2.19	540 MB	0	5.43	
CGPDE	13	08	75	23	07	43.2	43.2	4+	5.0+250N	90+000W	2.56	470 MB	0	4.28	
CGPDE	30	09	75	19	01	25.7	43.2	4+	5.0+250N	90+000W	2.98	430 MB	0	3.62	
CGS-B	01	07	66	20	17	4.9+0	4.4+370N	0	2.0+0	4.0+30W	2.01	460 MB	0	4.12	
CGS-B	09	12	64	14	22	0.3+0	4.4+320N	0	2.0+0	390 MB	0	3.00			
ERL	4.5	28	07	72	13	53.4	02	4.2+0	5.0+250N	90+000W	2.09	440 MB	0	*3.79	
CGS-B	25	12	08	71	05	59	0.0+1	4.2+0	5.0+250N	90+000W	2.10	400 MB	0	3.12	
ERL	6.0	19	01	67	02	1.9+0	2.0+0	2.0+0	4.2+0	5.0+250N	90+000W	2.01	470 MB	0	4.28
GUTE	24	02	34	05	33	30.3	4.2+0	4.2+0	4.2+0	5.0+250N	90+000W	2.00	600 PAS	0	6.95
CGS-B	09	12	64	14	22	0.3+0	4.4+320N	0	2.0+0	390 MB	0	3.00			
ERL	4.5	28	07	72	13	53.4	02	4.2+0	5.0+250N	90+000W	2.09	440 MB	0	*3.79	
CGS-B	25	12	08	71	05	59	0.0+1	4.2+0	5.0+250N	90+000W	2.10	400 MB	0	3.12	
ERL	6.0	19	01	67	02	1.9+0	2.0+0	2.0+0	4.2+0	5.0+250N	90+000W	2.01	470 MB	0	4.28
GUTE	24	02	34	05	33	30.3	4.2+0	4.2+0	4.2+0	5.0+250N	90+000W	2.00	600 PAS	0	6.95
CGS-B	09	12	64	14	22	0.3+0	4.4+320N	0	2.0+0	390 MB	0	3.00			
ERL	4.5	28	07	72	13	53.4	02	4.2+0	5.0+250N	90+000W	2.09	440 MB	0	*3.79	
CGS-B	25	12	08	71	05	59	0.0+1	4.2+0	5.0+250N	90+000W	2.10	400 MB	0	3.12	
ERL	6.0	19	01	67	02	1.9+0	2.0+0	2.0+0	4.2+0	5.0+250N	90+000W	2.01	470 MB	0	4.28
GUTE	24	02	34	05	33	30.3	4.2+0	4.2+0	4.2+0	5.0+250N	90+000W	2.00	600 PAS	0	6.95
CGS-B	09	12	64	14	22	0.3+0	4.4+320N	0	2.0+0	390 MB	0	3.00			
ERL	4.5	28	07	72	13	53.4	02	4.2+0	5.0+250N	90+000W	2.09	440 MB	0	*3.79	
CGS-B	25	12	08	71	05	59	0.0+1	4.2+0	5.0+250N	90+000W	2.10	400 MB	0	3.12	
ERL	6.0	19	01	67	02	1.9+0	2.0+0	2.0+0	4.2+0	5.0+250N	90+000W	2.01	470 MB	0	4.28
GUTE	24	02	34	05	33	30.3	4.2+0	4.2+0	4.2+0	5.0+250N	90+000W	2.00	600 PAS	0	6.95
CGS-B	09	12	64	14	22	0.3+0	4.4+320N	0	2.0+0	390 MB	0	3.00			
ERL	4.5	28	07	72	13	53.4	02	4.2+0	5.0+250N	90+000W	2.09	440 MB	0	*3.79	
CGS-B	25	12	08	71	05	59	0.0+1	4.2+0	5.0+250N	90+000W	2.10	400 MB	0	3.12	
ERL	6.0	19	01	67	02	1.9+0	2.0+0	2.0+0	4.2+0	5.0+250N	90+000W	2.01	470 MB	0	4.28
GUTE	24	02	34	05	33	30.3	4.2+0	4.2+0	4.2+0	5.0+250N	90+000W	2.00	600 PAS	0	6.95
CGS-B	09	12	64	14	22	0.3+0	4.4+320N	0	2.0+0	390 MB	0	3.00			
ERL	4.5	28	07	72	13	53.4	02	4.2+0	5.0+250N	90+000W	2.09	440 MB	0	*3.79	
CGS-B	25	12	08	71	05	59	0.0+1	4.2+0	5.0+250N	90+000W	2.10	400 MB	0	3.12	
ERL	6.0	19	01	67	02	1.9+0	2.0+0	2.0+0	4.2+0	5.0+250N	90+000W	2.01	470 MB	0	4.28
GUTE	24	02	34	05	33	30.3	4.2+0	4.2+0	4.2+0	5.0+250N	90+000W	2.00	600 PAS	0	6.95
CGS-B	09	12	64	14	22	0.3+0	4.4+320N	0	2.0+0	390 MB	0	3.00			
ERL	4.5	28	07	72	13	53.4	02	4.2+0	5.0+250N	90+000W	2.09	440 MB	0	*3.79	
CGS-B	25	12	08	71	05	59	0.0+1	4.2+0	5.0+250N	90+000W	2.10	400 MB	0	3.12	
ERL	6.0	19	01	67	02	1.9+0	2.0+0	2.0+0	4.2+0	5.0+250N	90+000W	2.01	470 MB	0	4.28
GUTE	24	02	34	05	33	30.3	4.2+0	4.2+0	4.2+0	5.0+250N	90+000W	2.00	600 PAS	0	6.95
CGS-B	09	12	64	14	22	0.3+0	4.4+320N	0	2.0+0	390 MB	0	3.00			
ERL	4.5	28	07	72	13	53.4	02	4.2+0	5.0+250N	90+000W	2.09	440 MB	0	*3.79	
CGS-B	25	12	08	71	05	59	0.0+1	4.2+0	5.0+250N	90+000W	2.10	400 MB	0	3.12	
ERL	6.0	19	01	67	02	1.9+0	2.0+0	2.0+0	4.2+0	5.0+250N	90+000W	2.01	470 MB	0	4.28
GUTE	24	02	34	05	33	30.3	4.2+0	4.2+0	4.2+0	5.0+250N	90+000W	2.00	600 PAS	0	6.95
CGS-B	09	12	64	14	22	0.3+0	4.4+320N	0	2.0+0	390 MB	0	3.00			
ERL	4.5	28	07	72	13	53.4	02	4.2+0	5.0+250N	90+000W	2.09				

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APPENDIX E

COMPUTER PROGRAMS

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***** STANFORD UNIVERSITY *****
C PACRAY GENER-MAG
C THIS PROGRAM REPLACES THE LETTERS EARTHQUAKE RICHTER MAGNITUDES
C A,B,C,D,E FROM THE NAMES & MORE MICRAGUA EARTHQUAKE CATALOGI
C BY A RICHTER MAGNITUDE OBTAINED THROUGH RANDOM NUMBER GENERATION.
C ***** INPUT FORMAT
C ***** COL VARIABLE NAME VARIABLE DESCRIPTION
C 1.- EARTHQUAKE RECORD CARD 1 CARD PER RECCRC (10A4)
C 1-36 XMAG(1-9) COL 1/36 READ IN A FORMAT
C 37-40 XMAG(10) LETTER EARTHQUAKE MAGNITUDE
C
C OUTPUT
C *****
C THE OUTPUT IS SAVED ON DISK, IT CONTAINS THE SAME INFORMATION AS
C THE INPUT, BUT THE LETTER MAGNITUDE HAS BEEN REPLACED BY THE
C RICHTER MAGNITUDE.
C *****
C DATA B,C,D,E/, E/, C*, D*, E/
C DIMENSION XMAG(10)
C *****
C
C IY=1
C CALL RAND(IY,YFL,0)
C DO 100 I=1,100
C READ(5,1000,END=99)XMAG
C 100C FORMATT(10A4)
C CALL RAND(IY,YFL,0)
C
C IF (XMAG(1).EQ.-E) GO TO 110
C IF (XMAG(1).EQ.0) GO TO 120
C IF (XMAG(1).EQ.-C) GO TO 130
C IF (XMAG(1).EQ.B) GO TO 140
C
C WRITE(10,2000)XMAG,XMAG(9)
C 200C FORMATT(10A4,31X,A4)
C GO TO 100
C
C 110 MAG=13.5+YFL*1.8)*100.
C GC TO 109
C
C 120 MAG=5.3+YFL*.7)*100.
C GC TC 109
C
C 130 MAG=(6.0+YFL)*100.
C GC TC 1C5
C
C 140 MAG=(7.0+YFL*.7)*100.
C 105 WRITE(10,2020)XMAG,MAG
C 2020 FORMAT(10A4,31X,14)
C 10C CONTINUE
C 99 RETURN
C END
C SUBROUTINE RANDK (IY,YFL,INDEX)
C THIS SUBROUTINE GENERATES RANDOM NUMBERS
C
C IY=IY*314159269*453806245
C 4 IF(IY.GE.0) GO TO 6
C 5 IY=IY*2147483647+1
C 6 CONTINUE
C IF (INDEX.GT.0) GO TO 8
C YFL=FLOAT(IY)*.456613E-9
C 8 RETURN
C END
C
C $DATA

```

```

***** STANFORD UNIVQSITY *****
C 200 CONTINUE
C RETURN
END
$DATA
***** SORTS EARTHQUAKES BY DEPTH. THE BAND THICKNESS IS SET
SET TO 10 KM.
***** INPUT FORMAT
***** READ FROM DISK. ONE CARD PER EVENT. (8A4,13,1I4)
C   C   VARIABLE NAME      VARIABLE DESCRIPTION
C   C   1-32    XIN1          COL 1/32 READ IN A FORMAT
C   C   33-35   NDP           DEPTH OF hypocenter
C   C   36-79    XIN3          COL 36/79 READ IN A FORMAT
C
C   OUTPUT
C   ***** THE OUTPUT IS SAVED ON DISK AND CONTAINS THE DATA SORTED BY
C   ***** RANK WITHIN 10 KM STARTING AT DEPTH 0
C
C   ***** DIMENSION XINF1(21,180,8),NDEP(21,180)
C   * XINF3(21,180,11),IND(21),XIN1(18),XIN3(11)
C
C   00 120 I=1,21
C   130 IND(I)=0
C   100 READ(5,1000,END=99)XIN1,NDP,XIN3
C   1000 FORMAT(8A4,13,1I4)
C   N=IND(I)+1
C   T=N,G=21,N=21
C   IND(N)=IND(N)+1
C   NDEPIN(N,NDP)=NDP
C   DJ=400,J=1,8
C   DO 400 XIN1(N,NDP,J)=XIN1(J)
C   400 XIN1(N,NDP,J)=XIN3(J)
C   ND=500,J=1,11
C   500 XIN3(N,NDP,J)=XIN3(J)
C   GO TO 100
C
C   99 CONTINUE
C   DC 200 N=1,21
C   NDP=(N-1)*10
C   NED=IND(N)
C   IF(NEDEQ.0) GO TO 210
C   WRITE(11,2010) NED,NDP
C   2010 FORMAT(15,'EQ AT DEPTH ',15,' PLUS ',1,
C   POINT,'ED.', 'EQ AT DEPTH ',NDP,' DLTUS',
C   DR 220 I=1,NED
C   WRITE(11,2000)(XINF1(N,I,J),J=1,8),NDEPIN,11,
C   1 (XINF3(N,I,J),J=1,11)
C   2000 FORMAT(1,'8A4,13,1I4),
C   220 CONTINUE
C   PRINT*, ''
C   PRINT*, ''
C   GO TO 200
C   GO TO 200 EQ AT DEPTH ,NDP, PLUS*
C
C

```

```

***** STANFORD UNIVERSITY *****

C PROGRAM LOCATE LINE
C THIS PROGRAM FITS A LINE THROUGH A SET OF POINTS IN A PLANE USING
C REGRESSION ANALYSIS. IT IS USED TO LOCATE A SEISMIC LINE SOURCE
C GIVEN A NUMBER OF EPICENTERS.
C INPUT FORMAT
C *****
C COL VARIABLE NAME VARIABLE DESCRIPTION
C 1.- NUMBER OF SOURCES 1 CARD (15)
C NBST NB OF SOURCES TO BE LOCATED
C 2.- SOURCE IDENTIFICATION 1 CARD ((15,18A4))
C NBRC NB OF RECORDS IN THE SOURCE
C 6-77 HED1 IDENTIFICATION OF THE SOURCE
C 3.- RECORD CARDS NBRC CARDS (4F10.0)
C 1-10 Y1 LATITUDE OF THE EPICENTER
C 11-20 X1 LONGITUDE OF THE EPICENTER
C 21-30 D1 DEPTH OF THE HYPOCENTER
C 31-40 XM1 PICTER MAGNITUDE
C REPEAT CARDS 2/3 NRST TIMES FOR THF NBST SOURCES
C OUTPUT
C *****
C THE OUTPUT DISPLAYS THE SOURCE IDENTIFICATION, THE NUMBER OF
C RECORDS FOR IN THE SOURCE, THE COORDINATES OF THE CENTROID
C OF THE DATA AS WELL AS THE SLOPE AND THE DEPTH OF THE LINE.
C
C IMPLICIT REAL*8 (A-H, C-Z)
C DIMENSION HED1(18), X(200), Y(200), D(200), XM1(200)
C REAL*4 HED1
C *****
C
C READ(5,1000)NBST
C DO 100 IS=1,NBST
C XA=0.0D0
C YA=0.0D0
C DB=0.0D0
C Y2=0.0D0
C X2=0.0D0
C XY=0.0D0
C XT=0.0D0
C YT=0.0D0
C XYT=0.0D0
C READ(5,1000)NBRC,HED1
C 1000 FORMAT(15,18A4)
C WRITE(6,2000)NBRC,HED1
C 2000 FORMAT(I,NUMBER OF RECORDS,I6,3X,18A4)
C ON 110 IR=1,NR0C
C READ(5,1010)Y(10),X(10),D(10),XM1(10)
C 1010 FORMAT(19X,F5.3,3X,F5.3,14X,F3.0,25X,F3.2)
C X(IR)=-(X(10)

```

```

*****  

C PROGRAM LINE.INTP  

C *****  

C THIS PROGRAM COMPUTES THE INTERSECTION OF TWO LINES IN THE SAME  

C PLANE. IT IS USED TO OBTAIN THE INTERSECTION OF TWO LINE SOURCES  

C AT THE SAME DEPTH.  

C C INPUT FORMAT  

C *****  

C COL VARIABLE NAME VARIABLE DESCRIPTION  

C 1.- IDENTIFICATION 1 CARD (20A4)  

C 1-80 HED1 IDENTIFICATION OF THE FIRST LINE  

C 2.- LINE PARAMETERS 1 CARD (3F10.0)  

C 1-10 X1 COORDINATE OF A POINT OF THE LINE  

C 11-20 Y1 Y COORDINATE OF THE SAME POINT  

C 21-30 AL1 SLOPE OF THE LINE  

C 3.- IDENTIFICATION 1 CARD (20A4)  

C 1-90 HED2 IDENTIFICATION OF THE SECOND LINE  

C 2.- LINE PARAMETERS 1 CARD (3F10.0)  

C 1-10 X2 COORDINATE OF A POINT OF THE LINE  

C 11-20 Y2 Y COORDINATE OF THE SAME POINT  

C 21-30 AL2 SLOPE OF THE LINE  

C  

C OUTPUT  

C *****  

C THE OUTPUT DISPLAYS THE IDENTIFICATION OF THE LINES AND  

C THEIR COORDINATES OF THEIR INTERSECTION POINT.  

C *****  

C DIMENSION HED1(20),HED2(20)  

C *****  

C 100 READ(5,1000 END=99)HED1  

C READ(5,1001)X1,Y1,AL1  

C DO 200 I=1,2  

C READ(5,1000)HED2  

C READ(5,1001)X2,Y2,AL2  

C 1000 FORMAT(20A4)  

C 1001 FORMAT(8F10.0)  

C  

C X=(AL1*X1-AL2*X2-Y1+Y2)/(AL1-AL2)  

C IF(Y2,EQ.0.) X=X2  

C Y=AL1*(X-X1)+Y1  

C  

C WRITE(6,2000)HED1,HED2  

C 2000 FORMAT('20A4')  

C PRINT,'LINE1',X1,Y1,AL1  

C PRINT,'LINE2',X2,Y2,AL2  

C PRINT,'INTERSECTION',X,Y  

C 200 CONTINUE  

C GO TO 100  

C 99 RETURN  

C END

```

```

***** STANFORD UNIVERSITY *****
C PROGRAM LINE LENGTH
C THIS PROGRAM COMPUTES THE LENGTH OF A SEGMENT OF LINE BETWEEN
C TWO HORIZONTAL COORDINATES.
C INPUT FORMAT
C *****
C CCL VARIABLE NAME VARIABLE DESCRIPTION
C 1.- IDENTIFICATION 1 CARD (20A4)
C 1-80 HED1 LINE IDENTIFICATION
C 2.- LINE PARAMETERS 1 CARD (5F1.C)
C 1-10 X1 X COORDINATE OF A POINT OF THE LINE
C 11-20 Y1 Y COORDINATE OF THE SAME POINT
C 21-30 ALPH SLOPE OF THE LINE
C 31-40 XS X COORD OF ORIGIN OF SEGMENT
C 41-50 YX X COORD OF END OF SEGMENT
C
C OUTPUT
C *****
C THE OUTPUT DISPLAYS THE LINE IDENTIFICATION AND PARAMETERS AS
C WELL AS THE LENGTH OF THE SEGMENT.
C *****
C DIMENSION HED1(20)
C *****
C 100 READ(5,1001,END=99)HED1
1001 FORMAT(20A4)
      READ(5,1000)X1,Y1,ALPH,XS,XE
1000 FORMAT(8F1.0)
      YS=ALPH*(XS-X1)+Y1
      YE=ALPH*(XE-X1)+Y1
      XLEN=SQRT((XS-XE)*(XS-XE)+(YS-YE)*(YS-YE))
      WRITE(6,2000)HED1
2000 FORMAT('O',20A4)
      PRINT,'DATA',X1,Y1,ALPH
      PRINT,'ORIGIN END LENGTH ',XS,YS,XE,YE,XLEN
      GO TO 100
99 RETURN
END
$DATA

```



```

      WRITE(6,2112)RMNN,RMCK
2112 FORMAT('MINIMUM MAGNITUDE',12X,F10.2)
      *0MAGNITUDE INCREMENT FOR CDF ,F10.2)
      DO 120 I=1,50
      RMCK=NMCK+NBIC
      IF(RMCK.LT.(RMCK-.05)) GO TO 121
      RMCK=RMCK
120  WRITE(6,2220) RMNN,RMCK

2220 FORMAT(' THERE IS NO RECORD BETWEEN MAGNITUDE',F8.2,' AND',F8.2)

C 121 CONTINUE
      WRITE(6,2004)
2004 FORMAT('EARTHQUAKE MAGNITUDES')
      WRITE(6,2003)(RM(IX),IX=1,NRRC)
2003 FORMAT(' ',13F10.2)
      RMIC2=RMIC*.5
      X(1)=NMCK1+RMIC2
      NBIC=0
      DO 102 I=1,49
      X(I+1)=X(I)+RMIC
      102 THIS(I)=0
C
      RMNN1=NMCK1-.1
      DO 103 IX=1,NRRC
      NBKC=(IPM(IX)-RMNN1)/RMIC)+.999
      THIS(NBKC)=THIS(NBKC)+1
      IF(NBKC.GT.NBIC) NBIC=NBKC
103  CONTINUE
C
      IF(SKIPDZ.EQ.1.0).SKIPC0.FQ.1) GO TO 133
C
      I=0
C
      131 I=I+1
      IF(I.GE.NBIC) GO TO 133
      IF(THIS(11)-NE.0) GO TO 131
      NBIC=NBIC-1
      DO 132 K=1,NBIC
      X(K)=X(K+1)
      THIS(K)=THIS(K+1)
      NBIC=NBIC-1
      GO TO 134
C
      133 CONTINUE
      IF(NRIC.LT.-2) GO TO 111
      COMPUTE CDF
      YY(NBIC)=THIS(NBIC)
      NBIC1=NRIC+1
      DO 130 IX=2,NRIC
      XX=NRIC1-IX
      YY(IX)=YY(IX+1)+FLOAT(THIS(IX))
130  YY(IX)=YY(IX)*10X,CUMULATIVE FREQUENCY /
      *INTERVAL .5X,FREQUENCY .10X,MCURRENTS ABOVE RM /)
      DO 160 IX=1,NRIC
      RM1=X(IX)-PRIC2
      RM2=X(IX)+PRIC2-.01
160  WRITE(6,2005)
      FORMAT(' ',0,'IX,RM',10X,'INTERVAL',10X,'CUMULATIVE FREQUENCY',/
      *INTERVAL .5X,FREQUENCY .10X,MCURRENTS ABOVE RM /)
      RM1=X(IX)-PRIC2
      RM2=X(IX)+PRIC2-.01
160  WRITE(6,2006)RM1,PM2,THIS(IX),YY(IX),RM1
      WRITE(6,2007)ALPHAN,ALPHA*,F10.6//OBETA *,F10.6 //)
      C
      IF(A.EQ.0..OR.T.EQ.0..) GO TO 360
      ALPHAN=ALPHA*(T)-ALNG(A*T)
      WRITE(6,2008)ALPHAN
      2008 FORMAT('NORMALIZED ALPHA',F10.6 //)

```

```

C 360 CONTINUE
C      IF(IAKA.NE.0) GO TO 330
C      IF(I1R.EQ.1) GO TO 111
C
C      COMPUTE INTERSECTION POINT IF THERE ARE TWO LINES
C      XM=ABS((ALPHA(11)-ALPHA(21))/(BETA(11)-BETA(21))
C      XLN=ALPHA(11)*BETA(11)*XM
C      WRITE(6,2120)XM,XLN
C 2120 FORMAT(10INTERSECTION POINT// MAGNITUDE*,F8.2,/,LN OF N *,F8.2)
C
C      GO TO 111
C9  RETURN
END
$DATA

```

```

***** STANFORD UNIVERSITY *****  

PROGRAM ACC LINE AREA  

***** THIS PROGRAM COMPUTES PROBABILITY OF EXCEEDANCE OF GIVEN PEAK *****  

GROUND ACCELERATIONS AT SPECIFIED SITES DUE TO LINE OR AREA *****  

SEISMIC SOURCES. *****  

***** INPUT FORMAT *****  

*****  

1.- IDENTIFICATION CARD 1 CARD 20A4  

1-80 HED1 TITLE  

2.- ATTENUATION CONSTANTS 1 CARD 3F10.0  

1-10 B1  

11-20 B2  

21-30 B3  

31-40 B4  

41-50 DELTA1 STEP SIZE FOR LINE INTEGRATION  

51-60 DELTAC STEP SIZE FOR CIRCLE INTEGRATION  

UNITS OF LENGTH USED IN THE PROGRAM  

ATTENUATION FORMULA OF THE TYPE  

ACC=51*EXP(B2*MAG)/(R+B4)**B3  

3.- PROBLEM DESCRIPTION 4 CARDS  

CARD 1 415 NUMBER OF LINE SOURCES  

1- 5 NL NUMBER OF AREA SOURCES  

6-10 NA NUMBER OF TIME PERIODS  

11-15 NT NUMBER OF ACC  

16-20 NY  

CARD 2 TIME PERIODS, NT VALUES, 8 VALUES PER CARD 8F10.0  

1-10 T(1) PERIOD 1  

11-20 T(2) PERIOD 2  

**** T(NT) PERIODS NT  

CARD 3 PEAK GROUND ACC., NY VALUFS, 8 VALUES PER CARD 8F10.0  

1-10 Y(1) PGA 1  

11-20 Y(2) PGA 2  

**** Y(NY) PGA NY  

CARD 4 SEARCHING GRID DESCRIPTION 8F10.0  

1-10 XBEGIN ORIGIN OF GRID X COORD  

11-20 YBEGIN ORIGIN OF GRID Y COORD  

21-30 XEND END OF GRID X COORD  

31-40 YEND END OF GRID Y COORD  

ORIGIN AND END SHOULD COINCIDE IF ONLY  

ONE LOCATION IS REQUIRED  

X INCREMENT  

Y INCREMENT  

DX AND DY MUST BE DIFFERENT FROM ZERO  

4.- LINE SOURCES PROPERTIES 3 CARDS PER SOURCE

```

```

      YL1(I,L)=YL2(I,L)
      XL2(I,L)=XN
      YL2(I,L)=YD
      1040 FORMAT(10401HED1
      1041 FORMAT(10,1041HED1,
      1042 FORMAT(0,1041HED1,
      1043 READ(5,10401HED1
      1044 WRITE(6,1041HED1,
      1045 READ(5,1001)NL,NA,NT,NY
      1046 WRITE(6,1001)NL,NA,NT,NY
      1047 CONTINUE
      610 IF(NA.LE.0) GO TO 551
      C   READ AND WRITE AREA SOURCE PROPERTIES
      C   T=NL+1
      NM=NL+NA
      WRITE(6,2124) NM
      2124 FORMAT(10,2124) AREA SOURCES//***** */
      C
      DD 615 I=NL,NM
      READ(5,10401HED2
      READ(5,1000) ALPHA1(I),BETA1(I),XL1(I),YL1(I),XL2(I),YL2(I)
      READ(5,1000) ALPHA2(I),BETA2(I),RML(I)
      WRITE(6,1041HED2
      WRITE(6,2120)
      WRITE(6,1023) ALPHAL(I),BETAL(I),XL1(I),YL1(I),XL2(I),YL2(I)
      WRITE(6,2123)
      WRITE(6,1023) ALPHA2(I),BETA2(I),RML(I)
      615 CONTINUE
      C   ITERATION ON TIME PERIODS
      C   2120 FORMAT(10,2X,'ALPHA',10X,'BETA',11X,'X0'
      C   & 12X,Y0,12X,'R',12X,'MA')
      2123 FORMAT(10,2X,'SECOND REGRESSION CONSTANTS',/,*2X,'ALPHA2',10X,
      & BETA2,10X,'MR')
      551 CONTINUE
      C
      650 370 IT=1,NT
      WRITE(6,2251)
      2250 FORMAT(10,2X,'TIME PERIODS')
      2251 FORMAT(10,2X,'ACCELERATIONS')
      2252 FORMAT(10,2X,'SITE LOCATION',/,*1X,*F10.3,5X,*Y= *F10.3)
      2253 READ(5,1000) XBEGIN,YBEGIN,XEND,YEND,DX,DY
      2254 DMAX=DARS((XBEGIN-XEND)/DX)+1,200
      NMAX=MAX((YBEGIN-YEND)/DY)+1,200
      NMAX=MAX(NMAX,NMAX,NMAX,NMAX,NT)
      WRITE(6,1021) NMAX,NMAX,NMAX,NMAX,NT
      1021 FORMAT(10,1000) ALPHA1(I),BETAL(I),XL1(I),YL1(I),
      & NMAX,X= *15.5X,*NMAX=*,15.5X,INT=*,15 / )
      IF(NL.LE.0) GO TO 610
      C   READ AND WRITE LINE SOURCE PROPERTIES
      C
      WRITE(6,1024)
      1024 FORMAT(10LINE SOURCES//***** */
      DO 616 I=NL
      READ(5,10401HED2
      READ(5,1000) ALPHA2(I),BETA2(I),RML(I)
      XL2(I,L)=HL(I)
      WRITE(6,1021) ALPHA1(I),BETAL(I),XL1(I),YL1(I),XL2(I),YL2(I),HL(I)
      WRITE(6,1022)
      1022 FORMAT(10,2X,'ALPHAL',10X,'BETAL',11
      CX,*XL1*,11X,XL2*,11X,YL1*,11X,YL2*,12X,*HL*)
      WRITE(6,1023) ALPHA2(I),BETA2(I),RML(I)
      WRITE(6,1024) ALPHA2(I),BETA2(I),RML(I)
      1031 FORMAT(10,2X,'SECOND REGRESSION CONSTANTS',/,*2X,'ALPHA2',9X,
      & L2*,10X,'MR')
      616 CONTINUE
      DO 700 IL=1,NL
      IF(XL1(IL).LT.XL2(IL)) GO TO 700
      XD=XL1(IL)
      YD=YL1(IL)
      XL1(IL)=XL2(IL)
      C   ITERATION ON LINE SOURCES
      C   IF(NL.EQ.0) GO TO 441
      DO 351 IL=1,NL
      CALL CONST(X,Y,XL1(IL),YL1(IL),XL2(IL),YL2(IL)),

```

```

1 AL1=HL(IL)
GAM22=DEXP(ALPHA2(IL))
DEL22=BETA2(IL)/R2
RHO22=DEL22*B3
FFY=DL1*DL1+HL1*HL1
RH=(LQSRT(FFY)*B4)*RHO22
EXCRH=GAM22*YGT1B1**DEL22*(AL2+AL1)*T(IL)
IF(EXCRH.LE.1.D-5) GO TO 351
CALL INTGIN(IL,XL1,YL1,XL2(IL),YL2(IL),AL1,AL2,DL1,
1 HL(IL),AL1,BIL,YGT1,X,Y,FFY,GAM22,DEL22,RHO22,FF,FF1)
SUM=SUM+FF
SUM1=SUM1+FF1

C 351 CONTINUE
C
C 441 IF(NA.EQ.0) GO TO 442
I1=NL+1
IEND=NL+NA
DO 850 TA=I1,IEND
C CHECK WHETHER THE CIRCLE CONTRIBUTES ANY TO THE PROB
RI=XL2(IL)
HL1=HL(IL)
GAM22=DEXP(ALPHA2(IL))
DEL22=RETA2(IL/B2)
RHO22=DEL22*B3
DIST=DSORT((X-ML1(IL))*((X-XL1(IL))+(Y-YL1(IL)))
R1=10*ST*B4)**RHO22
EXCRK=RHM22*YGT1B1**DEL22*(6.2831D0*RI*RI*T(IL)
IF(EXCRK.LE.1.D-5) GO TO 850
SUM=0.00
SUM1=0.00
NR=RI/DELTAC+0.900
DR=RI/ANR
DR1=DP*+.500

C ITERATION ON THE NB OF SEGMENTS IN THE CIRCLE
DO 202 IR1,NR
DL1=DABS(DIST-DR1)
DL2=DIST+DR1
AL=DSCRT(PIA*PIA-DR1*DR1)
FFY=DL1*DL1+HL1*HL1
CALL INTGIN(IL,0.0,DL1,0.0,DL1,0.0,DL1,0.0,0.0,DL1,
1 YGT1,0.0,0.0,0.0,FFY,GAM22,DEL22,RHO22,FF,FF1)
SUM1=SUM1+FF*2.00
DR1=DP1+DR
SUM1=SUM1+FF1*2.00
FF*DL2*DL2+HL1*HL1
CALL INTGIN(IL,0.0,DL2,0.0,DL2,0.0,DL2,0.0,0.0,DL2,
1 YGT1,0.0,0.0,0.0,FFY,GAM22,DEL22,RHO22,FF,FF1)
SUM1=SUM1+FF*2.00
SUM1=SUM1+FF1*2.00
DR1=DP1+DR

C 202 CONTINUE
SUMP=SUMP+DP
SUMP1=SUMP1*DR
SUM=SUM+SUMP
SUM1=SUM1*SUMP1

C 850 CONTINUE
F1(IL)=DEXP(-SUM*(IT))
F2(IL)=1.0D0-FY1(IL)
PDF(IL)=SUM1*T(IL)*FY1(IL)
360 CONTINUE
C
C WRITE(110,50001)(FY2(J),J=2,NY)
NY=(NY-1)/10+1
K=2
NY1=11
DO 375 J=1,NY
IF(JJ.EQ(NY)) NY1=NY
WRITE(6,226)(YG(J),J=K,NY1)
WRITE(6,227)(FY2(J),J=K,NY1)
WRITE(6,228)(FY1(J),J=K,NY1)
WRITE(6,229)(PDF(J),J=K,NY1)
NY1=NY1+10
K=K+10
375 CONTINUE
5010 FORMAT(' SITE ',2F10.2)
5000 FORMAT(10F8.5)
226 FORMAT(' P(G)= ',10F10.4)
227 FORMAT(' P(Y>Y0)',10F10.4)
228 FORMAT(' P(Y<Y0)',10F10.4)
229 FORMAT(' P(D(Y)',10F10.4)

C MOVE TO THE NEXT POINT ALONG THE GRID
380 X=X+DX
385 Y=Y+DY
370 CONTINUE
365 CONTINUE
225 FORMAT(11,'***** PROBABILITY DISTRIBUTION *****
61 ON OFF PEAK GROUND ACCELERATION *****
2*****,//)
STOP
END
C SUBROUTINE CONSTL(X,Y,XL1,YL1,XL2,YL2,
1 AL1,AL2,DL1,AM,B)
C
C IMPLICIT REAL*8 (A,H,O-Z)
C
C AN = SLOPE OF LINE DEFINED FY (X1,Y1),(X2,Y2)
C B = Y-INTERCEPT
C
C AN=(YL2-YL1)/(XL2-XL1)
C B=YL1-AN*XL1
C XBAR=(X+AM*Y-B*AM)/(AM*AM+1.0D0)
C YBAR=(AM*X+Y-AM*AM*AM)/(AM*AM+1.0D0)
C DL1=DSCRT((X-XBAR)*(X-XBAR)+(Y-YBAR)*(Y-YBAR))
C
C CHECK POSITION OF (YBAR,YBAR) ALONG LINE W.R.T. END
C OF POINTS OF LINE SOURCE
C AL1=DSORT((XL1-XBAR)*(XL1-XBAR)+(YL1-YBAR)*(YL1-YBAR))
AL1=NSORT((XL2-XBAR)*(XL2-XBAR)+(YL2-YBAR)*(YL2-YBAR))
C
C IF(XBAR.LT.XL1) AL1=-AL1

```



```

C DO 900 IXSG=1,NBSG
C TERM=0.0D
C IX=IXSG
C IF(IX.EQ.3) IX=1
C COMPUTE LENGTH OF SEGMENT CONSIDERED AND DETERMINE
C NUMBER OF INCREMENTS
C XLEN=DQRT((XX(IXSG)-XX(IXSG+1))*((XX(IXSG)-XX(IXSG+1))+  

C (YY(IXSG)-YY(IXSG+1))*(YY(IXSG)-YY(IXSG+1)))
C NBIC=IXLEN/DELTAL+.9D0
C IF(NBIC.EQ.0) NBIC=1
C XNBIC=NBIC
C DELTA=ALEN/XNBIC
C START DO LOOP AT 1/2 DELTA
C DELD0=EL(IX)
C RHOODERHO(IX)
C AL=AL+.5D0*DELTAL
C DO 100 IXIC=1,NBIC
C RH=AL*(L+FFY
C TERM=TERM+ID SORT(RH)+B*)**RHODO
C AL=AL+DELTAL
C 100 CONTINUE
C AL=AL-.5D0*DELTAL
C TERM=TERM*GAM((IX)*(YG1)/R1)**OELDO*I*DELTAL
C FF=FF+TERM
C FF1=FF1+TERM*DELD0/YG1
C 900 CONTINUE
C RETURN
C END
C $DATA

```

```

*****  

      PR(1)=1.00  

      AC(1)=0.0  

      AC(2)=ACST  

      DD 120 I=2,NBAC  

      120 AC(I+1)=AC(I)+AC  

      C   PRINT,AC*,AC  

      C   DO 100 I=P+1,NBPD  

      C   READ(5,100)HED1  

      1000 FORMAT(20A4)  

      WRITE(6,2000)HED1  

      2000 FORMAT(1X,20A4)  

      WRITE(10,1000)HFD1  

      DD 100 IY=1,NBYY  

      DD 110 IX=1,NBXX  

      C   READ(5,1020)HED1  

      C1020 FORMAT(1X,20A4)  

      C   DD 150 KP=2,NBAC1  

      C   IF(PBCK.GE.,PB(KP)) GO TO 160  

      1030 FORMAT(10F.0)  

      C   READ(5,1030)(PB(I),I=2,NBAC1)  

      C   150 CONTINUE  

      C   WRITE(6,2020)HFD1  

      2020 FORMAT(10*16 ATTENTION ***/* EXCEDANCE IS LARGER THAN THE INPUT *  

      1 /* '20A4)  

      ACPB(IX)=AC(NBAC1)  

      GO TO 110  

      C   160 ACPB(IY)=AC(KP-1)+((AC(KP)-AC(KP-1))*(PB(KP-1)-PB(K))/  

      1 *(PB(KP-1)-PB(KP))  

      110 CONTINUE  

      C   WRITE(6,2040)(ACPBI(),I=1,NBXX)  

      2040 FORMAT(1X,12F10.6)  

      WRITE(10,3000)(ACPRI(),I=1,NBXX)  

      3000 FORMAT(18F10.6)  

      100 CONTINUE  

      RETURN  

      END  

      $DATA  

*****  

      THE OUTPUT IS SAVED ON DISK AND CONTAINS THE RUN IDENTIFICATION  

      AND THE ACCELERATION AT EACH NODE CORRESPONDING TO THE GIVEN  

      PROBABILITY OF EXCEDANCE. FORMAT (8F10.6)  

*****  

      IMPLICIT REAL*8 (A-H,0-2)  

      DIMENSION AC(30),PB(30),ACPBA(40),HED1(20),HED2(20)  

*****  

      C   READ(5,1010)NBPD,NBAC,NBXX,NBYY  

      1010 FORMAT(8I5)  

      WRITE(6,2010)NBPD,NBAC,NBXX,NBYY  

      2010 FORMAT(11,0,16,0,16,  

      C   READ(5,1040)ACST,AC,PCBK  

      1040 FORMAT(16F10.0)  

      WRITE(6,2030)ACST,AC1C,PCBK  

      2030 FORMAT(11,0,ACST,F10.2,  

      C   NBAC1=NBAC+1

```

```

***** *****
C PROGRAM ACC.TD.INT
C ***** *****
C THIS PROGRAM FINDS THE MODIFIED MERCALLI INTENSITIES CORRESPONDING
C TO A CDF ON ACCELERATION USING THE GUTHENBERG-RICHTER RELATIONSHIP
C A MONTE-CARLO PROCESS IS USED. CDF PIA.GT.A01 IS INPUT.
C ***** *****
C INPUT FORMAT
C ***** *****
C COL VARIABLE NAME VARIABLE DESCRIPTION
C 1.- GENERAL 1 CARD (315)
C   1- 5 NB (NUMBER OF POINTS IN CDF AS READ IN)*2
C   6-10 TY RANDOM GENERATOR INITIATOR
C   11-15 NRPT NB OF CDF TO BE READ IN
C 2.- ACCELEPATIONS 8 VALUES PER CARD (8F10.0)
C   1-10 X(4) SMALLST ACC IN CDF
C   ... X(1) LARGEST ACC IN CDF
C   ... X(NB+2) AN ACC EQUAL OR LARGER THAN X(NB+2)*
C   ... X(NB+4) MAXIMUM POSSIBLE ACC TO BE USED IN CASE
C           INPUT CDF DOES NOT GO UP TO 100K
C
C THF REMAINING DATA IS READ FROM DISK
C 3.- NODE IDENTIFICATION 1 CARD IMAGE (20A4)
C   1 80 HED1 IDENTIFICATION
C
C 4.- CDF PIA.GT.A01 8 VALUES PER CARD IMAGE (8F10.0)
C   1-10 X(3) P(A.GT.X(4))
C   ... X(1) P(A.GT.X(NB+2))
C   ... X(NB+1) P(A.GT.X(NB+2))
C
C OUTPUT
C ***** *****
C THE OUTPUT DISPLAYS THE INPUT. THE NUMBER OF OCCURRENCES OF
C THE MODIFIED MERCALLI INTENSITY IN THE SIMULATION AND THE
C CORRESPONDING PERCENTAGE.
C ***** *****
C DIMENSION XI(44),INTEN(15),HED1(20)
C COMMON/PDT/Y,DELPNT
C ***** *****
C READ(5,1000)NB,1Y,NBPT
C
C NB2=NB+4
C NR1=NR+1
C XI(1)=0.
C XI(2)=0.
C
C 1000 FORMAT(315)
C READ(5,1100)(XI(I),I=4,NB2+2)
C
C 1103 FORMAT(RFD,0)
C XI(NB+3)=100.
C CALL RAND(Y,VFL,0)
C VFL=-.01
C
C RFA(5,1100)(XI(I),I=4,NB2+2)
C WRITE(6,1010) HED1
C D 400 IP=,NBPT
C ***** *****

```

```

1 264 LAST PERCENTAGE INPUT IS NOT 100.01 //1
110 IF(IICK-1) 200,300,120
200 ANS = X1(2*I-1)
RETURN

C LINEAR FITTING HALF DISTANCE BETWEEN INPUT POINTS
120 IFLT=EQK1N100 TO 140
121 IFLT=LT_31 GO TO 130
X=(L(X1(2*I-1))+L(X1(2*I-2)))*.5
Y=X1(2*I-1)
XR=(X1(2*I-2)+X1(2*I-3))*5
YA=X1(2*I-4)
GR TN 150
GO TO 130

C 130 XT=(X1(2*I-1)+X1(2*I-2))*5
YT=X1(2*I-1)
XR=X1(2*I-2)
YA=0.
GR TN 150
140 XT=X1(2*I-1)
YT=100.
160 XR=(X1(2*I-4)+X1(2*I-3))*5
YT=X1(2*I-4)
YB=X1(2*I-3)
GR TN 150

C LINEAR FIT THROUGH INPUT POINTS
300 XT=X1(2*I-1)
YT=X1(2*I-1)
XR=X1(2*I-2)
YA=X1(2*I-3)
C CHECK FOR VERTICAL INTERPOLATION
150 ANS=XT
IF((XT-X81.LT..00001) RETURN
C A=(YT-YA)/(XT-XB)
ANS=(YFL+100.-(YT-A*XT))/A
RETURN
SUBROUTINE RAND (IV,YFL,INDEX)
C THIS SUBROUTINE GENERATES RANDOM NUMBERS
C IV=Y*314159269+453806245
6 IF(IV.GE.0) GO TO 6
5 IV=IV+247483647+1
6 CONTINUE
7 IF(INDEX.GT.0) GO TO 8
YFL=FLD((IV*.4656613E-9
8 RETURN
END
$DATA

```

```

***** STANFORD UNIVERSITY *****
PROGRAM PLOT.EPI ***** STANFORD UNIVERSITY *****
THIS PROGRAM PLOTS THE EARTHQUAKE EPICENTERS ON A GRID (MAP),
GIVEN THE LATITUDE AND LONGITUDE. DIFFERENT SYMBOLS ARE
USED FOR THE DIFFERENT MAGNITUDES.
THE PLOT WILL BE ROTATED 90 DEGREES COUNTERCLOCKWISE IF IT IS TOO
HIGH TO FIT ON THE PLOTTER. IF BOTH DIMENSIONS ARE TOO BIG, AN
MESSAGE WILL BE GENERATED AND THE PLOT ABORTED.
***** INPUT FORMAT *****

COL VARIABLE NAME VARIABLE DESCRIPTION
1.- IDENTIFICATION CARD 1 CARD (80A1)
1-80 HED RUN IDENTIFICATION

2.- PLOT FLAGS 1 CARD (715)
1- 5 NBPL NR OF PLOTS
6-10 SKIP3 20, WILL PLOT EQ OF MAG 3
           31, WILL NOT PLOT EQ OF MAG 3
11-15 SKIP4 EQ OF MAG 4
16-20 SKIP5 EQ OF MAG 5
21-25 SKIP6 EQ OF MAG 6
26-30 SKIP7 EQ OF MAG 7
31-35 SKIP8 EQ OF MAG 8

3.- GRID PARAMETERS 1 CARD (16F10.0)
XWIN] THE ORIGIN OF THE GRID IS DEFINED AS
YWIN] THE BOTTOM LEFT CORNER; THE END AS THE
XAXI] TOP RIGHT. THE VERTICAL HAS TWO POINT
YMAX] NORTH FOR THE DATA TO BE READ IN
INDGX] CONSISTENTLY.
X COORD OF ORIGIN
Y COORD OF ORIGIN
X COORD OF END
Y COORD OF END
LENGTH OF ONE DEGREE IN INCHES FOR
THE PLOT (X DIRECTION).
LENGTH OF ONE DEGREE IN INCHES FOR
THE PLOT (Y DIRECTION).
1-10 INDGY
11-20 CAL THE PLOTTER TO BE USED (10, OR 29.)
21-30 XAXI IF ZERO, WILL BE SET TO 10.
31-40 YMAX] THE FRAME COORDINATES ARE ROUNDED OFF
41-50 INDGX TO THE CLOSEST DEGREE(DIV1).
51-60
61-70 DIVR IF INPUT AS 0, WILL RF SFT TO 1
71-80

3.- EPICENTER COORDINATES 1 EPICENTER PER CARD IMAGE (3F10.0)
X( ) INFORMATION READ FROM DISK
Y( ) X COORDINATE (LONGITUDE) OF EPICENTER
Z( ) Y COORDINATE (LATITUDE) OF EPICENTER
MAG( ) MAGNITUDE

OUTPUT *****
***** A FILE IS CREATED ON DISK, IT HAS TO BE TRANSFERRED ON TAPE AND
PLOTTED ON A 10 OR 29 INCH CALCOMP PLOTTER

```

```

YMAX=X=YMAX1
X=XMIN1
YMIN=YMIN1
C
READ 10,10201,NBRC,1HE0
1030 FORMAT(15.70A1)
C
IF(YRAS.LE.(CAL*1NPAS1)) GO TO 100
DO 120 IX=1,NBRC
120 QFAD10,1010IX,Y(IX),Y(IX),RM(IX)
1010 FNPW1(119,F5.3,3X,F5.3,42X,F3.2)
GO TO 130
C
100 CONTINUE
DO 140 IX=1,NBRC
140 READ 10,1D10Y(IX),X(IX),RM(IX)
C
CHECK SIGNS
130 IF(XMAX1.GT.XMM1) GO TO 150
C
XMAX=-XMAX1
XMIN=-XMIN1
DO 160 IX=1,NBRC
160 X(IX)=X(IX)
C
150 XFM=X=XMAX+DELTAX*.25/.75
XFMTN=XMIN-DELTAX
XF(YMAX1.GT.YMM1) GO TO 170
YMAX=X-YMAX1
YMIN=-YMIN1
C
DC 180 IX=1,NBRC
180 Y(IX)=-Y(IX)
170 YFM=X=YMAX+DELTAY
YMIN=YMM-DELTAY
552 WRITE(6,551)(X(IX)*Y(IX),RM(IX),IX=1,NBRC)
551 FORMAT(' ',12F10.3)
C
AMODES(19)=XMAS
AMODES(119)=YMAS
CALL SUREG(AMODES,XFMIN,YFMAX,XFMAS,YMAS)
CALL OBJEC(AMODES,0.,0.,XMAS,YMAS)
C
IF(AMODES(112).NE.AMODES(13))WRITE(6,2102)AMODES(112),AMODES(13)
2102 FORMAT('THE SCALES ARE NOT THE SAME,2E10.6')
FND1(1)=IFIX(YMIN)
FND2(1)=IFIX(XMAX)
END(12)=IFIX(YMAX)
END(22)=IFIX(XMIN)
C
DELTAY=1./18.*TNDGX
XMSL=4.
DELTA1=4.*DELTAY
DELTAY3=3.*DELTAY
SIGN=-1.
C
I=1
VAR=END1(12)
FIX=FND2(12)
C
DO 400 K=1,2
C
401 XX(I)=FIX
YY(I)=VAR
XX(I+1)=FIX+DELTAY
YY(I+1)=VAP
XX(I+2)=FIX
YY(I+2)=VAR
INTG=ABS(VAR)
XC=FIX-DELTAY*XMSL
YC=VAP
C
CALL INUMBG(AMODES,XC,YC,3,INTG)
WRITE(6,550)XC,YC,INTG
I=I+3
IF(VAR.EQ.END1(K)) GO TO 402
VAR=VAR*SIGN
GO TO 401
C
402 STOR=VAR
VAR=FIX
FIX=STOR
404 XX(I)=VAR
YY(I)=FIX
XX(I+1)=VAR
YY(I+1)=FIX+DELTAY
XX(I+2)=VAR
YY(I+2)=FIX
INTG=ABS(VAR)
XC=VAR
YC=FIX-DELTAY
C
CALL INUMBG(AMODES,XC,YC,3,INTG)
WRITE(6,550)XC,YC,INTG
I=I+3
IF(VAR.EQ.END2(K)) GO TO 403
VAR=VAR*SIGN
GO TO 404
C
403 SIGN=-SIGN
STOR=VAR
VAR=FIX
FIX=STOR
DELTAY2=-DELTAY
XMSL=2.
DELA3=-DELTAY
C
400 CONTINUE
C
DO 500 K=1,1
C
500 WRITE(6,550)XX(K),YY(K)
T=-3
FORMAT(' ',2E0.3,'16')
CALL LINESG(AMODES,I,XX,YY)
C
I3=0
I4=0
I5=0
I6=0
I7=0
I8=0
DO 300 IX=1,NBRC
IRH=RHM(IX)-1.99

```

```

IF(IRM.LT.1) GO TO 300
GO TO (230,240,250,260,270,280),IRM
230 I3=I3+1
XX3(I3)=X(I1X)
YY3(I3)=Y(I1X)
GO TO 300
240 I4=I4+1
XX4(I4)=X(I1X)
YY4(I4)=Y(I1X)
GO TO 300
250 I5=I5+1
XX5(I5)=X(I1X)
YY5(I5)=Y(I1X)
GO TO 300
260 I6=I6+1
XX6(I6)=X(I1X)
YY6(I6)=Y(I1X)
GO TO 300
270 I7=I7+1
XX7(I7)=X(I1X)
YY7(I7)=Y(I1X)
GO TO 300
280 I8=I8+1
XX8(I8)=X(I1X)
YY8(I8)=Y(I1X)
300 CONTINUE
AMODES(40)=8.*INRAS/100.
AMODES(41)=14.*INPAS/100.
C AMODES(45)=.5
AMODES(84)=12.
IF(SKIP3.EQ.0.AND.I3.GT.0) CALL POINTGAMODES,I3,XX3,YY3
IF(SKIP3.EQ.0.AND.I3.GT.0) PRINT,I3,I3 HAS BEEN CALLED
IF(SKIP4.EQ.0.AND.I4.GT.0) CALL POINTGAMODES,I4,XX4,YY4
IF(SKIP4.EQ.0.AND.I4.GT.0) PRINT,I4,I4 HAS BEEN CALLED
AMODES(84)=15.
IF(SKIP5.EQ.0.AND.I5.GT.0)CALL POINTGAMODES,I5,XX5,YY5
IF(SKIP5.EQ.0.AND.I5.GT.0)PRINT,I5 HAS BEEN CALLED
C AMODES(45)=.35
AMODES(84)=40.
IF(SKIP6.EQ.0.AND.I6.GT.0)CALL POINTGAMODES,I6,XX6,YY6
IF(SKIP6.EQ.0.AND.I6.GT.0) PRINT,I6 HAS BEEN CALLED
C AMODES(45)=2.0
AMODES(84)=12.
IF(SKIP7.EQ.0.AND.I7.GT.0)CALL POINTGAMODES,I7,XX7,YY7
IF(SKIP7.EQ.0.AND.I7.GT.0) PRINT,I7 HAS BEEN CALLED
C AMODES(45)=.65
AMODES(84)=40.
IF(SKIP8.EQ.0.AND.I8.GT.0)CALL POINTGAMODES,I8,XX8,YY8
IF(SKIP8.EQ.0.AND.I8.GT.0) PRINT,I8 HAS BEEN CALLED
C XT=XMAX-.5/INDGX
CALL VECSGAMODES,XT,YT,MAGNITUDE 4+0
C XT=XT+DELTAI
CALL VECSGAMODES,XT,YT,MAGNITUDE 5+0
C XT=XT+DELTAI
AMODES(45)=.35
CALL VECSGAMODES,XT,YT,MAGNITUDE 6+0
C XT=XT+DELTAI
AMODES(45)=1.0
CALL VECSGAMODES,XT,YT,MAGNITUDE 7+0
C XT=XT+DELTAI
AMODES(45)=.55
CALL VECSGAMODES,XT,YT,MAGNITUDE 8+0
C XT=XT+DELTAI
AMODES(45)=.5
CALL VECSGAMODES,XT,YT,MAGNITUDE 9+0
C XT=XT+DELTAI
CALL PICTRGAMODES
PRINT,"MAGNITUDE 3"
IF(I3.NE.DIMRITE(6,555)(XX3(I1),YY3(I1),I=1,I3)
PRINT,"MAGNITUDE 4 "
IF(I4.NE.DIMRITE(6,555)(XX4(I1),YY4(I1),I=1,I4)
PRINT,"MAGNITUDE 5 "
IF(I5.NE.DIMRITE(6,555)(XX5(I1),YY5(I1),I=1,I5)
PRINT,"MAGNITUDE 6 "
IF(I6.NE.DIMRITE(6,555)(XX6(I1),YY6(I1),I=1,I6)
PRINT,"MAGNITUDE 7 "
IF(I7.NE.DIMRITE(6,555)(XX7(I1),YY7(I1),I=1,I7)
PRINT,"MAGNITUDE 8 "
IF(I8.NE.DIMRITE(6,555)(XX8(I1),YY8(I1),I=1,I8)
500 CONTINUE
555 FORMAT(' ',12F10.3)
CALL EXITGAMODES
RETURN
END
$DATA
```

```

***** STANFORD UNIVERSITY *****
C PROGRAM BY SAC.C.DUR
C THIS PROGRAM COMPUTES THE PROBABILITY OF EXCEDANCE OF GIVEN PEAK
C GROUND ACCELERATIONS AND STRONG MOTION DURATION AT SPECIFIED
C SITES DUE TO LINE OR AREA SEISMIC SOURCES.
C CDF OF PGA IS COMPUTED WITH INCREMENTS OF .02 G
C CDF OF DURATION IS COMPUTED WITH INCREMENTS OF 2 SEC
C INPUT FORMAT
C ****
C 1.- IDENTIFICATION CARD          1 CARD   20A4
C 1-80  HED1  TITLE
C 2.- ATTENUATION CONSTANTS        1 CARD   3F10.0
C
C 1-10  B1
C 1-20  B2
C 21-30 B3
C 31-40 B4
C 41-50 DELTA1
C 51-60 DELTAC
C
C STEP SIZE FOR LINE INTEGRATION
C STEP SIZE FOR CIRCLE INTEGRATION
C UNITS OF LENGTH USED IN THE PROGRAM
C ATTENUATION FORMULA OF THE TYPE
C ACC-B1*EXP(B2*RAG)/(R+B4)*B3
C RATIO OF UPPER BOUND TO MEAN OF UNIFORM
C DISTRIBUTION PLACED ON PGA ATTENUATION
C RELATIONSHIP
C RATIO OF UPPER BOUND TO MEAN OF UNIFORM
C DISTRIBUTION PLACED ON DURATION ATTENUATION
C RELATIONSHIP
C
C 3.- PROBLEM DESCRIPTION          3 CARDS
C
C CARD 1  615
C 1-5    NL      NUMBER OF LINE SOURCES
C 6-10   NA      NUMBER OF AREA SOURCES
C 11-15  NT      NUMBER OF TIME PERIODS
C 16-20  NY      NUMBER OF ACC
C 21-25  NBGD    NUMBER OF GRIDS
C 26-30  SKSAVE  IF 0 WILL SAVE RESULTS ON DEVICE SPECIFIED
C                 BY JCL FOR DEVICES 11,12 ETC. FOR FIRST,
C                 SECOND, ETC. TIME PERIODS.
C                 IF 1 WILL NOT SAVE RESULTS
C
C CARD 2  TIME PERIODS, NT VALUES, 8 VALUES PER CARD  8F10.0
C 1-10  T(1)  PERIOD 1
C 11-20 T(2)  PERIOD 2
C .....
C T(N)  PERIODS NT
C
C 4.- LINE SOURCES PROPERTIES      AT LEAST 3 CARDS PER SOURCE
C
C CARD 1  HED2  IDENTIFICATION OF THE LINE SOURCE
C 1-80
C CARD 2  7F10.0,110 GEOMETRIC DESCRIPTION AND PARAMETERS OF POISSON MODEL

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C 1-10  XXOR1  X COORD OF ORIGIN
C 11-20  XXED1  X COORD OF END
C 21-30  YYOR1  Y COORD OF ORIGIN
C 31-40  YYED1  Y COORD OF END
C 41-50  DP1    DEPTH OF LINE
C 51-60  TDAL1  TIME DATA BASE
C 61-70  XNBD1  NO OF EVENTS RECORDED ABOVE M=4 ON THIS SOURCE
C 71-80  NB     NO OF DIFFERENT MAG GREATER OR EQUAL TO 4.0
C                         ON THIS SOURCE (.25 INCREMENT)
C
C CARD 3  8 VALUES PER CARD  8F10.0
C PARAMETERS OF BERNOULLI MODEL
C XNBV1  NO OF EVENTS BERNOULLI TRIAL IS BASED ON
C XNBMG1  NO OF SUCCESSES FOR M=4.0
C XNBMG2  NO OF SUCCESSES FOR M=4.25
C
C 1-10  XXOR1  X COORD OF CIRCLE CENTER
C 11-20  XXED1  Y COORD OF CIRCLE CENTER
C 21-30  YYOR1  RADIUS OF CIRCLE
C 31-40  YYED1  LEAVE BLANK
C 41-50  DP1    DEPTH OF CIRCLE
C 51-60  TDAL1  TIME DATA BASE
C 61-70  XNBD1  NO OF EVENTS RECORDED ABOVE M=4 ON THIS SOURCE
C 71-80  NB     NO OF DIFFERENT MAG GREATER OR EQUAL TO 4.0
C                         ON THIS SOURCE (.25 INCREMENT)
C
C CARD 4  20A4  AREA SOURCE IDENTIFICATION
C
C CARD 2  7F10.0,110 GEOMETRIC DESCRIPTION AND PARAMETERS OF POISSON MODEL
C 1-10  XXOR1  X COORD OF CIRCLE CENTER
C 11-20  XXED1  Y COORD OF CIRCLE CENTER
C 21-30  YYOR1  RADIUS OF CIRCLE
C 31-40  YYED1  LEAVE BLANK
C 41-50  DP1    DEPTH OF CIRCLE
C 51-60  TDAL1  TIME DATA BASE
C 61-70  XNBD1  NO OF EVENTS RECORDED ABOVE M=4 ON THIS SOURCE
C 71-80  NB     NO OF DIFFERENT MAG GREATER OR EQUAL TO 4.0
C                         ON THIS SOURCE (.25 INCREMENT)
C
C CARD 3  8 VALUES PER CARD  8F10.0
C PARAMETERS OF BERNOULLI MODEL
C XNBV1  NO OF EVENTS BERNOULLI TRIAL IS BASED ON
C XNBMG1  NO OF SUCCESSES FOR M=4.0
C XNBMG2  NO OF SUCCESSES FOR M=4.25
C
C 1-10  XXOR1  NO OF SUCCESSES FOR LARGEST M ON THIS SOURCE
C 11-20  XXED1  2 CARDS PER GRID
C 21-30  YYOR1
C 31-40  YYED1
C
C 6.- GRID DESCRIPTION
C
C CARD 1  IDENTIFICATION CARD  20A4
C 1-80  HED2  NAME OF GRID
C
C CARD 2  SEARCHING GRID DESCRIPTION  8F10.0
C 1-10  XBEGIN  ORIGIN OF GRID X COORD
C 11-20  YBEGIN  ORIGIN OF GRID Y COORD
C 21-30  XEND   END OF GRID X COORD
C 31-40  YEND   END OF GRID Y COORD
C
C CORNERS
C 41-50  DX    X INCREMENT
C 51-60  DY    Y INCREMENT
C
C IF ONLY ONE LOCATION IS REQUIRED, INPUT THE
C SITE COORDINATES AS XBEGIN AND XEND, AND
C INPUT YEND,DX,DY AS ZEROS
C
C

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      WRITE(6,104)HED2
      WRITE(6,120)
      2120 FORMAT(' ',7X,'XOR',7X,'OR',4X,'RADIUS',3X,'DEPTH',
     1 5X,'OCCURRENCE TIME',5X,'TIME BASE',5X,'NB OF OCC')
      WRITE(6,1027)XOR(L1),YOR(L1),XXD(L1),YYD(L1),TMD(L1),
     1 XNDA(L1)
      1027 FORMAT(' ',4F10.3,25X,2F10.3)
      WRITE(6,1031)XBTP(L1),XBDEV(L1)
      IXED=0
      DO 615 IXST=1,NB,10
      IXED=IXED+10
      IF(NB-IXST.LT.9)IXED=NB
      WRITE(6,1025)XNGST(L1),I=IXST,IXED)
      615 WRITE(6,1025)XNBMGLX(L1),I=IXST,IXED)
      123 PBLRC(I,J)=0.00
      C 551 CONTINUE
      C
      C COMPUTE DISTRIBUTION ON THE NUMBER OF EVENTS FOR EACH SOURCE
      C
      CALL INITIA
      DO 100 IASC=NBSCL,NBLN,TM(L1),TMDA(L1,XSC),XNBEV(L1,XSC))
      CALL BERNU(L1,XSC,NBLN,TM(L1),TMDA(L1,XSC),XNBEV(L1,XSC))
      100 CONTINUE
      C
      C ITERATION ON THE NUMBER OF GRIDS
      DO 365 IGD=1,NBGD
      WRITE(6,125)
      225 FORMAT('1',*****PROBABILITY DISTRIBUTION *****
     1 0 N O F P E A K G R O U N D A C C E L E R A T I O N *****
     2***,/,/)
      READ(5,104)HED2
      WRITE(6,104)HED2
      READ(5,100)YBEGIN,XEND,YEND,DY,DY
      IF(YEND.EQ.0.0)YEND=XBEGIN
      IF(YEND.EQ.0.0)YEND=YBEGIN
      IF(DY.EQ.0.0)DY=1.00
      IF(DY.EQ.0.0)DY=1.00
      NMMAX=DABSL(XBEGIN-XEND)/DY+1.900
      NMMAX=DABSL(XBEGIN-YEND)/DY+1.900
      WRITE(6,1021)NBLN,NBLN,NMAX,NMAX,NBTM
      1021 FORMAT('0',*GEOMETRIC CONSTANTS',//,*NL*',15,5X,*NA = ',15,5X,
     6*NMAX = ',15,5X,*NYMAX X = ',15,5X,*NT = ',15 / )
      C
      IF(1KSAVE.NE.0)GO TO 651
      DO 650 IT=L,NBTM
      IWT=10+2*(IT-1)
      IWT=IWT+1
      WRITE(L,104)HED1
      WRITE(L,104)HED1
      WRITE(L,104)HED1
      WRITE(L,104)HED1
      WRITE(L,104)HED1
      WRITE(L,104)HED1
      WRITE(L,104)HED1
      WRITE(L,104)HED1
      650 FORMAT('ACCELERATION DISTRIBUTION')
      5021 FORMAT('ACCELERATION DISTRIBUTION')
      5022 FORMAT('DURATION DISTRIBUTION')
      WRITE(L,104)TM(L1),XBEGIN,XEND,YBEGIN,YEND,DY,DY
      WRITE(L,104)TM(L1),XBEGIN,XEND,YBEGIN,YEND,DY,DY
      WRITE(L,104)TM(L1),XBEGIN,XEND,YBEGIN,YEND,DY,DY
      XB=*,F7.3,* XY=*,F5.2,* YZ=*,F5.2,*

      5020 FORMAT('TIME PERIOD',F6.2,' ',X=*,F7.3,* Y=*,F5.2,*,
     6 , YE=*,F7.3,* DX=*,F5.2,*,
     650 CONTINUE
     651 CONTINUE
      C
      C ITERATION ON GRID
      C
      C ITERATION IN THE Y DIRECTION
      Y=YBEGIN
      DO 385 IV=1,NYMAX
      C
      C ITERATION IN THE X DIRECTION
      X=XBEGIN
      DO 380 IX=1,NXMAX
      WRITE(6,1222) X,Y
      C
      C INITIALIZE ACCELERATION DISTRIBUTION
      ACDF(I1)=0.00
      DRDF(I1)=0.00
      DO 390 IXSC=1,NBSCL
      ACDF(I1)=1.00
      DO 391 IXSC=1,NB
      DRDF(I1)=1.00
      C
      C ITERATION ON LINE SOURCES
      IF(NBLN.EQ.0) GO TO 441
      C
      DO 351 IXSC=1,NBSCL
      CALL CONSTL(IXSC,X,Y)
      351 CONTINUE
      C
      C *** AREA SOURCE COMPUTATION
      441 IF(NBLN.EQ.0) GO TO 851
      SIGN=-1.D0
      I1=NBLN+1
      IEND=NBLN+NBAR
      DO 850 IXSC=I1,IEND
      C
      C CHECK WHETHER THE CIRCLE CONTRIBUTES TO SITE ACCELERATION
      RIA=XXD(IXSC)
      HLI=DP(IXSC)
      DIST=SQRT((X-XXOR(IXSC))*(X-XXOR(IXSC))+(Y-YFOR(IXSC))*
     1 (Y-YFOR(IXSC)))
      850 IF(IXSC.EQ.0) GO TO 851
      C
      C *** DETERMINE WHETHER CIRCLE WILL BE CONSIDERED
      RMM=3.75D0*XNBTM(IXSC)*.25D0
      RH=DIST-LIA
      IF(RH.LT.0.D0) RH=0.00
      RH=DSQR(TRH*RH*HLI*LIL)
      YGC=(B1*DEXP(B2*RMM))/((RH+B4)**B3)
      IF(YGC.LT.YGMCK) GO TO 850
      C
      NR=RIA/DELTAC*.5D0
      DR=DELTAC*.5D0
      C
      C ITERATION ON THE NB OF SEGMENTS IN THE CIRCLE
      DO 202 IR=1,NR
      DO 203 IS=1,L2
      DR1=SIGN*DR1
      DR12=(DIST+UR1)*(DIST+DR1)

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AL=IRIA*RIA-DR1*DRI1
TF(ALL,EQ.0.00) GO TO 203
AL=DSORTIAL
RHT2=DL1*2+HL1*HL1
C COMPUTE EPICENTRAL DISTANCE RECK AT WHICH AN EARTHQUAKE
C OF LARGEST MAGNITUDE RMXX WILL GENERATE AN ACCELERATION
C YG(I,I)
C IF RECK IS IMAGINARY OR SMALLER THAN DL1, THE SOURCE IS
C DISREGARDED. OTHERWISE SE PINTS WHERE THE CONTRIBUTION BECOMES
C ZERO ARE DETERMINED ON THE LINE. SEGMENTS BEYOND THESE POINTS
C ARE DISREGARDED.
TERM=BL1*EXP(1.0*B2*RMXX)/YGANKJ*(1.00/B3)
IF (TERM-B3)*LE.HL1J .GT. 0.0001 GO TO 203
TERM=(TERM-B3)*(TERM-B3)-4*HL1*HL1
C THE EPICENTRAL DISTANCE IS NOT IMAGINARY
C IF (TERM LE.DL1) GO TO 203
C THE SEGMENT WILL BE PARTIALLY OR TOTALLY CONSIDERED
C DETERMINE INTERSECTION BETWEEN RMXX AND THE SEGMENT
C AL1=D2*QT1*TERM-DL1*2
C AL1=D2*QT1*TERM-DL1*2
C AL1=DL1.LT.AL1.AL1=AL1
C DUMMY=0.DD
CALL INTGL(LXSCAL,DUMMY,RHT2)

C 203 CONTINUE
DRI=DBS1*DL1
DRI=DRI+DELTAC
C 202 CONTINUE
C
850 CONTINUE
851 CONTINUE
C *** COMPUTE CDF OF EXCEEDING AT LEAST ONE TIME
C N1=100
N2=101
DO 442 I=1,99
N1=N1-1
N2=N2-1
ACPDF(N1)=ACPDF(N1)*ACPDF(N2)
DRPDF(N1)=DRPDF(N1)*DRPDF(N2)
ACPDF(N2)=1.00-ACPDF(N2)
442 DRPDF(N<J)=1.00-DRPDF(N2)
ACPDF(IJ)=1.00-ACPDF(IJ)
DRPDF(IJ)=1.00-DRPDF(IJ)
WRITE(6,2501)
2501 FORMAT('ACCELERATIONS',)
IST=1
IED=10
XIC=.200
ACST=.0200
DO 400 IS=1,3
DO 410 IW=1,10
XIX=IW
ACUT(IW)=ACST*XIX*XIC
410 CONTINUE
WRITE(6,2500) ACUT
WRITE(6,2500) ACUT
WRITE(6,2500) ACUT
2300 FORMAT(' ',10FL2.5)
IST=IST+10

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C2010 FORMAT( ' ',15,2F14.5,3I10)
100 CONTINUE
C *** COMPUTE HANd & PROB OF UNIF DISTRIBUTION FOR DURATION
C
      XC=2.0D
      DO 110 IX=1,51
      XI=IX-1
      ACV=X*XC
      HIGH=ACV*UPDR
      IXACST=(1.50*HIGH)+1.51
      IXACST=IX+IX-IXACED
      NBRAV(IX,1)=IX
      NBRAV(IX,2)=IX*ACST
      NBRAV(IX,3)=IX*ACED
      XIXX=IXACED-IXACST+1
C
      PBORAV(IX)=1.0D/XIXX
      WRITE(6,2G10.1) X,ACV,PBORAV(IX),(NBRAV(IX,1),I=1,3)
C2010 FORMAT( ' ',15,2F14.5,3I10)
110 CONTINUE
      XXST=3.75U
      XXIC=2500
      DO 200 IX=1,20
      XI=IX
      XMG=XXST*(IX*XXIC)
      B1EB2M(IX)=61*DEXP((B2*XMG)
      WRITE(6,20) X,XMG,B1EB2M(IX)
      C2020 FORMAT( ' ',F10.2,E15.7)
200 CONTINUE
C
      IXACST=(50.0D*YGMN)+1.5DD
      DO 300 IX=2,30
      IXACMN=IX
      IF(INBACAV(IX,3).GT.IXACST) GO TO 310
300 CONTINUE
310 IXACMN=NBAV(IXACMN-1,1)
C
      RETURN
END
SUBROUTINE BERNU(IXSC,NBLN,TMRK,TMPR,XNPR,EVNB)
C
C***** ****
C
C IMPLICIT REAL*8 (A-H,P-Z)
COMMON/ATEN/BL,B2,B3,B6,DELTAC,YGMN,YGMNCK,JPAC,UPDR
COMMON/BRNU/ANBEV(30),ANBMG(30),ANBMG(30,201)
COMMON/ACVR/PBACAV(10),PBIEB2(120),PBZROC(30,20)
1,ACPDF(200),DRPDF(100),PBORAV(51),DR50(201)
COMMON/GEUM/YXUR(30),YFOR(30),XXED(30),YED(30),
1,TMDA(30),XNUA(30)
DIMENSION PBNG(5,200),PBMGTT(13)
LOGICAL*1 NUNE
C
C1001 FORMAT(5F10.0,2I5)
C INITIALIZE VARIABLES
DO 101 IX=1,1000
101 PBOC(IX)=0.0D
DO 102 IX=1,200
102 IX=1.3
C
102 PBMG(IX,IX)=0.0D
DO 103 IX=1,3
103 PBMGTT(IX)=0.0D
C *** SOURCE IXSC
C *** COMPUTE PROBABILITY OF 0,1,2...N OCCURRENCES OF ANY MAG
C *** ON THIS SOURCE. REPLACE TWO SMALL PROB BY ZERO
C *** XNPR=NB OF UCC. IN TIME TMDA-TMPR = DATA BASE + BAYES
C *** TMPK = RISK PERIOD CONSIDERED
C *** NBOC = NB OF UCC. INCLUDING ZERO. NBOC=6+5 EVENTS
C
C NORMALIZE RATE OF OCCURRENCE FOR DELTA IN LINE SOURCES
C
      T(IIXSC)=T(NBLN) GO TO 220
      XLEN=D4RT(IIXSC)*KAEJ(IIXSC)*(KGR(IIXSC)-XXED(IIXSC))
      XNPR=XNPR*XLEN/ALEN
      LLYOR(IIXSC)=YYUR(IIXSC)-YYED(IIXSC)
      GO TO 210
C
C *** NORMALIZE RATE OF OCCURRENCE FOR AREA SOURCES
      220 XNPR=XNPR*DELTAC*DELTA/(3.14159D0*XXED(IIXSC)*XXED(IIXSC))
C
C 210 TMTR=TMRK+TMPK
C
C *** PROB OF ZERO JCC. ON THE SOURCE
C *** P(0)=1.4PR/TMTR*XNPR
      PRB=XNPR*DLOG(TMTR/TMTR)
      IF(PRBLT>10.0D) GU TO 110
      PBUC(1)=DEXP(IPROB)
110 CONTINUE
      IXOC=0
      PBTT=PBUC(1)
      TMTR=1
      DO 120 IX=1,999
      XN=N
      XNTR=XN*XNPR
      XNTT=XN*XNPR
      XN=N
      NBDC=N-1
      NBDC=N-1
      C
      *** PROB OF 1,2,...N UCC. ON THE SOURCE
      C *** PROB(N)=G(N+N)/((N+1)*G(N))**T*N)*(T**N)/(T*T)**(N+N)
      PBCT=XNPR*DLOG(TMPR)-DLGAMA(XNPR)
      DLTR=DLOG(TMRK)
      DLTT=DLOG(TMTR)
      NONE=.TRUE.
      TMTR=1
      DO 120 IX=1,999
      XN=N
      NBDC=N-1
      NBDC=N-1
      C
      *** PROB=PCT+DLGAMA(XNTT)-DLGAMA(XN+1.0D)+(XN*DLTR)-(XNTT*DLTT)
      IF(PRBLT>10.0D) GO TO 115
      PBDC=NBDC=DEXP(IPROB)
      PBDC=PBCT+PBCD(NBDC)
      NONE=.FALSE.
      GO TO 120
      115 IF(.NOT.NUNE) GO TO 130
      1XST=1XST+1
      120 CONTINUE
      130 CONTINUE
      NBDC=NBDC-1
C
C *** INTRODUCE DATA ABOUT MAGNITUDE
C *** OBTAIN PROB OF 0,1,2,...N EVENTS FOR EACH OF THE NBMG MAGS
C *** XETA = TOTAL NUMBER OF EVENTS
C *** XXXI = NUMBER OF EVENTS OF EACH MAG.
      NBMG=KNBTP(IIXSC)+5
      XETA=ANDE(IIXSC)

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DO 200 IXMG=1,NBMC
XXI=XNMG*(XSC,XMG)
C ***
PBC=(ETA)/(IXSC)*(ETA-XII)
PBC=DGAMMA(XTAI)-DGAMMA(XXXI)-DLGAMA(XETA-XXXI)
C ***
PROB OF ZERO SUCCESS GIVEN N OCCUR.
C P(0/N)=EXP(PBC)*XI*(N+ETA-XI)*(N+ETA)
C PROB OF K SUCCESSES GIVEN N OCCUR.
C P(K/N)=CNR*EXP(PBC)*GIR(XI)*(N+ETA-R-XI)*(N+ETA)
NBDC=NSOL-1
DO 300 N=1,NST, NBDC
XTETT=0.0D0
XN=N
C ***
PROB OF ZERO SUCCESS
XNPR=XN*XA(XTAI-XXXI)
DEN=DGAMMA(XN*ETAJ)
PROB=PBC+DLGAMA(XXXI)+DLGAMA(XNPR)-DEN
IF(PROBLT=1.0) DO1 GU TO 300
PZR=ROC(XSC,XMG)=PBZROC(IXSC,IXMG)*DEXP(PR0BJ)*PB0C(N+1)
PBMG((IXN,1)=PBMG(IXMG,1))+UEXP(PR0B)*PB0C(N+1)
C
XTETT=DEXP(PR0B)
XTETT=XTETT*TEST
PBMG(IXMG)=PBMG(IXMG)+DEXP(PR0B)*PB0C(N+1)
GO TO 300
C
C *** DO LOOP ON THE NUMBER OF SUCCESSES
C ***
TERM=CNR
C *** PROB OF 1 SUCCESS GIVEN N OCCUR.
305 TERM=XN
NONE=.TRUE.
PROB=DLG(TERM)+PBCT+DLGAMMA(L+XXXI)+DLGAMA(XNPR-1)-DEN
IF(PROBLT=1.0) DO1 GU TU 312
PBMG((IXN,2)=PBMG(IXMG)+DEXP(PR0B)*PB0C(N+1)
PBMG(IXMG)=PBMG(IXMG)+DEXP(PR0B)*PB0C(N+1)
XTETT=DEXP(PR0B)
XTETT=XTETT*TEST
312 IF(N.EQ.LGU) TU 300
C ***
PROB OF 2,3,...N SUCCESSES GIVEN N OCCUR.
DO 310 IR=2,N
XI=N-IR+1
XIR=IR
TERM=TERM*XI/XIR
DLTER=DL0(ITERMB)
PROB=DLTEK+PCT+DLGAMMA(XR+XXXI)+DLGAMA(XNPR-XIR)-DEN
IF(PROBLT=1.0) DO1 GO TO 311
PBMG((IXN,IR)=PBMG(IXMG,IR)+DEXP(PR0B)*PB0C(N+1)
PBMG(IXMG)=PBMG(IXMG)+DEXP(PR0B)*PB0C(N+1)
XTETT=DEXP(PR0B)
XTETT=XTETT*TEST
NONE=.FALSE.
WRITE(6,20031)K,PBMG(IXMG,IR+1)
GO TO 310
311 IF(.NOT.NUNE) GU TO 300
310 CONTINUE
300 CONTINUE
C PBTT=0.0D0
DG 313 IX=1,NBUC
C PBTT=PBTT+PB0C(IXMG,IX)
C 313 WRITE(6,20031)X,PBMG(IXMG,IX)

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TERM3=AL1*AL1*AL1
TERM2=DSQRT(TERM1*TERM2*TERM1*TERM2*TERM3)
C
C COORDINATES OF THE POINTS OF INTERSECTION
C
X11=(-TERM1+TERM2)/TERM3
X12=(-TERM1-TERM2)/TERM3
Y11=AL1*X11+B1L
Y12=AL1*X12+B1L
IF(X11.LT.X12) GO TO 150
STOR=X11
X11=X12
X12=STOR
STOR=Y11
Y11=Y12
Y12=STOR
150 CONTINUE
C
C CHECK WHETHER THE POINTS OF INTERSECTION ARE ON THE SEGMENT
C CONSIDERED AND DETERMINE THE USE OF ALPHA AND BETA
C
IF(X11.LT.X11) GO TO 800
IF(X11.GT.X12) GO TO 830
IF(X12.GT.X12) GO TO 820
C
CASE=1
C *** SOURCE TOTALLY INCLUDED
GO TO 850
C 800 IF(XL2.LT.X11) GO TO 830
IF(XL2.LT.X12) GO TO 840
C
CASE=5
C *** SOURCE CUTOFF ON LEFT AND RIGHT ENDS
XL2=X12
YL2=Y12
GO TO 860
C 830 CASE=3
C *** SOURCE TOTALLY DISREGARDED
RETURN
C
820 CASE=2
C *** SOURCE CUTOFF ON THE RIGHT END
XL2=X12
YL2=Y12
GO TO 850
C
840 CASE=4
C *** SOURCE CUTOFF ON THE LEFT END
B60 XL1=X11
YL1=Y11
C 850 CONTINUE
C
C CHECK POSITION OF (XBAR,YBAR) ALONG LINE W.R.T. END
C OF POINTS OF LINE SOURCE
C AL1=0.5*(XL1-YBAR)*(YL1-YBAR)+(YL1-YBAR)*(YL2-YBAR)
C AL2=0.5*(XL2-XBAR)*(YL2-YBAR)+(YL2-XBAR)*(YL2-YBAR)
C XLEN=DSQRT((AL2-XL1)*(YL2-YL1)+(YL2-XL1)*(YL2-YL1))
C
IF(XBAR.GT.X11) AL1=AL1
C
CALL INTLN(CIL,XLEN,ALL,RHITZ)
C
RETURN
END
SUBROUTINE INTGLN(IASC,XLEN,AL,RHITZ)
C
C *****
C
IMPLICIT REAL*8(A-H,O-Z)
COMMON/ACN/PBACV(100),BLD2M(20),PROC(1000),PBZRDC(30,20)
1,ACPDFLUJ,DPDFUJ,BDRAV(100),DR50(120)
COMMON/ACN/IXACN,NBACAV(100,3),NBDRAV(150,3)
COMMON/ATEN/B1,82,83,84,DELTA,C,YGMCK,JPAIC,UPDR
COMMON/BRN/ABEV(100),XNBTP(30),XNBIG(30,20)
COMMON/BRN/ABEV(100),XNBTP(30),XNBIG(30,20)
DIMENSION ACUT(10)
C
C *****
C
NBAG=XNBTP(IASC)
NBIC=(XLEN/DELTAL)*500
IF(NBIC.EQ.0) GU TO 110
C
C START DO LOOP AT 1/2 DELTAL
AL=AL+500*DELTAL
C
DO 100 IXAC=1,NBIC
  RHR=D5RTR(RH)
  RH=(RHR*B4)*RHR*B4
  RHR=RHR*108.88D0
C
C DO LOOP ON ALL POSSIBLE MAGNITUDES
IXAC=NBRG+1
DO 200 IXAC=1,NBNG
  IXMG=IXMG-1
  AC=B1E82M1*IXMG/KH
  IXAC=(50.0*AC)+1*500
  PRINT',IXAC,IXMG',AC,IXAC,IXMG
  IF(IXAC.LT.IXACMN) GO TO 100
  IXACP8=NBACAV(IXAC,1)
  IXACST=NBACAV(IXAC,2)
  IXACED=NBACAV(IXAC,3)
200 CONTINUE
C
PBZR=PZRUC(IASC,IXMG)
PROB=1.0D-(PBACV(IACPB)*PBZR)
PRINT',300',IXACPV,IXACST,PROB,PBACV(IACPB),PBZRUC(IASC,IXMG)
DO 300 IXACVR=IXAC5,IXACD
  ACDF(IXACVR)=ACPDF(IXACVR)*PROB
300 CONTINUE
C
C *** DURATIONS USING THE SAME VARIABLES AS ACC
C
IF(RHR.GT.50.0) AC=DR50*IXMG/(.0011200*(RHR-50.0))
1. (RHR-50.0)*1.00
  IXAC=AC*.500 + 1.5100
  IXACPB=NBDRAV(IXAC,1)
  IXACST=NBDRAV(IXAC,2)
  IXACED=NBDRAV(IXAC,3)
C
PROB=1.0D-(PBACV(IACPB))*PROB
DO 310 IXACVR=IXAC5,IXACD
310 IXACVR=IXACST,IXACED
C

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```

DRPDF(IXACVRI)=DRPDF(IXACVRI)*PRUB
310 CONTINUE
C
200 CONTINUE
100 AL=AL+DELTAL
DO 500 IX=1,100
IF(ACPDF(IX).LT.1.0E-5) ACPDF(IX)=0.00
500 CONTINUE
C
110 CONTINUE
C
1ST=1
C
1ED=10
C
XIC=2.00
C
ACST=-2.00
C
DO 400 IS=1,3
C
DO 410 IX=1,10
C
XIX=IX
C
ACUT(IX)=ACST*XIX*XIC
C
410 CONTINUE
C
WRITE(6,1000) ACUT
C
WRITE(6,10000)(DRDF(I),I=1ST,IED)
C1000 FORMAT(1X,1.0F12.5)
C
IST=IST+10
C
IED=IED+10
C
ACST=ACUT(10)
C
400 CONTINUE
RETURN
END
SUBROUTINE INTGCL(IXSC,XLEN,AL,RHT)
C
*****+
C
IMPLICIT REAL*8(A-H,O-)
COMMON/ACR/PA(110),BLB2M(120),PB0C(1000),PBZROC(10,20)
L_1*ACPDF(200),URPDF(100),PBDRAV(51),DR50120,
COMMON/ACIN/IXACIN,NBACAV(10,3),NBDRAV(51,3),
COMMON/ATTEN/81,B3,B3,DELTA,IGNH,YGMNCK,JPAC,JPDR
COMMON/BNU/IXNBE(130),XNBT(30),XNBMG(130,20)
DIMENSION ACUT(10)
*****
C
NBMG=XNBT(1,IXSC)
NBIC=(XLEN/DELTAL)+500
IF(NBIC.EQ.0) GO TO 110
C
START DO LOOP AT 1/2 DELTAL
AL=AL+500*DELTAL
C
DO 110 IXIC=1,NBIC
RHT=DQRTR(KH)
RH=(RHT+B4)*(RHT+B4)
RHT=RHT*108.8800
C
DO LOOP ON ALL POSSIBLE MAGNITUDES
IXMG=NBIC+1
DO 200 IX=1,NBMG
IXMG=IXMG-1
C *** ACCELERATIONS
AC=B1E2M(IXMG)/RH
PRINT,'IXAC,IXMG',AC,IXAC,IXMG
IF(IXAC.LT.IXCMN) GO TO 100
IXACP(B)=NBACAV(IXAC,1)
IXACST=NBACAV(IXAC,2)
IXACFD=NBACAV(IXAC,3)
C
PBZR=PBZROC(IXSC,IXMG)
PROB=1.0D-IPBACAV(IXACPBJ)*PBZR
C
PRINT,' 300, IXACB, IXACST, PROB, PBACAV(IXACPBJ), PBZROC(IXSC,IXMG)*
DO 300 IXACVR=IXACST,IXACED
ACPDF(IXACVR)=ACPDF(IXACVR)*PROB*PROB
300 CONTINUE
C
*** DURATIONS USING THE SAME VARIABLES AS ACC
C
AC=DR501IXMG)+15D0*(50.00-RHT)
IF(RHT.GT.50.00) AC=DR501IXMG/(1.00112D0*(RHT-50.00)*
1
(RHT-50.00)+1.0D0)
IXAC=AC*500 + 1.54D0
IXACP(B)=NBDRAV(IXAC,1)
IXACST=NBDRAV(IXAC,2)
IXACED=NBDRAV(IXAC,3)
C
PROB=1.0D-(PBDRAV(IXACPBJ)*PBZR)
C
DO 310 IXACVR=IXACST,IXACED
DRPDF(IXACVR)=DRPDF(IXACVR)*PROB*PROB
310 CONTINUE
C
200 CONTINUE
100 AL=AL+DELTAL
C
DO 500 IX=1,100
IF(ACPDF(IX).LT.1.D-5) ACPDF(IX)=0.0D
IF(DRPDF(IX).LT.1.D-5) DRPDF(IX)=0.0D
500 CONTINUE
C
110 CONTINUE
C
IST=1
C
IED=10
C
XIC=2.00
ACST=-2.00
C
DO 400 IS=1,3
C
DO 410 IX=1,10
C
XIX=IX
ACUT(IX)=ACST*XIX*XIC
C
410 CONTINUE
C
FORMAT(1X,1.0F12.5)
C
IST=IST+10
C
ACUT=ACUT(10)
C
400 CONTINUE
RETURN
END
//GO.FT10F001 I0 UNIT=DISK,DCH=CARU,DSN=YL,EL=G35,AG300,
DISP=NEW,KEEPI,SPACE=13120,(10,5),RLSEJ,VOL=SER=PB0005
//GO.FT11F001 UD UNIT=DISK,DCH=CARD,DSN=YL,EL=G35,DG300,
DISP=(NEW,KEEP),SPACE=3120,(10,5),RLSEI,VOL=SER=PB0005
//GO.SYSIN DD *

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***** PROGRAM PLOT.ISO *****
***** STANFORD UNIVERSITY *****
***** THIS PROGRAM PLOTS THE ISO INTENSITY OR ISO ACCELERATION LINES OVER A GRID (MAP) GIVEN THE ACCELERATIONS AT EACH NODE OF THE GRID. THE ORIGIN OF THE GRID IS THE BOTTOM LEFT CORNER, THE END OF THE TOP RIGHT CORNER. THE VALUES HAVE TO BE INPUT FILLING COLUMNS FROM BOTTOM TO TOP AS ONE MOVES FROM LEFT TO RIGHT : FIRST VALUE INPUT IS THUS THE ORIGIN AND LAST THE END. THE PLOT WILL BE ROTATED 90 DEGREES COUNTERLOCWSE IF IT IS TOO HIGH TO FIT ON THE PLOTTER. IF BOTH DIMENSIONS ARE TOO BIG, AN MESSAGE WILL BE GENERATED AND THE PLOT ABORTED.

COL VARIABLE NAME          VARIABLE DESCRIPTION
1- IDENTIFICATION CARD   1 CARD (80AL)
1-80 HED                  RUN IDENTIFICATION

2- PLOT FLAGS             1 CARD (1515,6F10.0)
1- 5 SKIPAC               =0, WILL PLOT ISO ACCELERATION LINES
                           *1, WILL PLOT ISO INTENSITY LINES
                           NB OF FAILURES TO BE PLOTTED
                           NB OF PLOTS
                           NB OF ROWS IN INPUT
                           NB OF COLUMNS IN INPUT
                           DISTANCE (DEGREE) BETWEEN TWO COLUMNS
                           DISTANCE (DEGREE) BETWEEN TWO ROWS
                           INTERVAL BETWEEN TWO ISO LINES
                           MULTIPLE OF CONTOUR TO BE LABELED (1.2
                           FORMAT) FOR EXAMPLE IF XMDC=2*, EVERY OTHER DC WILL BE LABELED

3.- GRID PARAMETERS        1 CARD (10F0.0)
                           THE ORIGIN OF THE GRID IS DEFINED AS THE BOTTOM LEFT CORNER; THE END AS THE TOP RIGHT. THE VERTICAL HAS TO POINT NORTH FOR THE DATA TO BE READ IN CONSISTENTLY.
                           X COORD OF ORIGIN
                           Y COORD OF ORIGIN
                           X COORD OF END
                           Y COORD OF END
                           LENGTH OF ONE DEGREE IN INCHES FOR THE PLOT (X DIRECTION).
                           LENGTH OF ONE DEGREE IN INCHES FOR THE PLOT (Y DIRECTION)
                           PLOTTER TO BE USED (11, OR 33+)
                           IF ZERO, WILL BE SET TO 11.
                           THE FRAME COORDINATES ARE ROUNDED OFF TO THE CLOSEST (DEGREE/DIV)
                           IF INPUT AS 0, WILL BE SET TO 1

3.- FAULT COORDINATES      NF CARDS (6F10.0) NO INPUT IF NF=0
                           1-10 YFI1 ) Y COORD OF ORIGIN OF FAULT 1
                           11-20 XF11 ) X COORD OF ORIGIN OF FAULT 1
                           21-30 YF21 ) Y COORD OF END OF FAULT 1

***** 4.- ACCELERATION AT THE NUDGES 8 VALUES PER CARD (8F10.0)
***** 1-10 ACC(1,IC) ACC AT TOP OF FIRST COLUMN (IC=1)
*****    ACC(1R,IC) ACC AT TOP OF FIRST COLUMN (IC=1)
*****    ACC(INR,IC) REPEAT NC TIMES

***** OUTPUT
***** *****
***** COMMON /PLT/ CONTX(100),CONY(100),DX,DY,ERROR,OFFJLY
***** DIMENSION AMODESS(200),END(12),END(22),XX(1000),YY(1000),A(1600)
***** 1,A(14,40),XF1(120),XF2(120),YF1(120),YF2(120),MED1(20)
***** INTEGER** NR,NC,ROTA,SKIPAC
***** REAL RASIN,INDG,INDXA,INDGY
***** LOGICAL** MED2(80)
***** EQUIVALENCE(A(1),AA(1,1))
***** *****
***** READ(5,1001)HED1
***** 1001 FORMAT(10A4)
***** WRITE(6,2202)HED1
***** 2202 FORMAT(1,*20A4)
***** READ(5,1003)SKIPAC,NF,NBPL,NR,NC,DX,DY,DC,XMDC
***** READ(5,1000)XM IN YMIN,XMAX,YMAX,INDGX,INDGY,CAL,DIVR
***** IF(DIVR.EQ.0.0) DIVR=1.
***** IF(DIVR.EQ.0,J CAL=11.
***** PRINT XMIN,YMIN,XMAX,YMAX,INDGX,INDGY,CAL,DIVR
***** PRINT,NBPL,NR,NC,DX,DY,DC
***** 1000 FORMAT(8F10.0)
***** 1003 FORMAT(8I3,8F10.0)
***** RASIN=100.
***** ICAL=1
***** IF(CAL.EQ.33.) RASIN=200.
***** IF(CAL.EQ.33.) ICAL=2

***** DIVR IS USED TO TO BE ABLE TO PLOT PARTS OF DEGREES, USUALLY DEGREE/10
***** XMIN=IFIX(XMIN*DIVR+.5)
***** YMIN=IFIX(YMIN*DIVR+.5)
***** XMAX=IFIX(XMAX*DIVR+.5)
***** YMAX=IFIX(YMAX*DIVR+.5)
***** INDGX=(INDGX/DIVR
***** INDGY=(INDGY/DIVR
***** DX=DX/DIVR
***** DY=DY/DIVR
***** XRAS=(BS1(XMAX-XMIN)*(INDGX+.5)*RASIN
***** YRAS=(BS1(YMAX-YMIN)*(INDGY+.5)*RASIN
***** ROTA=0

***** ORIENTE FRAME
***** DELTAX=.75/INDGX
***** DELTAY=.75/INDGY
***** IF(YRAS.LE.(CAL*RASIN)) GO TO 100
***** IF(XRAS.LE.(CAL*RASIN)) GO TO 110
***** WRITE(6,2100)

***** 
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2100 FORMAT('THE SCALE IS TOO LARGE FOR THE PLOTTER, REDUCE INU, PER U
DEGREE,')
STOP
C
C INTERCHANGE X AND Y SCALES
C
110 WRITE(6,201)
2101 FORMAT('THE PLOT WILL BE ROTATED BY 90 DEGREES COUNTERCLOCKWISE')
ROTATE=1
STORE=DX
DX=DY
DY=STORE
STORE=DELTA_X
DELTA_X=DELTA_Y
DELTA_Y=DELTA_X
STORE=STOR
STORE_XMIN
XMIN=XMAX
YMAX=XMAX
XMAX=XMIN
YMIN=STOR
STORE_YRAS
YRAS=YRAS+3.5*RASIN
YRAS=STOR
IF(NF.EQ.0) GO TO 144
C
READ FAULTS
READ(5,1007)(YF1(I),XF1(I),YF2(I),XF2(I),I=1,NF)
1007 FORMAT(4F10.0)
C
GO TO 130
C
100 XRAS=XRAS+3.5*RASIN
C
1F(NF.EQ.0) GO TO 144
READ(5,1007)(XF1(I),YF1(I),XF2(I),YF2(I),I=1,NF)
C
130 DO 143 I=1,NF
XF1(I)=XF1(I)*DIVR
YF1(I)=YF1(I)*DIVR
XF2(I)=XF2(I)*DIVR
YF2(I)=YF2(I)*DIVR
143 CONTINUE
144 CONTINUE
C
CHECK SIGNS
I(XMAX.GT.XMIN) GO TO 150
XMIN=-XMIN
XMAX=XMIN
IF(NF.EQ.0) GO TO 150
DO 145 I=1,NF
XF1(I)--XF1(I)
XF2(I)--XF2(I)
145 CONTINUE
C
150 XMAX=XMAX+DELTA_X*4.25/.75
XMIN=XMIN+DELTA_X
IF(YMAX.GT.YMIN) GO TO 170
YMAX=-YMAX
YMIN=-YMIN
IF(NF.EQ.0) GO TO 170
DO 146 I=1,NF
YF1(I)=-YF1(I)
YF2(I)=-YF2(I)
146 CONTINUE
C
170 YMAX=YMAX+DELTA_Y
YMIN=YMIN+DELTA_Y
C
C FIND INTERSECTION BETWEEN FAULTS AND FRAME
IF(NF.NE.0)CALL FRAME(INF,YMIN,XMIN,YMAX,XF1,YF1,XF2,YF2)
C
CALL STARTGAMODES,ICAL,I,LUSBLK,LUSBLK,'LUSBLK'
AMODES(97)=XRAS
AMODES(1179)=YRAS
IF(CAL.EQ.33) AMODES(45)=AMODES(45)*1.5
CALL OBJECTGAMODES,O,O,O,XRAS,YRAS
IF(ROTA.EQ.0) GO TO 610
NSTOR=NC
NC=NR
NR=NSTOR
610 CONTINUE
C
DO 600 I=1,NBPL
C
CALL READ(AA,HED2,NR,NC,RUTA)
C
FIND LOWER AND UPPER CONTOUR VALUES
NBVL=NR*NC
CALL LIMITSIA,NBVL,SKIPAC,CHIN,DC,CHMAX
C
AMODES(50)=0.
AMODES(46)=0.
CALL SUBJEGAMODES,XMIN,YMIN,XFMAX,YFMAX)
C
WRITE(6,202)AMODES(12),AMODES(13)
2102 FORMAT('THE SCALES ARE ',Z16.5)
C
PLOT GRID
END(1)=FIX(YMIN)
END(11)=FIX(XMAX)
END(12)=FIX(YMAX)
END(22)=FIX(XMIN)
C
DELTA2=1./(.8.*INDGX)
XMUL=.4.
DELTA1=.4.*DELTA2
DELTA3=.3.*DELTA2
DELTA4=.15*AMODES(40)*AMODES(45)/(RASIN*INDGX)
DELTA5=.2.*DELTA4
SIGN=-1.
C
I=1
VAR=END1(2)
FIX=END2(2)
C
DO 400 K=1,2
C
401 XX(I)=FIX
YY(I)=VAR
XX(I+1)=IX+DELTAX
YY(I+1)=VAR
XX(I+2)=IX
YY(I+2)=VAR
XVAR=ABS(YAK/D)IVR
INTG=XVAR+.01

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XC=FIX-DELTAZ*Xmul
YC=VAR
C
IF(IFIX(XVAR+.91).EQ.INTG)CALL INUMBG(AMODES,XC,YC,2,INTG)
I=I+3
IF(VAR.EQ.END1(k)) GO TO 402
VAR=VAR*SIGN
GO TO 401
C
402 STOR=VAR
VAR=FIX
FIX=STOR
404 XX(I)=VAR
YY(I)=FIX
XX(I+1)=VAR
YY(I+1)=FIX*DELTAX
XX(I+2)=VAR
YY(I+2)=FIX
XVAR=ABS(VAR/DIVR)
INTG=XVAR+.01
XC=VAR-DELTAX
YC=FIX-DELTAX
C
IF(IFIX(XVAR+.91).EQ.INTG)CALL INUMBG(AMODES,XC,YC,3,INTG)
I=I+3
IF(VAR.EQ.END2(k)) GO TO 403
VAR=VAR*SIGN
GO TO 404
C
403 SIGN=-SIGN
STOR=VAR
VAR=FIX
FIX=STOR
DELTAX2=-DELTAX
XMUL=2.
DELTAX3=-DELTAX
C
400 CONTINUE
DO 500 K=1,I
C 500 WRITE(6,550)XX(K),YY(K)
550 FORMAT(' ',2F0.3,' ',F10.3)
I=I-3
CALL LINESG(AMODES,I,XX,YY)
C PLOT TITLE
XT=XFMAX-3.*DELTAX
AMODES(50)=90.
AMODES(46)=90.
YT=PMINDELTAX
CALL VECSC(AMODES,XT,YT,+8,HE02)
C IF(NF.EQ.0) GO TO 142
C PLOT FAULTS
C PRINT, 'FAULTS AS THEY ARE PLOTTED'
WRITE(6,*2011)I,XF1(I),YF1(I),YF2(I),I=1,NF
2011 FORMAT(' ',15,F10.3)
CALL SEGNTG(AMODES,NF,XF1,YF1,XF2,YF2)
142 CONTINUE
C CHANGE SUBJECT SPACE TO PLOT CONTOURS, THE BOTTOM LEFT CORNER
C HAS TO HAVE (0.,0.) COORDINATES
XFMAX1=AMODES(XFMAX-XFMIN)-DELTAX
YMAX1=AMODES(YFMAX-YFMIN)-DELTAY
PRINT, *DELTAX,DELTAY,XFMAX,YFMAX
XFMIN1=DELTAX
YFMIN1=DELTAY
C PRINT, 'RASOR,XRAS,YRAS,YRAS,YRAS'
CALL SUBSEG(AMODES,XFMIN1,YFMIN1,XFMAX1,YFMAX1)
PRINT, *SCALE, AMODES(12),AMODES(13).
C CREATE CONTOURS AND PLOT THEM
AMODES(<5)=AMODES(<5)*.6
CALL CONTR(AMODES,SA,NC,DC,XMDC,DELTAS)
AMODES(<5)=AMODES(<5)/.6
C CALL PICTRG(AMODES)
600 CONTINUE
C CALL EXIT(AMODES)
RETURN
END
SUBROUTINE READAA(HE02,NROW,NCOL,NROT)
C *****
C ***** DIMENSION AA(NROW,NCOL)
C ***** DIMENSION AA(NROW,NCOL)
C ***** LOGICAL *1 HE02(80)
C *****
C THIS SUBROUTINE READS THE DATA TO BE INTERPOLATED
READ(5,1002)HE02
1002 FORMAT(80A1)
C IF(NROT.EQ.1) GO TO 110
C THE DATA IS STORED IN A TWO DIMENSIONAL ARRAY, A(1,1) BEING AT
C THE ORIGIN OF THE PLOT (BOTTOM LEFT CORNER)
C DO 100 IR=1,NROW
100 READ(5,1001)(AA(IR,IC),IC=1,NCOL)
GO TO 99
C THE PLOT HAS BEEN ROTATED BY 90 DEGREES COUNTERCLOCKWISE,
C THE ROWS AND COLUMNS ARE INVERTED IN THE CALL STATEMENT
C 110 CONTINUE
DO 200 I=1,NCOL
IC=NCOL-I+1
200 READ(5,1000)(AA(IR,IC),IR=1,NROW)
99 CONTINUE
C DO 300 IR=1,NROW
300 WRITE(6,1001)(AA(IR,IC),IC=1,NCOL)
1001 FORMAT(' ',13F10.3)
1000 FORMAT(5F10.5)
C RETURN
END
SUBROUTINE LIMITS1(A,NBV1,SKIPAL,CMIN,UC,CMAX)
C *****
C DIMENSION A(1)

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***** INTEGER SKIPAC *****
C
C SEARCH FOR LOW BOUND
C IF(SKIPAC.NE.1) GO TO 110
C FIND INTENSITIES USING RICHTER GUTENBERG RELATIONSHIP
C DO 99 IV =1,NBVL
A(IV)=3.*ALOG(GLOTA(IV)*1000.)1+.5)
99 CONTINUE
C
C 110 CONTINUE
XLOW=A(1)
DO 100 IV=2,NBVL
I(IFL(IV).LT.XLOW) XLOW=A(IV)
100 CONTINUE
C
C SEARCH FOR UPPER BOUND
XHIGH=A(1)
DO 210 IV=2,NBVL
I(IFL(IV).GT.XHIGH) XHIGH=A(IV)
200 CONTINUE
C
C FIND LOW AND HIGH CONTOUR LINES TO BE PLOTTED
CMIN=DC
300 CMIN=MIN+DC
I(FCMIN.LT.XLOW) GO TO 300
C
CMAX=CMIN-DC
400 CMAX=CHAX+DC
I(FCMAX.LT.XHIGH) GO TO 400
CMAX=CMAX-DC
C
PRINT *,XLOW,XHIGH,CMIN,CMAX,XLOW,XHIGH,CMIN,CMAX
C
C RETURN
END
SUBROUTINE FRAME(INF,XMIN,YMIN,XMAX,YMAX,XFL1,YFL1,XF2,YF2)
C
C ***** DIMENSION XF1(1),XF2(1),YF1(1),YF2(1) *****
C LIMIT THE FAULTS TO THE AREA UNDER CONSIDERATION
DO 500 IF=LNF
C
C CHECK FOR LOWER AND UPPER BORN
SIGN=L_
CHECK=XMIN
C
C CHECK FOR ORIGIN AND END OF SEGMENT
DO 100 K=1,2
XF=XFL1(IF)*SIGN
C
C CHANGE COORDINATES
XF1(IF)*SIGN*CHECK
YFL1(IF)=ASL*XFL1(IF)+BET
GO TO 200
300 XF2(IF)*SIGN*CHECK
YF2(IF)=ASL*XF2(IF)+BET
200 XF=XF2(IF)*SIGN
SIGN=-1.
100 CHECK=-XMAX
C
C CHECK THE Y COORDINATES OF THE FAULT
SIGN=1.
CHECK=YMIN
C
C CHECK FOR LOWER AND UPPER BORN
DO 110 K=1,2
YF=YFL1(IF)*SIGN
C
C CHECK FOR ORIGIN AND END OF SEGMENT
DO 210 I=1,2
IF(YF.GE.CHECK) GO TO 210
ASL=(YFL1(IF)-YF2(IF))/XFL1(IF)-XF2(IF)
BET=YFL1(IF)-ASL*XFL1(IF)
IF(I.EQ.2) GO TO 310
C
C CHANGE COORDINATES
YFL1(IF)=(YFL1(IF)-BET)/ASL
GO TO 210
310 YF2(IF)*SIGN*CHECK
XF2(IF)=YF2(IF)-BET/ASL
210 YF=YF2(IF)*SIGN
SIGN=-1.
110 CHECK=-YMAX
C
C 500 CONTINUE
RETURN
END
SUBROUTINE CONTR(ANODES,FXY,IROM,ICDM,ICDM,ICDM,DELTAS)
C
C *** PASS A SET OF MESH POINTS FXY(ICDM,ICDM) WITH INCREMENTS
C *** OF DX AND DY FOR AN AREA OF SEARCH IROM, ICDM WITH
C *** ORIGIN AT THE BOTTOM LEFT CORNER. FIND A SPECIFIC
C *** CONTOUR LEVEL CTLV
C
C DIMENSION FXY(IROM,ICDM),ANODES(200)
COMMON APLT/CONTX(100),CONTY(100),DX,DY,ERROR,DPJLY
COMMON NBPT,NBPTS,IROWCOL(100),IROW,NBPTT,NBSURF,
1 ISQROW(100),ISQCOL(100),IXSQBD(20),IXSQBD(20),
LOGICAL*I OPEN
C
C *** SEARCH *RID FOR SMALLEST AND LARGEST VALUE
C *** SET VALUES OF CONTOURS TO BE PLOTTED
PTLVAD=XHDC*CTIC
PTLVAD=PTLVAD
DSMN=(DX*DX+DY*DY)
OEPOLY=SQR(T(DSMN))*0.0001
XINC=SQR(T(DSMN))*0.05
DSMN=DSMN*.02
OFFSET=CTIC*.001
ERROR=CTIC*.001

```

```

IORDER=3
NBPT=6
CTLM=FX(1,1)
CTHG=FX(1,1)
DO 21 IR=1,IRDN
  DO 21 IC=1,ICDN
    CTCK=FX(IR,IC)
    IF(CTCK.LT.CTLW)CTLM=CTLK
    IF(CTCK.GT.CTHG)CTHO=CTLK
  21 CONTINUE

C *** DETERMINE FIRST CONTOUR TO BE LABELED
IF(XMDC.EQ.0.) GO TO 300
DO 310 IR=1,100
  IF(PTLV.EQ.CTLW) GO TO 301
  310 PTLV=PTLV+PTLVA
  301 CONTINUE

C
  IF(CTIC.GT.1.) GO TO 300
  PTLV=PTLV/100.
  PTLVA=PTLVA/100.
  300 CONTINUE

C *** DETERMINE NUMBER OF CONTOURS
NBCT=CTLM/CTIC
IF(NBCT.LT.1) NBC=NBC+1
CTLV=FLOAT(NBCT)*CTIC
NBHG=CTHG/CTIC
IF(NBHG.LT.0) NBHG=NBHG+1
NBCT=NBHG-NBCT+1
CTLV=.4
NBC=1
PRINT *,LM,HG,IC,NBCT*,CTLM,CTHG,CTIC,CTLV,NBCT
DO 9 IR=1,IRDN
  WRITE(6,111)(XY(IR,IC),IC=1,ICDN)
  9 CONTINUE
  111 FORMAT(' ',10F10.2)

C *** FIND EACH CONTOUR, STARTING WITH SMALLEST CTLV
DO 26 ICT=1,NBCT
  PRINT *****,CTLV
  26 CONTINUE

C *** CHECK IF ANY NODE HAS A VALUE EQUAL TO CTLV, IF SO CHANGE BY OFFSET
DO 22 IR=1,IRDN
  DO 22 IC=1,ICDN
    IF(FXY(IR,IC).EQ.CTLV) FAY(IR,IC)=FXY(IR,IC)+OFFSET
  22 CONTINUE

C *** SEARCH ALONG BOUNDARIES FOR SQUARES CONTAINING CONTOUR EXTREMITIES
NBC=0
NBP>0
NBS=0
NBSFR=0
IRH=IRDN-1
ICM=ICDN-1

C *** SEARCH ALONG VERTICAL BOUNDARIES
IC=1
IKBD=0
DO 100 IR=1,IRDN
  DO 110 IC=1,ICDN
    110 IKBD=ICM-1

C *** SEARCH ALONG HORIZONTAL BOUNDARIES
IR=1
IKBD=0
DO 120 IP=1,2
  DO 130 IC=1,ICMX
    IF((FXY(IR,IC)).LT.CTLW)AND(FAY(IR,IC)).LT.CTLW)OR.
      1 (FAY(IR,IC)).GT.CTLW)AND(FAY(IR,IC)).GT.CTLW) GO TO 130
    110 NBS=NBST+1
    110 IKBD=NBST+1
    110 IC=ICM-1
    110 CONTINUE
    IR=IRDN
    120 IKBD=1
    IF(NBS.EQ.0) GO TO 151

C *** START SEARCH WITH BOUNDARY SQUARES CONTAINING CONTOUR EXTREMITIES
DO 150 IBSU=1,NBSQ
  IC=(IBSQD*IBSQ-1)/IRMX
  IR=IBSQD*IBSQ-1-IC*IRMX
  IC=IC+1

C *** CHECK IF SQUARE HAS ALREADY BEEN SEARCHED. IN THIS PART OF THE
C *** PROBLEM ALL THE SEARCHES ARE SUCCESSFUL AND SEARCH GOES FROM
C *** SQUARE TO SQUARE, ONLY FURTHER IS SEARCH EXHAUSTIVE
  IF(NBP>0) GO TO 161
  DO 160 IXP=1,NBP
    IF(ISQR((IXPT).EQ.IR.AND.ISCOL((IXPT).EQ.IC)) GO TO 150
    160 CONTINUE
    150 CONTINUE

C *** THIS SQUARE IS THE FIRST OF THE CURVE, TWO INTERSECTIONS SHOULD
C *** BE FOUND: ONE WITH BOUNDARY AND THE OTHER LEADING TO THE SEARCH
C *** WITHIN THE GRID
C   1 IR,IC,IRMX,ICMX
    CALL SQR((IXY,IRDN,ICDN,IR,IC,0,CTLV)
    161 CONTINUE

C
    IP=NBPTS+1
    GO TO (150,11,12,11,14),IP

C *** ONE OR THREE POINTS: END CURVE
  11 NBCVNBCV+1
    EXCV$INBCV+=-(NPNT-IP+1)
    GO TO 150

C *** FOUR POINTS: SKIP THE SQUARE AT THIS TIME NOT TO START
C *** SEARCH WITH FOUR POINT SQUARE
  12 IPTBND=0
    14 NPNT=NPNT-4
    GO TO 150

C *** TWO POINTS : CHECK WHICH ONE IS ON THE BOUNDARY. SET IT FIRST
C *** IN ARRAY. START SEARCH WITHIN THE GRID
  12 IPTBND=0
    DO 15 IP=1,2
      IF((IR.EQ.1).AND.ISIDE(IP).EQ.1).OR.


```

```

1   ((R.EQ.ICMX.ANU.ISIDE(IP).EQ.3).OR.
2   ((C.EJ.IAND.ISIDE(IP).EQ.4).OR.
3   ((C.EJ.ICMX.ANU.ISIDE(IP).EQ.2).OR.TBNJ+1P
15 CONTINUE
C
IP=IPTBND+1
GO TO 150
C
C *** THE SECOND POINT IS ON THE BOUNDARY
32 STORJ=CONTX(NBPT)
STORY=CONTY(NBPT)
CONTX(NBPT)=CONTX(NBPT-1)
CONTY(NBPT)=CONTY(NBPT-1)
CONTX(NBPT-1)=STORX
CONTY(NBPT-1)=STORY
ISIDE(2)=ISIDE(1)
C
C *** THE FIRST POINT IS ON THE BOUNDARY (OR BOTH, IP=3)
C *** SET COUNTER TO NEGATIVE VALUE SINCE CURVE IS OPEN
31 NBCV=NBCV+1
IXCVST(NBCV)=-1(NBPT-1)
IS=ISIDE(2)
C
CALL SEARCH(FXY,IRDM,ICDM,IR,JC,IS,NBCV,CTLV)
C
C *** SEARCH FOR PRESENT CURVE COMPLETE. GO TO NEXT CURVE
C *** AT SAME CTLY
C
ICNT=1ABS(IXCVST(NBCV))
GO TO 150
C
150 CONTINUE
151 CONTINUE
C
C *** SEARCH EXHAUSTIVELY THE REMAINING SQUARES
C
DO 170 ICDO=1,ICMX
DO 170 IRDO=1,IRMX
IC=ICDO
IR=IRDO
C
C *** TEST IF THE SQUARE HAS BEEN SEARCHED BEFORE
IF (NBPT.EQ.0) GO TO 172
DO 171 IXP=1,NBPT
IF (ISURJW(IXPT).EQ.JR.AND.1SWCOL(IXPT).EQ.JC) GOTO 170
171 CONTINUE
172 CONTINUE
C
CALL SQUARI(FXY,IRDM,ICDM,IR,JC,0,CTLV)
C
IF (NBPTSW.NE.2) GO TO 170
NBCV=NBCV+1
IXCVST(NBCV)=NBPT-1
IS=ISIDE(2)
C
CALL SEARCH(FXY,IRDM,ICDM,IR,JC,IS,NBCV,CTLV)
ICNT=1ABS(IXCVST(NBCV))
C
170 CONTINUE
C
C *** KEEP LAST POINT + 1 IN MEMORY

```

ALL THE CURVES CORRESPONDING TO THIS CTRY ARE DETERMINED  
START PLOTTING, IF NBCV.EQ.0 SKIP PLOTTING  
REPLACE POINTS THAT TOO CLOSE TOGETHER BY THEIR AVERAGES  
IXST=1(XCVST)  
IXSK=0  
DO 200 IXCV=1,NBCV  
OPEN=FALSE.  
IXED=1ABS(IXCVST)(IXCV+1)-1  
IF (IXST.GT.0) GO TO 250  
OPEN=.TRUE.  
IXST=1XST  
250 STX=CONTX(IXST)  
STY=CONTY(IXST)  
IXP=IXST-XSK  
IXST=IXST+1  
C \*\*\* CHECK DISTANCE BETWEEN POINTS OF THE SAME CURVE  
DO 210 IX=IXST,IXED  
IXNW=IX-IXSK-1  
DXX=CONTX(IX)-SXIX  
DYY=CONTY(IX)-SYIX  
IF ((DXX\*DXX)+(DYY\*DYY).GT.DSMN) GO TO 220  
CONTX(IXNW)=STXX+5\*DAX  
CONTY(IXNW)=STY+5\*DYY  
STXX=CONTX(IXNW)  
STYY=CONTY(IXNW)  
IXSK=IXSK+1  
GO TO 210
C
220 CONTX(IXNW)=STXX  
CONTY(IXNW)=STYY  
STXX=CONTX(IX)  
STYY=CONTY(IX)  
210 CONTINUE
C
IF (OPEN) GO TO 225
DXX=CONTX(IXP)-CONTX(IXED)  
DYY=CONTY(IXP)-CONTY(IXED)  
IF ((DXX\*DAX+DYY\*DYY).GT.DSMN) GO TO 225  
CONTX(IXP)=CONTX(IXP)+5\*DAX  
CONTY(IXP)=CONTY(IXP)+5\*DYY  
IXSK=IXSK+1
C
C \*\*\* STORE LAST POINT OF CURVE AND START NEW ONE
225 CONTX(IXN+1)=CONTX(IXED)  
CONTY(IXN+1)=CONTY(IXED)  
IXCCK=IXCVST(1ACV+1)  
IXST=1XCVLK  
IF (IXCVLK.GT.0)IXCVCK=IXCVK-1XSK  
IF (IXCVLK.LT.0) IXCVK=IXCVCK+1XSK  
IXCVST(1IXCV+1)=IXCVK  
IXPP=1ABS(IXCVCK)-1  
200 CONTINUE
C
C \*\*\* PLOT THE CURVES. CHECK WHETHER LEVEL VALUES SHOULD BE PLOTTED
INLY=0
IF (PTLV.EQ.0) GO TO 201

```

CCTCK=CTLV
IF(CTCK.LT.1.) CTCK=CTCK*100.
IF(CTCK.GT.PTLV) GO TO 201
INLV=CTCK*.1
PTLV=PTLV*PTLVAD
201 CALL PLOT(AMODES,NBCV,INV,XINC,DELTA5)
10 CONTINUE
C CTLV=NCTLV+CTLC
26 CONTINUE
RETURN
END
SUBROUTINE SEARCH(FXV,IRDM,ICDM,IR,IC,IS,NBCV,CTLV)
C *** THE SUBROUTINE STARTS WITH A RECTANGLE HAVING ONE INTERSECTION WITH
C *** WITH CONTOUR AND SEARCHES THE WHOLE CONTOUR
C
COMMON /PLT/,CONTX100,CONTY100,DY,DX,ERROR,DFPOLY,
COMMON NBPT,NBPTSQ,ISIDE(4),LORDER,NBPTI,NBSQFR,
1 ISQRON(100),ISQCOL(100),IXCVST(20),ISQBD(20),
DIMENSION FXV(100),ICDM(100)
C *** DETERMINE NEXT SQUARE TO SEARCH
20 CONTINUE
GO TO (31,32,33,34),IS
31 IR=IR-1
GO TO 35
32 IC=IC+1
GO TO 35
33 IR=IR+1
GO TO 35
34 IC=IC-1
C *** TEST IF THE SQUARE IS WITHIN THE MESH. IF NOT END CURVE
C *** START NEXT CURVE FOR SAME CTLV
35 CONTINUE
IF(LC.LT.1.OR.LC.GT.(ICOM-1).OR.LT.1.OR.LR.GT.(IRD-1))GO TO 9
C *** DETERMINE IF CURVE CLOSES ON ITSELF. IF SO DELETE LAST POINT
C *** (SAME AS FIRST), END CURVE AND GO TO NEXT ONE FOR SAME CTLV
ISCK=ABS(IXCVST(NBPT))
IF(LR.EQ.ISQRON(100).AND.IC.EQ.ISQCOL(100))
1 GO TO 8
C *** DETERMINE SIDE NOT TO BE SEARCHED IN NEXT SQUARE
ISKIP=IS*2
IF(ISKIP.GT.4) ISKIP=ISKIP-4
C *** SEARCH NEXT SQUARE
CALL SQURAR(FXV,IRDM,ICDM,IR,IC,IS,ISKIP,CTLV)
IP=NBPITSQ+1
GO TO (7,11,7,6),IP
C *** NO POINTS ARE FOUND, THERE IS A PROBLEM
C *** END CURVE AND GO TO THE NEXT ONE FOR THE SAME CTLV
C *** ONE POINT FOUND, FLAG THE SIDE AND CONTINUE SEARCH
11 IS=ISIDE(1)
GO TO 20
GO TO 10
C
C *** COMPUTE ANGLE, CHOOSE SMALLEST
50 X2=CONTX(NBPT)
Y2=CONTY(NBPT)
X1=CONTX(NBPT-1)
Y1=CONTY(NBPT-1)
GO TO 50
40 CONTINUE

```

```

DXX=X2-X1
DY=Y2-Y1
IF(DXX.EQ.0.) GO TO 100
A1=ATAND(Y/DXX)
A1D=A1*D
GO TO 110

C *** THE SEGMENT IS VERTICAL A1=90.
100 A1=1.57085

C *** COMPUTE DIFFERENCE OF SLOPES BETWEEN PREVIOUS 2 POINTS (SEGMENT)
C *** AND SEGMENT (NBPT, NBPT+1)
110 DO 130 I=1,2
  XC=X(I,X)
  YC=Y(I,X)
  A2=ATAN((YC-Y2)/(XC-X2))
  A2D=A2*D
  DISTX=ABS(XA2-A1)
  DSIG=DIST(XI)*DG
  130 CONTINUE

C *** CHOOSE SMALLEST ONE, STORE REJECTED POINTS FOR LATER
  IXCK=1
  IF(DISTX).GT.DIST(2)) IXCK=2
  NBPT=NBP+1
  CONIX(NBPT)=XX(IXCK)
  CONY(NBPT)=YY(IXCK)

C *** STORE INFORMATION FOR LATER
  NBSQFR=NBSQFR+1
  IRFR(NBSQFR)=IR
  ICFR(NBSQFR)=IC
  IX=1
  IF(IXCK.EQ.1) IX=2
  ISFR2(NBSQFR)=ISIDE(IX)
  ISIDE(1)=ISIDE(IXCK)
  GO TO 99

C *** THIS RECTANGLE HAS ALREADY BEEN SEARCHED. TWO POINTS HAVE
C *** BEEN CHOSEN. CHOOSE THE JNE DIFFERENT FROM ISKIP
30 ISCK=ISR(IXSQ)
  IF(ISKIP.EQ.ISCK) ISCK=ISFR2(IXSQ)
  DO 60 IX=1,3
    IXP=IX
    IF(IXCK.EQ.1) IX=2
    ISFR2(NBSQFR)=ISIDE(IX)
    ISIDE(1)=ISIDE(IXCK)
    GO TO 99

C *** STORE THE POINT IN VECTOR
30 IXSQ=ISR(IXSQ)
  CONIX(NBPT)=XX(IXPT)
  CONY(NBPT)=YY(IXPT)
  ISIDE(1)=ISIDE(IXPT)

C 99 RETURN
END
SUBROUTINE SQUARIFYX(IRDM,ICDM,IR,IC,ISKIP,CTLV)
DIMENSION FXY(IRDM,ICDM)
COMMON /PLTT/ CONTX(100),CONTY(100),DX,DY,DX,DY,ERROR,OPFLY
COMMON /NPTR/ NPTRSW,ISIDE(4),IORDR,NBPTT,NBSQFR,
COMMON /NPTR/ NPTRSW,ISNCOL(100),IXCST(20),IXSQBD(20),
  1 ISQRD(100),ISNCOL(100),IXCST(20),IXSQBD(20),
  1 REAL FXY(IRDM,ICDM),MESH,X(100,5),QR(10,5),AS(5),DUM100,J,F15,
C *** DETERMINE REFERENCE CONER, DIRECTION OF INTERPOLATION
C *** AND MESH BOUNDARIES
  GO TO (1,2,1,2),IS
C *** HORIZONTAL SIDE
  1 MESH=DX
    IRE=IC
    IFC=IC
    IFC=IC
    IBC=ICDM
    GO TO 5

```

```

C *** VERTICAL SIDE
C 2 MESH-DY
  IREF=IR
  IFC=IC
  IBND=IDIM

C *** CHECK THAT ALL INTERPOLATION POINTS ARE WITHIN THE MESH
C *** IF NOT END INTERPOLATION AT THE BOUNDARY AND DECREASE
C *** CORRESPONDINGLY THE ORDER OF THE POLYNOMIAL
  5 IPAD=(NBPTIT-2)/2
  ILOR=IREF-IPAD
  IMGE=IREF+IPAD+1
  IF(ILOW.LT.ILOW) GO TO 1
  NPTS=IHIGH-ILOR+1
  IDIM=NPTS+1-(NBPTIT-IDIM)
  IF(IORDMAX.EQ.0) IORDMAX=1
  IF(IORDMAX.EQ.1) IDIM=1

C *** DETERMINE SIDE INDEX AND EVALUATE POLYNOMIAL AT THE POINTS
  IF(1.S-EJ-2.OR.1.S-EQ.3) IFCT=IFCT+1
  ICN=0
  DO 10 IAPT=ILDM,IGH
    ICN=ICN+1
    XINTER=1.1
    IF(IORDMX.LT.-2) GO TO 7
    DO 6 ICN=2,IORDMX
      6 XINTER(IORD)=FLOAT(ICN-1)*MESH**((IORD-1)
    7 CONTINUE
    GO TO (1.12,1.11,1.12,1) IS
    11 FICNT=FXY(IFCT,XAPT)
    GO TO 10
    12 FICNT=FXY(IXPCT,IFCT)
  10 CONTINUE

C *** CALL SOLVER
  CALL LSQ(X,QR,A,F,DUM,NPTS,10,IRDUM,5,1,130)
  XINTER=(FLOAT(IREF-ILDIM)+.5)*MESH
  TORMX=IRDUM**-1
  DO 20 ICN=1,20
    DERVF= A(12)
    FPOLY = A(11)
    DO 21 I = 1,IRDUM
      FPOLY = FPOLY + A(1+I)*XINTER**I
    21 CONTINUE
    DO 22 I=2,IRDUM
      DERVF = DERVF + FLOAT(I)*(A(1+I)*XINTER**I-1)
    22 CONTINUE
    DIFF=CTLY-FPOLY
    IF (ABS(DIFF).LT.ERROR) GO TO 25
    XINTER = XINTER + DIFF / DERVF
    2C CONTINUE
    25 XINTER=XINTER+FLOAT(ILDM-L)*MESH
    GO TO 31
  31 RETURN

SUBROUTINE LSQA(JRX,X,B,ABNRNM,M,MDIM,N,NDIM,P,*)
  INTEGER M,N,P,PIVOT100,MDIM,N,NDIM,I,J,K
  REAL QR(MDIM,N),ALPHA(N,R),Y(100),QR(M,N),ALPHAI(100),Y(100),
  1E100J,RL(100),DBL*8,DBLSUM*8,YNORM,ENORM,AXBNRNM,P,RNRSW
  C
  2 MESH-DY
  DO 100 I=1,M
    DO 11 J=1,N
      DO 12 I=1,M
        12 R(I,J)=A(I,J)
        1 CALL DCP(M,MDIM,N,QR,ALPHAI,PIVOT,130)
        DO 10 K=1,P
          ABNRNM(K)=0.
        DO 21 I=1,M
          R(I)=B(I,K)
        21 CALL SLV(M,MDIM,N,QR,ALPHA,PIVOT,R,Y)
        DO 31 I=1,M
          DBLSUM=-B(I,K)
          DO 32 J=1,N
            DBLA(A(I,J))
          32 DBLSUM=DBLSUM + DBL*Y(J)
        31 R(I)=DBLSUM
        CALL SLV(M,MDIM,N,QR,ALPHA,PIVOT,R,E)
        YNORM=0.
        DO 41 I=1,N
          YNORM=YNORM + Y(I)**2
        41 ENORM=ENORM + E(I)**2
        IF(ENORM.GT.YNORM*0.0625) GO TO 10
        DO 51 I=1,N
          Y(I)=Y(I) + E(I)
        51 DBLSUM = -B(I,K)
        DO 77 J=1,N
          DBLA(I,J)
        77 DBLSUM=DBLSUM+DBL*Y(J)
        77 R(I)=DBLSUM
        CALL SLV(M,MDIM,N,QR,ALPHA,PIVOT,R,E)
        ENORM=0.
        DO 81 I=1,N
          ENORM=ENORM + E(I)**2
        81 IF(ENORM.GT.1.5E-8*YNORM) GO TO 5
        DO 91 I=1,N
          RNRSQ=0.
          XLI(K)=Y(I)
        91 CONTINUE
        RNRSQ=SQRT(RNRSQ)
        11 ABNRNM(K)=SQRT(RNRSQ)
        11 RNRSQ=RNRSW*R(I)**2
        10 CONTINUE
        RETURN
      30 RETURN
    END
    SUBROUTINE SLV(M,MDIM,N,QR,ALPHA,PIVOT,R,Y)
    INTEGER M,N,PIVOT100,PIVOT100,I,J,I1,I2,NMIN
    REAL QR(MDIM,N),ALPHA(N,R),Y(100),Z(100),DBL*8,DBLSUM*8,GAMMA
    DO 1 J=1,N
      DBLSUM=0.
      DO 11 I=1,M
        DBL=QR(I,J)
        DBLSUM=DBLSUM+DBL*R(I)
      11 GAMMA=UBLSUM/(ALPHA(I)*QR(I,J))
      00 12 IJ=J
      12 RI(J)=R(I,J)+GAMMA*QR(I,J)
    1 CONTINUE

```

```

2*(N+R(N)/ALPHA(N)
NMN1=N-1
DO 2 I=1,NM1N,
I=N-1
I=I+1
DBLSUM=R(I)
DO 22 J=1,I,N
DBL=QR(I,J)
DBLSUM=DBLSUM + DBL*L(J)
22 Z(I)=DBLSUM+ALPHA(I)
DO 3 I=1,N
PIVOTI=PIVOT(I)
3 Y(PIVOTI)=Z(I)
RETURN
END
SUBROUTINE DCP(M,MDIM,N,QR,ALPHA,PIVOT,*)
INTEGER M,N,PIVOT1(100),MDIM,I,J,K,JBAR,K1
REAL QR(MDIM,N),ALPHA(N),SUM(100),Y(100),DBL,SUM*8,BETA,
1 SIGMA,ALPHAK,QRKK
DO 1 J=1,N
PIVOT(J)=J
DBLSUM=0.
DO 11 I=1,M
DBL=QR(I,J)
DBL=QR(I,J)
DBLSUM=DBLSUM + DBL**2
11 SUM(J)=DBLSUM
DO 8 K=1,N
SIGMA=SUM(K)
JBAR=K
IF(K.EQ.N) GO TO 5
K=K+1
DO 2 J=K1,N
IF(SIGMA.GE.SUM(J)) GO TO 2
SIGMA=SUM(J)
JBAR=J
CONTINUE
3 IF(JBAR.EQ.K) GO TO 5
I=PIVOT(K)
PIVOT(K)=PIVOT(JBAR)
PIVOT(JBAR)=I
SUM(JBAR)=SUM(K)
SIGMA=SIGMA
DO 4 I=1,M
SIGMA=QR(I,K)
QR(I,K)=QR(I,JBAR)
QR(I,JBAR)=SIGMA
DBLSUM=0.
DO 55 I=K,M
DBL=QR(I,K)
DBLSUM=DBLSUM+DBL**2
55 SIGMA=DBLSUM
IF(SIGMA.EQ.0.) RETURN
5 QRK=QR(K,K)
ALPHAK=SQR(SIGMA)
IF(QRK.GE.0.) ALPHAK=-ALPHAK
ALPHAK=ALPHAK
QR(K,K)=QRK-ALPHAK
BETA1=-SIGMA-QRK*ALPHAK
IF(K.EQ.N) GO TO 8
DO 6 J=K1,N
DBLSUM=0.

```

```

C *** TO THE MAIN SYSTEM. OBTAIN WEIGHTED AVERAGES CNTX(), CNTY()
C *** XNO, YNO AND XNM, YNM
C *** 400 XBG=.
C *** IPT=1
C *** NBIC=ABS(XA2/XINC)+1
C *** DETERMINE WEIGHTING FACTOR AND ANGLE OF NEW WITH RESPECT
C *** TO OLD SYSTEM
C *** IF(INDAVG) GO TO 430
C *** IF(NBIC.EQ.0) GO TO 430
C *** NBIC=XNBIC
C *** MNOD=1.-MN
C *** SLNO=SLNW-SL00
C *** SLNO=SLNO*DEG
C *** CSNO=COS(SLNO)
C *** SNNO=SIN(SLNO)
C *** DETERMINE IN OLD SYSTEM THE SLOPE OF THE VERTICAL IN NEW
C *** VERTI=.TRUE.
C *** IF(SLNO.EQ.0..OR.ABS(SLNO).EQ.PI) GO TO 430
C *** VERTI=.FALSE.
C *** AVT=TAN(SLNO+P12)
C *** SLV=SLN(P12)
C *** NBIC=NBIC+1
C *** DETERMINE INTERSECTIONS IN NEW AXIS
C *** 430 CNTX11=X(IPT)
C *** CNTY11=Y(IPT)
C *** NBIC=NBIC+1
C *** IF(NBIC.LT.2) GO TO 410
C *** XIC=XX2/XNBIC
C *** DO 420 IXIC=2,NBIC
C *** XNM=XBIG*FLOAT(IXIC(C-1)*XXIC
C *** YNM=A3NM*XNM*A2NM*XNM
C *** TPXN1IXIC=XXNM
C *** TPYN1IXIC=YNM
C *** TPXNM(IXIC)=XORMH+XXNM*CSNM-YNM*SNM
C *** TPYNM(IXIC)=YORMH+XXNM*SNNM+YNM*CSNM
C *** DO NOT AVERAGE IF IT IS THE FIRST OR LAST LEG OF AN OPEN CURVE
C *** IF(INDAVG) GO TO 480
C *** DETERMINE THE COORD OF XXNM,YN IN OLD SYSTEM TO OBTAIN
C *** THE B COEFF. (BVT) IN OLD OF VERTICAL IN NEW
C *** X00-DXNO*XXNM*CSNO
C *** IF(VERTI) GO TO 425
C *** YY0=DYN0*XXNM*SNM
C *** BVT=Y00-AVT*XX00
C *** AA=AVT-A200
C *** RTAD=(SQT(AA*AA+4.*A300*BVT))/(2.*A300)
C *** XX00-AA+RTAD
C *** AA=AA*.5/.300
C *** XXXX-AA-RTAD
C *** IF((XX00.LT.0.) GO TO 426
C *** IF((XX0D-LT.DXNO).OR.
C *** 1.(XX0D.GT.(DX3))XX0D=AA-RTAD
C *** GO TO 425
C *** 426 IF((XX0J.GT.DXNO).OR.

C SD0=0.
C PIC=0
C *** TRANSFER THE POINTS OF INTEREST INTO A SMALL ARRAY
C *** 110 IXED=IXST
C *** IPIC=IPIC+1
C *** IF(IXED.GT.IXHG)IXED=IXLN
C *** DO 300 IX=1,3
C *** XX(IX)=CONTX(IXED)
C *** YY(IX)=CONTY(IXED)
C *** IXED=IXED+1
C *** IF(IXED.GT.IXHG) IXED=IXLN
C *** 300 CONTINUE
C *** OPERATE TRANSFER OF AXIS
C *** XXND=(XX(13)*XX(1))+*.5
C *** YYND=(YY(13)*YY(1))+*.5
C *** YYDU=YY(10)-YY(12)
C *** XXDU=XX(12)-XX(10)
C *** CHECK IF AXIS SHOULD BE VERTICAL
C *** IF(ABS(YYDU).GE.OFPOLY) GO TO 310
C *** IF MD AND 2 CO INCIDE, THE SLOPE IS TO BE DETERMINED
C *** BETWEEN MD AND 1
C *** IF(ABS(XXDU).GE.OFPOLY) GO TO 311
C *** YYDU=YY(1)-YY(11)
C *** XXDU=XX(11)-XX(1)
C *** IF(ABS(YYDU).GE.OFPOLY)SLNW=ATANI(XXDU/YYDU)
C *** GO TO 311
C *** 310 SLNW=ATANI(XXDU/YYDU)
C *** DETERMINE COORDINATES OF THE THREE POINTS IN NEW SYSTEM
C *** 311 XORNW=XX(11)
C *** YORNW=YY(11)
C *** CSMH=XCOS(SLNW)
C *** SNHW=XSIN(SLNW)
C *** XX2=(XX(12)-XX(11))+CSNW*(YY(2)-YY(1))*SNHW
C *** IF((ABS(XX2).LT.OFPOLY) XX2=SIGN(OFPOLY)*XX2)
C *** YY2=(YY(2)-YY(1))+CSNW*(YY(2)-YY(1))*CWNW
C *** YY3=(XX(13)-XX(11))+CSNW*(YY(3)-YY(1))*CWNW
C *** IF((YY2-.5*YY3).EQ.0.) YY2=YY2+OFPOLY
C *** SLDNW=SLNW*DEG
C *** OBTAIN THE COEFFICIENTS OF THE 2 ORDER POLYNOMIAL
C *** PASSING THROUGH THE 3 POINTS, ALIN=0.
C *** A2NW=(2.*YY2-.5*YY3)/XX2
C *** A3NW=(.5*YY3-YY2)/XX2*XX2
C *** IF THIS IS THE FIRST LEG OF A CLOSED CURVE, GO TO NEXT POINT
C *** IF(SKIP) GO TO 120
C *** IN NEW AXIS, OBTAIN NUMBER OF INCREMENTS (NBIC), INCREMENT
C *** SIZE (IXIC), COORD OF POINT ON CURVE (XNM,YNM), TRANSFER
C *** TO MAIN AXIS (CNTX(CNTY)).
C *** IN OLD (PREVIOUS) SYSTEM, FIND THE CORRESPONDING OF X COORDINATE
C *** (XX0) OF XNM,YNM. OBTAIN THE INTERSECTION OF THE VERTICAL AT
C *** AT XX0 WITH THE OLD CURVE XX0D,YY0D. TRANSFER THE INTERSECTION

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1. (XX0D+LT*(DXX3)) XX0D=AA=R7AU
C Y0D=A3DD*XX0D*XX0D+A200*XX0D
C *** TRANSFER TO MAIN AXIS
480 TP0(IIXIC)=Y0D
TP0(IIXIC)=Y0D
TP0M(IIXIC)=XR0D+XX0D*CS0D-Y0D*SN0D
TP0M(IIXIC)=XR0D+XX0D*CS0D-Y0D*SN0D
CNY(IIXIC)=MNH*(XR0N+XN*CSN*-YYNN*SNNN)*
CNY(IIXIC)=MNH*(XR0D+X0D*CS0D-Y0D*SN0D)
MNHD*(XR0D+X0D*CS0D-Y0D*SN0D)
CNY(IIXIC)=MNH*(YR0N+XN*CSN*-YYNN*SNNN)*
MNHD*(YR0D+X0D*SN0D-Y0D*CS0D)

C *** COMPUTE NEW WEIGHTS
419 MNH=MNH*MNH
MNHD=MHD-MH
420 CONTINUE
410 CNIX(NBICIT)=XX(IPIT+1)
CNY(NBICIT)=Y(IPIT+1)

C *** PLOT ARRAY OF NBIC POINTS GOING FROM IXST TO IXST+1
C* WRITE(6,10001)(TPXN(I),TPYN(I),I=2,NBIC)
C* WRITE(6,10001)(TPXQ(I),TPYQ(I),I=2,NBIC)
C* WRITE(6,10001)(TPXN(I),TPYN(I),I=2,NBIC)
C* WRITE(6,10001)(TPXQ(I),TPYM(I),I=2,NBIC)
C* WRITE(6,10001)(TPXM(I),TPYQ(M(I)),I=2,NBIC)
C* WRITE(6,10001)(CNTX(I),CNY(I),I=1,NBICTT)
C* FORMAT(' ',1OF10.3)
1000 FORMAT(' ',1OF10.3)
1 IF NOT LEVEL GO TO 119
C *** WRITE CONTOUR LEVEL
IF(XX(2).EQ.0.OR.IPIC.LT.ILPT) GO TO 119
XLB=DELTA*DELTA
IF((XX(2)*XX(2)+YY(2)*YY(2)).LT.XLAB) GO TO 119
IPIC=0
119 KICK=XI1IPIT+1
YICK=YY1IPIT+1
DO 125 I=2,NBICTT
IACK=NBICIT-IX+1
DELTA=(XC(K)-CNTX(IACK))*XX(C-K-CNTX(IACK))+  

1 (YC(K)-CNTY(IACK))*YY(C-K-CNTY(IACK))
IF(DELTA.GT.XLAB) GO TO 126
125 CONTINUE
C
126 ANGL=ATAN((CNY(IACK)-YICK)/(CNTX(IACK)-XX(C-K)))
AMODES(150)=ANGL*180./3.14159
AMODES(4)=AMODES(150)
XLDY=DELTAS/1.5
XLDX=XLDY

C
118 ILB=IXCK
C IF(IX2.LT.0.)ILB=-NBICIT
XLB=CN(X(IILB))*ALD*A*COS(ANGL)
YLB=CN(Y(IILB))+XLDB*SIN(ANGL)
CALL INJRGAMODES,XLB,YLB,2,[NLV]

C
NBICIT=LACK
IF(NBICIT.LT.-2) GO TO 120
119 CALL LINESGAMODES,NBICIT,CNTX,CNTY
C *** MOVE TO NEXT POINT
C *** STORE PARAMETERS FROM NM TO JD
120 IXST=IXST+1

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