NSF-RA-E-73-540

OPTIMUM SEISMIC PROTECTION FOR NEW BUILDING CONSTRUCTION IN EASTERN METROPOLITAN AREAS

NSF Grants GK-27955 and GI-29936

Internal Study Report No. 27

DAMPING CAPACITY OF BOSTON BLUE CLAY

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Any opinions, findings, conclusions or recommendations expressed in this publication are those of the author(s) and do not necessarily reflect the views of the National Science Foundation.

January 1973

Department of Civil Engineering Massachusetts Institute of Technology Cambridge, Massachusetts

ASRA INFORMATION RESOURCES NATIONAL SCIENCE FOUNDATION

50272 -101			
REPORT DOCUMENTATION 1. REPORT NO. PAGE NSF-RA-E-73-	540	2.	3. Recipients Accession No. 50
4. Title and Subtitle Damping Capacity of Boston Blue Cl	ay (Optimum Se	ismic Protec-	5. Report Date January 1973
tion for Low Building Construction	in Eastern Me	tropolitan	6.
7. Author(s)			8. Performing Organization Rept. No.
P.J. Trudeau			Internal Study Report 27
9. Performing Organization Name and Address			10. Project/Task/Work Unit No.
Massachusetts Institute of Technol Department of Civil Engineering	ogy		11. Contract(C) or Grant(G) No.
Cambridge, Massachusetts 02139			(C)
			^(G) GI29936, GK27955
12. Sponsoring Organization Name and Address	<u> </u>		13. Type of Report & Period Covered
Applied Science and Research Applic National Science Foundation	ations (ASRA)		
1800 G Street, N.W.			14.
Washington, D.C. 2000			1
13. Supplementary notes			
This report presents the results o	f the determin	ation of the d	amping capacity of un-
alsturbed samples of Boston Blue C nology's Hardin Oscillator Two m	lay obtained under the second se	sing Massachus rmining damnin	etts Institute of lech-
illustrated. In the Log Decrement	method a phot	ograph of the	decaving vibration is
used to determine the log decremen	t of the appar	atus and of the	e system with specimen
at the resonant frequency. Equati	ons are then u	sed to compute	the damping ratios. In
the Frequency and Amplitude of Vib	ration method,	the frequenci	es and amplitudes of
capacity. The values of damping o	of Boston Blue	Clav as determ	ined by this testing
program are between 1 percent and	3 percent.		
17 Document Anglusis - a Descriptors			
Farthquake resistant structures	Soil dyna	mics	Soil tests
Damping capacity	Soil prop	erties	Clay soils
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b. Identifiers/Open-Ended Terms			
a COSATI Field (Craup	•		
18. Availability Statement	/	19. Security Class (Th	is Report) 21. No. of Pages
NTIS	C		11
		20. Security Class (Th	is Page) 22. Price $AAT = AAT$
(See ANSI-Z39.18)	See Instructions on R	everse	ΟΡΤΙΟΝΑL FORM 272 (4-77
			(Formerly NTIS-35)

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⁽Formerly NTIS-35) Department of Commerce

CAPITAL SYSTEMS GROUP, 1990. 6110 EXECUTIVE BOULEVARD SUITE 260 ROCKVILLE, MARYLAND 20852

List of Internal Study Reports

- 1. "Preliminary Work Plans and Schedules"
- 2. "Background for Preliminary Expected Future Loss Computations"
- 3. "Identification of Typical Soil Profiles in the Boston Basin Area"
- 4. "Comparison of Wind and Seismic Forces on Tall Buildings"
- 5. "Contribution to State-of-the-Art Report of the Earthquake Committee of the IABSE-ASCE Tall Buildings Committee ---Economic and Social Aspects"
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- 7. "Analysis of the Seismic Risk on Firm Ground for Sites in the Central Boston Metropolitan Area
- 8. "1967 Caracas Venezuela Earthquake Tall Building Damage Review"
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- 14. "Elastic Analysis of Pilot Building"
- 15. "Soil-Amplification Analyses on Typical Boston Soil Profiles"
- 16. "Description and User's Manual of the Inelastic Dynamic Analysis Program"
- 17. "Ground Motions Measured by VM-1"
- 18. "Borings on MIT Campus near Westgate II"
- 19. "Evaluation of Expected Losses and Total Present Cost: Further Preliminary Sensitivity Analyses"
- 20. "Damage Prediction of Three Types of Buildings Based on Dynamic Analysis"
- 21. "Incident Losses: Identification and Evaluation Methods to to be Employed"
- 22. "1957 San Francisco Earthquake Tall Building Damage Review"
- 23. "1965 Puget Sound, Washington, Earthquake Tall Building Damage Review"
- 24. "Evaluation of Risky Public Projects"
- 25. "Microtremor Measurements in the Boston Area"
- 26. "In Situ Shear Wave Velocity Measurements on MIT Campus"

This report presents the results of the determination of the damping capacity of Boston Blue Clay obtained using MIT's Hardin Oscillator. For a description of the testing program see Trudeau (1973). The procedures used in calculating the damping capacity are outlined in Hardin (1970). A sample calculation for Test G-1 is included in Appendix A of this report.

These tests were performed on undisturbed samples of Boston Blue Clay. The purpose of these tests was to determine the shear wave velocity of Boston Blue Clay to be used in soil amplification studies in the design of structures in the Boston area against earthquakes; however, values of damping capacity of the specimens tested were also obtained. Inexperience and faulty electronic equipment led to erroneous values of damping for the earlier tests. However, the values of damping reported in Table 1 were obtained after the electronic equipment was repaired and more experience had been gained. Therefore, it is the opinion of the author that these values are good.

Two methods of determining damping were used and are illustrated in Appendix A. In the Log Decrement method a photograph of the decaying vibration is used to determine the log decrement of the apparatus and of the system (with specimen) at the resonant frequency. Equations presented in Hardin (1970)

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are then used to compute the damping ratios. In the Frequency and Amplitude of Vibration method, the frequencies and amplitudes of vibration at resonance and at a higher frequency (equal to f_n^2/f_0) are used to determine the damping capacity.

As can be seen in Table 1, the values of damping of Boston Blue Clay as determined by this testing program are between 1 and 3%. The values for Tests L-1 and AA-1 of 1-1.5% were obtained after an elapsed time of approximately one day whereas the values of 3% for Test G-1 were obtained after four days of testing. Past testing of clay soils (Hardin and Drnevich, 1972; and Richart, Hall, and Woods, 1970) has indicated damping ratios within this range; therefore, these values appear to be reasonable.

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LIST OF REFERENCES

- Hardin, Bobby O., "Suggested Methods of Test for Shear Modulus and Damping of Soils by the Resonant Column," ASTM, STP 479, pp. 516 - 529, 1970.
- Hardin, Bobby O., and Drnevich, V. P., "Shear Modulus and Damping in Soils: Measurement and Parameter Effects," JSMFD, ASCE, Volume 98, SM 6, June, 1972.

Richart, F. E., Jr.; Hall, J. R., Jr.; and Woods, R. D.; <u>Vibrations of Soils and Foundations</u>, Prentice - Hall, New Jersey, 1970.

Trudeau, P. J., "The Shear Wave Velocity of Boston Blue Clay," MIT Department of Civil Engineering Research Report R73-12, Soils Publication # 317, 1973.

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DAMPING CAPACITY

OF

BOSTON BLUE CLAY

TABLE I

TEST	LOG DECREMENT	FREQ. 4 AMP. OF VIBRATION	ELAPSED
	•/。	%	DAYS
L-1	1.20	1.02	1
AA-I	1.57	1.41	
G-1	2.91	2.92	4

NOTE: VALUES OBTAINED AT IN SITU To

5

APPENDIX A

CALCULATION OF DAMPING FOR TEST G-I

CALCULATION OF DAMPING FOR TEST G-I

$$S = \frac{2 K_{0}L}{R C_{m} G_{v}^{4} q_{81} \times 10^{3}} = \frac{(2)(3.389 \times 10^{9})(8.00)}{R(1.005)(483)(1.74)^{4} q_{81} \times 10^{3}}$$
$$S = 3.53$$
$$D = \frac{1}{2\pi} \left[S_{s} (1+5) - S_{A} S \right] \times 100\%$$
$$S_{A} = 0.029 \qquad S_{s} = 0.063$$
$$\longrightarrow D = 2.91\%$$

LOG DECREMENT

CALCULATION OF DAMPING FOR TEST G-1

$$T_{n} = 4.787 \text{ m} \text{Sec} \longrightarrow f_{n} = 208.9 \text{ cPS}$$

$$T_{0} = 5.33 \text{ m} \text{Sec} \longrightarrow f_{0} = 187.4 \text{ cPS}$$

$$\frac{f_{n}}{f_{0}} = 1.114 \longrightarrow C_{1} = 0.867 \quad C_{2} = 1.25$$

$$R = \frac{A_{T_{n}}}{A_{T_{1}}} = \frac{27.0}{2.95} = 9.15$$

$$D = \frac{1}{2} \left[\frac{C_{1}}{R} - C_{2} \text{ S}_{A} \right] \times 100\%$$

- D = 2.92 %

FREQUENCY AND AMPLITUDE OF VIBRATION

8.