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EARTHQUAKE ENGINEERING RESEARCH CENTER

# TLUSH:

A COMPUTER PROGRAM FOR THE  
THREE-DIMENSIONAL DYNAMIC ANALYSIS  
OF EARTH DAMS

by

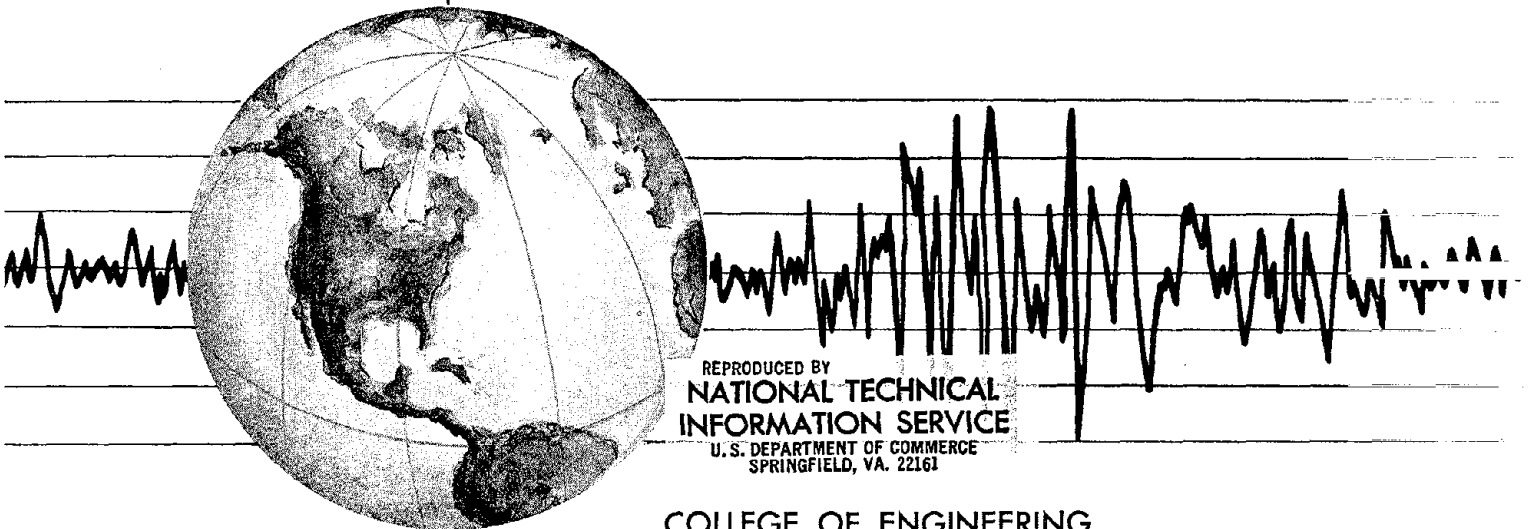
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## 1. INTRODUCTION

Significant progress has been made over the past two decades in the development of analytical procedures for evaluating the response and stability of earth dams subjected to seismic loads. Current methods of stability analysis involve procedures such as that proposed by Seed et al., (1973) which consists of the following steps:

1. Determination of the initial stresses existing throughout the dam and the foundation before the earthquake.
2. Determination of the characteristics of the earthquake motions that are likely to affect the dam.
3. Computation of the response of the embankment and foundation to the selected earthquake motions.
4. Determination in the laboratory or by means of empirical correlations of the response to the induced dynamic stresses of representative samples of the embankment and foundation materials.
5. Evaluation of the overall deformations and stability of the embankment dam.

Due to the fact that the finite element method can easily handle geometrical irregularities, complex material behavior and arbitrary boundary conditions it is perhaps the most flexible tool currently available to perform the dynamic response analysis of an earth dam.

Limitations of computer speed and storage capacity have restricted until recently the use of the finite element method to two-dimensional problems. Although many earth dams fall within this category, there are also many cases in which the assumption of plane strain behavior gives



only approximate results and therefore a full three-dimensional analysis is warranted. Thus, the availability of a numerical procedure for the dynamic analysis of earth dams in three-dimensions seems desirable.

Two-dimensional finite element techniques which use the complex response method and therefore permit variations in modulus and damping in different elements of a soil structure, were developed by Lysmer et al., (1974,1975). These procedures were extended to three dimensions, with a constraint on the possible deformations of the finite element model, by Kagawa (1977). The present version of the computer program TLUSH constitutes a further development of these procedures (Mejia, 1981) and incorporates additional features among which are the following:

- 1) Complete freedom for the selection of the direction of the earthquake motions,
- 2) Complete freedom in the deformational modes of the model,
- 3) A new interpolation scheme,
- 4) A nodal point and element data generation routine,
- 5) More efficient element stiffness generation routines and
- 6) A more efficient program structure that has lower memory requirements.

The program TLUSH can take into account the strong nonlinear effects characteristic of soil masses subjected to strong earthquake motions. This is achieved by a combination of the equivalent linear method (Seed and Idriss, 1969) and the complex response method. Typical relationships between stiffness, damping and effective shear strains for sand and clay are provided within the program. Special options that permit creation of a permanent record of both input and basic information on the complete solution, and recovery of this information for iteration and output purposes are available within the program.



## 2. ANALYTICAL PROCEDURE

Within the framework of the finite element techniques employed by the computer program TLUSH a typical earth dam will be modeled by an assemblage of elements as shown in Fig. 1. Eight node isoparametric brick elements with three degrees of freedom per node and linear interpolation on the displacement field are used in the model. Nodal points are defined only at the intersection of three of the planes defining an element. It is assumed that the walls of the valley in which the dam is located are rigid and therefore all points on these boundaries move in phase and with the same displacement amplitudes. Interaction between the dam and its abutments and between the dam and the reservoir is neglected. Material behavior is assumed to be characterized by an equivalent linear elastic shear modulus and an equivalent fraction of damping.

### 2.1 Equation of Motion

Considerations of dynamic equilibrium of the model shown in Fig. 1 lead to the following equations of motion:

$$[M]\{\ddot{u}\} + [K]\{u\} = -[M]\{r\}\ddot{y}(t) \quad (1)$$

where:  $\{u\}$  = the nodal point displacements relative to the rigid boundary,

$\{\ddot{u}\}$  = the corresponding accelerations

$[K]$  = the complex stiffness matrix

$[M]$  = the mass matrix

$\ddot{y}(t)$  = the input rigid boundary acceleration

$\{r\}$  = the load vector that gives the direction of the input motion.





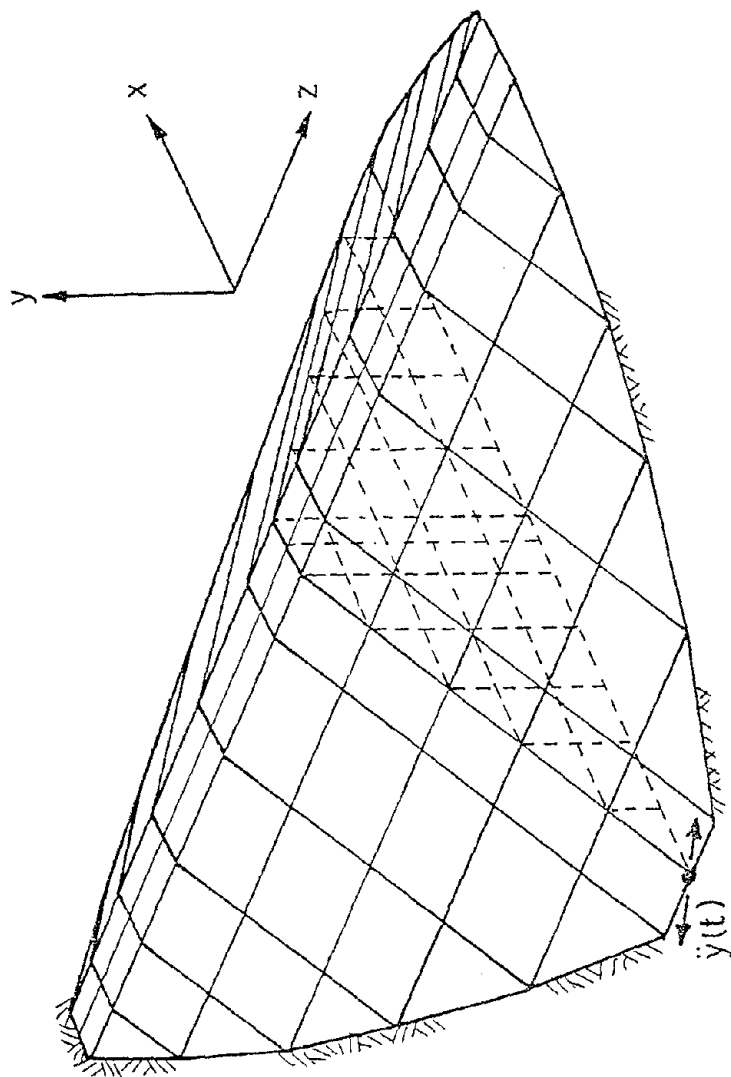


FIG. 1 TYPICAL 3-D FINITE ELEMENT MODEL OF AN EARTH DAM



The load vector  $\{r\}$  is given by:

$$\{r\} = C_x \begin{Bmatrix} 1 \\ 0 \\ 0 \\ 1 \\ 0 \\ 0 \\ \cdot \\ \cdot \\ \cdot \\ 1 \\ 0 \\ 0 \\ 0 \end{Bmatrix} + C_y \begin{Bmatrix} 0 \\ 1 \\ 0 \\ 0 \\ 1 \\ 0 \\ \cdot \\ \cdot \\ \cdot \\ 0 \\ 1 \\ 0 \\ 0 \end{Bmatrix} + C_z \begin{Bmatrix} 0 \\ 0 \\ 1 \\ 0 \\ 0 \\ 1 \\ \cdot \\ \cdot \\ \cdot \\ 0 \\ 0 \\ 0 \\ 1 \end{Bmatrix} \quad (2)$$

where  $C_x$ ,  $C_y$  and  $C_z$  are scalar constants that determine the magnitude of the components of the input motion in the x, y and z directions respectively.

The global mass and stiffness matrices are assembled from the corresponding element matrices following standard finite element procedures and the direct stiffness method (Zienkiewicz, 1977). Viscous damping is introduced by the use of complex shear moduli in forming the element stiffness matrices:

$$G^* = G(1 - 2\beta^2 + 2i\beta\sqrt{1 - \beta^2}) \quad (3)$$

where:  $G$  = the element shear modulus

$\beta$  = the element fraction of damping

and therefore, the stiffness matrix for the system will have complex coefficients.

## 2.2 The Method of Complex Response

Use of the complex response method is made in order to solve the equations of motion for the system (equation (1)). In this method it is assumed that the system is linear and that the input motion is periodic.



As a consequence the principle of superposition is applicable and the input accelerations can be expressed as a sum of harmonics:

$$\ddot{y}(t) = R_e \sum_{s=0}^{N/2} \ddot{Y}_s \exp(i\omega_s t) \quad (4)$$

where:  $N$  = the number of digitized points in the input motion.

$$\omega_s = \frac{2\pi s}{N\Delta t} \quad (5)$$

$\Delta t$  = the time step of digitization

$\ddot{Y}_s$  = the complex Fourier amplitudes which are given by:

$$\left. \begin{aligned} \ddot{Y}_s &= \frac{1}{N} \sum_{K=0}^{N-1} \ddot{y}_k \exp(-i\omega_s k\Delta t) \quad \text{for } s = 0, s = N/2 \\ \ddot{Y}_s &= \frac{2}{N} \sum_{K=0}^{N-1} \ddot{y}_k \exp(-i\omega_s k\Delta t) \quad \text{for } 1 \leq s \leq N/2 \end{aligned} \right\} \quad (6)$$

The complex Fourier amplitudes,  $\ddot{Y}_s$ , can be computed efficiently using the Fast Fourier Transform algorithm developed by Cooley and Tukey (1965) which requires that  $N$  be a power of 2. This requirement is generally not a drawback since trailing zeroes usually need to be added at the end of the earthquake excitation to provide for an adequate quiet zone that allows the decay of free vibrations of the model.

Substitution of equation (4) into equation (1) leads to:

$$[M]\{\ddot{u}\} + [K]\{u\} = -[M]\{r\}R_e \left( \sum_{s=0}^{N/2} \ddot{Y}_s \exp(i\omega_s t) \right) \quad (7)$$

The steady-state solution of this equation can also be expressed as a sum of harmonics. That is:



$$\{u\} = R_e \sum_{s=0}^{N/2} \{U_s\} \exp(i\omega_s t) \quad (8)$$

The complex displacement amplitudes  $\{U_s\}$  can be obtained by substituting equation (8) into equation (7). Since the principle of superposition is applicable this leads to the following matrix equation for each frequency  $\omega_s$ :

$$([K] \cdot \omega_s^2 [M]) \{U_s\} = -[M]\{r\}\ddot{Y}_s \quad (9)$$

This is a system of linear algebraic equations with complex coefficients that can be solved by gauss elimination. Solution of equation (9) for each frequency  $\omega_s$  constitutes a formidable task since it is not uncommon for an input record to have 2048 digitized points and therefore 1025 frequency points in its Fourier spectrum. In order to simplify the computational effort a cut-off frequency is usually selected. This frequency is chosen at a value such that the response of the dam to higher frequencies is small and does not need to be computed. Additionally, the following equation:

$$([K] - \omega_s^2 [M]) \{A_s\} = -[M]\{r\} \quad (10)$$

is solved instead of equation (9). A limited number of frequencies is selected for solution of equation (10) and a special interpolation technique is used to compute intermediate values of  $\{A_s\}$ . The complex displacement amplitudes can then be obtained from the following expression:

$$\{U_s\} = \{A_s\}\ddot{Y}_s \quad (11)$$





### 2.3 Interpolation in the Frequency Domain

Linear interpolation on the inverse of the amplification functions has been used in the past as the technique to obtain intermediate values of  $\{A_s\}$  (Lysmer et al., 1974,1975). However, this interpolation scheme does not perform well when there is a close spacing between natural frequencies which seems to be the case of three-dimensional systems.

A sophisticated technique that makes use of the transfer function for a two-degree-of-freedom system to interpolate between the amplification function values  $\{A_s\}$  has been developed by Tajirian (1981) and has been incorporated to the computer program TLUSH. The transfer function for a two-degree-of-freedom system has typically two frequency peaks and is given by an expression of the form:

$$A(\omega) = \frac{C_1\omega^4 + C_2\omega^2 + C_3}{\omega^4 + C_4\omega^2 + C_5} \quad (12)$$

where:  $\omega$  = angular frequency

$C_1, C_2, C_3, C_4, C_5$  = complex constants.

The values of the amplification function  $\{A_s\}$  at five particular frequencies spanning the range where interpolation is to be performed are necessary to determine the five constants that define the transfer function (equation 12), which is then used to compute the intermediate values of  $\{A_s\}$ .

The nodal displacements  $\{u\}$ , which completely define the response of the dam, can be computed from equations (11) and (8). Velocities, accelerations and strains can be directly computed in the frequency domain from the displacement amplitudes  $\{U_s\}$  and can then be transformed into the time domain.



## 2.4 The Equivalent Linear Method

Due to the extensive use of superposition, the procedures previously outlined are only applicable to linear systems. However, the response of soils to dynamic loading is highly non-linear in nature and must be accounted for in order to obtain meaningful results from the dynamic analysis of soil structures.

The approximate method proposed by Seed and Idriss (1969) to take into account the non-linear behavior of soils is used in TLUSH. According to this method the non-linear response of a soil structure can be approximated by a linear analysis for which the stiffness and damping are compatible with the induced strains at every point of the system.

An extensive summary of data on strain compatible moduli and damping for clays and sands was presented by Seed and Idriss (1970) from which the curves shown in Fig. 2 were developed. Starting from selected initial moduli and damping values for each element in the model, these curves can be used iteratively to reach compatibility between the properties used in the analysis and the computed strains.

The relationships between stiffness, damping and effective shear strains for sands and clays proposed by Seed and Idriss (1970) have been provided within the program but other modulus attenuation curves can readily be read into the program. The following procedure is employed to obtain an approximate non-linear solution:

1. Read in initial assumed values of shear modulus and damping for all elements.
2. Solve equation (1) and use equation (11) to obtain the complex displacement amplitudes.
3. Compute strain amplitudes for all elements.



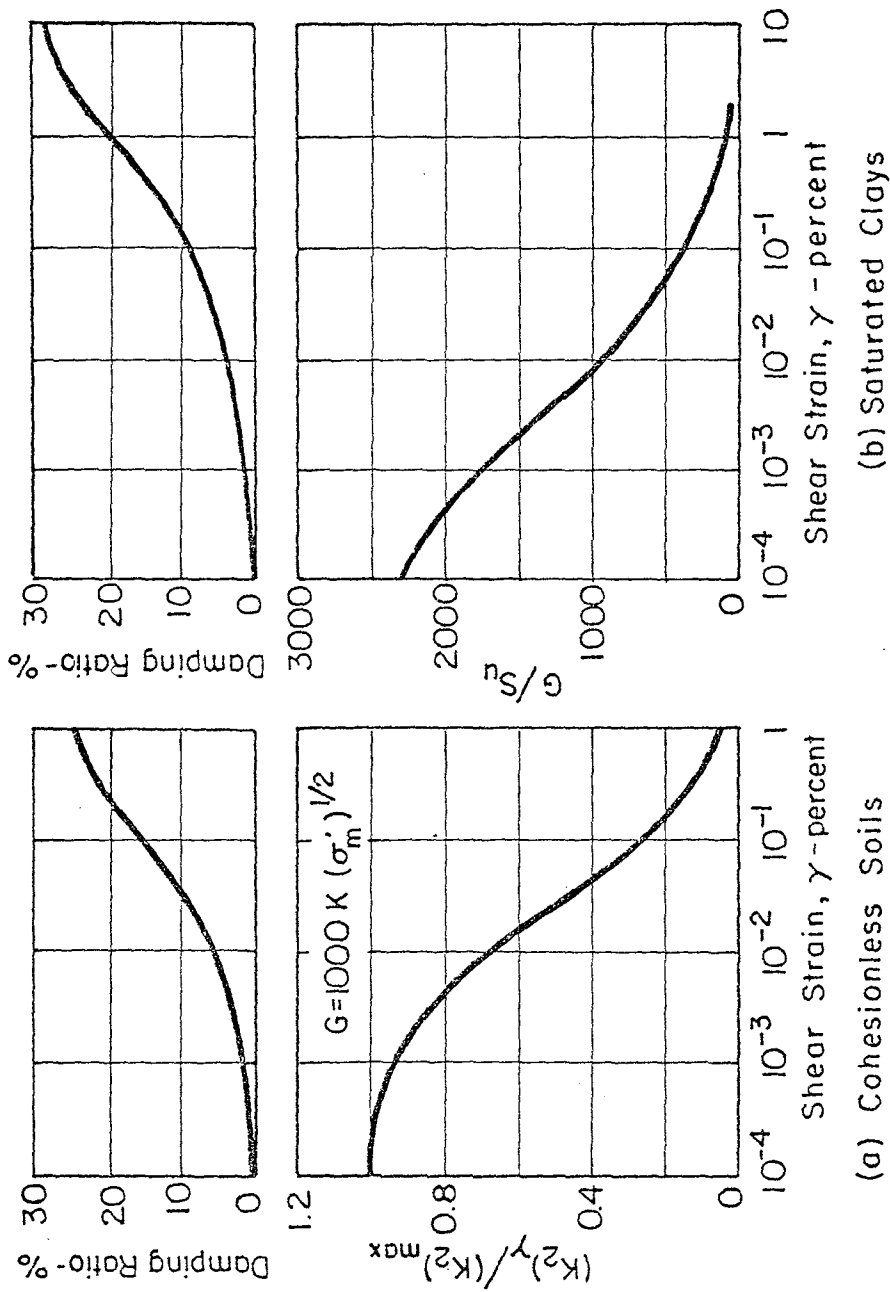


FIG. 2 AVERAGE SHEAR MODULI AND DAMPING CHARACTERISTICS OF SOILS

(After Seed and Idriss, 1970)



4. Compute the effective shear strains for all elements.
5. Determine new strain compatible values of shear modulus and damping by using the built in relationships of Fig. 2 or others supplied externally.
6. Repeat steps 2 through 5 until compatibility between the material properties used in the analysis and the computed shear strains is reached.

### 2.5 Effective Shear Strain

The three-dimensional strain state at a point can be described by a second order strain tensor with 6 independent terms which in a Cartesian coordinate system can be written as:

$$\underline{\varepsilon} = \begin{bmatrix} \varepsilon_{xx} & \varepsilon_{xy} & \varepsilon_{xz} \\ \varepsilon_{yx} & \varepsilon_{yy} & \varepsilon_{yz} \\ \varepsilon_{zx} & \varepsilon_{zy} & \varepsilon_{zz} \end{bmatrix} \quad (13)$$

During the seismic response of a dam the strain state varies with time at each point and therefore the selection of a single value to represent both the directional and time variation of strain level is difficult.

Soils are materials with a complex constitutive behavior which, strictly speaking, depends on all terms in expression (13). However, the state of present knowledge on constitutive laws for soils does not permit a complete characterization of material behavior. The method proposed by Seed and Idriss (1970) which consists of using the maximum shear strain as the parameter indicative of strain level is used in the computer program TLUSH. As a consequence the strain level will be given by an effective shear strain defined as:

$$\gamma_{\text{eff}} = 0.65 \cdot \max_t |\gamma_{\text{max}}| \quad (14)$$





Time variation in the strain level is taken into account by the factor 0.65 which is purely empirical but is assumed to be representative of an average time value of the maximum shear strain. The value of this factor is not critical in view of the fact that the computed motions are not very sensitive to moderate variations in the magnitude of the effective shear strain.

The peak maximum shear strain used in equation (14) can be computed by either of two methods available as two options of the computer program TLUSH. The first method consists of obtaining the maximum and minimum principal strains from a solution to the following equation for each time step.

$$\begin{vmatrix} \epsilon_{xx} - \epsilon & \epsilon_{xy} & \epsilon_{xz} \\ \epsilon_{yx} & \epsilon_{yy} - \epsilon & \epsilon_{yz} \\ \epsilon_{zx} & \epsilon_{zy} & \epsilon_{zz} - \epsilon \end{vmatrix} = 0 \quad (15)$$

Once the principal strains have been determined at each time step the peak maximum shear strain can easily be found. The second method makes use of the fact that the peak maximum shear strain can be estimated from the root-mean-square value of the maximum shear strain as follows:

$$\max_t |\gamma_{\max}| \approx C \cdot \text{RMS} (\gamma_{\max}) \quad (16)$$

where C is a constant approximately given by:

$$C = \max_t |\ddot{y}| / \text{RMS}(\ddot{y}) \quad (17)$$

The root-mean-square values of  $\gamma_{\max}$  and  $\ddot{y}$  can be conveniently evaluated by making use of Parseval's identity which for an arbitrary function  $f(t)$  is given by:



$$\text{RMS}^2(f) = \frac{1}{2} \sum_{s=0}^{N/2} |F_s|^2 \quad (18)$$

where  $F_s$ ,  $s=0,1,2, N/2$  are the complex Fourier amplitudes of the function  $f(t)$ , that is

$$f(t) = \text{Re} \sum_{s=0}^{N/2} F_s \exp(i\omega_s t) \quad (19)$$

This method is substantially faster than the first one since equation (15) can be solved in terms of the strain amplitudes for each frequency and all computations can be performed in the frequency domain without the need for numerous Fourier transforms.

### 3. PROGRAM DESCRIPTION

#### 3.1 Program Structure

The computer program TLUSH is written in standard FORTRAN IV language and the CDC 7600 version consists of a main program TLUSH and 42 subroutines which will be described under the next subheading. The program is easily convertible to most modern computer systems that have more than 100K words of high speed storage. It has been designed so that it can operate in any of three modes all of which produce a listing of the relevant input data. The type of mode to be used in a particular run can be specified by the value of the variable NOPT on the first input data card.

#### MODEL - The Initiation Mode

MODEL is the mode to be specified when running the program for the first time on a particular problem. In this mode all input comes from cards; the program will set up the equations of motion and solve them for



a specified number of iterations using in each iteration after the first, material properties compatible with the strains computed in the previous iteration. After the last iteration any output desired will be generated. A special option permits the creation of a permanent record on a magnetic tape, TAPE1, of all input data and of the complete solution. This information can subsequently be easily recovered in MODE2 or MODE3 runs. An additional option permits the storage of the element mass matrices, the normalized element stiffness matrices and the load vector on a magnetic tape, TAPE5, in this way making it possible to avoid their generation in subsequent MODE1 or MODE2 runs.

#### MODE2 - The Iteration Mode

In order to monitor the convergence of strain compatible properties in real size jobs it is convenient to perform the iterations on soil properties one at a time. In this way it is possible to speed up the rate of convergence of the solution and to make more efficient use of the program. MODE2 provides the re-start capability required to perform further iterations if necessary. In this mode the contents of TAPE1 are read and copied onto TAPE2, the equations of motion are set up and solved using the improved soil properties computed in the previous iteration, additional iterations can be performed, and any desired output can be generated. Just as in MODE1 a permanent record of the complete solution can be stored on TAPE1 for further reference. TAPE2 serves as a back-up in case the contents of the original TAPE1 need to be preserved.

#### MODE3 - The Extraction Mode

This mode is useful for the extraction of additional information on the solution to a certain problem which has been stored on TAPE1. In this



mode, the program will read the TAPE1 generated in previous MODEL or MODE2 runs and produce the additional output specified, without performing any finite element computations. Hence, the contents of TAPE1 are unaltered in this mode.

#### Data Check Run

An additional mode is provided to permit the revision of the input data for MODEL. All input data is read from cards, just as in MODEL, and this information is printed out.

From the previous discussion it follows that the main difference between MODEL and MODE2 is the form in which the input data is read in. Both modes follow approximately the same numerical procedure which can be summarized in the following steps:

1. Read in control data and set up dynamic storage allocation.
2. Read in finite element model data and generate missing information.
  - a. Element data.
  - b. Nodal point data.
3. Read in earthquake motion, transform to frequency domain and apply base line correction.
4. Print out all input data.
5. Form element mass matrices, normalized element stiffness matrices and load vector.
6. Compute element stiffness matrices.
7. Set up blocks and profile for equation solution.
8. Form total mass and stiffness matrices by blocks.
9. Solve equation of motion (Eq. 10) for selected frequencies and store amplification functions.





10. Compute strain amplification functions for all elements.
11. Determine effective shear strains and strain compatible soil properties for all elements.
12. Repeat from step 6, using the improved soil properties, for the number of iterations specified.
13. Generate all nodal point and stress output.
14. Stop.

The above numerical procedure is schematically illustrated in Fig. 3 which shows a simplified flowchart of the program.

### 3.2 Description of Routines

The CDC 7600 version of TLUSH consists of a main program and 42 subroutines which carry out the numerical procedure previously described. Additionally there are three CDC system routines, LOCF, SECOND and SETFLS, which are called by the program subroutines, LOC, TIME and MCORE respectively. The calling sequence for the program subroutines along with the tapes referenced by each routine is given in Table 1. A brief description of each routine follows:

TLUSH	This is the main program. It consists mostly of comment cards that describe the input data and it calls the initiation sub-routine INPT.
ADJUST	This subroutine applies base line correction in the frequency domain to the input acceleration time history.
ARRNG	It establishes the order in which the element mass and stiffness matrices will be generated.



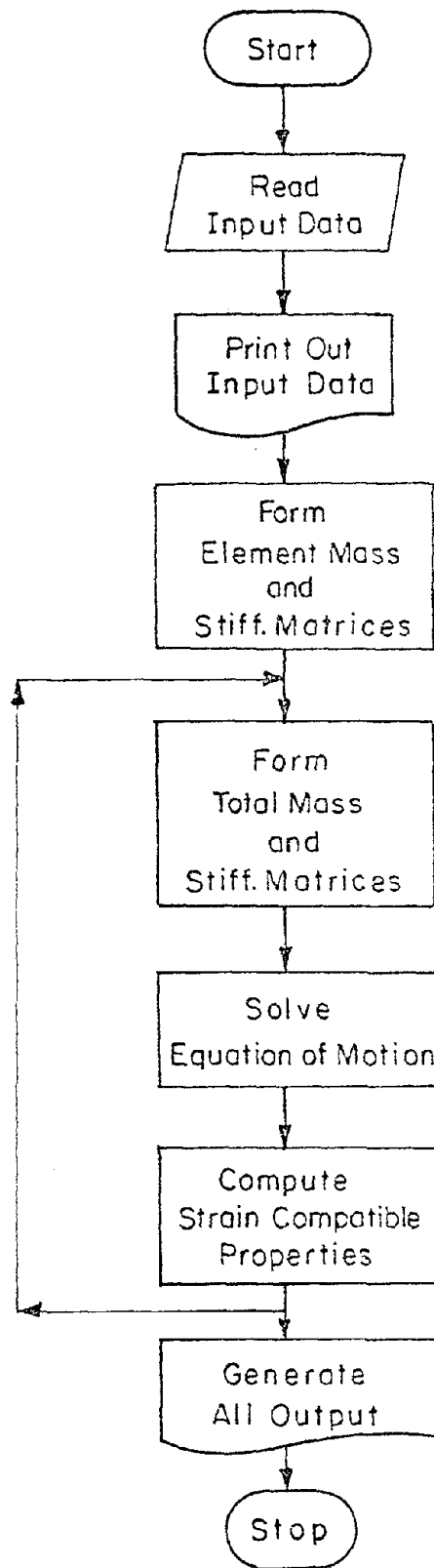


FIG. 3 · SIMPLIFIED FLOWCHART FOR TLUSH



Table 1. Calling Sequence for Subroutines and Tapes

Program	Calls	Called by	Tapes Referenced
TLUSH	INPT,SECOND	This is the main program	
ADJUST		VEDDA	
ARRNG		VEDDA	
BLOCK	RDSTF	GSTIF	6,7,8
CALBN	RDSTF	FRMSTF	5
CMPMAX		DRCTSP	
CNSTNTS	CSOLVE	INSET	
CSOLVE		CNSTNTS	
CURV52		STRAIN	
DRCTSP	CMPMAX,PLOT	MOTION	
ELSTRN		STRNAP	
EXPND		INSET	
FCONTL		INPT	
FFT		RFFT,RFSN	
FORM	LUMP	FRMSTF	
FRMSTF	CALBN,FORM,RDSTF	SOLVEB	5,6,10
FSET		MOTION	
GSTIF	BLOCK,TOTSTF	SOLVEB	
INPT	FCONTL,MCORE,VEDDA	TLUSH	1,2
INSET	CNSTNTS,EXPND	INTPL	
INTPL	INSET	MOTION,STRAIN	
LOC	LOCF	MCORE	
LUMP	VOLUME	FORM	



Table 1. (continued)

Program	Calls	Called by	Tapes Referenced
MCORE	LOC, SETFLS	INPT, SOLVEB, VEDDA	
MOTION	DRCTSP, FSET, INTPL PLOT, PROUT, RFSN TIME	VEDDA	3
PLOT		DRCTSP, MOTION, STRESS	
PRINT		VEDDA	
PROUT		MOTION	
RDSTF		BLOCK, CALBN, FRMSTF TOTSTF	
RFFT	FFT	VEDDA	
RFSN	FFT	MOTION, STRAIN, VEDDA	
ROOTME		STRNAP	
RW		SLOWR, SOLVEB, STRNAP TOTSTF, VEDDA	
SHMAX		STRAIN	
SLOWR	RW	SOLVEB	7,9,11,13,14 15,19,20,21,22
SOLVEB	FRMSTF, GSTIF, MCORE RW, SLOWR, TIME	VEDDA	1,12,9
STRAIN	CURV52, INTPL, RFSN SHMAX, TIME	VEDDA	4,17,18
STRESS	PLOT, TIME	VEDDA	17
STRNAP	ELSTRN, ROOTME, RW	VEDDA	1,3,4,18,19
TIME	SECOND	MOTION, SOLVEB, STRAIN STRESS, VEDDA	





Table 1. (continued)

Program	Calls	Called by	Tapes Referenced
TOTSTF	RDSTF, RW	GSTIF	6,7,8,9 10,11
VEDDA	ADJUST, ARRNG, MCORE MOTION, PRINT, RFFT RFSN, RW, SOLVEB STRAIN, STRESS, STRNAP, TIME	INPT	1,2,3 12,16
VOLUME		LUMP	



BLOCK Sets up the equation profile and the blocking of the global mass and stiffness matrices for the out-of-core solver.

CALBN Reorders the rows and columns in the element mass and stiffness matrices.

CMPMAX Determines the maximum values in the computation of response spectra.

CNSTNTS Sets up the five equations necessary for the determination of the constants in the interpolation function.

CSOLVE Determines the five constants in the interpolation function by solving a system of simultaneous equations.

CURV52 Computes updated strain-compatible soil properties from material curves.

DRCTSP Computes and plots acceleration and velocity response spectra.

ELSTRN Computes strain application functions at the centroid of an element.

EXPND Determines intermediate values in the amplification functions by interpolation.

FCONTL Determines the frequencies for which the equations of motion are to be solved.

FFT This is a standard routine for complex Fast Fourier Transform.

FORM Forms the mass and normalized stiffness matrices for an element.



FRMSTF Generates the element mass and stiffness matrices and the load vector.

FSET Determines the frequencies at which response spectral values are to be computed and sets the time array for plotting.

GSTIF Drives the routines that set up the equation profile, calculate blocking information and assemble the global mass and stiffness matrices.

INPT Reads control data from cards or TAPE1, writes this information on TAPE1 or TAPE2 and sets up field lengths and dynamic storage allocation.

INSET Drives the interpolation routines which compute the interpolation function and expand the amplification function.

INTPL Prepares amplification functions for interpolation and controls interpolation procedure.

LOC Calls the system routine LOCF to find out the address of a certain variable.

LUMP Computes the lumped mass matrix for an element.

MCORE Calls the system routine SETFLS which sets the core memory field length.

MOTION Controls the output at specified nodal points. Computes and plots amplification functions, response spectra and acceleration time histories.



PLOT Performs printer plotting of amplification functions, response spectra, acceleration time histories and stress time histories.

PRINT Prints out all input data.

PROUT Prints out the peak accelerations at specified nodal points.

RDSTF Controls reading and writing of element mass and stiffness matrices.

RFFT This is a standard routine for Fast Fourier Transform from the time domain into the frequency domain.

RFSN This is a standard routine for Fast Fourier Transform from the frequency domain into the time domain.

ROOTME Computes the square of the root-mean-square of the maximum shear strain for an element in the frequency domain.

RW Controls the reading, writing, rewinding and back-spacing of tapes.

SHMAX Computes the maximum shear strain at the centroid on an element in the time domain.

SLOWR This is an out-of-core equation solver for symmetric linear algebraic equations in complex variables. It uses the active column method of gaussian elimination.

SOLVEB Drives the routines that set up the equations of motion and solve them.





STRAIN     Determines effective shear strains and updated strain-compatible soil properties for all elements and computes stress time histories at specified points.

STRESS     Controls the output of element stress time histories.

STRNAP     Computes and writes strain amplification functions on tape and sets up amplification functions for subroutine MOTION.

TIME       Calls the system routine SECOND which gives the time since the start of execution.

TOTSTF     Assembles in blocks the global mass and stiffness matrices.

VEDDA      This is the main subroutine which controls the flow of the program. It reads the finite element input data, reads and modifies the control motion, generates most of TAPE1 and TAPE2, controls the solution of the equations of motion and the iteration on soil properties, and controls output from the program.

VOLUME     Computes the volume of a tetrahedron.

### 3.3 Tape Usage

The computer program TLUSH uses a total of 22 tapes of which, three may need to be specified as physical tapes depending on the options used in the program. TAPE1 should be specified as a physical tape in MODE1 and MODE3. TAPE1 and TAPE2 should be specified as physical tapes in MODE2. TAPE5 needs to be specified as a physical tape if permanent storage of the element matrices is required. All other tapes are usually simulated on



magnetic discs or drums. The following is a description of the contents of the tapes as used by the program.

- TAPE1 This is the principal input tape for MODE2 and MODE3. It contains all input data, estimated and current soil properties, and the computed amplification functions.
- TAPE2 This is a copy of TAPE1 produced in the initial stages of MODE2.
- TAPE3 Stores the computed amplification functions for nodal points where output is required.
- TAPE4 Stores the computed strain amplification functions for all elements.
- TAPE5 Is used to store element mass and normalized stiffness matrices and the load vector.
- TAPE6 Stores element mass and stiffness matrices and boundary conditions.
- TAPE7 Is used to store information necessary for block generation.
- TAPE8 Is used to store information on equation blocking.
- TAPE9 Is used to store information on equation blocking.
- TAPE10 Stores the load vector.
- TAPE11 Is used to store global mass and stiffness matrices in blocks.



- TAPE12 Is used as temporary storage of the permanent data of the blank common.
- TAPE13 Is used as a scratch file by the out-of-core equation solver.
- TAPE14 Is used as a scratch file by the out-of-core equation solver.
- TAPE15 Is used as a scratch file by the out-of-core equation solver.
- TAPE16 Stores the updated material properties and the root-mean-square values of the maximum shear strain for all elements.
- TAPE17 Is used to store element stress time histories to be output.
- TAPE18 Stores strain amplification functions for elements where stress time histories are required.
- TAPE19 Is used to store the displacement amplification functions for the frequencies used in the solution of the equations of motion.
- TAPE20 Is used as a scratch file by the out-of-core equation solver.
- TAPE21 Is used as a scratch file by the out-of-core equation solver.
- TAPE22 Is used as a scratch file by the out-of-core equation solver.

### 3.4 Error Messages

TLUSH has the capability of detecting some errors in the input data. Once an error is detected the program will stop execution and issue an error message. A description of the errors which can be detected by TLUSH follows:



- ERROR NO. 1     The number of points in the input acceleration time history, N3, exceeds the number of points specified for the Fast Fourier Transform, KGMAX.
- ERROR NO. 2     The specified small core length, NSCM, is too short for the blank common required in the initial stages of the program.
- ERROR NO. 3     The specified small core length, NSCM, is too short for the blank common required by subroutine STRAIN.
- ERROR NO. 4     The specified small core length, NSCM, is too short for the buffer used in subroutine STRAIN.
- ERROR NO. 5     The specified small core length, NSCM, is too short for the blank common required by subroutine MOTION.
- ERROR NO. 6     The specified small core length, NSCM, is too short for the buffer used in subroutine MOTION.
- ERROR NO. 7     The number of additional displacement boundary conditions, NBP, does not agree with the total number of boundary conditions specified on the nodal point cards.
- ERROR NO. 8     The computed volume of a finite element is less than or equal to zero.
- ERROR NO. 9     There is an equation number for which all the corresponding matrix coefficients are zeroes.
- ERROR NO. 10    There is an equation number for which the diagonal element of the global stiffness matrix is zero.





### 3.5 Core Memory

TLUSH requires about 110000 (octal) words of memory to load and about 76000 (octal) words plus the length of blank common in words to execute on a CDC 7600 computer using the FTN4 compiler. The program uses dynamic storage allocation and therefore it automatically assigns the required dimension or length for each variable used. Three lengths of blank common which are used by the three main subroutines, VEDDA, STRAIN and MOTION, are computed internally and printed out by the program. The largest of these three, determines the minimum field length required to run the program. As mentioned above, the minimum field length for execution is obtained by adding the blank common length to the core memory occupied by the program (76000 octal words on a CDC 7600 system). This length should be shorter than the maximum field length available on the system, which in the case of the BKY CDC 7600 computer is 170000 (octal) words.

The user must specify in the first control card, the maximum length of blank common to be used by the program, NSCM. This number should be greater than the minimum blank common requirements printed at the beginning of execution of the program. However, it is usually convenient to specify the largest blank common length compatible with the maximum field length available on the system. In this way, the out-of-core solver has the largest possible high-speed storage space to work with. The largest small core length that can be specified on a CDC 7600 computer using the FTN4 compiler, is 30500 (decimal) words. The program adjusts the field length to fit its need at each stage of execution and therefore no idle space is wasted.



Problems may be encountered in satisfying the blank common requirements of large jobs in medium sized computers (CDC 6400). In these cases the following suggestions might prove helpful in reducing the core memory requirements:

1. Select the largest mesh size compatible with accuracy requirements.
2. Keep the maximum frequency of the analysis as low as possible.
3. Do not specify extensive output in MODEL or MODE2 runs.
4. Use the lowest possible number of points in the Fast Fourier Transform.

#### 4. COMMENTS ON INPUT

##### 4.1 Mesh Size Requirements

Use of the finite element method for the dynamic response analysis of an earth dam requires discretization of the dam by a finite element model. The definition of the model boundaries and the selection of a general mesh layout are the two main ingredients in the design of a model.

Difficulties are encountered in the definition of the model boundaries and in the characterization of the earthquake motions at the boundaries of open systems such as earth dams. The simplifying assumptions that the boundary of the finite element model of an earth dam is rigid, and that the earthquake excitation acts on this rigid boundary, are implicit in the analytical approach used by TLUSH (see Fig. 1). Therefore, it is appropriate to select the boundary for the model of a dam at the physical boundary that best resembles this condition. In many cases this boundary is provided by a rock interface in the valley where the dam is located.



Another aspect of importance in the design of a model for the dynamic analysis of an earth dam is the selection of an adequate mesh layout. Two criteria, which in essence are equivalent, are available to determine an appropriate mesh layout for the dynamic analysis of earth dams. The first criterion requires that the mesh layout be such, that a good approximation of the predominant mode shapes of vibration of the dam can be obtained. The second criterion requires that the element size should not be larger than a certain fraction of the shortest wavelengths to be transmitted through the finite element mesh (Lysmer et al., 1974). The following expression is generally accepted:

$$h_{\max} = \frac{1}{5} \frac{V_s}{v_{\max}} \quad (20)$$

where  $h_{\max}$  is the vertical element dimension,  $V_s$  is the shear wave velocity in the element, and  $v_{\max}$  is the highest frequency of the analysis, TOTFR. Recent studies by Mejia (1981) have shown that, in general, 10 to 15 elements in the cross valley direction of a dam give a good representation, in this direction, of the predominant mode shapes of vibration of dams in triangular canyons subjected to strong shaking.

#### 4.2 Identification of Nodes and Elements

Once the mesh layout for the finite element model of a dam is selected, identification of the nodal points and elements is required. The nodal points of the model should be numbered in such a way that the average bandwidth of the global stiffness matrix is minimized. Additionally nodal points on the rigid boundary have to be numbered last (see Figs. 5 and 6). The user specifies the first nodal point on the rigid boundary,



NB1, and all points with higher numbers are considered by the program to be on this boundary.

The program is designed to work with eight node brick isoparametric elements. Therefore elements may have any shape with a maximum of 8 nodal points and straight edges between these points. Elements are also numbered and identification of the nodal points defining an element must follow the rules shown in Fig. 4. This figure additionally shows two examples which illustrate element identification. The order of nodal point numbers which define an element should be such that NP1, NP2, ..., NP8 on an element card correspond to the numbers 1, 2, ..., 8 in Fig. 4(a).

The program has the capability of generating data for nodal points and elements not included in the input data. Nodal point coordinates are generated by interpolation along a straight line connecting the nodes on two successive input data cards. Element data is generated by using the same material properties and incrementing the nodes on the previous element card a specified number, LX.

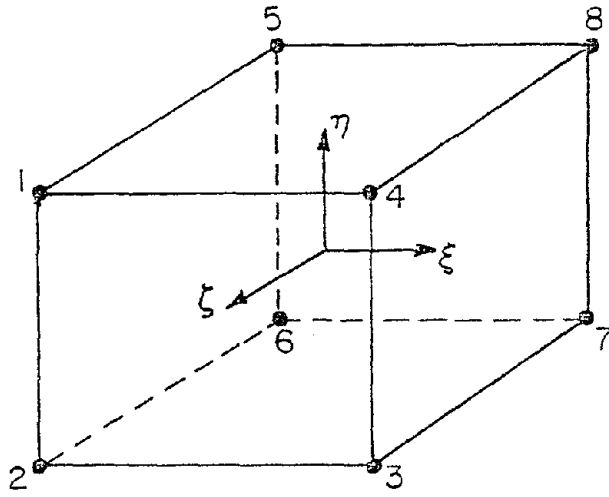
#### 4.3 Element Matrices

The computer program TLUSH generates the element stiffness and mass matrices in two steps. The first step consists of computing normalized matrices. In the second step these matrices are multiplied by the element complex shear moduli and density in order to obtain the element stiffness and mass matrices respectively.

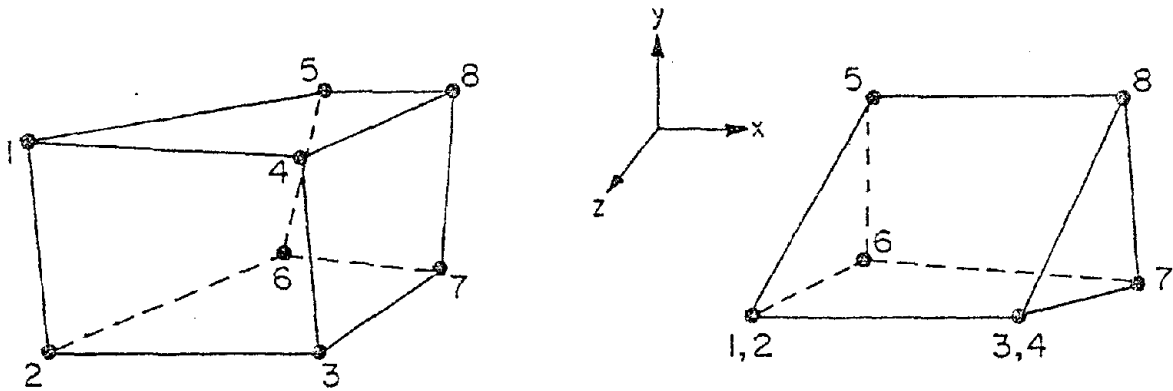
The normalized stiffness matrix for an element is computed following standard finite element procedures and corresponds to the element stiffness matrix for a shear modulus with a value of 1. It depends only on the geometry of the element and the value of Poisson's ratio. The







(a) Element Normalized Coordinate System



(b) Elements in Global Coordinate System

FIG. 4 ELEMENT IDENTIFICATION IN TLUSH



normalized mass matrix also depends only on the geometry of the element since it corresponds to the element mass matrix for a density with a value of 1. This matrix is computed as a combination of the consistent mass matrix and the lumped mass matrices for the element, according to the following expression:

$$[M] = \text{RATIA} * [M]^C + (1 - \text{RATIA}) [M]^L \quad (21)$$

where the parameter RATIA may range between 0 and 1.0 and is specified by the user. Values of RATIA between 0.5 and 0.75 optimize the ability of the element to transmit high frequencies.

It follows that elements which have the same geometry and value of Poisson's ratio will have identical normalized element matrices. To take advantage of this fact, an option in TLUSH permits normalized matrices to be duplicated for elements that have the same identification number, IDEL(N), different than zero.

The normalized element matrices are multiplied by the complex shear modulus:

$$G^* = G(1 - 2\beta^2 + 2i\beta \sqrt{1 - \beta^2}) \quad (22)$$

and the density,  $\rho$ , in order to obtain the element stiffness and mass matrices respectively. The parameter  $\beta$  corresponds to the element fraction of damping.

There are options in TLUSH which permit the storage of element mass and normalized stiffness matrices on TAPE5 and the recovery of these matrices in future runs. Use of these options are likely to be helpful in saving computer time and costs in some systems.



#### 4.4 Material Curves

As mentioned before TLUSH uses the equivalent linear method to arrive at an approximate non-linear solution. In order to perform the successive iterations on soil properties the program uses material curves that describe the relationships between normalized shear modulus and damping, and cyclic maximum shear strain for the types of soil composing the dam. Two options are available in the program for defining the material property curves for a certain problem and any of these options can be selected by appropriately specifying the parameter KMATYP.

One option permits the use of built-in material curves. These curves correspond to those proposed by Seed and Idriss (1970) for clays and sands (see Fig. 2). In the other option any number, MATYP, of such relationships may be read from input data cards. Each set of relationships for modulus and damping must have a sequence number. This material identification number is referenced as element data in the parameter MTYPE(N) for those elements composed of a particular material. Strain independent properties can be assigned to any element by specifying a value of 0 for MTYPE(N).

#### 4.5 Shear Strain Computation

As mentioned before two methods are available within TLUSH for the computation of the peak maximum shear strain. The first method uses a time domain approach. The time histories of the 6 components of strain at the centroid of each element are obtained from the inverse Fast Fourier Transform of the Fourier amplitudes of the strain components. A cubic equation in terms of the 6 components of strain at each time step is solved for the principal strains. From these, the maximum shear



strain is obtained for each time step and the peak value can be determined by direct comparison. The second method uses a frequency domain approach. Using the Fourier complex amplitudes of the 6 strain components at the centroid of each element, a cubic equation is set up for each frequency in the spectrum. From these equations the complex amplitudes of principal strains are obtained from which the amplitudes of the maximum shear strain can be determined. These amplitudes are used to compute the root-mean square value of the maximum shear strain function from which an approximation to the peak value of the function can be obtained.

Although the first method yields the theoretically exact value for the peak maximum shear strain it is much more lengthy than the second method since it involves numerous Fast Fourier Transforms. In addition to using a statistical procedure for determining the peak value, the second method uses a technique which assumes linearity to determine a nonlinear parameter such as the maximum shear strain. For these reasons the peak maximum shear strains computed by this method are different than the exact values. However, the final differences in the strain compatible soil properties computed by the two methods are not likely to be greater than 20%.

By specifying the input parameter KSTRN the user can select any of the following three options: a) No strain computation, b) Strain computation in the time domain and, c) Strain computation in the frequency domain. Additionally, the user must supply the value of the ratio between effective shear strain and peak maximum shear strain, FCT. It must be noted that in order to obtain output for stress time histories, any of the two strain computation options must be specified.





#### 4.6 Frequency Controls

The computer program TLUSH has several frequency controls which permit the user to select the highest frequencies for which the equations of motion are solved. One of these parameters is the cut-off frequency or maximum frequency of the analysis, TOTFR. No computations are performed by the program for frequencies above this value. That is, the displacement amplification functions are assigned a zero value and the complex Fourier amplitudes of displacement, acceleration and strain are assumed to be zero for frequencies above the cut-off frequency.

It follows that the maximum frequency of the analysis, TOTFR, should be higher than the natural frequencies corresponding to the predominant modes of vibration of the structure. That is, the response of the dam to motions at frequencies higher than the cut-off frequency should be negligible. An estimate of an appropriate value for the maximum frequency to be used in the dynamic analysis of an earth dam is given by a number 4 or 5 times higher than the fundamental frequency of vibration of the dam. Typical values for medium sized dams range between 8 and 10 Hz.

In view of the fact that a high degree of accuracy is not required during the first iterations on material properties the use of lower cut-off frequencies in these iterations is justified. TLUSH allows the user to specify a cut-off frequency for each iteration, STEP(I), and this permits the use of low cut-off frequencies in the initial stages of the solution.

#### 4.7 Interpolation

It was mentioned earlier that in order to reduce computational effort



and cost the equations of motion are solved only for a small number of selected frequencies, and a special interpolation scheme is used by the program to obtain intermediate values of the complex amplitudes of the displacement and strain amplification functions. The interpolation technique used by TLUSH has been previously described.

The frequencies for which the equations of motion are solved must be specified by the user. It is desirable to select the minimum number of frequencies that will yield an adequate picture of the displacement and strain amplification functions at all points in the dam. This number of frequencies will depend on the shape of the amplification functions and the distribution or spacing of the frequencies selected. A close spacing between solution frequencies should be selected in the frequency ranges where a high degree of accuracy is required and where the amplification functions have a complex shape; that is, a great number of frequency peaks. It is useful to remember that the transfer function for a two-degree-of-freedom system is used to interpolate on the amplification functions and that 5 frequency points are needed to define such function. Thus, normally 5 frequency points are needed in the vicinity of two single frequency peaks to get accurate interpolation of the amplification function values. A greater number of frequency points is needed if there are nearby frequency peaks (Tajirian, 1981). In general, a closer spacing between solution frequencies is needed in the vicinity of the predominant frequencies of vibration of a dam while a lower density may be used in the high frequency range. Also, 30 to 40 points usually suffice to adequately define the amplification functions at most points in a dam. However, the best way to determine an appropriate distribution of solution



frequencies is to carry out the iterations on soil properties one at a time and to have the amplification functions for several points printed out after each iteration.

TLUSH allows the user to specify a maximum number of 5 interpolation ranges. For each range the maximum frequency in that range, FPKINT(I), and the interpolation gap, KINT(I), must be specified. Typical values for KINT(I) vary between 4 and 16.

#### 4.8 Printer Plots

The following printer plots may be obtained from TLUSH:

1. Time histories of nodal point accelerations.
2. Acceleration and velocity response spectra for specified nodal point acceleration time histories.
3. Fourier spectra of acceleration time histories at specified nodal points.
4. Acceleration amplification functions for specified nodal points.
5. Time histories of the 6 components of the stress tensor at the centroid of specified elements.

The user may control the length and degree of refinement of these plots by specifying a value for the parameter NSKIP. The plotting routine will skip the specified number of points in between displayed points. It is important to mention that all accelerations output by the program are absolute accelerations and that the program has the capability of performing baseline correction on the input motions. The effect of baseline correction is small on the computed accelerations; however, it is rather significant on the corresponding velocities and displacements. The use of this option is recommended since it frees the computed motions of



integrated errors in the input motions and does not involve costly calculations.

#### 4.9 Punched Output

The following punched output can be generated by TLUSH:

1. Element data cards with improved strain-compatible soil properties.
2. Acceleration time histories at specified nodal points.
3. Time histories of the stress components at the centroid of specified elements.

It is useful to have the program punch element cards when it is desired to perform a new analysis with a slightly different finite element model. All data on the element cards is punched in the same format as that required for input to the program. The material properties punched are those computed in the last iteration.

Acceleration time histories are punched for the entire duration of the analysis, and at the time interval and with the same format used for the input motions (8F9.6). Stress time histories are punched in psf and with a format (8E9.3).

#### 4.10 Execution Time

It is difficult to estimate a priori the execution time of TLUSH for a given job since this time is a function of many parameters including the speed of the computer used, the available high speed storage capacity, the geometry of the finite element mesh, and the number of output options requested. It is convenient, therefore, to perform a trial run for one or two gaussian eliminations in order to estimate the execution time for a





real size job. This can easily be done since execution time for each gauss elimination is printed out by the program and the total number of eliminations can be estimated a priori. In general, three fifths to four fifths of the total execution time are spent in the solution of the equations of motion.

The following suggestions will prove to be helpful in reducing the execution time of the program:

1. Keep the maximum frequency of the analysis as low as possible.
2. Use interpolation control parameters, KINT(I), as large as possible.
3. Select the largest possible mesh size compatible with accuracy requirements.
4. Use the minimum possible number of points, KGMAX, for the Fast Fourier Transform.
5. Compute maximum shear strains by the RMS procedure.
6. Use option to duplicate element matrices.
7. Compute as few response spectra as possible in MODEL or MODE2 runs.



## 5. LISTING OF MAIN PROGRAM



PROGRAM TLUSH (INPUT, OUTPUT, PUNCH, TAPE1, TAPE2, TAPE3, TAPE4, TAPE5,	TLUS	1
1 TAPE6, TAPE7, TAPE8, TAPE9, TAPE10, TAPE11, TAPE12, TAPE13, TAPE14,	TLUS	2
2 TAPE15, TAPE16, TAPE17, TAPE18, TAPE19, TAPE20, TAPE21, TAPE22)	TLUS	3
-----		
C A COMPUTER PROGRAM FOR THE COMPLEX RESPONSE ANALYSIS OF	TLUS	4
C THREE DIMENSIONAL EARTH DAMS BY THE FINITE ELEMENT METHOD.	TLUS	5
C STRAIN-COMPATIBLE SOIL PROPERTIES BY THE EQUIVALENT LINEAR METHOD.	TLUS	6
C ANY COMBINATION OF LUMPED AND CONSISTENT MASS MATRIX.	TLUS	7
C INPUT ACCELERATION AT RIGID BOUNDARY OF FINITE ELEMENT MODEL.	TLUS	8
-----		
C OPERATION MODES - THE PROGRAM CAN OPERATE IN THREE MODES	TLUS	10
C MODE1 - IN THIS MODE, ALL INPUT DATA IS FROM CARDS. THE PROGRAM	TLUS	11
C WILL COMPUTE THE COMPLETE RESPONSE. IT WILL ITERATE A	TLUS	12
C SPECIFIED NUMBER OF TIMES TO OBTAIN STRAIN-COMPATIBLE	TLUS	13
C SOIL PROPERTIES. ANY OUTPUT CAN BE SPECIFIED IN THIS	TLUS	14
C MODE. DURING EXECUTION, ALL INPUT DATA AND THE FINAL	TLUS	15
C RESPONSE IN THE FREQUENCY DOMAIN OF ALL NODAL POINTS IS	TLUS	16
C WRITTEN ON TAPE 1 WHICH IS THEREFORE A COMPLETE PERMANENT	TLUS	17
C RECORD OF PROBLEM AND SOLUTION.	TLUS	18
C MODE2 - IN THIS MODE, THE CONTENT OF TAPE 1 WILL BE COPIED ONTO	TLUS	19
C TAPE 2 AND THE ADDITIONAL ITERATIONS ON SOIL PROPERTIES	TLUS	20
C CAN BE PERFORMED. THE CONTENT OF TAPE 1 WILL BE UPDATED	TLUS	21
C FOR THE FOLLOWING ITERATIONS.	TLUS	22
C MODE3 - IN THIS MODE THE INFORMATION ON TAPE 1 CAN BE RECOVERED	TLUS	23
C TO GENERATE ADDITIONAL OUTPUT WITHOUT REPEATING THE COSTLY	TLUS	24
C FINITE ELEMENT PROCEDURE USED IN MODE1 OR MODE2.	TLUS	25
-----		
C INPUT DATA	TLUS	26
-----		
C 0. OPERATION MODE CARD (15,110)	TLUS	28
C 1- 5 NOPT IF 0 STOP ( LAST DATA CARD )	TLUS	29
C IF 1 MODE1	TLUS	30
C IF 2 MODE2	TLUS	31
C IF 3 MODE3	TLUS	32
C IF 4 DATA CHECK RUN	TLUS	33
C 6-15 NSCM AVAILABLE SMALL CORE LENGTH IN DECIMAL	TLUS	34
-----		
C INPUT DATA FOR MODE1	TLUS	35
-----		
C 1. JOB IDENTIFICATION CARD (12A6, 18)	TLUS	37
C 1-72 TITLE (12) JOB IDENTIFICATION	TLUS	38
C 73-80 ITER RUN NUMBER - IDENTIFICATION ONLY	TLUS	39
-----		
C 2. CONTROL CARD FOR JOB SIZE OF SYSTEM (815)	TLUS	40
C 1- 5 NELM TOTAL NUMBER OF ELEMENTS	TLUS	41
C 6-10 NDPT TOTAL NUMBER OF NODAL POINTS	TLUS	42
C 11-15 NB1 FIRST NODAL POINT NUMBER ON RIGID BOUNDARY	TLUS	43
C I.E. ALL NODAL POINTS WITH HIGHER NUMBERS ARE	TLUS	44
C ASSUMED TO BE ON RIGID BOUNDARY	TLUS	45
C 16-20 NBP NUMBER OF ADDITIONAL DISPLACEMENT BOUNDARY	TLUS	46
C CONDITIONS.	TLUS	47
C NOTE***EVERY DEGREE OF FREEDOM SUPRESSED	TLUS	48
C COUNTS ONE	TLUS	49
C 21-25 NUMBER NUMBER OF ITERATIONS ON SOIL PROPERTIES	TLUS	50
C ( INITIAL SOLUTION COUNTS ONE )	TLUS	51
C 26-30 MATYP TOTAL NUMBER OF STRAIN-DEPENDENT MATERIALS	TLUS	52
C 31-35 KMATYP =0 READ MATERIAL CURVES FROM CARDS	TLUS	53
C =1 USE STANDARD BUILT IN MATERIAL CURVES	TLUS	54
C 36-40 NFORM 0 -- FORM MATRICES AND LOAD VECTOR	TLUS	55
C 1 -- FORM MATRICES AND LOAD VECTOR	TLUS	56
C WRITE ON TAPE 5	TLUS	57
C	TLUS	58
C	TLUS	59
C	TLUS	60
C	TLUS	61



C		2 -- READ MATRICES AND LOAD VECTOR FROM TAPE 5	TLUS	62
C		WHEN NFORM=1 OR 2, TAPES SHOULD BE A PHYSICAL	TLUS	63
C		TAPE	TLUS	64
C			TLUS	65
C	3. CONTROL CARD FOR	INPUT MOTION (2I5,3F10.4,I5)	TLUS	66
C	1- 5 KGMAX	TOTAL NUMBER OF POINTS USED IN FAST FOURIER	TLUS	67
C		TRANSFORMS, MUST BE A POWER OF TWO.	TLUS	68
C	6-10 N3	LAST POINT OF EARTHQUAKE RECORD USED IN ANALYSIS	TLUS	69
C	11-20 DT	TIME STEP OF DIGITIZED EARTHQUAKE RECORD. -SEC.	TLUS	70
C	21-30 EQMUL	EARTHQUAKE MULTIPLICATION FACTOR. IF RECORD IS	TLUS	71
C		CONTROLLED BY UGMAX, LEAVE BLANK.	TLUS	72
C	31-40 UGMAX	MAX. INPUT ACCELERATION USED IN ANALYSIS. -G	TLUS	73
C		IF EQMUL IS NOT ZERO, LEAVE BLANK.	TLUS	74
C	41-45 KBLINE	=1 APPLY BASELINE CORRECTION TO INPUT MOTION	TLUS	75
C		=0 DO NOT APPLY BASELINE CORRECTION	TLUS	76
C			TLUS	77
C	4. CONTROL CARD FOR	DIRECTION OF INPUT MOTION (3F10.4)	TLUS	78
C		THE ABOVE INPUT ACCELERATION ON THE RIGID BASE	TLUS	79
C		CAN BE SPECIFIED TO ACT IN ANY DIRECTION	TLUS	80
C		ACCORDING TO THE RULE	TLUS	81
C		X COMPONENT = HORX * ABOVE MOTION	TLUS	82
C		Y COMPONENT = VERT * ABOVE MOTION	TLUS	83
C		Z COMPONENT = HORZ * ABOVE MOTION	TLUS	84
C		HORX, VERT AND HORZ ARE ENTERED AS FOLLOWS	TLUS	85
C	1-10 HORX	FACTOR FOR X MOTION	TLUS	86
C	11-20 VERT	FACTOR FOR Y MOTION	TLUS	87
C	21-30 HORZ	FACTOR FOR Z MOTION	TLUS	88
C			TLUS	89
C	5. CONTROL CARDS FOR	FREQUENCY OF ANALYSIS.	TLUS	90
C	5A. MASTER CONTROL	(F10.4, I5)	TLUS	91
C	1-10 TOTFR	HIGHEST FREQUENCY TO BE CONSIDERED IN ANALYSIS.	TLUS	92
C		-HZ ( USED FOR DIMENSIONING ONLY, THE ACTUAL	TLUS	93
C		FREQUENCIES USED ARE SET BY THE VARIABLE	TLUS	94
C		STEP(I), SEE PT. 9 BELOW )	TLUS	95
C	11-15 KINTS	NUMBER OF VARIABLE INTERPOLATION RANGES (MAX=5)	TLUS	96
C	5B. (KINTS) CARDS	(F10.4, I5)	TLUS	97
C	1-10 FRKINT(I)	MAX. FREQ. IN INT. RANGE I	TLUS	98
C	11-15 KINT(I)	INT. CONTRCL IN INT. RANGE I	TLUS	99
C			TLUS	100
C	6. CONTROL CARD FOR	OUTPUT (7I5)	TLUS	101
C	1- 5 KDISP	IF 1, SAVE INPUT DATA AND THE COMPUTED AMPLIFIC-	TLUS	102
C		ATION FUNCT. ON TAPE 1.	TLUS	103
C	6-10 KSTRN	=0, NO STRAIN COMPUTATION	TLUS	104
C		=1, COMPUTE MAX. SHEAR STRAIN IN TIME DOMAIN TO	TLUS	105
C		ITERATE ON SOIL PROPERTIES	TLUS	106
C		=2, COMPUTE MAX. SHEAR STRAIN IN FREQ. DOMAIN TO	TLUS	107
C		ITERATE ON SOIL PROPERTIES	TLUS	108
C	11-15 KPNCH	IF 1, PUNCH ELEMENT CARDS WITH NEW SOIL PROPERT.	TLUS	109
C		FOR THE FOLLOWING ITERATION.	TLUS	110
C	16-20 NOUT	TOTAL NUMBER OF NODAL POINTS WHERE OUTPUT IS	TLUS	111
C		REQUIRED	TLUS	112
C	21-25 ND	NUMBER OF DAMPING VALUES FOR RESPONSE SPECTRA	TLUS	113
C	26-30 NSKIP	EVERY NSKIP-TH POINT OF TIME HISTORY IS PLOTTED	TLUS	114
C	31-35 NELS	NO. OF ELEMENTS AT WHICH STRESS TIME HISTORIES	TLUS	115
C		ARE ASKED	TLUS	116
C			TLUS	117
C	7. CONTROL CARD FOR	MASS MATRIX AND EFFECTIVE SHEAR STRAIN (2F10.4)	TLUS	118
C	1-10 RATIA	RATIO OF CONSISTENT MASS MATRIX TO FORM MASS	TLUS	119
C		THE MASS MATRIX USED IS (1.-RATIA) * LUMPED MASS	TLUS	120
C		MATRIX + RATIA * CONSISTENT MASS MATRIX + A	TLUS	121
C		LUMPED MASS MATRIX FORMED FROM THE CONCENTRATED	TLUS	122
C		MASSES SPECIFIED ON THE NODAL POINT CARDS (SEE	TLUS	123





C		PT.15 BELCW). TYPICAL VALUES ARE 0.5-0.75	TLUS 124
C	11-20 FCT	RATIO OF EFFECTIVE SHEAR STRAIN TO MAX. SHEAR	TLUS 125
C		STRAIN. (TYPICAL VALUES ARE 0.6 TO 0.7)	TLUS 126
C		USED FOR DETERMINING STRAIN COMPATIBLE SOIL	TLUS 127
C		PROPERTIES.	TLUS 128
C			TLUS 129
C	8. CONTROL CARD FOR DAMPING CHARACTERISTICS (F10.4)		TLUS 130
C	1-10 SDAMP	IF UNIFORM DAMPING IS USED IN ALL ELEMENTS,	TLUS 131
C		PUNCH THE UNIFORM DAMPING VALUE, OTHERWISE	TLUS 132
C		LEAVE BLANK. ( SIMULATION OF MODAL ANALYSIS )	TLUS 133
C		DAMPING VALUE MUST BE PUNCHED ON ELEMENT CARDS	TLUS 134
C			TLUS 135
C	9. CONTROL CARD FOR FREQUENCY OF ANALYSIS IN EACH ITERATION(8F10.4)		TLUS 136
C	1-10 STEP(1)	HIGHEST FREQUENCY OF ANALYSIS DURING FIRST	TLUS 137
C		ITERATION. -HZ	TLUS 138
C			TLUS 139
C		STEP(NUMBER)=HIGHEST FREQUENCY OF ANALYSIS FOR THE LAST	TLUS 140
C		ITERATION. - HZ(SHOULD BE EQUAL TO TOTFR)	TLUS 141
C			TLUS 142
C	10. CONTROL CARDS FOR RESPONSE SPECTRA.		TLUS 143
C		NOTE***SKIP THESE CARDS IF ND=0	TLUS 144
C	10A. FREQUENCY CONTROL (2F10.4, I5)		TLUS 145
C		IF THIS CARD IS BLANK THE STANDARD VALUES	TLUS 146
C		FSTRT=0.4,FLAST=40.,NINT=40 ARE ASSUMED	TLUS 147
C		THIS WILL LEAD TO A PLOT WHICH FILLS ONE PAGE.	TLUS 148
C	1-10 FSTRT	FIRST FREQUENCY USED IN RESPONSE SPECTRUM	TLUS 149
C		ANALYSIS. -HZ	TLUS 150
C	11-20 FLAST	LAST FREQUENCY USED IN RESPONSE SPECTRUM	TLUS 151
C		ANALYSIS. -HZ	TLUS 152
C	21-25 NINT	TOTAL NUMBER OF FREQUENCY STEP FOR RESPONSE	TLUS 153
C		SPECTRA.	TLUS 154
C	10B. DAMPINGS (8F10.4)		TLUS 155
C	1-10 DAMP(1)		TLUS 156
C		* DAMPING RATIOS FOR RESPONSE SPECTRA	TLUS 157
C		CAMP(ND)	TLUS 158
C			TLUS 159
C	11. NODAL POINT OUTPUT CONTROL CARDS (4I5) - NOUT CARDS		TLUS 160
C		NOTE***SKIP THESE CARDS IF NOUT=0.	TLUS 161
C	1- 5 M	NODAL POINT NUMBER WHERE OUTPUTS ARE REQUIRED.	TLUS 162
C		M MUST BE LESS THAN NBI	TLUS 163
C	6-10 KEYSFC(3M-2)	OUTPUT CONTROL IN X DIRECTION	TLUS 164
C	11-15 KEYSFC(3M-1)	OUTPUT CONTROL IN Y DIRECTION	TLUS 165
C	16-20 KEYSFC(3M)	OUTPUT CONTROL IN Z DIRECTION	TLUS 166
C			TLUS 167
C		CODES FOR KEYSFC - VALUES	TLUS 168
C		0--MAXIMUM ACCELERATION ONLY	TLUS 169
C		1--PLOT ACC. TIME HISTORY	TLUS 170
C		10--PLOT ACC. AND VELOCITY RESPONSE SPECTRUM	TLUS 171
C		100--PUNCH ACC. TIME HISTORY	TLUS 172
C		1000--PLOT FOURIER AMP. OF ACCELERATION	TLUS 173
C		10000--PLOT AMPLIFICATION FUNCT.	TLUS 174
C			TLUS 175
C		SUM THESE VALUES FOR SEVERAL OPTIONS	TLUS 176
C			TLUS 177
C	12. OUTPUT CONTROL CARDS FOR STRESSES -NELS CARDS (2I5)		TLUS 178
C	1- 5 NSTR(N)	ELEMENT NUMBER AT WHICH STRESS TIME HISTORY IS	TLUS 179
C		ASKED	TLUS 180
C	6-10 KEYSTR(N)	OUTPUT CONTROL OF STRESS TIME HISTORY	TLUS 181
C		CODES FOR KEYSTR	TLUS 182
C		1--PLOT TIME HISTORIES OF STRESSES	TLUS 183
C		10--PRINT TIME HISTORIES OF STRESSES	TLUS 184
C		100--PUNCH TIME HISTORIES OF STRESSES	TLUS 185



C			TLUS 186
C		SUM THESE VALUES FOR COMBINED OPTIONS	TLUS 187
C			TLUS 188
C	13. ELEMENT CARDS (9I4,3I2,F4.3,3F10.4,F4.3)		TLUS 189
C	1- 4 N	ELEMENT NUMBER	TLUS 190
C	5- 8 NP1(N)	SEQUENCE NO. OF NODAL POINT 1	TLUS 191
C	9-12 NP2(N)	SEQUENCE NO. OF NODAL POINT 2	TLUS 192
C	13-16 NP3(N)	SEQUENCE NO. OF NODAL POINT 3	TLUS 193
C	17-20 NP4(N)	SEQUENCE NO. OF NODAL POINT 4	TLUS 194
C	21-24 NP5(N)	SEQUENCE NO. OF NODAL POINT 5	TLUS 195
C	25-28 NP6(N)	SEQUENCE NO. OF NODAL POINT 6	TLUS 196
C	29-32 NP7(N)	SEQUENCE NO. OF NODAL POINT 7	TLUS 197
C	33-36 NP8(N)	SEQUENCE NO. OF NODAL POINT 8	TLUS 198
C	37-38 MTYPE(N)	MATERIAL TYPE NUMBER. 0 MEANS MATERIAL WITH STRAIN-INDEPENDENT PROPERTIES.	TLUS 199
C		IF KMATYP=1, 1 MEANS CLAY AND 2 MEANS SAND.	TLUS 200
C	39-40 LX	GENERATION INCREMENT	TLUS 201
C	41-42 IDEL(N)	IDENTIFICATION NO. OF THE ELEMENT	TLUS 202
C	43-46 PO(N)	POISSON'S RATIO. MUST BE LESS THAN .5	TLUS 203
C	47-56 DENS(N)	UNIT WEIGHT -PCF	TLUS 204
C	57-66 S3(N)	MAX. SHEAR MODULUS -KSF	TLUS 205
C	67-76 G(N)	INITIAL ESTIMATE OF SHEAR MODULUS -KSF	TLUS 206
C	77-80 XL(N)	INITIAL ESTIMATE OF FRACTION OF CRITICAL DAMPING	TLUS 207
C			TLUS 208
C		ELEMENT CARDS MUST BE IN NUMERICAL SEQUENCE. IF	TLUS 209
C		CARDS ARE OMITTED THE ELEMENT DATA WILL BE	TLUS 210
C		GENERATED WITH THE SAME MATERIAL PROPERTIES AND	TLUS 211
C		WITH NODES INCREMENTED BY LX ON THE PREVIOUS	TLUS 212
C		ELEMENT. LAST ELEMENT CARD MUST BE PROVIDED.	TLUS 213
C		ELEMENTS WITH THE SAME ID NO. (DIFFERENT THAN 0)	TLUS 214
C		SHOULD HAVE THE SAME GEOMETRY. NORMALIZED STIF.	TLUS 215
C		AND MASS MATRICES WILL THEN BE DUPLICATED.	TLUS 216
C			TLUS 217
C			TLUS 218
C	14. MATERIAL CURVES -MATYP GROUPS OF THREE CARDS		TLUS 219
C		NOTE***SKIP IF MATYP=0 OR KMATYP=1	TLUS 220
C	14A. IDENTIFICATION (I5,12A6)		TLUS 221
C	1- 5 N	MATERIAL TYPE NUMBER	TLUS 222
C	6-77 TTL	MATERIAL IDENTIFICATION	TLUS 223
C	14B. SHEAR MODULUS REDUCTION FACTORS (11F5.3)		TLUS 224
C	1- 5	REDUCTION FACTOR AT 10**(-4.0) PER CENT STRAIN	TLUS 225
C	6-10	REDUCTION FACTOR AT 10**(-3.5) PER CENT STRAIN	TLUS 226
C	11-15	REDUCTION FACTOR AT 10**(-3.0) PER CENT STRAIN	TLUS 227
C	16-20	REDUCTION FACTOR AT 10**(-2.5) PER CENT STRAIN	TLUS 228
C	21-25	REDUCTION FACTOR AT 10**(-2.0) PER CENT STRAIN	TLUS 229
C	26-30	REDUCTION FACTOR AT 10**(-1.5) PER CENT STRAIN	TLUS 230
C	31-35	REDUCTION FACTOR AT 10**(-1.0) PER CENT STRAIN	TLUS 231
C	36-40	REDUCTION FACTOR AT 10**(-0.5) PER CENT STRAIN	TLUS 232
C	41-45	REDUCTION FACTOR AT 10**(-0.0) PER CENT STRAIN	TLUS 233
C	46-50	REDUCTION FACTOR AT 10**( 0.5) PER CENT STRAIN	TLUS 234
C	51-55	REDUCTION FACTOR AT 10**( 1.0) PER CENT STRAIN	TLUS 235
C	14C. FRACTIONS OF CRITICAL DAMPINGS IN PER CENT (11F5.3)		TLUS 236
C	1- 5	DAMPING AT 10**(-4.0) PER CENT STRAIN	TLUS 237
C	6-10	DAMPING AT 10**(-3.5) PER CENT STRAIN	TLUS 238
C	11-15	DAMPING AT 10**(-3.0) PER CENT STRAIN	TLUS 239
C	16-20	DAMPING AT 10**(-2.5) PER CENT STRAIN	TLUS 240
C	21-25	DAMPING AT 10**(-2.0) PER CENT STRAIN	TLUS 241
C	26-30	DAMPING AT 10**(-1.5) PER CENT STRAIN	TLUS 242
C	31-35	DAMPING AT 10**(-1.0) PER CENT STRAIN	TLUS 243
C	36-40	DAMPING AT 10**(-0.5) PER CENT STRAIN	TLUS 244
C	41-45	DAMPING AT 10**(-0.0) PER CENT STRAIN	TLUS 245
C	46-50	DAMPING AT 10**( 0.5) PER CENT STRAIN	TLUS 246
C	51-55	DAMPING AT 10**( 1.0) PER CENT STRAIN	TLUS 247



C			TLUS 248
C	15. NODAL PCINT CARDS (15,3F10.4,15,F10.4,15)		TLUS 249
C	1- 5 M	NODAL POINT NUMBER	TLUS 250
C	6-15 XORD(M)	X-COORDINATE -FT	TLUS 251
C	16-25 YORD(M)	Y-COORDINATE -FT	TLUS 252
C	26-35 ZORD(M)	Z-COORDINATE -FT	TLUS 253
C	36-40 KEYBC(M)	KEY FOR DISPLACEMENT BOUNDARY CONDITIONS	TLUS 254
C		RELATIVE TO MOVING RIGID BASE	TLUS 255
C		0 - FREE POINT AND PCINTS ON RIGID BOUNDARY	TLUS 256
C		1 - CANNOT MOVE IN X-DIRECTION	TLUS 257
C		2 - CANNOT MOVE IN Y-DIRECTION	TLUS 258
C		3 - CANNOT MOVE IN Z DIRECTION	TLUS 259
C		4 - CANNOT MOVE IN X OR Y DIRECTION	TLUS 260
C		5 - CANNOT MOVE IN Y OR Z DIRECTION	TLUS 261
C		6 - CANNOT MOVE IN Z OR X DIRECTION	TLUS 262
C		7 - FIXED	TLUS 263
C	41-50 SMASIM)	WEIGHT OF MASS ACTING AT NODAL POINT M -K IPS	TLUS 264
C	51-55 NG	GENERATOR INCREMENT	TLUS 265
C			TLUS 266
C		NODES NEED NOT BE IN ORDER. NODAL COORDINATES	TLUS 267
C		CAN BE GENERATED ALONG A STRAIGHT LINE CONNECT-	TLUS 268
C		ING THE NODES ON TWO SUCCESSIVE CARDS. NODE	TLUS 269
C		NUMBERS ARE COMPUTED AS M+NG, M+2NG ETC. USING	TLUS 270
C		M AND NG ON THE FIRST CARD. MASS IS INTERPOLATED	TLUS 271
C		FRM THE TWO END VALUES AND THE BOUNDARY	TLUS 272
C		CONDITIONS ARE SET EQUAL TO THOSE ON THE FIRST	TLUS 273
C		CARD IF INPUT POSITIVE OR SET TO ZERO OTHERWISE.	TLUS 274
C			TLUS 275
C	16. EARTHQUAKE RECCRD CARDS		TLUS 276
C	16A. EARTHQUAKE NAME CARD (12A6)		TLUS 277
C	1-72 EQN(12)	IDENTIFICATION OF EARTHQUAKE RECCRD	TLUS 278
C	16B. EARTHQUAKE RECORD (BF9.6) - (N3+71/8 CARDS		TLUS 279
C	U2G(I)	INPUT ACCELETATIONS-G	TLUS 280
C		DIGITIZED AT THE SPECIFIED TIME INTERVAL DT.	TLUS 281
C			TLUS 282
C	INPUT DATA FOR MODE2		TLUS 283
C			TLUS 284
C	0. OPERATION MODE CARD (15,110)	SEE PT.0 ABOVE	TLUS 285
C			TLUS 286
C	1. CONTROL DATA (1015)		TLUS 287
C	1- 5 KDISP	SEE PT.6 ABOVE	TLUS 288
C	6-10 KSTRN	SEE PT.6 ABOVE	TLUS 289
C	11-15 KPNCH	SEE PT.6 ABOVE	TLUS 290
C	16-20 NOUT	SEE PT.6 ABOVE	TLUS 291
C	21-25 ND	SEE PT.6 ABOVE	TLUS 292
C	26-30 NSKIP	SEE PT.6 ABOVE	TLUS 293
C	31-35 NELS	SEE PT.6 ABOVE	TLUS 294
C	36-40 NUMBER	SEE PT.2 ABOVE	TLUS 295
C	41-45 KBLINE	SEE PT.3 ABOVE	TLUS 296
C	46-50 NFORM	SEE PT.2 ABOVE	TLUS 297
C			TLUS 298
C	2. FREQUENCY OF ANALYSIS CARD		TLUS 299
C	2A. MASTER CONTROL (F10.4, 15)		TLUS 300
C	1-10 TOTFRN	NEW TCTFR, SEE PT.5A ABOVE	TLUS 301
C	11-15 KINTSN	NEW KINTS, SEE PT.5A ABOVE	TLUS 302
C		IF BLANK, PREVIOUS INTERPOLATION RANGES WILL BE	TLUS 303
C		USED. IN THIS CASE, SKIP 2B.	TLUS 304
C	2B. (KINTS) CARDS (F10.4, 15)		TLUS 305
C	1-10 FRKINT(I)	SEE PT.5B ABOVE	TLUS 306
C	11-15 KINT(I)	SEE PT.5B ABOVE	TLUS 307
C			TLUS 308
C	3. CONTROL CARD FOR FREQUENCY OF ANALYSIS IN EACH ITERATION (8F10.4)		TLUS 309



C	SEE PT.9 ABOVE	TLUS 310
C		TLUS 311
C	4. CONTROL CARD FOR RESPONSE SPECTRA (2F10.4,15) AND (8F10.4)	TLUS 312
C	SEE PT.10A AND 10B ABOVE	TLUS 313
C		TLUS 314
C	5. OUTPUT CONTROL CARDS (415) - NOUT CARDS	TLUS 315
C	SEE PT.12 ABOVE	TLUS 316
C	6. OUTPUT FOR STRESSES (215) -NELS CARDS	TLUS 317
C	SEE PT.12 ABOVE	TLUS 318
C		TLUS 319
C		TLUS 320
C	INPUT DATA FOR MODE3	TLUS 321
C		TLUS 322
C	0. OPERATION MODE CARD (15), SEE PT.0 ABOVE	TLUS 323
C		TLUS 324
C	1. CONTROL DATA (715)	TLUS 325
C	1-5 KPACH SEE PT.6 ABOVE	TLUS 326
C	6-10 NOUT SEE PT.6 ABOVE	TLUS 327
C	11-15 ND SEE PT.6 ABOVE	TLUS 328
C	16-20 NSKIP SEE PT.6 ABOVE	TLUS 329
C	21-25 NELS SEE PT.6 ABOVE	TLUS 330
C	26-30 KBLINE SEE PT.3 ABOVE	TLUS 331
C	31-35 KSTRN SEE PT.6 ABOVE	TLUS 332
C		TLUS 333
C	2. CONTROL CARDS FOR RESPONSE SPECTRA (2F10.4,15) AND (8F10.4)	TLUS 334
C	SEE PT.10A AND 10B ABOVE	TLUS 335
C		TLUS 336
C	3. OUTPUT CONTROL CARDS (415) - NOUT CARDS	TLUS 337
C	SEE PT.11 ABOVE	TLUS 338
C		TLUS 339
C	4. OUTPUT FOR STRESSES (215) -NELS CARDS	TLUS 340
C	SEE PT.12 ABOVE	TLUS 341
C		TLUS 342
C		TLUS 343
C	COMMON XX(1)	TLUS 344
C		TLUS 345
C	CALL SECCND(TIM1)	TLUS 346
C	PRINT 6100,TIM1	TLUS 347
1	CALL INPT (XX,TIM1)	TLUS 348
C	GO TO 1	TLUS 349
C	6100 FORMAT (10X,32H)TIME REQUIRED FOR COMPILATION = .F10.3,6H SEC	TLUS 350
C	END	TLUS 351





## 6. EXAMPLE PROBLEM

### 6.1 Problem Description

A fictitious earth dam founded in a V-shaped valley will be used as an example problem for TLUSH. The geometry of the dam along with the finite element model used in the analysis are shown in Figs. 5, 6 and 7. It can be seen that the dam is geometrically symmetrical with respect to the A-A and E-E planes. Since the analysis will be performed for earthquake motions in the x-direction the deformation pattern of the dam will be symmetrical with respect to the A-A plane and anti-symmetrical with respect to the E-E plane. Accordingly, only one quarter of the model needs to be considered in the analysis, and symmetry and anti-symmetry boundary conditions have to be provided on the A-A and E-E planes respectively.

The response of the dam will be evaluated for an earthquake motion digitized in 64 points at a time interval of 0.04 seconds. A quiet zone consisting of 64 zeroes will be added at the end of the record to give a total analysis duration of 5.12 seconds. The original motion has a peak acceleration of 0.05g but will be scaled to a peak value of 0.10g and will be baseline corrected. The highest frequency in the Fourier spectrum of this motion is 12.5 Hz. However, a cut-off frequency of 10 Hz will be used in the first two iterations and one of 12.0 Hz will be used in the third iteration.

The shells and core of the dam are assumed to be composed of sands and clays respectively. These materials are assumed to have strain compatible properties defined by the relationships built in the program,



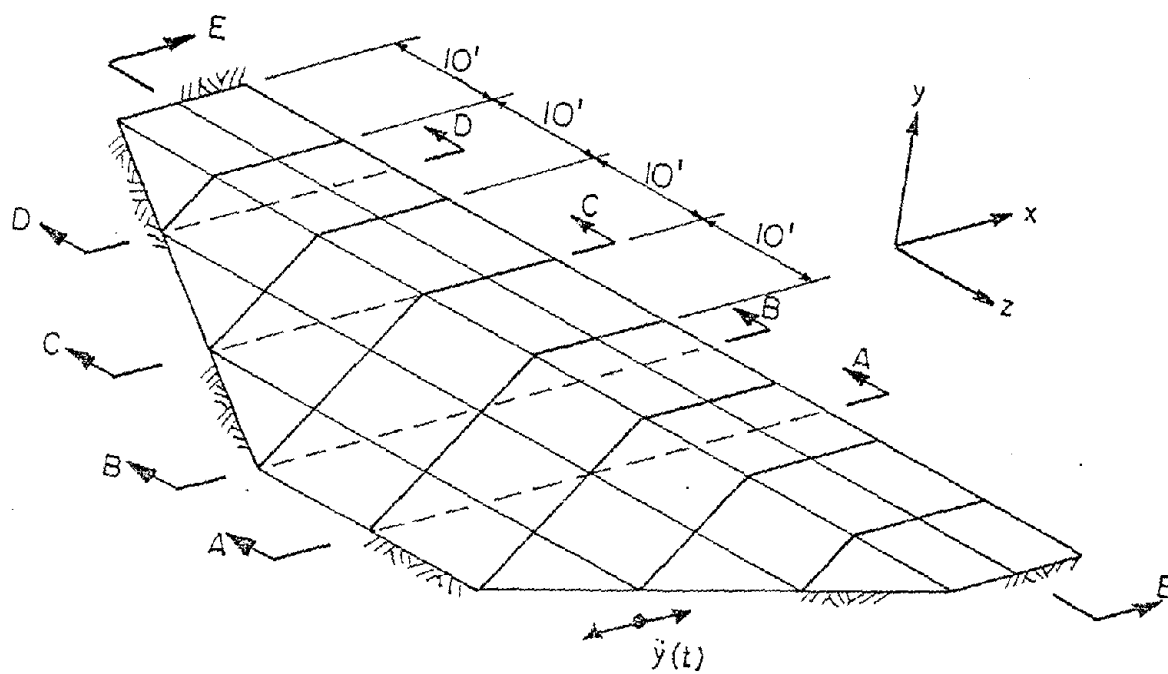


FIG. 5 FINITE ELEMENT MODEL FOR EXAMPLE PROBLEM



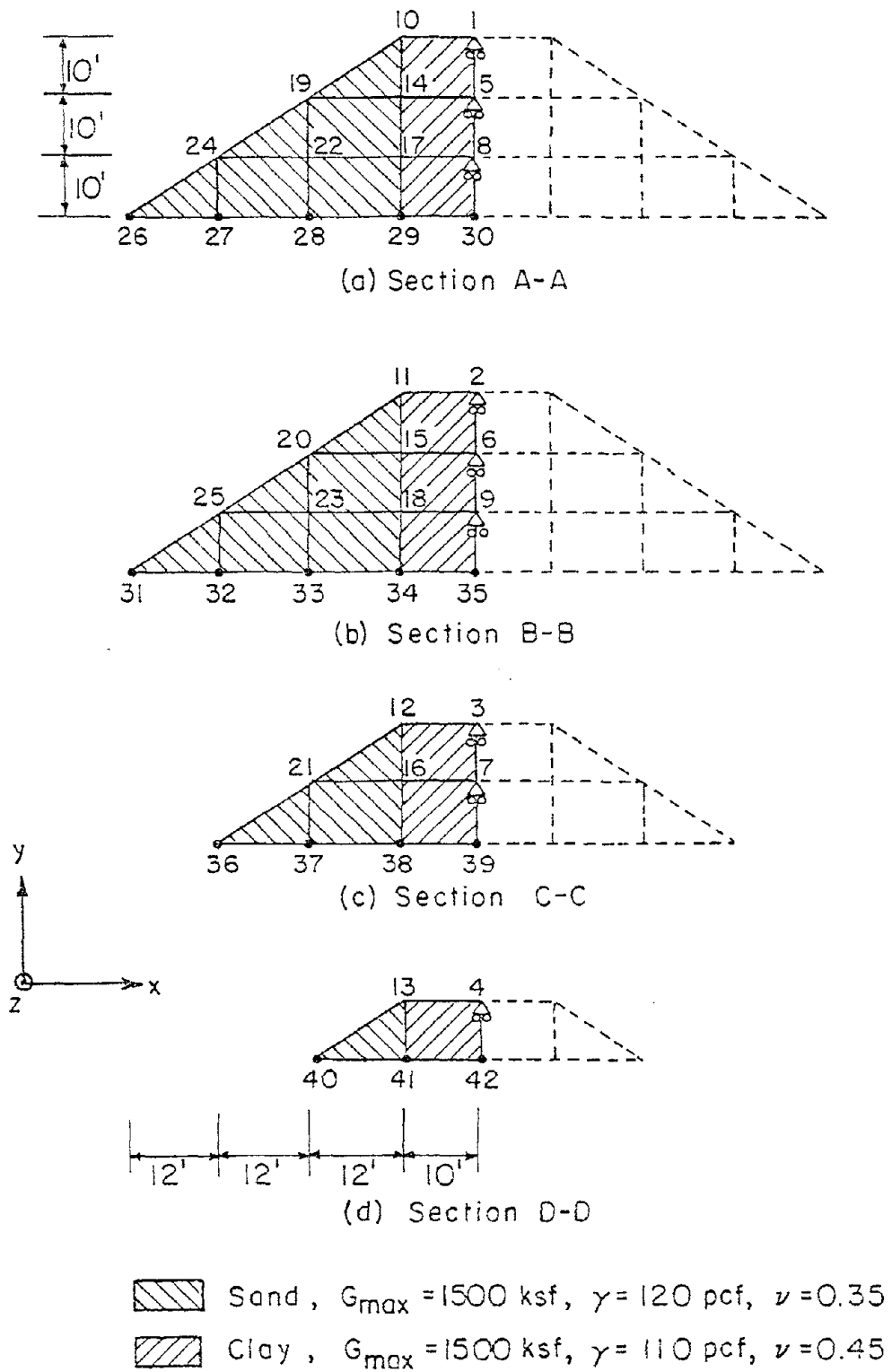
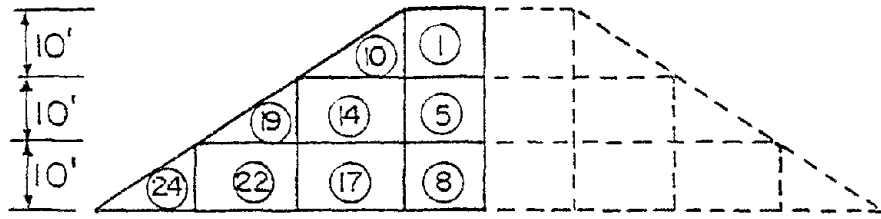
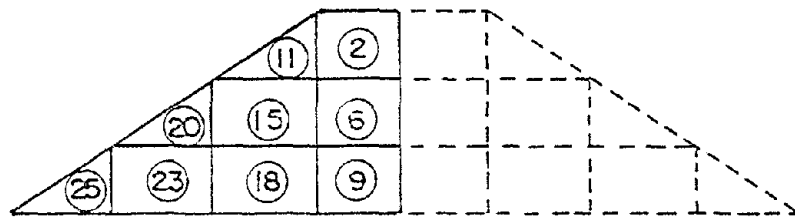


FIG. 6 NODAL POINT NUMBERING SYSTEM AND MATERIAL PROPERTIES

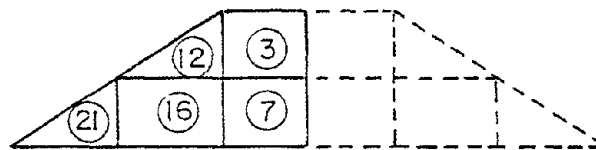




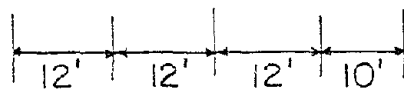
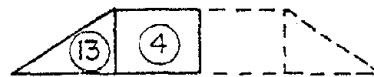
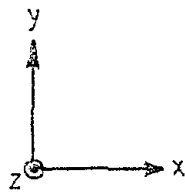
(a) Section A-A



(b) Section B-B



(c) Section C-C



(d) Section D-D

FIG. 7 ELEMENT NUMBERING SYSTEM





and to have a shear modulus at low strains of 1500 ksf. Unit weights of 120 and 110 pcf have been assigned to the shell sands and the core clays respectively.

Although convergence of the strain-compatible soil properties might be achieved in two iterations, three iterations will be performed to illustrate operation of the program in MODE1 and MODE2. Two iterations will be performed using MODE1 and one iteration will be done in MODE2. The acceleration amplification functions, the Fourier spectra and the acceleration time histories for nodal points 1, 3 and 19 will be computed and plotted. The RMS procedure will be used for computing maximum shear strains in MODE1 and the time domain approach will be used in MODE2. Additionally MODE3 will be used to generate the Fourier spectrum and the response spectra for the acceleration time history at nodal point 1 and the stress time histories for element number 14.

## 6.2 Input Data Cards

Images of the data cards required for the example run are shown in the following pages. The data is shown exactly as it appears on the data cards and should be compared with the detailed description of the data format given in section 5.











### 6.3 Computer Output

The printed output corresponding to the example problem is shown in the following pages, exactly as produced by a CDC 7600 computer, except for some blank space in between tables. It is once again noted that a complete record of the input data and solution remains stored on TAPE1. This additional output of the program is disposed onto a magnetic tape or disc.





\*\*\*\*\*  
\* MODEL \*  
\*\*\*\*\*

\*\*\* BLANK COMMON REQUIREMENTS \*\*\*

LENGTH OF BLANK COMMON =	15000
BLANK COMMON REQUIRED FOR VEDDA =	1499
BLANK COMMON FOR PERMANENT DATA =	940
BLANK COMMON REQUIRED FOR STRAIN =	1757
BLANK COMMON FOR BBBC =	6621
MAX. WORDS REQUIRED FOR BBBC =	72
BLANK COMMON REQUIRED FOR MOTION =	863
BLANK COMMON FOR AAAC =	7068
MAX. WORDS REQUIRED FOR AAAC =	12



RUN NUMBER = 1

EXAMPLE PROBLEM FOR TLUSH

```

*** INPUT DATA ***
TOTAL NUMBER OF ELEMENTS = 25
TOTAL NUMBER OF NODAL POINTS = 74
1ST NODAL POINT OF RIGID BASE = 29
DEGREES OF FREEDOM = 75
NUMBER OF BOUNDARY CONDITIONS = 18
NUMBER OF ITERATIONS = 2
NUMBER OF MATERIAL TYPES = 2

```

```

*** INPUT MOTION ***
TOTAL NO. OF POINTS USED IN FET = 128
LAST POINT TO BE READ = 64
TIME STEP OF RECORD = .040 SEC.
DURATION OF RECORD = 2.560 SEC.
QUIET ZONE OF RECORD = 2.560 SEC.
TOTAL DURATION OF ANALYSIS = 5.120 SEC.
EQ. MULTIPLICATION FACTOR = 2.0000
MAX. ACCEL. AFTER SCALING = .0585 (G)
MOTION IN X-DIRECTION = 1.0000 TIMES INPJT MOTION
MOTION IN Y-DIRECTION = 0.
MOTION IN Z-DIRECTION = 0.

```

BASE LINE CORRECTION WILL BE APPLIED

```

*** FREQUENCY CONTENT OF ANALYSIS ***
HIGHEST FREQUENCY OF ANALYSIS = 10.000 HZ
NO. OF INTERPOLATION RANGES = 4
0. -- 3.000 HZ(KINT= 5)
3.000 -- 6.000 HZ(KINT= 3)
6.000 -- 8.000 HZ(KINT= 5)
8.000 -- 10.000 HZ(KINT= 3)
FREQUENCY USED FOR ITERATION 1 = 10.000 HZ
FREQUENCY USED FOR ITERATION 2 = 10.000 HZ

```

```

*** OUTPUT CONTROL ***
SAVING AMP. FUNCTION ON TAPE = YES
COMPUTING NEW SOIL PROPERTIES = YES
PUNCHING NEW SOIL PROPERTIES = NO
STEPS TO FIND MAX. RESPONSE = 128
SKIP POINTS FOR PLOTTING = 2
FACTOR FOR EFF. SHEAR STRAIN = .6500

```

```

*** MASS MATRIX ***
FRACTION OF CONSISTENT MASS = .5000
FRACTION OF LUMPED MASS = .5000

```

```

*** DAMPING CHARACTERISTICS ***
UNIFORM DAMPING USED = NO

```



## \*\*\* OUTPUT REQUIREMENTS \*\*\*

CODES FOR KEYSPO  
 0--MAXIMUM ACCELERATION ONLY  
 1--PLOT ACC. TIME HISTORY  
 10--PLOT ACC. AND VELOCITY RESPONSE SPECTRUM  
 100--PUNCH ACC. TIME HISTORY  
 1000--PLOT FOURIER AMP. OF ACCELERATION  
 10000--PLOT AMPLIFICATION FUNCTION

## VALUES OF KEYSPO

N.P.NO.	X-OUTPUT	Y-OUTPUT	Z-OUTPUT
1	1001	0	0
3	1001	0	0
19	1001	0	0

## \*\*\* SHEAR MODULUS AND DAMPING CURVES FOR MATERIALS WITH STRAIN-DEPENDENT PROPERTIES \*\*\*

STANDARD BUILT IN CURVES ARE USED

## MAT. TYPE -- IDENTIFICATION

1	SEED AND IORISS (1970) -- CLAY
2	SEED AND IORISS (1979) -- SAND

## SHEAR MODULUS CURVES

## SHEAR STRAIN IN PERCENT

MAT.	10**(-4.0)	10**(-3.5)	10**(-3.0)	10**(-2.5)	10**(-2.0)	10**(-1.5)	10**(-1.0)	10**(-0.5)	10**( 0.0)	10**( 0.5)	10**( 1.0)
1	.22E+01	.91E+01	.76E+00	.57E+00	.40E+00	.25E+00	.15E+00	.76E-01	.37E-01	.13E-01	.40E-02
2	.10E+01	.28E+00	.43E+00	.83E+00	.56E+00	.44E+00	.25E+00	.12E+00	.49E-01	.49E-01	.49E-01

## DAMPING CURVES (PERCENT)

## SHEAR STRAIN IN PERCENT

MAT.	10**(-4.0)	10**(-3.5)	10**(-3.0)	10**(-2.5)	10**(-2.0)	10**(-1.5)	10**(-1.0)	10**(-0.5)	10**( 0.0)	10**( 0.5)	10**( 1.0)
1	.05E+01	.25E+01	.25E+01	.35E+01	.48E+01	.65E+01	.11E+01	.14E+02	.20E+02	.26E+02	.29E+02
2	.50E+00	.80E+00	.17E+01	.32E+01	.56E+01	.10E+02	.16E+02	.21E+02	.25E+02	.25E+02	.25E+02



\*\*\* ELEMENT DATA \*\*\*

NO.	NP1	NP2	NP3	NP4	NP5	NP6	NP7	NP8	NTYPE	ID.ELE	PD	UNIT WT. (PCF)	G-MAX (KSF)	G-USE (KSF)	DMP-USE (FRACTION)	VS-USE (FPS)	
1	10	14	5	1	11	15	6	2	1	1	1	.450	110.000	1500.000	900.000	.050	513.278
2	11	15	6	2	12	16	7	3	1	1	1	.450	110.000	1500.000	900.000	.050	513.278
3	12	16	7	2	13	17	8	4	1	1	1	.450	110.000	1500.000	900.000	.050	513.278
4	13	17	8	4	14	18	9	5	1	2	2	.450	110.000	1500.000	900.000	.050	513.278
5	14	18	9	5	15	19	10	6	1	1	1	.450	110.000	1500.000	900.000	.050	513.278
6	15	19	10	6	16	20	11	7	1	1	1	.450	110.000	1500.000	900.000	.050	513.278
7	16	20	11	7	17	21	12	8	1	2	2	.450	110.000	1500.000	900.000	.050	513.278
8	17	21	12	8	18	22	13	9	1	1	1	.450	110.000	1500.000	900.000	.050	513.278
9	18	22	13	9	19	23	14	10	1	2	2	.450	110.000	1500.000	900.000	.050	513.278
10	19	23	14	10	20	24	15	11	2	3	3	.350	120.000	1500.000	900.000	.050	491.426
11	20	24	15	11	21	25	16	12	2	3	3	.350	120.000	1500.000	900.000	.050	491.426
12	21	25	16	12	22	26	17	13	2	3	3	.350	120.000	1500.000	900.000	.050	491.426
13	22	26	17	13	23	27	18	14	2	4	4	.350	120.000	1500.000	900.000	.050	491.426
14	23	27	18	14	24	28	19	15	2	5	5	.350	120.000	1500.000	900.000	.050	491.426
15	24	28	19	15	25	29	20	16	2	5	5	.350	120.000	1500.000	900.000	.050	491.426
16	25	29	20	16	26	30	21	17	2	6	6	.350	120.000	1500.000	900.000	.050	491.426
17	26	30	21	17	27	31	22	18	2	6	6	.350	120.000	1500.000	900.000	.050	491.426
18	27	31	22	18	28	32	23	19	2	7	7	.350	120.000	1500.000	900.000	.050	491.426
19	28	32	23	19	29	33	24	20	2	8	8	.350	120.000	1500.000	900.000	.050	491.426
20	29	33	24	20	30	34	25	21	2	9	9	.350	120.000	1500.000	900.000	.050	491.426
21	30	34	25	21	31	35	26	22	2	9	9	.350	120.000	1500.000	900.000	.050	491.426
22	31	35	26	22	32	36	27	23	2	10	10	.350	120.000	1500.000	900.000	.050	491.426
23	32	36	27	23	33	37	28	24	2	10	10	.350	120.000	1500.000	900.000	.050	491.426
24	33	37	28	24	34	38	29	25	2	11	11	.350	120.000	1500.000	900.000	.050	491.426
25	34	38	29	25	35	39	30	26	2	11	11	.350	120.000	1500.000	900.000	.050	491.426

\*\*\* NODAL POINT DATA \*\*\*

N.P.	YORD (FT)	YORD (FT)	ZORD (FT)	MASS (KIPS)	BOUNDARY CONDITIONS		
					X-DIRECTION	Y-DIRECTION	Z-DIRECTION
1	46.000	30.000	40.000	-0.		FIXED	FIXED
2	46.000	30.000	30.000	-0.		FIXED	
3	46.000	30.000	20.000	0.		FIXED	
4	46.000	30.000	10.000	-0.		FIXED	
5	46.000	20.000	40.000	-0.		FIXED	FIXED
6	46.000	20.000	30.000	-0.		FIXED	
7	46.000	20.000	20.000	-0.		FIXED	
8	46.000	10.000	40.000	-0.		FIXED	FIXED
9	46.000	10.000	30.000	-0.		FIXED	
10	34.000	30.000	40.000	-0.			FIXED
11	34.000	30.000	30.000	0.			FIXED
12	34.000	30.000	20.000	0.			
13	34.000	30.000	10.000	-0.			
14	34.000	20.000	40.000	-0.			FIXED
15	34.000	20.000	30.000	0.			
16	34.000	20.000	20.000	-0.			
17	34.000	10.000	40.000	-0.			FIXED
18	34.000	10.000	30.000	-0.			
19	34.000	20.000	40.000	-0.			FIXED
20	24.000	20.000	30.000	0.			
21	34.000	20.000	20.000	-0.			
22	24.000	10.000	40.000	-0.			FIXED
23	24.000	10.000	30.000	-0.			
24	12.000	10.000	40.000	-0.			FIXED
25	12.000	10.000	30.000	-0.			
26	0.	0.	40.000	-0.		FIXED	FIXED
27	12.000	0.	40.000	0.	FIXED	FIXED	FIXED
28	24.000	0.	40.000	0.	FIXED	FIXED	FIXED
29	34.000	0.	40.000	-0.	FIXED	FIXED	FIXED
30	46.000	0.	40.000	-0.	FIXED	FIXED	FIXED
31	0.	0.	30.000	-0.	FIXED	FIXED	FIXED
32	12.000	0.	30.000	0.	FIXED	FIXED	FIXED
33	24.000	0.	30.000	0.	FIXED	FIXED	FIXED
34	34.000	0.	30.000	-0.	FIXED	FIXED	FIXED
35	46.000	0.	30.000	-0.	FIXED	FIXED	FIXED
36	12.000	10.000	20.000	-0.	FIXED	FIXED	FIXED
37	24.000	10.000	20.000	0.	FIXED	FIXED	FIXED
38	34.000	10.000	20.000	-0.	FIXED	FIXED	FIXED
39	46.000	10.000	20.000	-0.	FIXED	FIXED	FIXED
40	24.000	20.000	10.000	-0.	FIXED	FIXED	FIXED
41	34.000	20.000	10.000	-0.	FIXED	FIXED	FIXED
42	46.000	20.000	10.000	-0.	FIXED	FIXED	FIXED
43	34.000	30.000	0.	-0.	FIXED	FIXED	FIXED
44	46.000	30.000	0.	-0.	FIXED	FIXED	FIXED





\*\*\* EARTHQUAKE RECORD \*\*\*

\*\* EXAMPLE EARTHQUAKE \*\* 0.04 SEC 64 POINTS

INPUT ACCEL. PRIOR TO SCALING

MAX. ACC. = .0500 (G) AT TIME = .9600 SEC.

.000204	-.000191	.000058	.001460	.004111	.004390	.009534	-.003481	1
.013073	.009455	.000374	.018791	-.022305	-.018976	-.011557	-.048139	2
.013707	-.029759	.011100	-.025240	.015560	-.013417	.023272	-.013397	3
.050000	.000058	.012727	.006808	.010060	.010332	.006599	-.001122	4
.008954	-.020449	.015687	-.005003	-.017561	.003863	-.010918	-.008641	5
-.008450	-.010743	.001941	-.000952	.000968	-.001998	.010888	.008075	6
.009841	.003844	-.005115	.001176	-.000372	-.006514	.001117	-.000050	7
.000281	-.000221	.000197	-.000188	.000199	-.000188	.000193	-.000200	8
0.	0.	0.	0.	0.	0.	0.	0.	9
0.	0.	0.	0.	0.	0.	0.	0.	10
0.	0.	0.	0.	0.	0.	0.	0.	11
0.	0.	0.	0.	0.	0.	0.	0.	12
0.	0.	0.	0.	0.	0.	0.	0.	13
0.	0.	0.	0.	0.	0.	0.	0.	14
0.	0.	0.	0.	0.	0.	0.	0.	15
0.	0.	0.	0.	0.	0.	0.	0.	16

TIME REQUIRED FOR INPUT AND INITIALIZATION = .133 SEC

\*\*\* MATRICES AND LOAD VECTOR ARE FORMED IN THIS RUN \*\*\*

ITERATION 1  
 CORE SIZE AVAILABLE FOR EQUATION SOLUTION = 14957  
 TOTAL NUMBER OF EQUATIONS = 75  
 MAXIMUM NUMBER OF TERMS IN A BLOCK = 3731  
 MAXIMUM NUMBER OF COLUMNS IN A BLOCK = 75  
 NUMBER OF BLOCKS = 1  
 BANDWIDTH = 45  
 NUMBER OF TERMS IN STIFFNESS MATRIX = 1701

TIME REQUIRED FOR FORMATION OF STIFFNESS MATRIX = .303 SEC

\*\*\* TIME FOR GAUSSIAN ELIMINATION \*\*\*

FRQ NUMBER = 1	FRQ = 0.	HZ	CUMULATIVE TIME =	.0542	SEC.
FRQ NUMBER = 2	FRQ = .9766	HZ	CUMULATIVE TIME =	.1078	SEC.
FRQ NUMBER = 3	FRQ = 1.9531	HZ	CUMULATIVE TIME =	.1615	SEC.
FRQ NUMBER = 4	FRQ = 2.9297	HZ	CUMULATIVE TIME =	.2152	SEC.
FRQ NUMBER = 5	FRQ = 3.9156	HZ	CUMULATIVE TIME =	.2689	SEC.
FRQ NUMBER = 6	FRQ = 4.9016	HZ	CUMULATIVE TIME =	.3220	SEC.
FRQ NUMBER = 7	FRQ = 4.8875	HZ	CUMULATIVE TIME =	.3757	SEC.
FRQ NUMBER = 8	FRQ = 5.2734	HZ	CUMULATIVE TIME =	.4294	SEC.
FRQ NUMBER = 9	FRQ = 5.8594	HZ	CUMULATIVE TIME =	.4831	SEC.
FRQ NUMBER = 10	FRQ = 6.3354	HZ	CUMULATIVE TIME =	.5368	SEC.
FRQ NUMBER = 11	FRQ = 7.8125	HZ	CUMULATIVE TIME =	.5904	SEC.
FRQ NUMBER = 12	FRQ = 9.3750	HZ	CUMULATIVE TIME =	.6441	SEC.

NO. OF FRQ. STEPS = 9 (INCLUDING INTERPOLATED POINTS)



ELM	G-USFD (KSF)	G-NE# (KSF)	DIF-G (PERCENT)	DAMP-USED (FRACTION)	DAMP-NEW (FRACTION)	DIF-DAMP (PERCENT)	VS-USED (FPS)	VS-NEW (FPS)	DIF-VS (PERCENT)	MAX. S. STRAIN (PERCENT)
1	900.	701.	26.3	.050	.042	18.0	513.	453.	13.3	.00960
2	900.	740.	21.6	.050	.040	23.7	513.	456.	10.3	.00801
3	900.	557.	29.1	.050	.043	17.4	513.	452.	13.6	.00980
4	900.	796.	13.0	.050	.038	33.0	513.	483.	6.3	.00617
5	900.	601.	49.8	.050	.047	5.3	513.	419.	22.4	.01533
6	900.	585.	53.9	.050	.049	2.5	513.	414.	24.1	.01674
7	900.	594.	51.5	.050	.048	4.2	513.	417.	23.1	.01590
8	900.	623.	44.2	.050	.046	7.9	513.	427.	20.2	.01393
9	900.	558.	50.6	.050	.048	4.8	513.	418.	22.7	.01558
10	900.	1081.	-16.7	.050	.047	6.6	491.	539.	-8.7	.00994
11	900.	1020.	-11.8	.050	.053	-5.0	491.	523.	-6.1	.01308
12	900.	1025.	-12.2	.050	.052	-4.1	491.	524.	-6.3	.01278
13	900.	1142.	-21.2	.050	.041	21.6	491.	554.	-11.2	.00753
14	900.	998.	-9.3	.050	.055	-8.6	491.	517.	-5.0	.01466
15	900.	972.	-7.4	.050	.058	-13.3	491.	511.	-3.8	.01608
16	900.	593.	-9.4	.050	.055	-9.3	491.	516.	-4.8	.01475
17	900.	1017.	-11.5	.050	.053	-5.4	491.	522.	-5.9	.01324
18	900.	985.	-9.2	.050	.056	-10.0	491.	515.	-4.6	.01504
19	900.	995.	-9.6	.050	.055	-9.0	491.	517.	-4.9	.01464
20	900.	585.	-8.6	.050	.056	-10.6	491.	514.	-4.4	.01532
21	900.	989.	-9.0	.050	.055	-10.0	491.	515.	-4.6	.01507
22	900.	1058.	-14.9	.050	.049	2.0	491.	533.	-7.8	.01101
23	900.	1031.	-12.7	.050	.052	-3.1	491.	526.	-6.5	.01247
24	900.	1083.	-16.5	.050	.047	7.2	491.	539.	-8.9	.00982
25	900.	1059.	-15.0	.050	.049	2.2	491.	533.	-7.8	.01395

\*\*\* MAX. SHEAR STRAIN COMPUTATION IN FREQ DOMAIN

TIME REQUIRED FOR COMPUTING STRESS AND STRAIN = .956 SEC

ITERATION 2

TIME REQUIRED FOR FORMATION OF STIFFNESS MATRIX = .049 SEC

\*\*\* TIME FDP GAUSSIAN ELIMINATION \*\*\*

FRQ NUMBER	1	2	3	4	5	6	7	8	9	10	11	12
FRQ NUMBER =	1	2	3	4	5	6	7	8	9	10	11	12
FRQ =	1.9766	1.9531	2.5297	3.5156	4.1016	4.6875	5.2734	5.8594	6.4453	7.0312	7.6171	8.2030
FRQ =	1.9766	1.9531	2.5297	3.5156	4.1016	4.6875	5.2734	5.8594	6.4453	7.0312	7.6171	8.2030
CUMULATIVE TIME =	.0460	.0931	.1389	.1968	.2532	.3092	.3652	.4211	.4768	.5327	.5874	.6413

NO. OF FREQ. STEPS 49 (INCLUDING INTERPOLATED POINTS)



EL#	G-USED (KSF)	G-NEW (KSF)	DIF-G (PERCENT)	DAMP-JSED (FRACTION)	DAMP-NEW (FRACTION)	DIF-DAMP (PERCENT)	VS-USED (FPS)	VS-NEW (FPS)	DIF-VS (PERCENT)	MAX. S. STRAIN (PERCENT)
1	701.	571.	4.5	.062	.044	-3.5	453.	443.	2.2	.01106
2	740.	716.	3.3	.043	.042	-2.9	466.	458.	1.7	.00895
3	697.	692.	.7	.043	.043	-.5	452.	450.	.3	.01001
4	796.	800.	-4	.038	.037	.5	483.	484.	-.2	.00637
5	601.	577.	4.1	.047	.049	-3.9	419.	411.	2.0	.01745
6	585.	570.	2.5	.049	.050	-2.4	414.	409.	1.3	.01812
7	594.	584.	1.7	.048	.049	-1.7	417.	414.	.8	.01678
8	623.	610.	2.1	.046	.047	-1.4	427.	423.	1.0	.01466
9	598.	590.	1.3	.046	.048	-1.3	416.	416.	.6	.01624
10	1081.	1125.	-3.9	.047	.043	9.8	539.	549.	-2.0	.00813
11	1026.	1055.	-3.4	.053	.049	6.8	523.	532.	-1.7	.01115
12	1025.	1050.	-2.4	.052	.050	4.7	524.	531.	-1.2	.01142
13	1142.	1161.	-1.6	.041	.039	4.6	554.	558.	-.8	.00691
14	990.	1021.	-2.3	.055	.053	4.1	517.	523.	-1.1	.01323
15	972.	996.	-2.5	.058	.055	5.2	511.	517.	-1.2	.01457
16	993.	1007.	-1.4	.055	.054	2.5	516.	520.	-.7	.01384
17	1017.	1043.	-2.5	.053	.050	4.8	522.	529.	-1.2	.01179
18	989.	1008.	-1.5	.056	.054	3.4	515.	520.	-1.0	.01378
19	995.	1001.	-.6	.055	.054	1.1	517.	518.	-.3	.01422
20	985.	1000.	-1.5	.056	.054	2.7	514.	518.	-.8	.01428
21	989.	1022.	-1.4	.056	.054	2.4	515.	519.	-.7	.01416
22	1058.	1092.	-3.5	.049	.045	7.9	533.	542.	-1.7	.00928
23	1031.	1063.	-3.1	.052	.049	6.3	526.	534.	-1.5	.01077
24	1083.	1122.	-3.5	.047	.043	8.6	539.	549.	-1.8	.00824
25	1059.	1096.	-3.3	.049	.045	7.5	533.	542.	-1.7	.00929

\*\*\* MAX. SHEAR STRAIN COMPUTATION IN FREQ DOMAIN

TIME REQUIRED FOR COMPUTING STRESS AND STRAIN = .950 SEC

\*\*\* TAPE I HAS BEEN COMPLETED \*\*\*



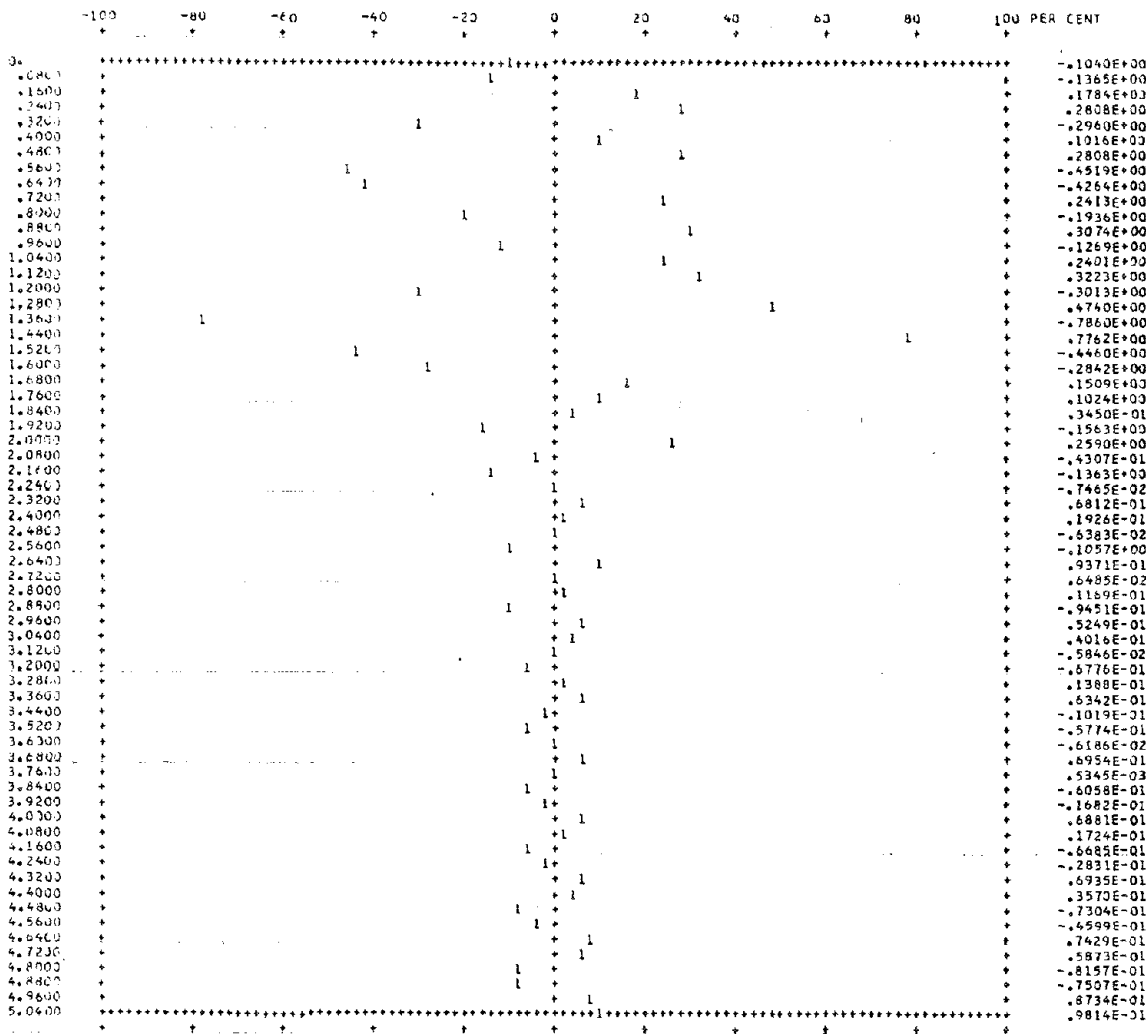






\*\*\* PLOT OF ACCELERATION. (BASE ACC.+REL. ACC.)

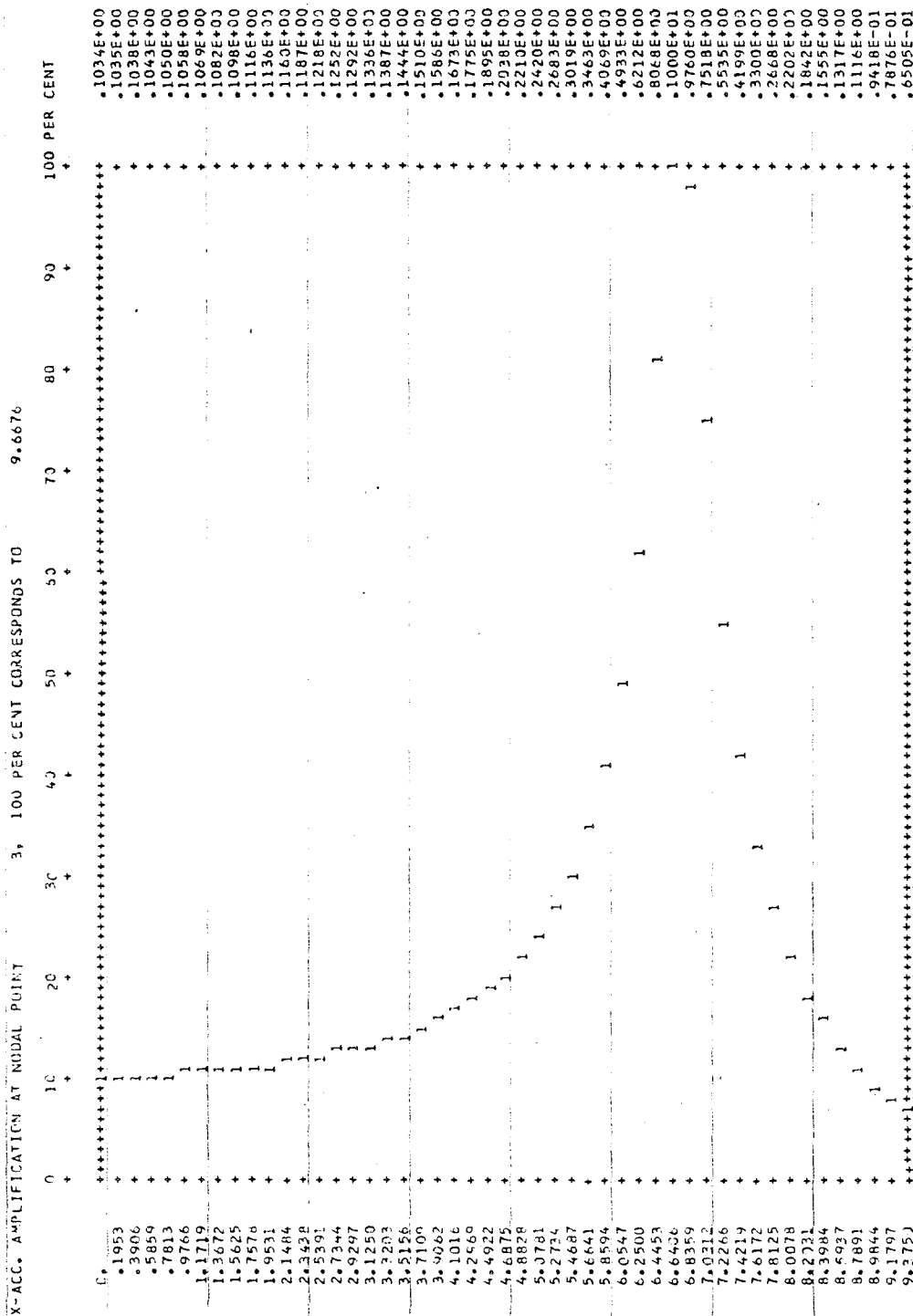
X-COMP. OF ACCELERATION AT NODAL POINT 1, 100 PER CENT CORRESPONDS TO .1875 G.  
 EVERY 2 IN POINTS ARE PLOTTED



\*\*\*\*\* TIME IN SECONDS



\*\*\* PLOT OF ACCELERATION AMPLIFICATION FUNCT.

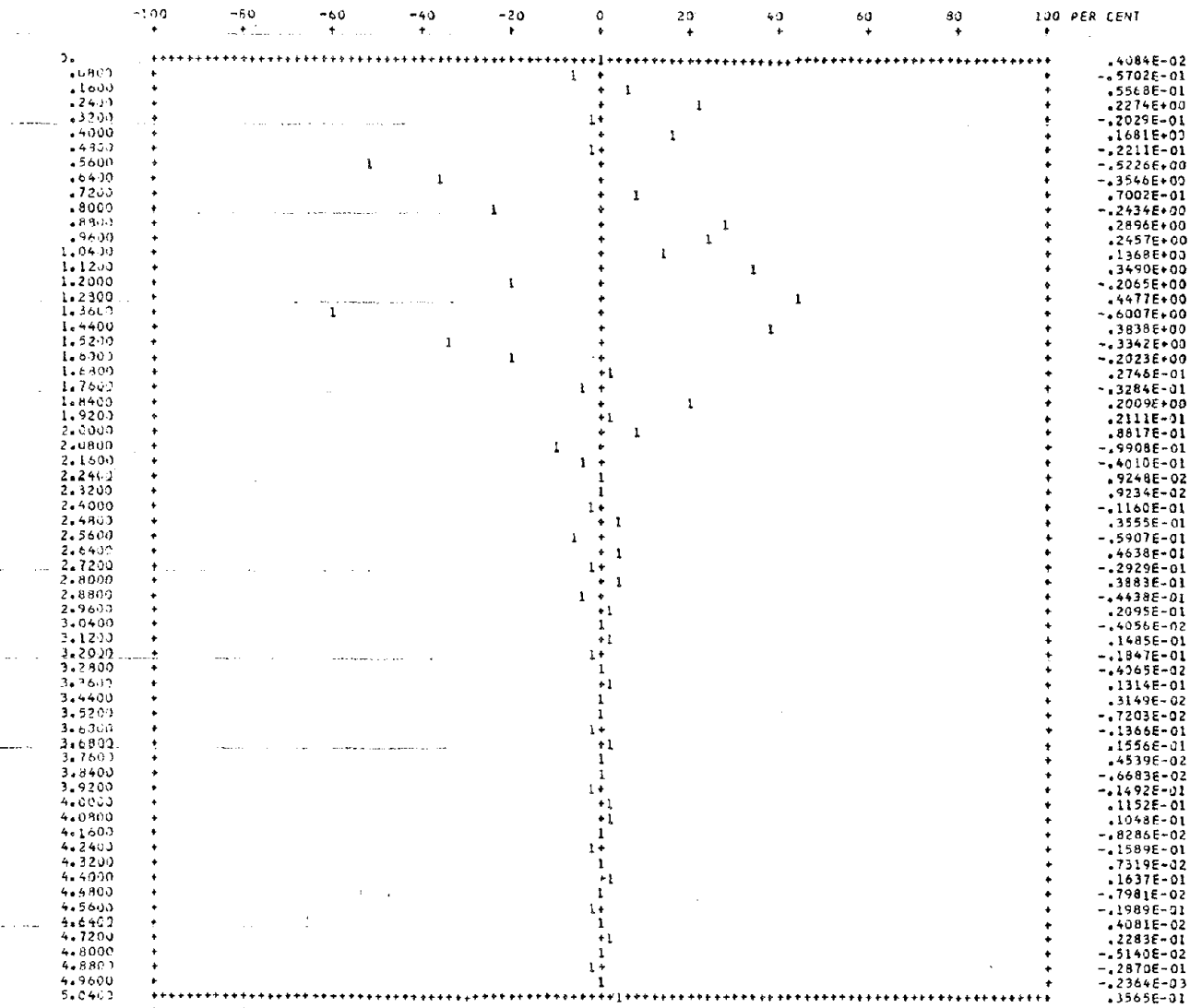


\*\*\*\*\* FREQ. IN HZ



\*\*\* PLOT OF ACCELERATION. (BASE ACC. FREL. ACC.)

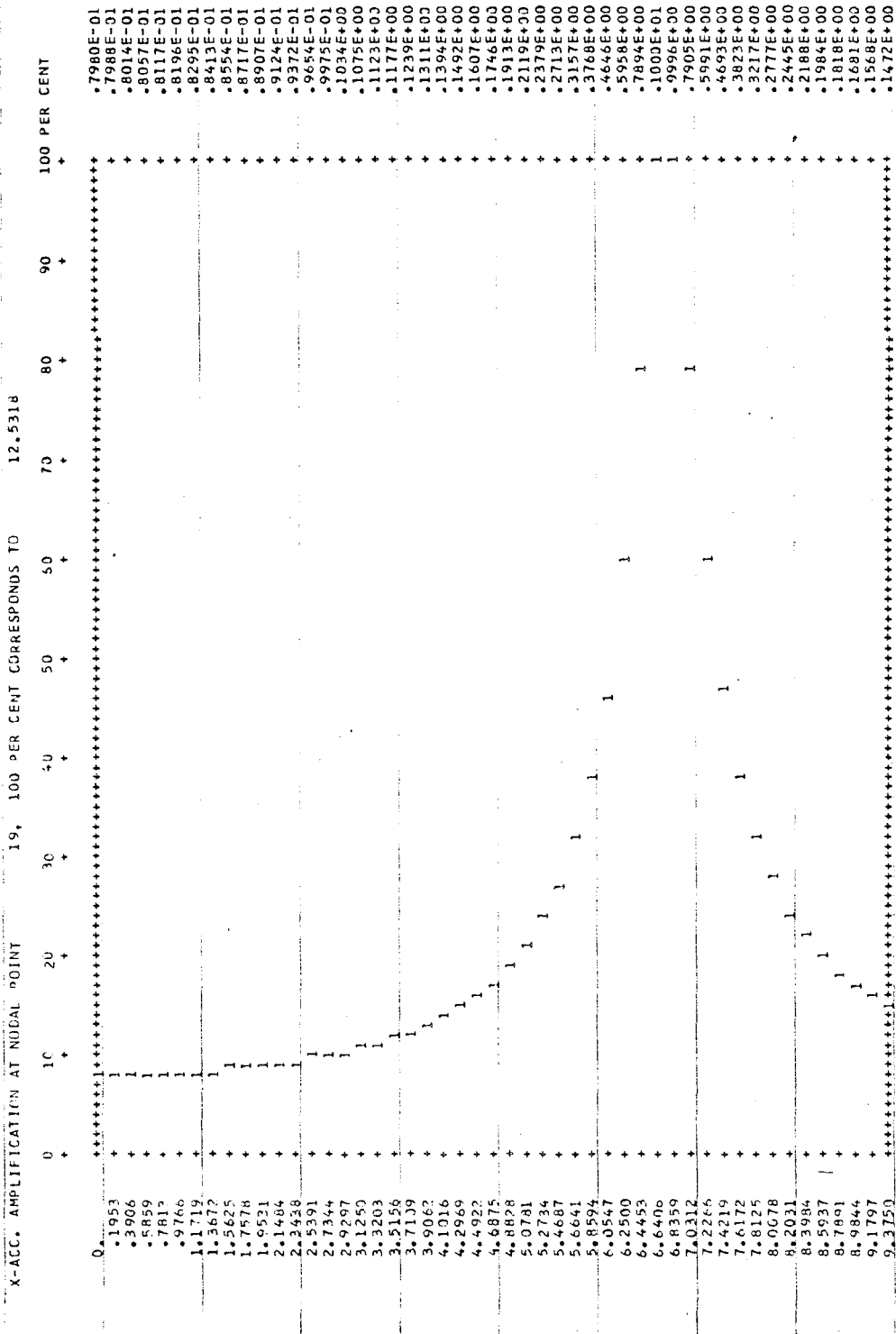
X-COMP. OF ACCELERATION AT NODAL POINT 3, 100 PER CENT CORRESPONDS TO .1163 G  
 EVERY 2 TH POINTS ARE PLOTTED



\*\*\*  
 \*\*\*\*\*  
 \*  
 \*\*\*\*\* TIME IN SECONDS



\*\*\* PLOT OF ACCELERATION AMPLIFICATION FACTOR



\*\*\*  
\*\*\*\*\*  
\*  
\*\*\*\*\* FREQ. IN HZ

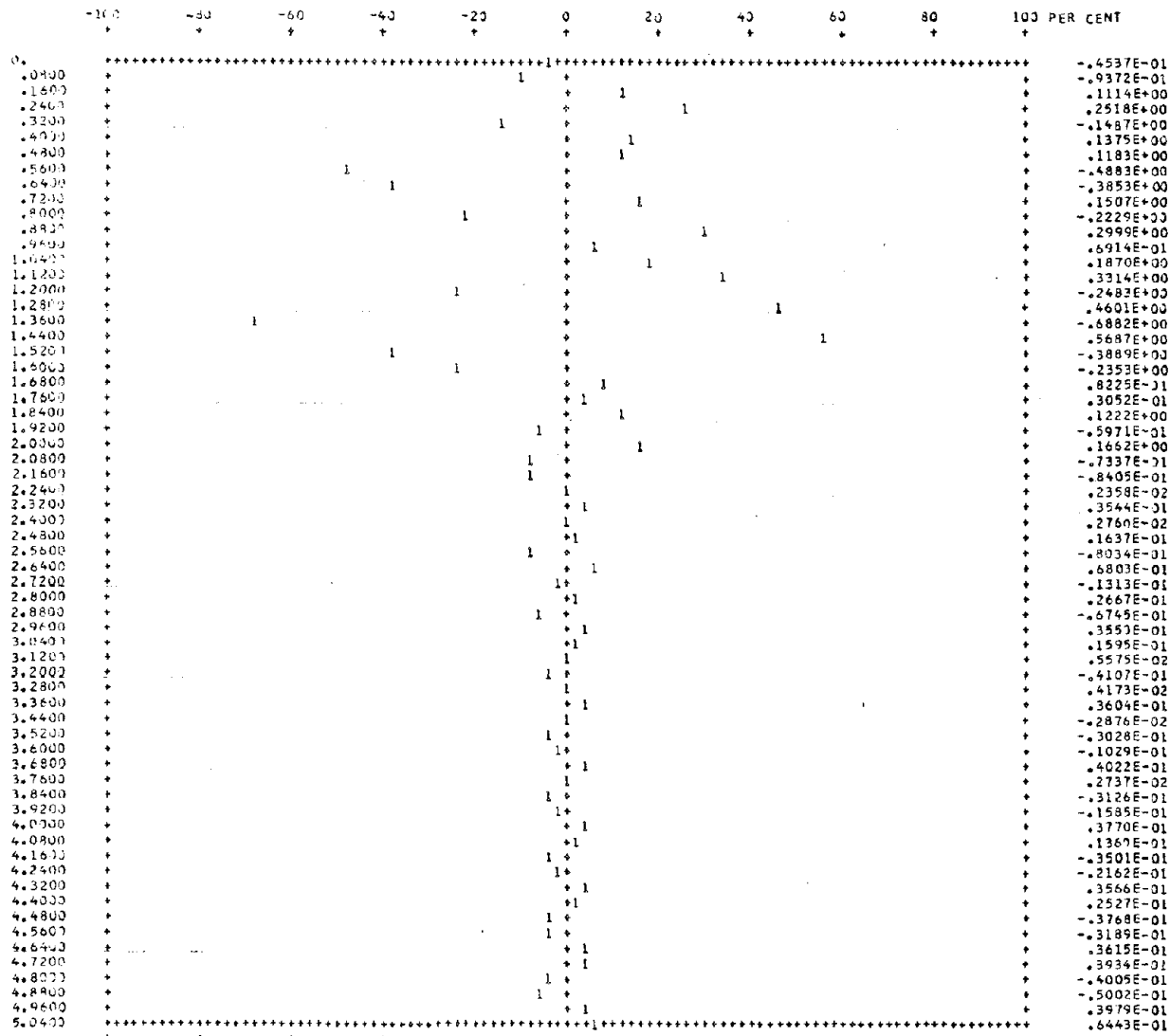




\*\*\* PLOT OF ACCELERATION (BASE ACC. PREL. ACC.)

X-COMP. OF ACCELERATION AT NODAL POINT 19, 100 PER CENT CORRESPONDS TO .1422 G

EVERY 2 TH POINTS ARE PLOTTED



\*\*\*  
\*\*\*\*\*  
\*  
\*\*\*\*\* TIME IN SECONDS



MAXIMUM ABSOLUTE ACCELERATIONS(G)

ITERATION NO. 2  
 MAX. INPUT BASE ACC. = .0585 (G) IN X DIRECTION  
 MAX. INPUT BASE ACC. = 0. (G) IN Y DIRECTION  
 MAX. INPUT BASE ACC. = 0. (G) IN Z DIRECTION

N.P.	XORD	YORD	ZORD	X-ACC	AT TIME	Y-ACC	AT TIME	Z-ACC	AT TIME
1	46.00	30.00	40.00	.1875	1.0000	0.	.9600	0.	.9600
3	46.00	30.00	20.00	.1163	1.0000	0.	.9600	.0492	1.0800
19	24.00	20.00	40.00	.1422	1.0000	.0132	1.4400	0.	.9600

TIME REQUIRED FOR COMPUTATION AND OUTPUT OF ACCELERATIONS AT 5 DEG. OF FOM. = .161 SEC



\*\*\*\*\*  
\* MODEZ \*  
\*\*\*\*\*

\*\*\* BLANK COMMON REQUIREMENTS \*\*\*

LENGTH OF BLANK COMMON =	15000
BLANK COMMON REQUIRED FOR VEDCA =	1481
BLANK COMMON FOR PERMANENT DATA =	922
BLANK COMMON REQUIRED FOR STRAIN =	1740
BLANK COMMON FOR BBBC =	6630
MAX. WORDS REQUIRED FOR BB7C =	96
BLANK COMMON REQUIRED FOR MUTICN =	904
BLANK COMMON FOR AAAC =	7048
MAX. WORDS REQUIRED FOR AAAC =	16

\*\*\* CONTENT OF TAPE 1 TRANSFERED TO TAPE 2 \*\*\*



EXAMPLE PROBLEM FOR TLUSH

RUN NUMBER = 1

## \*\*\* INPUT DATA \*\*\*

TOTAL NUMBER OF ELEMENTS = 25  
 TOTAL NUMBER OF NODAL POINTS = 44  
 1ST NODAL POINT ON RIGID BASE = 26  
 DEGREES OF FREEDOM = 75  
 NUMBER OF BOUNDARY CONDITIONS = 18  
 NUMBER OF ITERATIONS = 1  
 NUMBER OF MATERIAL TYPES = 2

## \*\*\* INPUT MOTION \*\*\*

TOTAL NO. OF POINTS USED IN FFT = 128  
 LAST POINT TO BE READ = 44  
 TIME STEP OF RECORD = .040 SEC.  
 DURATION OF RECORD = 2.560 SEC.  
 QUIET ZONE OF RECORD = 2.560 SEC.  
 TOTAL DURATION OF ANALYSIS = 5.120 SEC.  
 EQ. MULTIPLICATION FACTOR = 2.0000  
 MAX. ACCEL. AFTER SCALING = .0728 (G)  
 MOTION IN X-DIRECTION = 1.0000 TIMES INPUT MOTION  
 MOTION IN Y-DIRECTION = 0. TIMES INPUT MOTION  
 MOTION IN Z-DIRECTION = 0. TIMES INPUT MOTION

BASE LINE CORRECTION WILL BE APPLIED

## \*\*\* FREQUENCY CONTENT OF ANALYSIS \*\*\*

HIGHEST FREQUENCY OF ANALYSIS = 12.000 HZ  
 NO. OF INTERPOLATION RANGES = 3

0. -- 5.000 HZ(KINT= 5)  
 5.000 -- 10.000 HZ(KINT= 3)  
 10.000 -- 12.000 HZ(KINT= 5)  
 FREQUENCY USED FOR ITERATION 1 = 12.000 HZ

## \*\*\* OUTPUT CONTROL \*\*\*

SAVING AMP. FUNCTION ON TAPE = YES  
 COMPUTING NEW SOIL PROPERTIES = YES  
 PUNCHING NEW SOIL PROPERTIES = NO  
 STEPS TO FIND MAX. RESPONSE = 128  
 SKIP POINTS FOR PLOTTING = 2  
 FACTOR FOR EFF. SHEAR STRAIN = .6500

## \*\*\* MASS MATRIX \*\*\*

FRACTION OF CONSISTENT MASS = .5000  
 FRACTION OF LUMPED MASS = .5000

## \*\*\* DAMPING CHARACTERISTICS \*\*\*

UNIFORM DAMPING USED = NO

## \*\*\* OUTPUT REQUIREMENTS \*\*\*

CODES FOR KEYSPC  
 0--MAXIMUM ACCELERATION ONLY  
 1--PLOT ACC. TIME HISTORY  
 10--PLOT ACC. AND VELOCITY RESPONSE SPECTRUM  
 100--PUNCH ACC. TIME HISTORY  
 1000--PLOT FOURIER AMP. OF ACCELERATION  
 10000--PLOT AMPLIFICATION FUNCTION

## VALUES OF KEYSPC

N.P.NO.	X-OUTPUT	Y-OUTPUT	Z-OUTPUT
1	11001	0	0
3	11001	0	0
19	11001	0	0





\*\*\* SHEAR MODULUS AND DAMPING CURVES FOR MATERIALS WITH STRAIN-DEPENDENT PROPERTIES \*\*\*

STANDARD BUILT IN CURVES ARE USED.

MAT. TYPE IDENTIFICATION

1	SEED AND IDRIS (1970) -- CLAY
2	SEED AND IDRIS (1970) -- SAND

SHEAR MODULUS CURVES

MAT. TYPE	SHEAR STRAIN IN PERCENT										
	10**(-4.0)	10**(-3.5)	10**(-3.0)	10**(-2.5)	10**(-2.0)	10**(-1.5)	10**(-1.0)	10**(-0.5)	10**( 0.0)	10**( 0.5)	10**( 1.0)
1	.10E+01	.91E+00	.76E+00	.57E+00	.40E+00	.26E+00	.15E+00	.76E-01	.37E-01	.13E-01	.40E-02
2	.10E+01	.78E+00	.93E+00	.83E+00	.66E+00	.44E+00	.25E+00	.12E+00	.49E-01	.49E-01	.49E-01

DAMPING CURVES (PERCENT)

MAT. TYPE	SHEAR STRAIN IN PERCENT										
	10**(-4.0)	10**(-3.5)	10**(-3.0)	10**(-2.5)	10**(-2.0)	10**(-1.5)	10**(-1.0)	10**(-0.5)	10**( 0.0)	10**( 0.5)	10**( 1.0)
1	.25E+01	.25E+01	.25E+01	.35E+01	.48E+01	.65E+01	.93E+01	.14E+02	.20E+02	.26E+02	.29E+02
2	.50E+00	.80E+00	.17E+01	.32E+01	.50E+01	.10E+02	.16E+02	.21E+02	.25E+02	.25E+02	.25E+02

\*\*\* ELEMENT DATA \*\*\*

NO.	NP1	NP2	NP3	NP4	NP5	NP6	NP7	NP8	MTYPE	IO,EL	PO	UNIT WT. (PCF)	G-MAX (KSF)	G-USE (KSF)	DMP-USE (FRACTION)	VS-USE (FPS)	
1	10	14	5	1	11	15	6	2	1	1	1	.450	110.000	1500.000	670.867	.044	443.156
2	11	15	6	2	12	16	7	3	1	1	1	.450	110.000	1500.000	716.409	.042	457.944
3	12	16	7	2	13	17	8	4	1	1	1	.450	110.000	1500.000	692.458	.043	450.224
4	13	17	8	4	14	18	9	5	1	2	1	.450	110.000	1500.000	799.629	.037	483.872
5	14	17	8	5	15	18	9	5	1	1	1	.450	110.000	1500.000	577.225	.049	411.063
6	15	18	9	6	16	19	10	6	1	1	1	.450	110.000	1500.000	570.316	.050	408.591
7	16	18	9	7	17	20	11	7	1	2	1	.450	110.000	1500.000	584.308	.049	413.573
8	17	19	10	8	18	21	12	8	1	1	1	.450	110.000	1500.000	610.302	.047	422.692
9	18	19	10	9	19	22	13	9	1	2	1	.450	110.000	1500.000	590.178	.048	415.646
10	19	19	10	10	20	23	14	10	2	3	1	.350	120.000	1500.000	1125.169	.043	549.473
11	20	20	11	11	21	24	15	11	2	3	1	.350	120.000	1500.000	1055.399	.049	532.164
12	21	21	12	12	22	25	16	12	2	3	1	.350	120.000	1500.000	1050.366	.050	530.818
13	20	20	11	13	23	26	17	13	2	4	1	.350	120.000	1500.000	1161.362	.039	558.240
14	19	22	17	14	24	27	18	14	2	5	1	.350	120.000	1500.000	1020.762	.053	523.364
15	20	23	18	15	25	28	19	15	2	5	1	.350	120.000	1500.000	996.117	.055	517.002
16	21	23	18	16	26	29	20	16	2	6	1	.350	120.000	1500.000	1007.374	.054	519.915
17	22	28	29	17	27	30	21	17	2	5	1	.350	120.000	1500.000	1043.016	.050	529.033
18	23	33	34	18	28	31	22	18	2	6	1	.350	120.000	1500.000	1008.343	.054	520.165
19	24	24	22	19	25	25	23	20	2	3	1	.350	120.000	1500.000	1001.486	.054	518.394
20	25	25	23	20	26	26	24	21	2	4	1	.350	120.000	1500.000	1000.438	.054	518.123
21	26	26	27	21	27	27	25	22	2	4	1	.350	120.000	1500.000	1032.356	.054	518.619
22	24	27	28	22	25	32	33	23	2	5	1	.350	120.000	1500.000	1096.019	.045	542.308
23	25	32	33	23	36	36	37	37	2	6	1	.350	120.000	1500.000	1062.467	.049	534.069
24	26	26	27	24	31	31	32	25	2	3	1	.350	120.000	1500.000	1122.382	.043	548.792
25	31	31	32	25	36	36	36	36	2	4	1	.350	120.000	1500.000	1055.690	.045	542.229



\*\*\* NODAL POINT DATA \*\*\*

N.P.	XORD(FT)	YORD(FT)	ZORD(FT)	MASS (K/PS)	BOUNDARY CONDITIONS		
					X-DIRECTION	Y-DIRECTION	Z-DIRECTION
1	46.000	30.000	40.000	-0.		FIXED	FIXED
2	46.000	30.000	30.000	-0.		FIXED	
3	46.000	30.000	20.000	0.		FIXED	
4	46.000	30.000	10.000	-0.		FIXED	
5	46.000	20.000	40.000	-0.		FIXED	FIXED
6	46.000	20.000	30.000	-0.		FIXED	
7	46.000	20.000	20.000	-0.		FIXED	
8	46.000	10.000	40.000	-0.		FIXED	FIXED
9	46.000	10.000	30.000	-0.		FIXED	
10	36.000	30.000	40.000	0.			FIXED
11	36.000	30.000	30.000	0.			
12	36.000	30.000	20.000	0.			
13	36.000	30.000	10.000	-0.			
14	36.000	20.000	40.000	-0.			FIXED
15	36.000	20.000	30.000	0.			
16	36.000	20.000	20.000	-0.			
17	36.000	10.000	40.000	-0.			FIXED
18	36.000	10.000	30.000	-0.			
19	24.000	20.000	40.000	-0.			FIXED
20	24.000	20.000	30.000	0.			
21	24.000	20.000	20.000	-0.			
22	24.000	10.000	40.000	-0.			FIXED
23	24.000	10.000	30.000	-0.			
24	12.000	10.000	40.000	-0.			FIXED
25	12.000	10.000	30.000	-0.			
26	0.	0.	40.000	-0.	FIXED	FIXED	FIXED
27	12.000	0.	40.000	0.	FIXED	FIXED	FIXED
28	24.000	0.	40.000	0.	FIXED	FIXED	FIXED
29	36.000	0.	40.000	-0.	FIXED	FIXED	FIXED
30	46.000	0.	40.000	-0.	FIXED	FIXED	FIXED
31	0.	0.	30.000	-0.	FIXED	FIXED	FIXED
32	12.000	0.	30.000	0.	FIXED	FIXED	FIXED
33	24.000	0.	30.000	0.	FIXED	FIXED	FIXED
34	36.000	0.	30.000	-0.	FIXED	FIXED	FIXED
35	46.000	0.	30.000	-0.	FIXED	FIXED	FIXED
36	12.000	10.000	20.000	-0.	FIXED	FIXED	FIXED
37	24.000	10.000	20.000	0.	FIXED	FIXED	FIXED
38	36.000	10.000	20.000	-0.	FIXED	FIXED	FIXED
39	46.000	10.000	20.000	-0.	FIXED	FIXED	FIXED
40	24.000	20.000	10.000	-0.	FIXED	FIXED	FIXED
41	36.000	20.000	10.000	-0.	FIXED	FIXED	FIXED
42	46.000	20.000	10.000	-0.	FIXED	FIXED	FIXED
43	36.000	30.000	0.	-0.	FIXED	FIXED	FIXED
44	46.000	30.000	0.	-0.	FIXED	FIXED	FIXED

\*\*\* EARTHQUAKE RECORD \*\*\*

\*\* EXAMPLE EARTHQUAKE \*\* 0.04 SEC 64 POINTS

INPUT ACCEL. PRIOR TO SCALING

MAX. ACC. = .0500 (G) AT TIME = .9600 SEC.

.000204	-.000191	.000058	.001460	.004111	-.004390	.009534	-.033481	1
.013073	.009455	.000374	.018791	-.022305	-.018476	-.011557	-.048139	2
.013707	-.029759	.011100	-.025240	.015560	-.013417	.023272	-.013347	3
.050000	.000068	.012727	.006808	.010060	.010332	.006599	-.001122	4
.008954	-.020449	.015887	-.005003	-.017581	.003863	-.010918	-.008041	5
-.008450	-.010743	.001941	-.000952	.000968	-.001998	.010888	.008075	6
.009841	.003844	-.005115	.001176	-.003725	-.006514	.001117	-.000450	7
.000281	-.000221	.000197	-.000188	.000185	-.000188	.000193	-.000200	8
0.	0.	0.	0.	0.	0.	0.	0.	9
0.	0.	0.	0.	0.	0.	0.	0.	10
0.	0.	0.	0.	0.	0.	0.	0.	11
0.	0.	0.	0.	0.	0.	0.	0.	12
0.	0.	0.	0.	0.	0.	0.	0.	13
0.	0.	0.	0.	0.	0.	0.	0.	14
0.	0.	0.	0.	0.	0.	0.	0.	15
0.	0.	0.	0.	0.	0.	0.	0.	16

TIME REQUIRED FOR INPUT AND INITIALIZATION = .136 SEC



\*\*\* MATRICES AND LOAD VECTOR ARE FORMED IN THIS RUN \*\*\*

ITERATION 1  
 CORE SIZE AVAILABLE FOR EQUATION SOLUTION = 14969  
 TOTAL NUMBER OF EQUATIONS = 75  
 MAXIMUM NUMBER OF TERMS IN A BLOCK = 3699  
 MAXIMUM NUMBER OF COLUMNS IN A BLOCK = 75  
 NUMBER OF BLOCKS = 1  
 BANDWIDTH = 65  
 NUMBER OF TERMS IN STIFFNESS MATRIX = 1701

TIME REQUIRED FOR FORMATION OF STIFFNESS MATRIX = .307 SEC.

\*\*\* TIME FOR GAUSSIAN ELIMINATION \*\*\*

FRQ NUMBER	FRQ	FRQ	FRQ	CUMULATIVE TIME	SEC.
1	0.	0.	0.	.0047	SEC.
2	.9766	0.	0.	.1034	SEC.
3	1.9531	0.	0.	.1622	SEC.
4	2.9297	0.	0.	.2160	SEC.
5	3.9063	0.	0.	.2670	SEC.
6	4.8828	0.	0.	.3230	SEC.
7	5.8594	0.	0.	.3774	SEC.
8	6.8359	0.	0.	.4311	SEC.
9	7.8125	0.	0.	.4849	SEC.
10	8.7891	0.	0.	.5387	SEC.
11	9.7656	0.	0.	.5925	SEC.
12	10.7422	0.	0.	.6462	SEC.
13	11.7188	0.	0.	.6995	SEC.
14	12.6953	0.	0.	.7533	SEC.
15	13.6719	0.	0.	.8071	SEC.
16	14.6484	0.	0.	.8609	SEC.

NO. OF FREQ. STEPS = 60 (INCLUDING INTERPOLATED POINTS)

EL#	G-USED (KSF)	G-NEW (KSF)	DIF-G (PERCENT)	DAMP-USED (FRACTION)	DAMP-NEW (FRACTION)	DIF-DAMP (PERCENT)	VS-USED (FPS)	VS-NEW (FPS)	DIFF-VS (PERCENT)
1	671.	635.	5.7	.044	.046	-0.0	443.	431.	2.8
2	716.	718.	-0.2	.042	.042	0.2	458.	458.	-0.1
3	692.	677.	2.3	.043	.044	-1.0	450.	445.	1.1
4	800.	775.	3.3	.037	.039	-3.3	407.	478.	1.6
5	577.	574.	0.5	.049	.050	-0.5	411.	410.	0.3
6	570.	560.	1.3	.050	.051	-1.0	409.	405.	1.0
7	584.	581.	4.1	.049	.051	-3.8	414.	405.	2.0
8	610.	584.	4.6	.047	.049	-3.9	423.	413.	2.3
9	590.	585.	4.4	.048	.050	-4.2	418.	407.	2.2
10	1125.	1097.	2.6	.043	.045	-3.9	549.	542.	1.3
11	1055.	1064.	-0.9	.049	.048	1.8	532.	544.	-0.4
12	1050.	1031.	1.8	.050	.052	-3.2	521.	520.	0.9
13	1141.	1136.	2.2	.039	.042	-5.7	558.	552.	1.1
14	1021.	1032.	-1.0	.053	.052	2.0	523.	526.	-0.5
15	986.	986.	1.0	.055	.056	-1.7	517.	514.	0.5
16	1007.	982.	2.6	.054	.056	-4.3	520.	513.	1.3
17	1043.	1017.	2.5	.050	.053	-4.6	527.	522.	1.3
18	1008.	983.	2.8	.054	.056	-4.4	520.	513.	1.3
19	1001.	1037.	-0.6	.054	.054	1.0	518.	520.	-0.3
20	1000.	994.	0.7	.054	.055	-1.2	518.	516.	0.3
21	1002.	982.	2.1	.054	.056	-3.7	519.	513.	1.1
22	1056.	1081.	1.4	.045	.047	-3.1	542.	539.	0.7
23	1063.	1045.	1.7	.049	.050	-3.4	534.	529.	0.9
24	1122.	1104.	1.6	.043	.043	-3.8	549.	544.	0.8
25	1096.	1078.	1.6	.045	.047	-3.3	542.	538.	0.8



MAX. ELEMENT STRESSES IN PSF AND MAXIMUM SHEAR STRAIN IN PERCENT

ELEM.	SIG-X	SIG-Y	SIG-Z	TAU-XY	TAU-YZ	TAU-ZX	MAX. SHEAR STRN	AT TIME
1	24.0	51.8	82.9	70.0	5.4	48.3	.01309	1.080
2	15.5	6.3	10.8	53.9	20.3	38.2	.00885	1.000
3	190.0	177.4	191.7	37.2	3.9	64.3	.01076	1.000
4	27.7	34.6	37.8	43.5	5.1	34.4	.00683	1.160
5	53.4	35.9	61.3	89.2	6.6	24.3	.01773	1.080
6	17.4	6.6	6.4	84.9	15.3	67.5	.01924	1.000
7	77.2	85.4	104.0	79.9	12.0	75.5	.01906	1.000
8	8.1	9.5	7.5	101.3	2.8	14.8	.01684	1.000
9	42.4	54.4	53.1	79.2	7.2	75.9	.01805	1.000
10	96.9	34.5	31.5	57.0	17.0	61.7	.00926	1.080
11	85.2	42.3	51.9	55.5	31.3	97.3	.01070	1.080
12	72.4	36.1	39.5	67.9	24.1	109.8	.01242	1.000
13	52.9	44.7	106.3	57.5	22.6	57.5	.00774	1.000
14	78.2	31.6	11.3	115.6	8.7	41.2	.01241	1.000
15	61.6	29.9	34.6	111.0	16.8	102.2	.01523	1.000
16	62.9	53.1	87.7	112.9	17.7	105.3	.01548	1.000
17	36.2	19.4	11.9	134.2	2.3	24.4	.01324	1.000
18	21.8	19.9	23.7	108.9	5.9	111.1	.01546	1.000
19	101.7	87.4	18.1	135.0	25.8	40.5	.01385	1.080
20	84.4	70.3	73.3	114.8	51.3	90.0	.01473	1.000
21	105.3	115.0	185.8	102.3	45.2	102.3	.01552	1.000
22	83.0	78.2	41.5	106.9	7.5	20.7	.03994	1.000
23	72.1	76.6	77.6	87.1	18.6	87.9	.01169	1.000
24	39.6	80.2	32.9	97.5	9.3	10.1	.00894	1.000
25	65.0	94.5	91.1	75.4	27.9	75.4	.01006	1.000

\*\*\* MAX. SHEAR STRAIN COMPUTATION IN TIME DOMAIN

TIME REQUIRED FOR COMPUTING STRESS AND STRAIN = 2.648 SEC

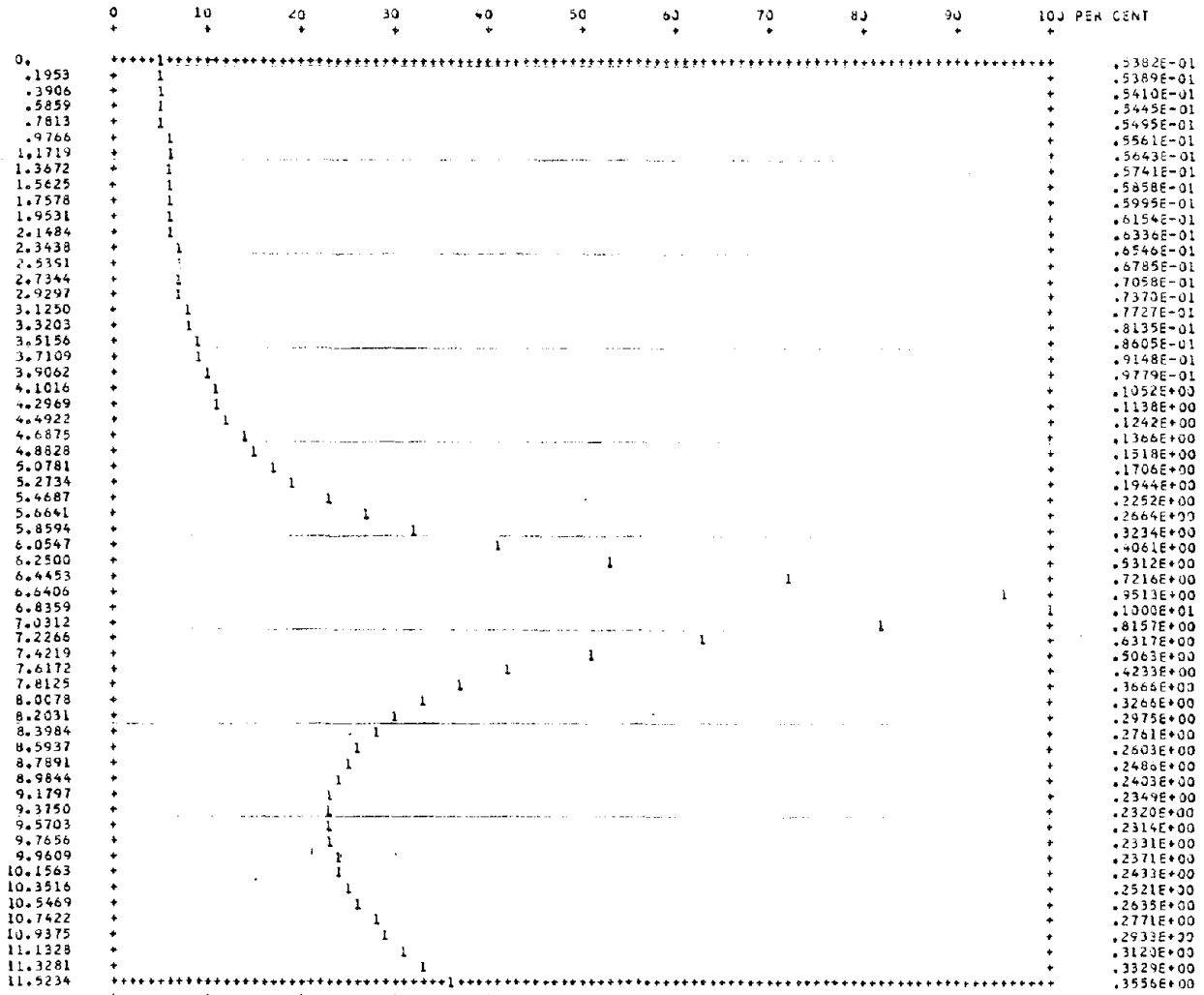
\*\*\* TAPE 1 HAS BEEN COMPLETED \*\*\*





\*\*\* PLOT OF ACCELERATION AMPLIFICATION FUNCT.

X-ACC. AMPLIFICATION AT NUCAL POINT 1, 100 PER CENT CORRESPONDS TO 1d.5812

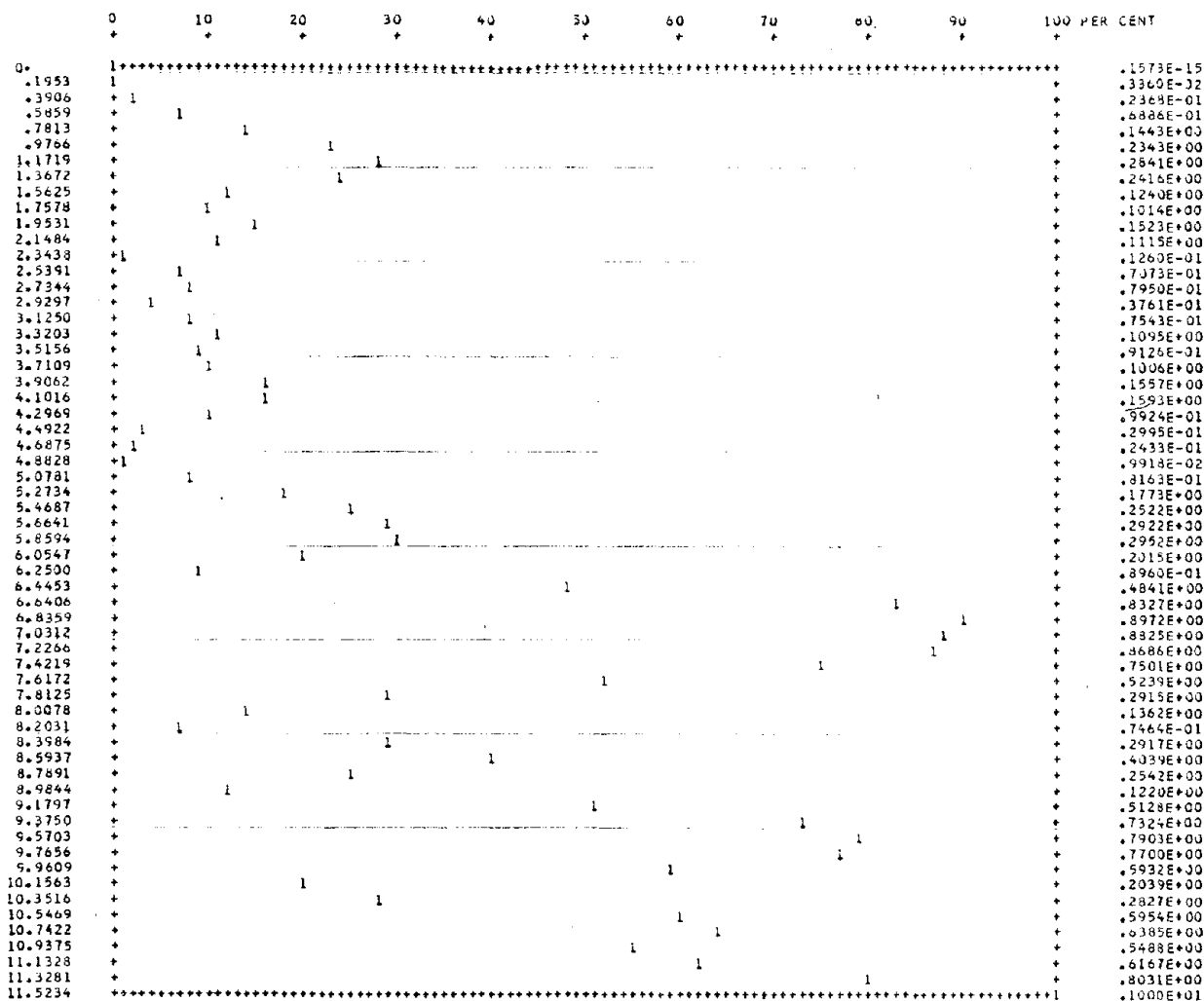


\*\*\*\*\*  
 \*  
 \*\*\*\*\* FREQ. IN HZ



\*\*\* PLOT OF FOURIER AMPLITUDE OF ACCELERATION

X-ACC. FOURIER SPECTRUM AT NODAL POINT 1, 100 PER CENT CORRESPONDS TO .0303 G

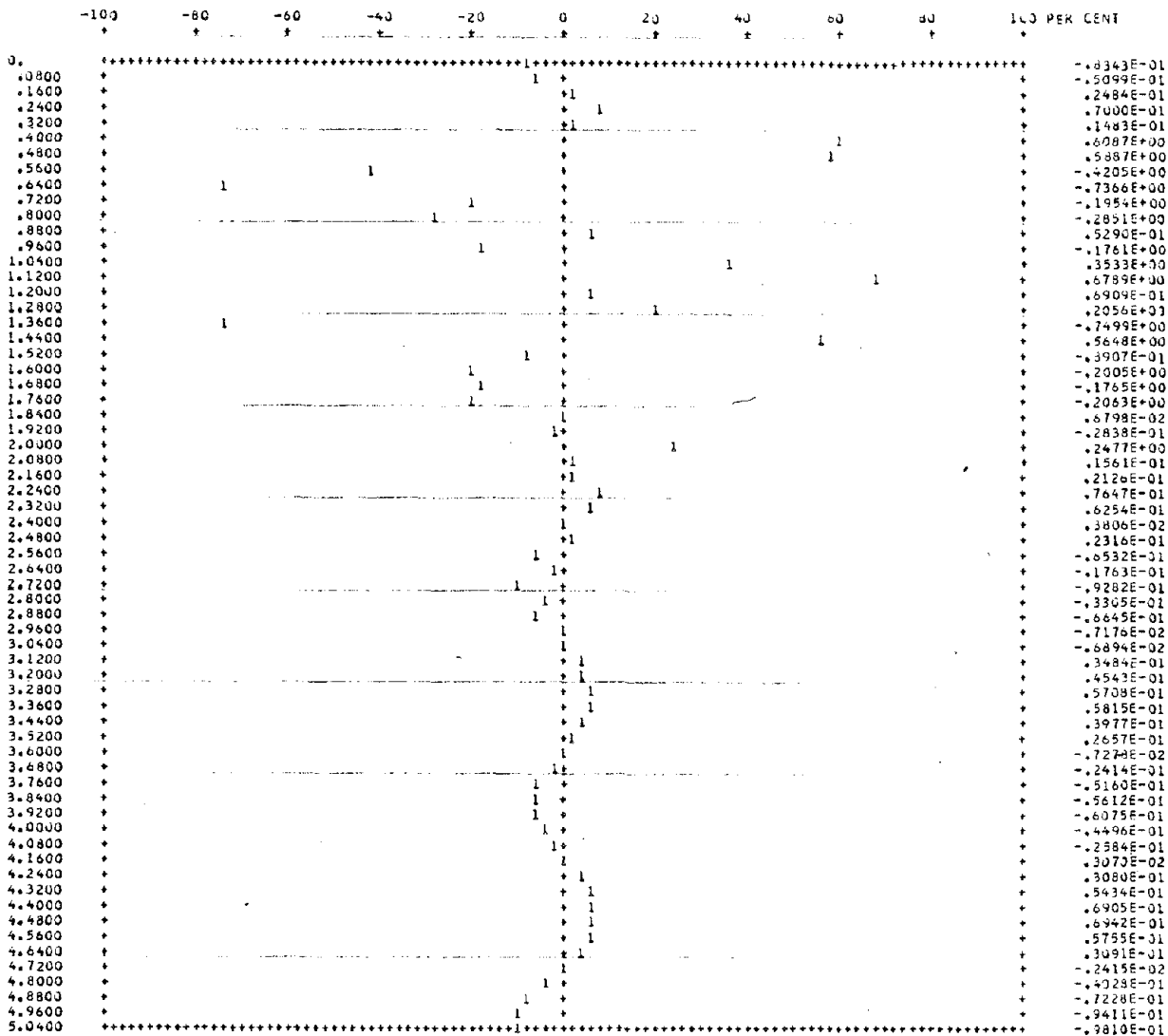


\*\*\*\*\* FREQ. IN HZ



\*\*\* PLOT OF ACCELERATION. (BASE ACC.+REL. ACC.)

X-COMP. OF ACCELERATION AT NODAL POINT 1, 100 PER CENT CORRESPONDS TO .2884 G  
 EVERY 2 TH POINTS ARE PLOTTED

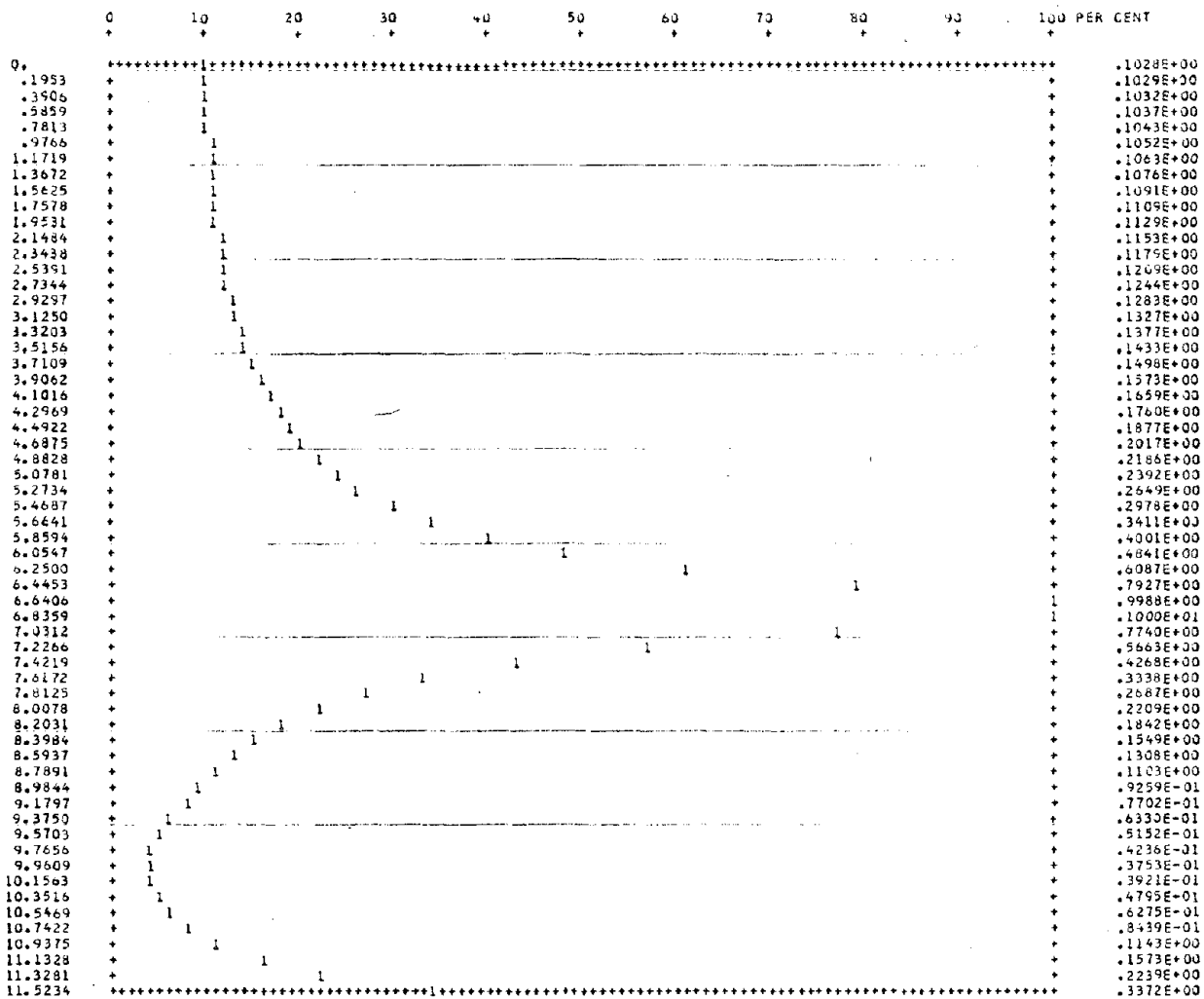


\*\*\*  
 \*\*\*\*\*  
 \*  
 \*\*\*\*\* TIME IN SECONDS



\*\*\* PLOT OF ACCELERATION AMPLIFICATION FUNCT.

X-ACC. AMPLIFICATION AT NUCCAL POINT 3, 100 PER CENT CORRESPONDS TO 9.7253

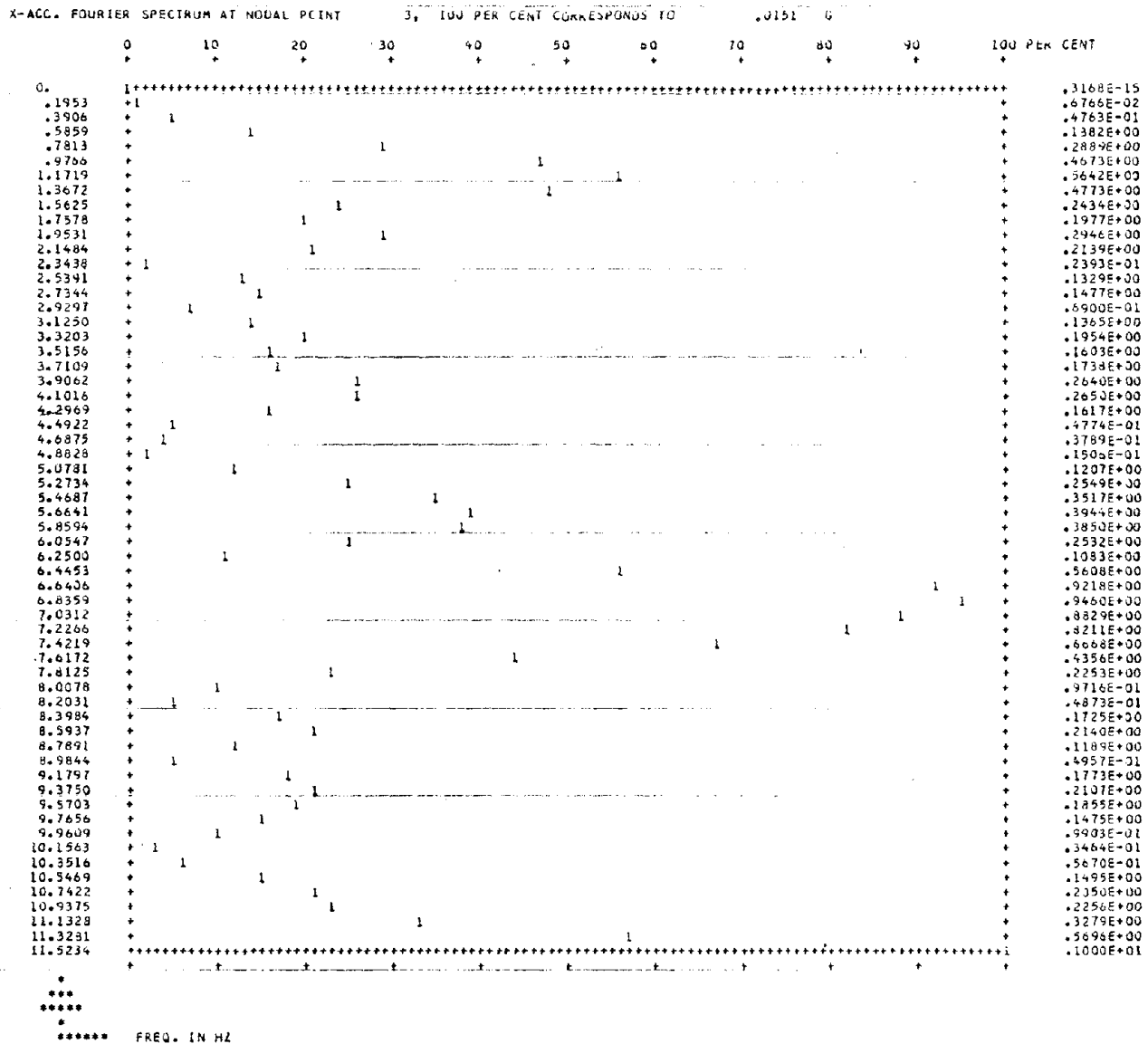


\*\*\*  
 \*\*\*\*\*  
 \*  
 \*\*\*\*\* FREQ. IN HZ





\*\*\* PLOT OF FOURIER AMPLITUDE OF ACCELERATION

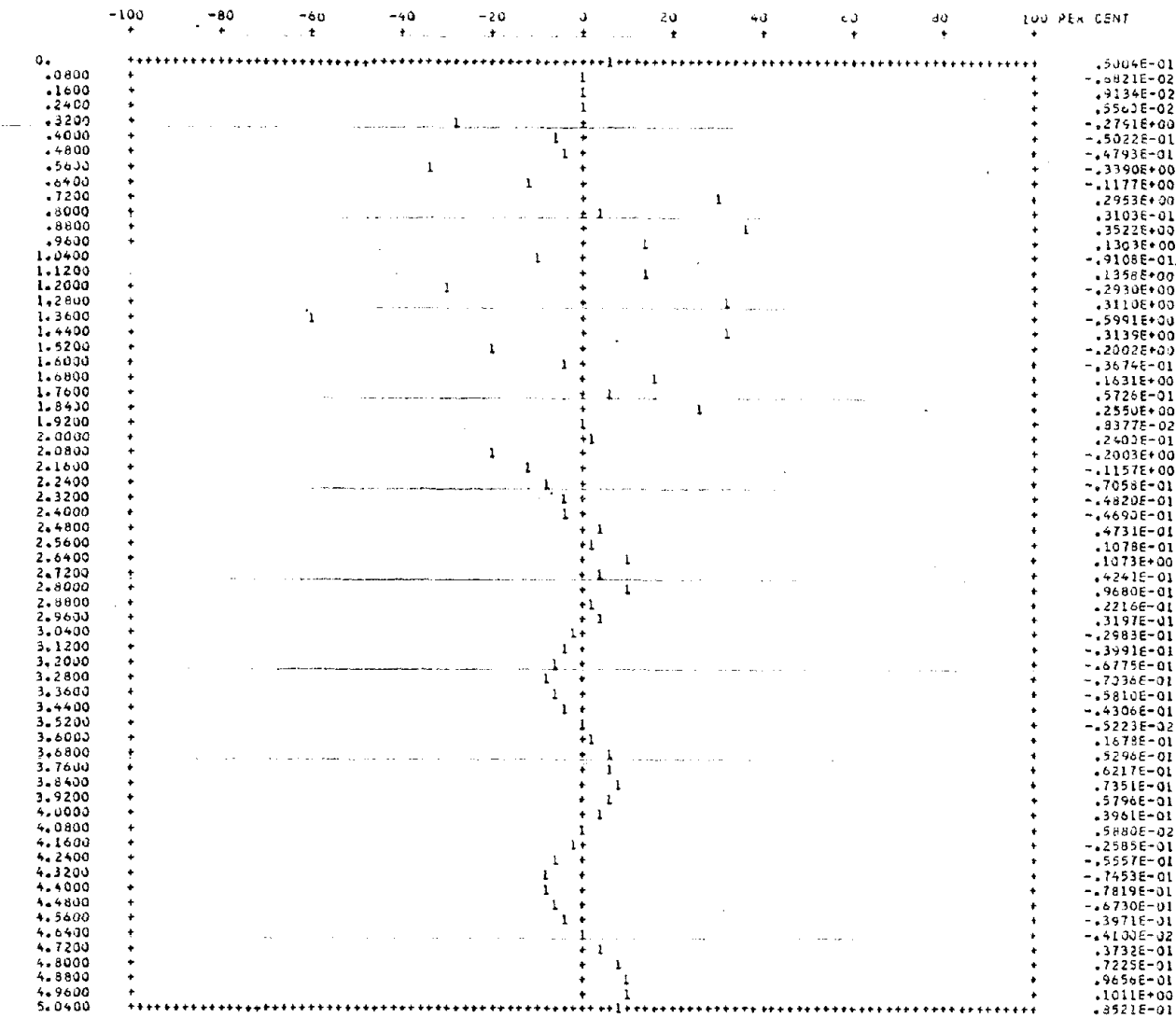




\*\*\* PLOT OF ACCELERATION, (BASE ACC.+REL. ACC.)

X-COMP. OF ACCELERATION AT NOCAL PCINT 3, 100 PER CENT CORRESPONDS TO .1384 G

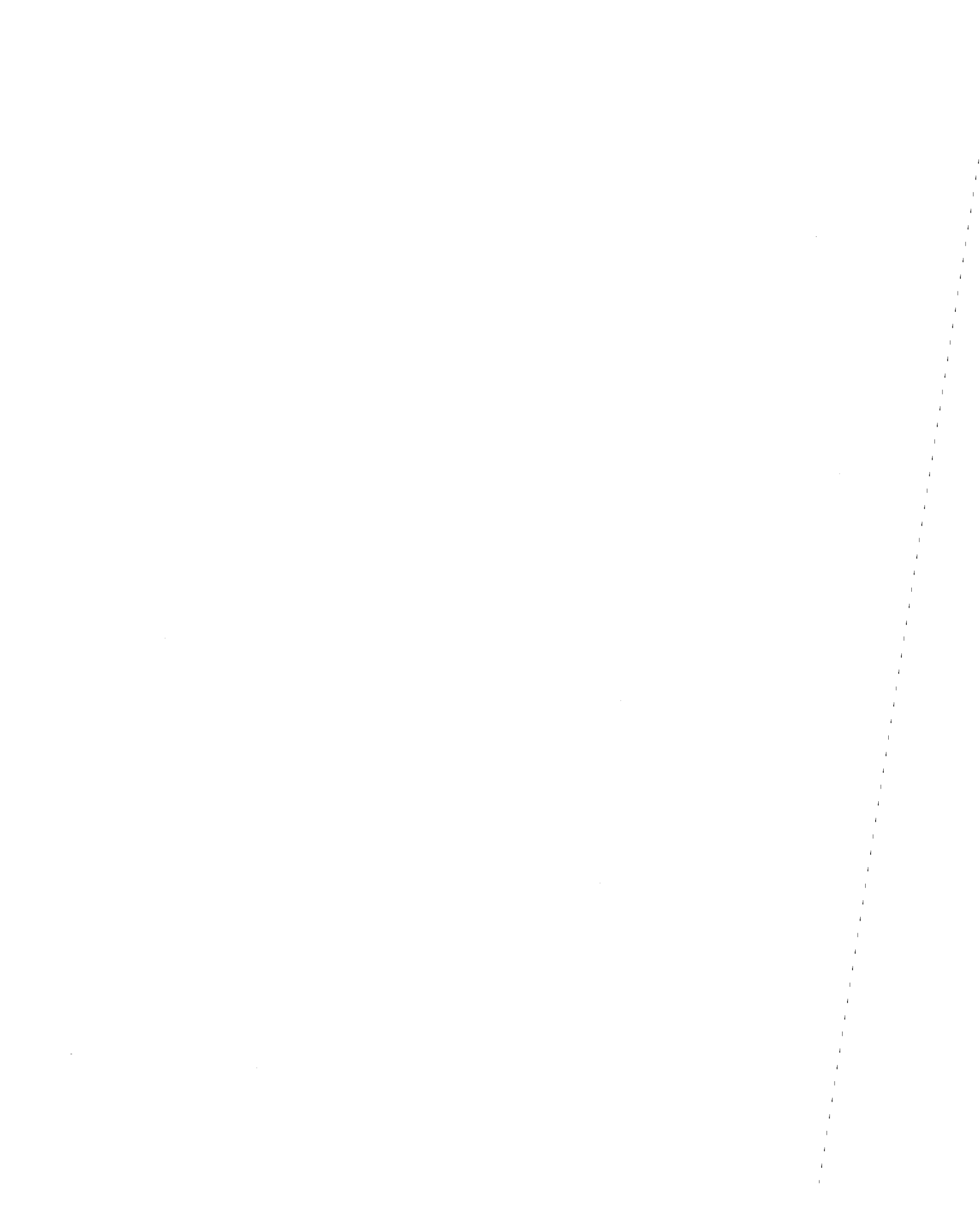
EVERY 2 TH POINTS ARE PLOTTED



\*\*\*  
 \*\*\*\*\*  
 \*  
 \*\*\*\*\* TIME IN SECONDS

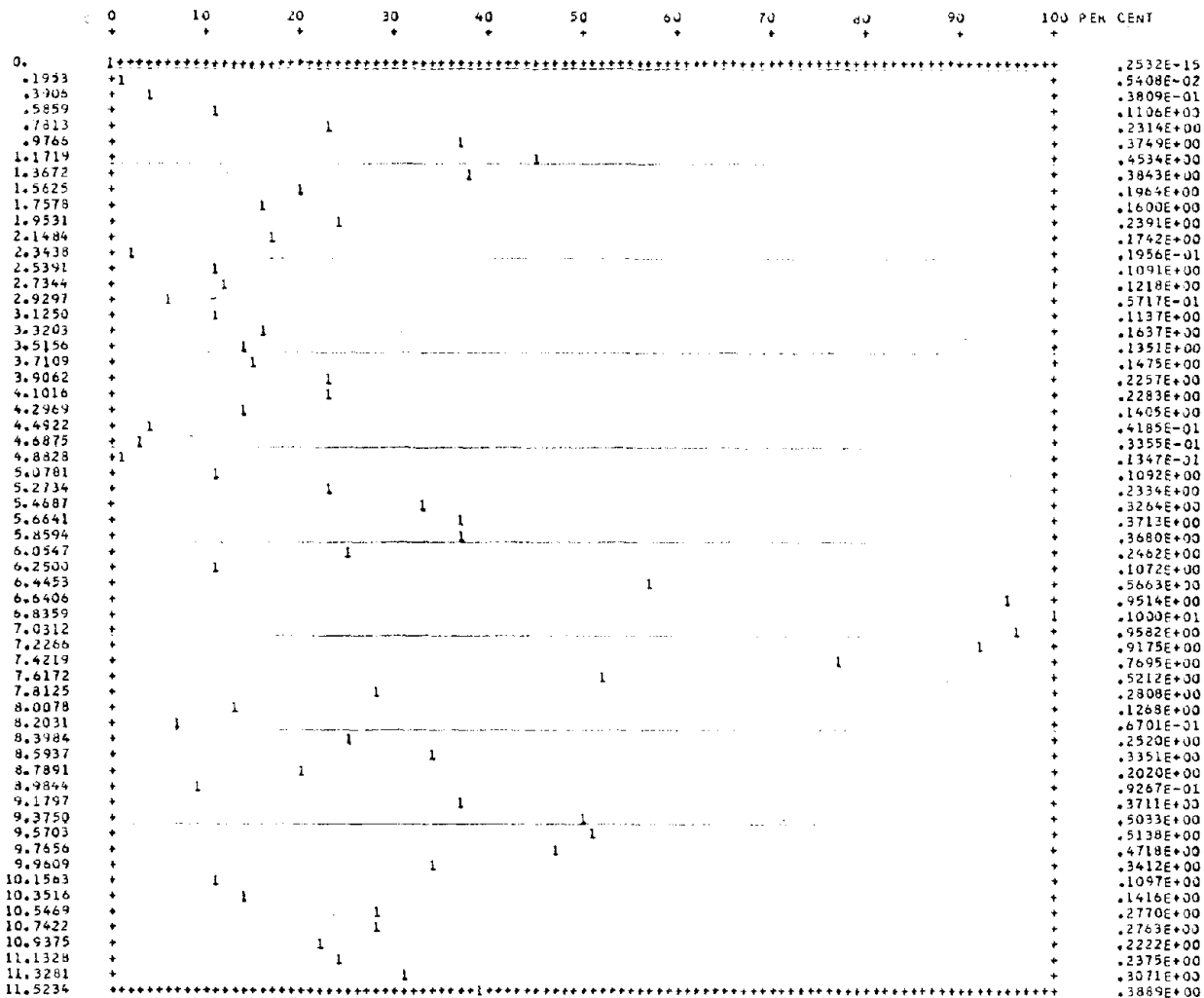






\*\*\* PLOT OF FOURIER AMPLITUDE OF ACCELERATION

X-ACC. FOURIER SPECTRUM AT NODAL PCINT 19, 100 PER CENT CORRESPONDS TO .0188 G



\*\*\*\*\*  
 \*\*\*\*\* FREQ. IN HZ

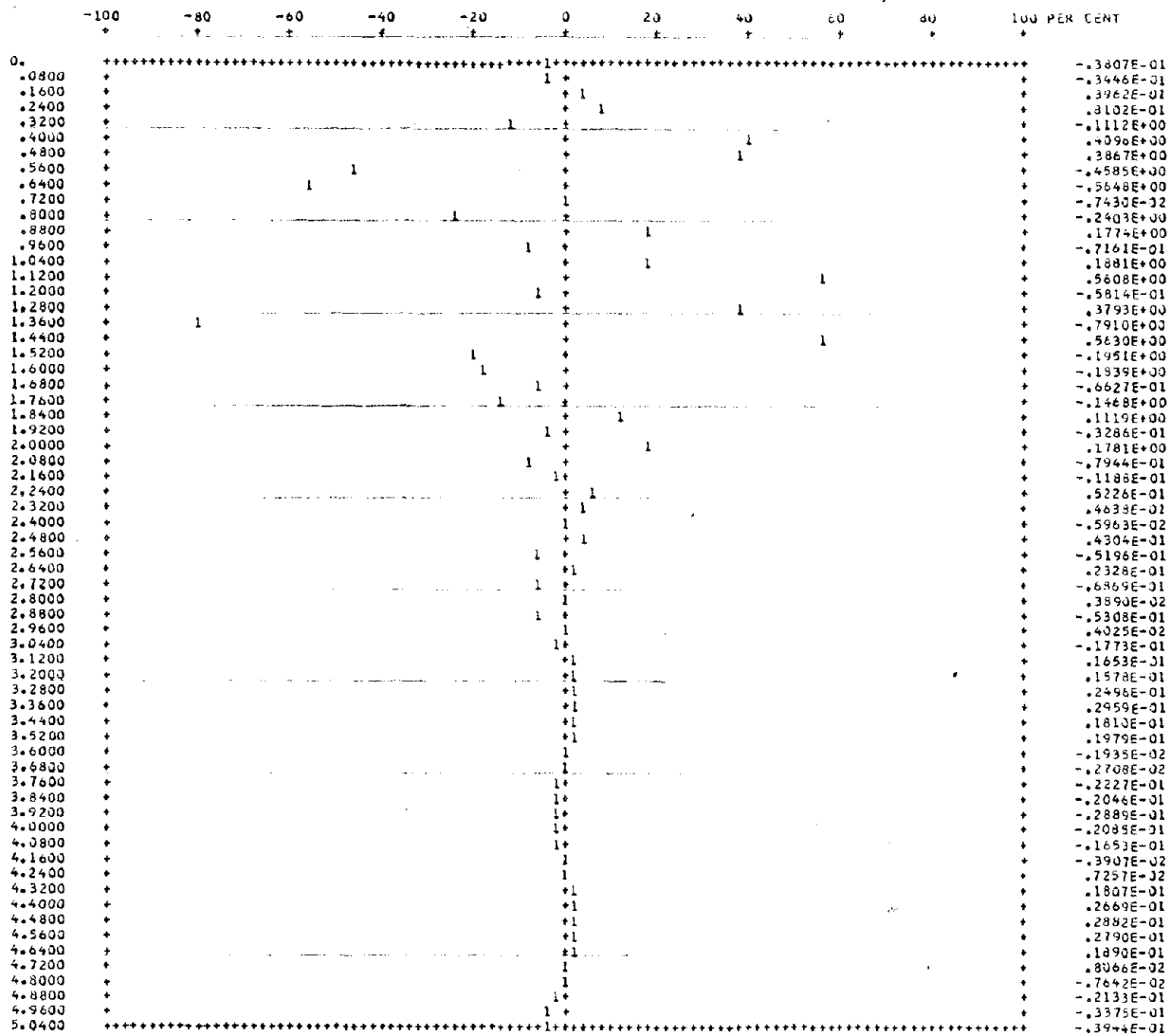




\*\*\* PLOT OF ACCELERATION. (BASE ACC.+REL. ACC.)

X-COMP. OF ACCELERATION AT NODAL POINT 19, 100 PER CENT CORRESPONDS TO .1553 G

EVERY 2 TH POINTS ARE PLOTTED



\*\*\*  
 \*\*\*\*\*  
 \*  
 \*\*\*\*\* TIME IN SECONDS



MAXIMUM ABSOLUTE ACCELERATIONS(G)

ITERATION NO. 1  
 MAX. INPUT BASE ACC. = .0728 (G) IN X DIRECTION  
 MAX. INPUT BASE ACC. = 0. (G) IN Y DIRECTION  
 MAX. INPUT BASE ACC. = 0. (G) IN Z DIRECTION

N.P.	XORD	YORD	ZORD	X-ACC	AT TIME	Y-ACC	AT TIME	Z-ACC	AT TIME
1	46.00	30.00	40.00	.2884	1.0800	0.	.8600	0.	.9600
3	46.00	30.00	20.00	.1384	1.0000	0.	.9600	.1113	1.0800
19	24.00	20.00	40.00	.1553	1.0000	.0374	1.0800	0.	.9600

TIME REQUIRED FOR COMPUTATION AND OUTPUT OF ACCELERATIONS AT 5 DEG. OF FDN. = .234 SEC



\*\*\*\*\*  
\* H0DE3 \*  
\*\*\*\*\*

\*\*\* BLANK COMMON REQUIREMENTS \*\*\*

LENGTH OF BLANK COMMON = 15000

BLANK COMMON REQUIRED FOR VEDDA = 1497

BLANK COMMON FOR PERMANENT DATA = 938

BLANK COMMON REQUIRED FOR STRAIN = 1767

BLANK COMMON FOR BBBC = 6616

MAX. WORDS REQUIRED FOR BBBC = 96

BLANK COMMON REQUIRED FOR MOTION = 1168

BLANK COMMON FOR AAAC = 6916

MAX. WORDS REQUIRED FOR AAAC = 16



EXAMPLE PROBLEM FOR TLUSH

RUN NUMBER = 1

\*\*\* INPUT DATA \*\*\*  
 TOTAL NUMBER OF ELEMENTS = 25  
 TOTAL NUMBER OF NODAL POINTS = 44  
 1ST NODAL POINT ON RIGID BASE = 26  
 DEGREES OF FREEDOM = 75  
 NUMBER OF BOUNDARY CONDITIONS = 18  
 NUMBER OF ITERATIONS = 1  
 NUMBER OF MATERIAL TYPES = 2

\*\*\* INPUT ACTION \*\*\*  
 TOTAL NO. OF POINTS USED IN FFT = 128  
 LAST POINT TO BE READ = 64  
 TIME STEP OF RECORD = .040 SEC.  
 DURATION OF RECORD = 2.560 SEC.  
 QUIET ZONE OF RECORD = 2.560 SEC.  
 TOTAL DURATION OF ANALYSIS = 5.120 SEC.  
 EQ. MULTIPLICATION FACTOR = 2.0000  
 MAX. ACCEL. AFTER SCALING = .0728 (G)  
 MOTION IN X-DIRECTION = 1.0000 TIMES INPUT MOTION  
 MOTION IN Y-DIRECTION = 0. TIMES INPUT MOTION  
 MOTION IN Z-DIRECTION = 0. TIMES INPUT MOTION

BASE LINE CORRECTION WILL BE APPLIED

\*\*\* FREQUENCY CONTENT OF ANALYSIS \*\*\*  
 HIGHEST FREQUENCY OF ANALYSIS = 12.000 HZ  
 NO. OF INTERPOLATION RANGES = 3  
 0. -- 5.000 HZ(KINT= 5)  
 5.000 -- 10.000 HZ(KINT= 3)  
 10.000 -- 12.000 HZ(KINT= 5)  
 FREQ. USED IN THE LAST ITERATION = 12.000 HZ

\*\*\* OUTPUT CONTROL \*\*\*  
 SAVING AMP. FUNCTION ON TAPE = NO  
 COMPUTING NEW SOIL PROPERTIES = YES  
 PUNCHING NEW SOIL PROPERTIES = NO  
 STEPS TO FIND MAX. RESPONSE = 128  
 SKIP POINTS FOR PLOTTING = 2  
 FACTOR FOR EFF. SHEAR STRAIN = .6500

\*\*\* MASS MATRIX \*\*\*  
 FRACTION OF CONSISTENT MASS = .5000  
 FRACTION OF LUMPED MASS = .5000





\*\*\* RESPONSE SPECTRUM \*\*\*  
 NUMBER OF DAMPING VALUES = 1  
 FRACTION OF CRITICAL DAMPING = .0500  
 FIRST FREQUENCY OF SPECTRUM = 1.4000 HZ  
 LAST FREQUENCY OF SPECTRUM = 40.0000 HZ  
 NO. OF FREQUENCY STEPS = 41

\*\*\* DAMPING CHARACTERISTICS \*\*\*  
 UNIFORM DAMPING USED = NO

\*\*\* OUTPUT REQUIREMENTS \*\*\*  
 CODES FOR KEYSPC  
 0--MAXIMUM ACCELERATION ONLY  
 1--PLOT ACC. TIME HISTORY  
 10--PLOT ACC. AND VELOCITY RESPONSE SPECTRUM  
 100--PUNCH ACC. TIME HISTORY  
 1000--PLOT FOURIER AMP. OF ACCELERATION  
 10000--PLOT AMPLIFICATION FUNCTION

VALUES OF KEYSPC

N.P.NO.	X-OUTPUT	Y-OUTPUT	Z-OUTPUT
1	1010	0	0

CODES FOR KEYSTR  
 1--PLOT TIME HISTORIES OF STRESSES  
 10--PRINT TIME HISTORIES OF STRESSES  
 100--PUNCH TIME HISTORIES OF STRESSES

VALUES OF KEYSTR

EL.NO.	KEYSTR
14	1

\*\*\* SHEAR MODULUS AND DAMPING CURVES FOR MATERIALS WITH STRAIN-DEPENDENT PROPERTIES \*\*\*

STANDARD BUILT IN CURVES ARE USED

MAT. TYPE -- IDENTIFICATION  
 1 SEED AND IDRISS (1970) -- CLAY  
 2 SEED AND IDRISS (1970) -- SAND

SHEAR MODULUS CURVES

MAT. TYPE	SHEAR STRAIN IN PERCENT											
	10**(-4.0)	10**(-3.5)	10**(-3.0)	10**(-2.5)	10**(-2.0)	10**(-1.5)	10**(-1.0)	10**(-0.5)	10**(0.0)	10**(0.5)	10**(1.0)	
1	.10E+01	.91E+00	.76E+00	.57E+00	.40E+00	.26E+00	.15E+00	.76E-01	.37E-01	.13E-01	.40E-02	
2	.10E+01	.98E+00	.93E+00	.83E+00	.66E+00	.44E+00	.25E+00	.12E+00	.49E-01	.49E-01	.49E-01	

DAMPING CURVES (PERCENT)

MAT. TYPE	SHEAR STRAIN IN PERCENT											
	10**(-4.0)	10**(-3.5)	10**(-3.0)	10**(-2.5)	10**(-2.0)	10**(-1.5)	10**(-1.0)	10**(-0.5)	10**(0.0)	10**(0.5)	10**(1.0)	
1	.25E+01	.25E+01	.25E+01	.35E+01	.48E+01	.65E+01	.93E+01	.14E+02	.20E+02	.26E+02	.29E+02	
2	.50E+00	.80E+00	.17E+01	.32E+01	.56E+01	.10E+02	.16E+02	.21E+02	.25E+02	.25E+02	.25E+02	



\*\*\* ELEMENT DATA \*\*\*

NO.	NP1	NP2	NP3	NP4	NP5	NP6	NP7	NP8	HTYPE	ID,EL	PD	UNIT WT. (PCF)	G-MAX (KSF)	G-USE (KSF)	DMP-USE (FRACTION)	VS-USE (FPS)	
1	10	14	5	1	11	15	6	2	1	1	1	.450	110.000	1500.000	670.887	.044	443.156
2	11	15	6	2	12	16	7	3	1	1	1	.450	110.000	1500.000	716.409	.042	457.944
3	12	16	7	2	13	17	8	4	1	1	1	.450	110.000	1500.000	692.458	.043	450.224
4	13	17	8	4	14	18	9	5	1	2	1	.450	110.000	1500.000	799.829	.037	483.872
5	14	17	8	5	15	18	9	6	1	1	1	.450	110.000	1500.000	577.235	.049	411.063
6	15	18	9	6	16	18	9	7	1	1	1	.450	110.000	1500.000	570.316	.050	408.591
7	16	18	9	7	17	19	10	8	1	2	1	.450	110.000	1500.000	584.308	.049	413.573
8	17	19	10	8	18	20	11	9	1	1	1	.450	110.000	1500.000	610.362	.047	422.693
9	18	19	10	9	19	21	12	10	1	2	1	.450	110.000	1500.000	590.178	.048	415.646
10	19	19	10	10	20	22	13	11	2	3	1	.350	120.000	1500.000	1125.169	.043	549.473
11	20	20	15	11	21	23	14	12	2	3	1	.350	120.000	1500.000	1055.399	.049	532.164
12	21	21	16	12	22	24	15	13	2	3	1	.350	120.000	1500.000	1050.066	.050	530.818
13	22	22	17	13	23	25	16	14	2	4	1	.350	120.000	1500.000	1161.362	.039	558.240
14	23	23	18	14	24	26	17	15	2	5	1	.350	120.000	1500.000	1020.782	.053	523.364
15	24	24	19	15	25	27	18	16	2	5	1	.350	120.000	1500.000	996.117	.055	517.002
16	25	25	20	16	26	28	19	17	2	6	1	.350	120.000	1500.000	1007.374	.054	519.915
17	26	26	21	17	27	29	20	18	2	5	1	.350	120.000	1500.000	1043.016	.050	529.033
18	27	27	22	18	28	30	21	19	2	4	1	.350	120.000	1500.000	1008.343	.054	520.165
19	28	28	23	19	29	31	22	20	2	3	1	.350	120.000	1500.000	1001.486	.054	518.394
20	29	29	24	20	30	32	23	21	2	3	1	.350	120.000	1500.000	1000.438	.054	518.123
21	30	30	25	21	31	33	24	22	2	4	1	.350	120.000	1500.000	1002.356	.054	518.619
22	31	31	26	22	32	34	25	23	2	5	1	.350	120.000	1500.000	1096.019	.045	542.308
23	32	32	27	23	33	35	26	24	2	6	1	.350	120.000	1500.000	1062.967	.049	534.069
24	33	33	28	24	34	36	27	25	2	3	1	.350	120.000	1500.000	1122.382	.043	548.792
25	34	34	29	25	35	37	28	26	2	4	1	.350	120.000	1500.000	1095.696	.045	542.229

\*\*\* NODAL PCENT DATA \*\*\*

N.P.	XORD(FT)	YORD(FT)	ZORD(FT)	MASS (KIPS)	BOUNDARY CONDITIONS		
					X-DIRECTION	Y-DIRECTION	Z-DIRECTION
1	46.000	30.000	40.000	-0.		FIXED	FIXED
2	46.000	30.000	30.000	-0.		FIXED	
3	46.000	30.000	20.000	0.		FIXED	
4	46.000	30.000	10.000	-0.		FIXED	
5	46.000	20.000	40.000	-0.		FIXED	FIXED
6	46.000	20.000	30.000	-0.		FIXED	
7	46.000	20.000	20.000	-0.		FIXED	
8	46.000	10.000	40.000	-0.		FIXED	FIXED
9	46.000	10.000	30.000	-0.		FIXED	
10	36.000	30.000	40.000	-0.		FIXED	FIXED
11	36.000	30.000	30.000	0.			
12	36.000	30.000	20.000	0.			
13	36.000	30.000	10.000	-0.			
14	36.000	20.000	40.000	-0.			FIXED
15	36.000	20.000	30.000	0.			
16	36.000	20.000	20.000	-0.			
17	36.000	10.000	40.000	-0.			FIXED
18	36.000	10.000	30.000	-0.			
19	24.000	20.000	40.000	-0.			FIXED
20	24.000	20.000	30.000	0.			
21	24.000	20.000	20.000	-0.			
22	24.000	10.000	40.000	-0.			FIXED
23	24.000	10.000	30.000	-0.			
24	12.000	10.000	40.000	-0.			FIXED
25	12.000	10.000	30.000	-0.			
26	0.	0.	40.000	-0.	FIXED	FIXED	FIXED
27	12.000	0.	40.000	0.	FIXED	FIXED	FIXED
28	24.000	0.	40.000	0.	FIXED	FIXED	FIXED
29	36.000	0.	40.000	-0.	FIXED	FIXED	FIXED
30	46.000	0.	40.000	-0.	FIXED	FIXED	FIXED
31	0.	0.	30.000	-0.	FIXED	FIXED	FIXED
32	12.000	0.	30.000	0.	FIXED	FIXED	FIXED
33	24.000	0.	30.000	0.	FIXED	FIXED	FIXED
34	36.000	0.	30.000	-0.	FIXED	FIXED	FIXED
35	46.000	0.	30.000	-0.	FIXED	FIXED	FIXED
36	12.000	10.000	20.000	-0.	FIXED	FIXED	FIXED
37	24.000	10.000	20.000	0.	FIXED	FIXED	FIXED
38	36.000	10.000	20.000	-0.	FIXED	FIXED	FIXED
39	46.000	10.000	20.000	-0.	FIXED	FIXED	FIXED
40	24.000	20.000	10.000	-0.	FIXED	FIXED	FIXED
41	36.000	20.000	10.000	-0.	FIXED	FIXED	FIXED
42	46.000	20.000	10.000	-0.	FIXED	FIXED	FIXED
43	36.000	30.000	0.	-0.	FIXED	FIXED	FIXED
44	46.000	30.000	0.	-0.	FIXED	FIXED	FIXED



\*\*\* EARTHQUAKE RECORD \*\*\*

\*\* EXAMPLE EARTHQUAKE \*\* 0.04 SEC 64 POINTS

INPUT ACCEL. PRIOR TO SCALING

MAX. ACC. = .0500 (G) AT TIME = .9600 SEC.

.300204	-.000191	.000058	.001460	.004111	.004390	.039534	-.003481	1
.013073	.009455	.000374	.018791	-.022305	-.018976	-.011557	-.048139	2
.013707	-.029759	.011100	-.025240	.015560	-.013417	.023272	-.013397	3
.050000	.000068	.012727	-.006808	.010056	.010332	.006599	-.031122	4
.008954	-.020449	.015687	-.005003	-.017561	.003863	-.010918	-.028641	5
-.008450	-.013743	.001941	-.000952	.000568	-.001998	.010888	.008075	6
.009841	.003844	-.005115	.001176	-.003725	-.005514	.001117	-.000450	7
.000281	-.000221	.000197	-.000188	.000185	-.000188	.000193	-.000200	8
0.	0.	0.	0.	0.	0.	0.	0.	9
0.	0.	0.	0.	0.	0.	0.	0.	10
0.	0.	0.	0.	0.	0.	0.	0.	11
0.	0.	0.	0.	0.	0.	0.	0.	12
0.	0.	0.	0.	0.	0.	0.	0.	13
0.	0.	0.	0.	0.	0.	0.	0.	14
0.	0.	0.	0.	0.	0.	0.	0.	15
0.	0.	0.	0.	0.	0.	0.	0.	16

TIME REQUIRED FOR INPUT AND INITIALIZATION = .088 SEC

ELM	G-USED (KSEI)	G-NEW (KSEI)	DIF-G (PERCENT)	DAMP-USED (FRACTION)	DAMP-NEW (FRACTION)	DIF-DAMP (PERCENT)	VS-USED (FPS)	VS-NEW (FPS)	DIF-VS (PERCENT)	MAX. S. STRAIN (PERCENT)
1	671.	611.	9.8	.044	.047	-6.4	443.	423.	4.8	.01461
2	716.	682.	5.0	.042	.043	-4.3	458.	447.	2.5	.01049
3	692.	677.	2.3	.043	.044	-1.8	450.	445.	1.1	.01075
4	800.	763.	4.8	.037	.039	-4.7	484.	473.	2.4	.00721
5	577.	546.	5.7	.049	.052	-5.5	411.	400.	2.8	.02070
6	570.	555.	2.7	.050	.051	-2.5	409.	403.	1.4	.01972
7	584.	573.	1.9	.049	.050	-1.9	414.	410.	1.0	.01784
8	610.	598.	2.1	.047	.048	-1.5	423.	418.	1.0	.01557
9	590.	580.	1.7	.048	.049	-1.7	416.	412.	.8	.01715
10	1125.	1075.	4.6	.043	.047	-9.9	549.	537.	2.3	.01019
11	1055.	1030.	2.5	.049	.052	-4.7	532.	525.	1.2	.01251
12	1050.	1039.	1.1	.050	.051	-2.0	531.	528.	.5	.01200
13	1161.	1129.	2.9	.039	.042	-7.2	558.	550.	1.4	.00800
14	1021.	998.	2.3	.053	.056	-3.9	523.	518.	1.1	.01443
15	996.	985.	1.1	.055	.056	-1.8	517.	514.	.5	.01529
16	1027.	996.	3.1	.054	.055	-2.0	520.	517.	.6	.01458
17	1043.	1035.	.8	.050	.051	-1.5	529.	527.	.4	.01224
18	1008.	999.	.9	.054	.055	-1.5	520.	518.	.4	.01435
19	1031.	969.	6.4	.054	.058	-6.5	518.	513.	1.7	.01627
20	1000.	986.	1.5	.054	.056	-2.5	518.	514.	.8	.01528
21	1002.	989.	1.3	.054	.055	-2.2	519.	515.	.7	.01501
22	1096.	1089.	.6	.045	.046	-1.4	542.	541.	.3	.00957
23	1063.	1056.	.7	.049	.049	-1.4	534.	532.	.3	.01113
24	1122.	1116.	.6	.043	.044	-1.5	549.	547.	.3	.00849
25	1096.	1089.	.6	.045	.046	-1.4	542.	541.	.3	.00958

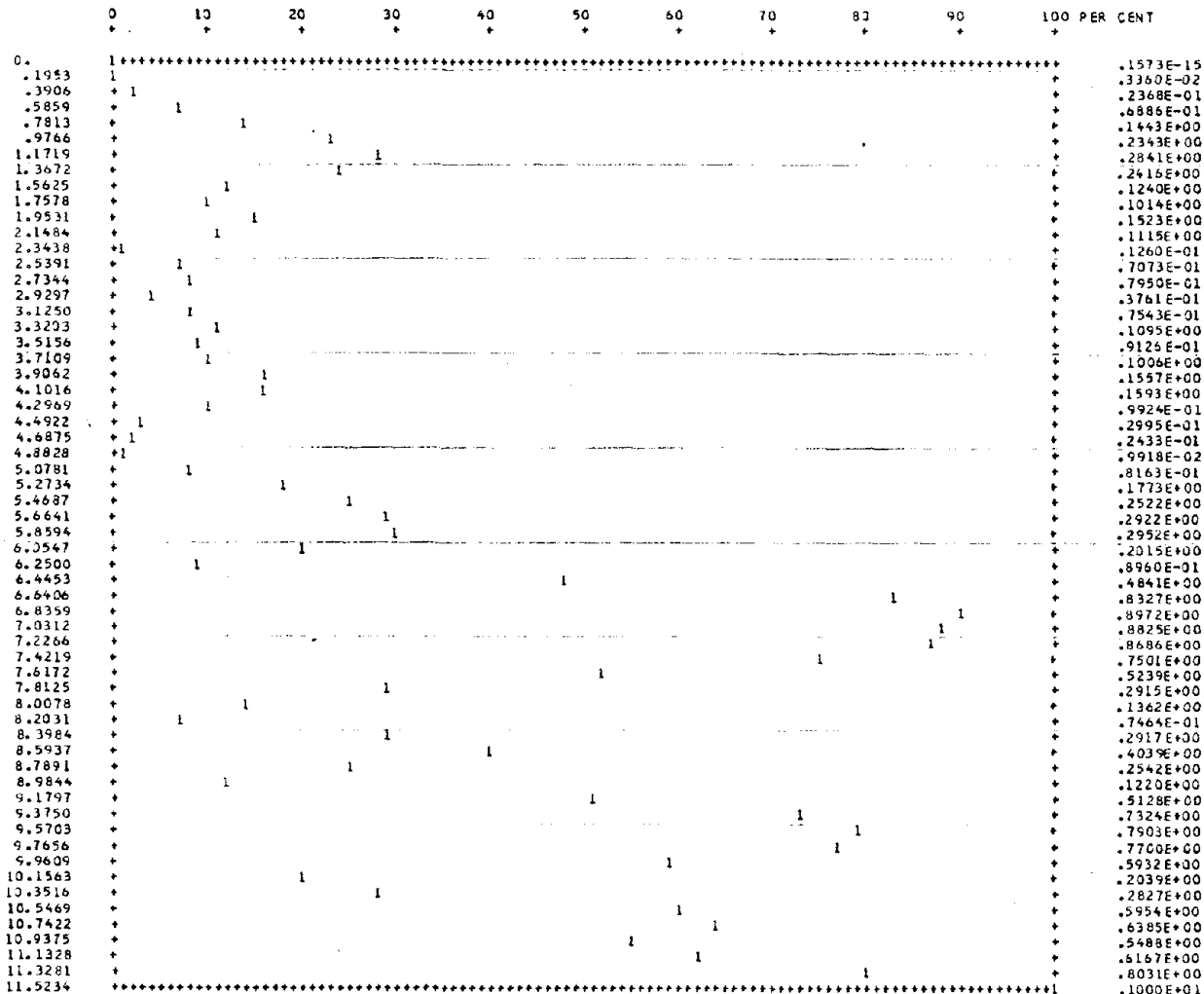
\*\*\* MAX. SHEAR STRAIN COMPUTATION IN FREQ DOMAIN

TIME REQUIRED FOR COMPUTING STRESS AND STRAIN = .452 SEC



\*\*\* PLOT OF FOURIER AMPLITUDE OF ACCELERATION

X-ACC. FOURIER SPECTRUM AT NCDAL PCINT 1, 100 PER CENT CORRESPONDS TO .0303 G



\*\*\*  
\*\*\*\*\*  
\*  
\*\*\*\*\* FREQ. IN HZ





EXAMPLE PROBLEM FOR TLUSH  
 X-COMPONENT OF MOTION AT NODAL POINT 1  
 MAXIMUM ACCELERATION = .2884  
 TIME INTERVAL = .0400 SEC.

DAMPING RATIO = .05

TIMES AT WHICH MAX. SPECTRAL VALUES OCCUR  
 TD = TIME FOR MAX. RELATIVE DISP.  
 TV = TIME FOR MAX. RELATIVE VEL.  
 TA = TIME FOR MAX. ABSOLUTE ACC.

PER = 2.5000	TIMES FOR MAXIMA	TD = .9600	TV = .7600	TA = .9600
PER = 2.2281	TIMES FOR MAXIMA	TD = .9600	TV = 1.2800	TA = .9600
PER = 1.9858	TIMES FOR MAXIMA	TD = .9600	TV = 1.2800	TA = .9600
PER = 1.7695	TIMES FOR MAXIMA	TD = .9500	TV = 1.2800	TA = .9600
PER = 1.5774	TIMES FOR MAXIMA	TD = 1.5600	TV = 1.1600	TA = 1.4400
PER = 1.4059	TIMES FOR MAXIMA	TD = 1.4400	TV = 1.1600	TA = 1.4400
PER = 1.2530	TIMES FOR MAXIMA	TD = 1.4400	TV = 1.6400	TA = 1.4400
PER = 1.1167	TIMES FOR MAXIMA	TD = 1.8400	TV = 1.6400	TA = 1.8400
PER = .9953	TIMES FOR MAXIMA	TD = 1.8000	TV = 1.4800	TA = 1.7600
PER = .8870	TIMES FOR MAXIMA	TD = 1.6800	TV = 1.8800	TA = 1.6800
PER = .7906	TIMES FOR MAXIMA	TD = 2.0000	TV = 1.0000	TA = 1.1600
PER = .7046	TIMES FOR MAXIMA	TD = 1.1200	TV = 1.0000	TA = 1.1200
PER = .6280	TIMES FOR MAXIMA	TD = 1.0400	TV = 1.2400	TA = 1.0400
PER = .5597	TIMES FOR MAXIMA	TD = 1.0400	TV = 1.0800	TA = 1.0000
PER = .4988	TIMES FOR MAXIMA	TD = .8400	TV = 1.0800	TA = .6400
PER = .4446	TIMES FOR MAXIMA	TD = .8400	TV = .7200	TA = .6400
PER = .3962	TIMES FOR MAXIMA	TD = .6400	TV = .6400	TA = .6400
PER = .3531	TIMES FOR MAXIMA	TD = .6400	TV = 1.0000	TA = .6400
PER = .3147	TIMES FOR MAXIMA	TD = 1.0400	TV = 1.0000	TA = 1.0400
PER = .2805	TIMES FOR MAXIMA	TD = 1.0400	TV = 1.0800	TA = 1.0000
PER = .2500	TIMES FOR MAXIMA	TD = 1.2000	TV = 1.1600	TA = 1.2000
PER = .2228	TIMES FOR MAXIMA	TD = .5600	TV = 1.2400	TA = .5600
PER = .1986	TIMES FOR MAXIMA	TD = 1.2000	TV = 1.2400	TA = 1.2000
PER = .1770	TIMES FOR MAXIMA	TD = 1.2400	TV = 1.2800	TA = 1.2400
PER = .1577	TIMES FOR MAXIMA	TD = 1.3200	TV = 1.2800	TA = 1.2400
PER = .1406	TIMES FOR MAXIMA	TD = 1.5600	TV = 1.6000	TA = 1.5600
PER = .1253	TIMES FOR MAXIMA	TD = 1.1200	TV = 1.1600	TA = 1.1200
PER = .1117	TIMES FOR MAXIMA	TD = 1.1200	TV = .5600	TA = 1.1200
PER = .0995	TIMES FOR MAXIMA	TD = .8800	TV = .6000	TA = .7200
PER = .0887	TIMES FOR MAXIMA	TD = 1.2800	TV = 1.0800	TA = 1.2800
PER = .0791	TIMES FOR MAXIMA	TD = 1.0400	TV = .4800	TA = 1.0400
PER = .0705	TIMES FOR MAXIMA	TD = 1.2000	TV = .6400	TA = .7600
PER = .0628	TIMES FOR MAXIMA	TD = .6400	TV = 1.0800	TA = .6400
PER = .0560	TIMES FOR MAXIMA	TD = 1.0800	TV = 1.5200	TA = 1.0800
PER = .0499	TIMES FOR MAXIMA	TD = 1.0400	TV = 1.0400	TA = 1.0400
PER = .0445	TIMES FOR MAXIMA	TD = 1.0400	TV = 1.0400	TA = 1.0400
PER = .0396	TIMES FOR MAXIMA	TD = 1.0400	TV = 1.0400	TA = 1.0400
PER = .0353	TIMES FOR MAXIMA	TD = 1.0400	TV = 1.0400	TA = 1.0400
PER = .0315	TIMES FOR MAXIMA	TD = 1.0400	TV = 1.0400	TA = 1.0400
PER = .0281	TIMES FOR MAXIMA	TD = 1.0400	TV = 1.0800	TA = 1.0400
PER = .0250	TIMES FOR MAXIMA	TD = 1.0400	TV = 1.0800	TA = 1.0400



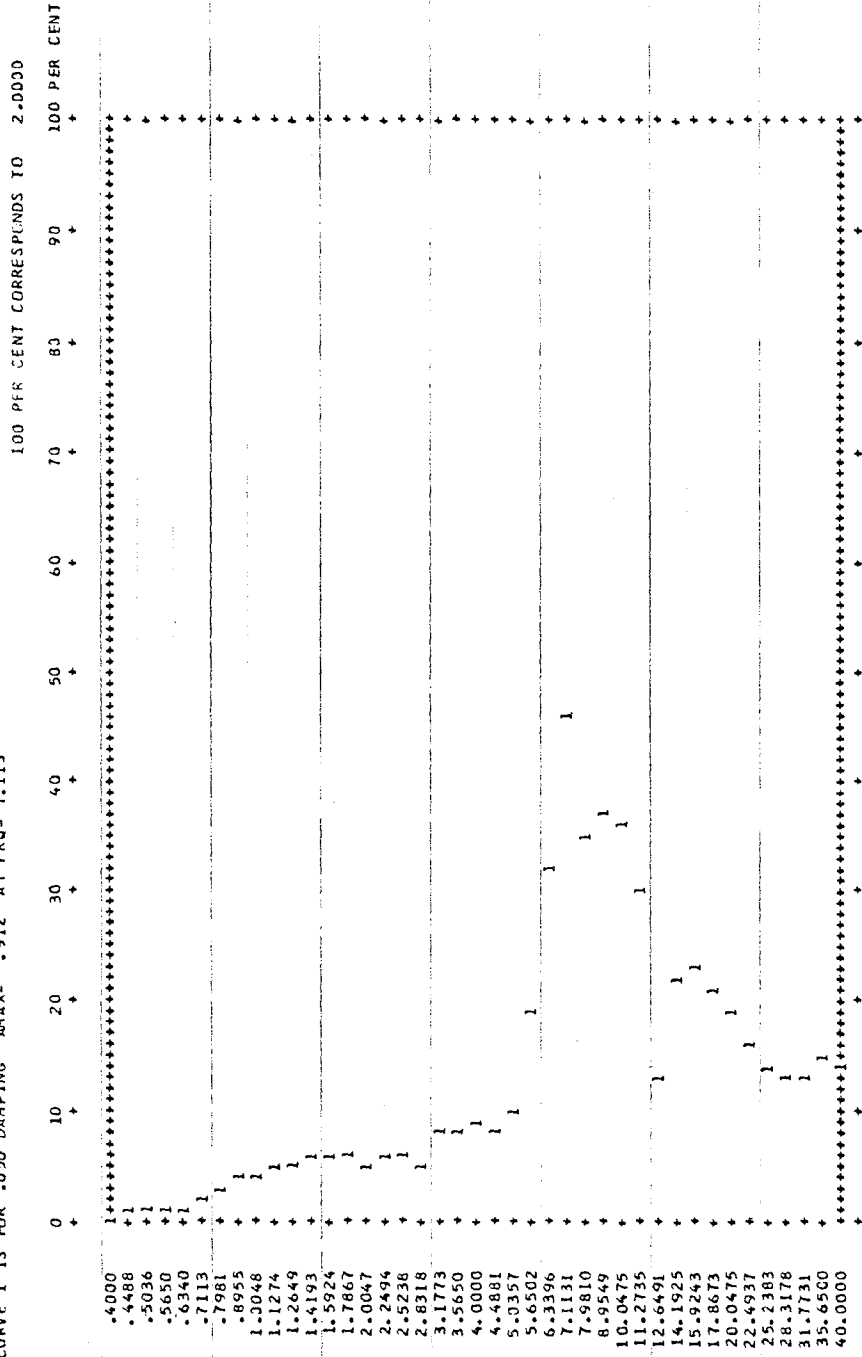
SPECTRAL VALUES  
DAMPING RATIO = .050

NO.	PERIOD (SEC)	REL. DISP. (FT)	REL. VEL. (F/SEC)	PSU. REL. VEL. (F/SEC)	ABS. ACC. (G/S)	PSU. ABS. ACC. (G/S)	FREQ. (HZ)
1	2.5000	.04483	.23270	.11266	.00926	.00879	.4000
2	2.2281	.04719	.24548	.13307	.01165	.01165	.4488
3	1.9858	.04937	.27277	.15621	.01570	.01535	.5036
4	1.7699	.05104	.30166	.18118	.02012	.01998	.5650
5	1.5774	.05447	.33476	.21697	.02764	.02684	.6340
6	1.4059	.06244	.37859	.27905	.04020	.03873	.7113
7	1.2530	.06689	.42458	.33541	.05225	.05223	.7981
8	1.1167	.07125	.48912	.40090	.07106	.07005	.8955
9	.9953	.07244	.48134	.45733	.08952	.08966	1.0048
10	.8870	.06931	.54892	.49093	.10817	.10800	1.1274
11	.7908	.05588	.50806	.44410	.10997	.10961	1.2649
12	.7046	.04445	.43642	.39640	.11010	.10978	1.4193
13	.6280	.04121	.38724	.34233	.12881	.12812	1.5924
14	.5597	.03068	.30630	.34439	.11723	.12007	1.7867
15	.4988	.02106	.30086	.26527	.10688	.10377	2.0047
16	.4446	.01791	.15497	.25311	.11212	.11110	2.2494
17	.3962	.01436	.19366	.22774	.11107	.11216	2.5238
18	.3531	.01105	.22465	.19662	.10549	.10865	2.8318
19	.3147	.01248	.24778	.24911	.15118	.15444	3.1773
20	.2805	.00998	.30136	.22353	.15463	.15949	3.5650
21	.2500	.00908	.24915	.22816	.17642	.17809	4.0000
22	.2228	.00652	.20412	.18372	.15554	.15089	4.4881
23	.1986	.00666	.24195	.21072	.19281	.20706	5.0357
24	.1770	.00905	.36878	.32120	.37902	.35412	5.6502
25	.1577	.01350	.56752	.53783	.64011	.65531	6.3396
26	.1406	.01444	.64357	.64552	.91197	.89596	7.1131
27	.1253	.00888	.38144	.44512	.70423	.69320	7.9810
28	.1117	.00749	.31623	.42151	.73998	.73654	8.9549
29	.0995	.00578	.35238	.36498	.72526	.71556	10.0475
30	.0887	.00386	.29290	.27341	.59121	.63146	11.2735
31	.0791	.00132	.23861	.10478	.25257	.25862	12.6491
32	.0705	.00171	.17589	.15278	.43702	.42309	14.1925
33	.0628	.00141	.09919	.14153	.45446	.43978	15.9243
34	.0560	.00104	.05693	.11656	.41955	.40639	17.8673
35	.0499	.00074	.01727	.09336	.37196	.36520	20.0475
36	.0445	.00052	.00552	.07314	.32346	.32103	22.4937
37	.0386	.00036	.00299	.05777	.28597	.28450	25.2383
38	.0353	.00026	.00450	.04554	.25415	.25166	28.3178
39	.0315	.00020	.01177	.04030	.25715	.24985	31.7731
40	.0281	.00019	.01705	.04190	.29727	.29147	35.6500
41	.0250	.00014	.01301	.03564	.28350	.27820	40.0000



RESPONSE SPECTRUM ANALYSIS FOR EXAMPLE PROBLEM FOR TLUSH  
 X-COMPONENT OF MOTION AT NODAL POINT 1  
 ABSOLUTE SPECTRAL ACCELERATIONS (G)

CURVE 1 IS FOR .050 DAMPING AMAX= .912 AT FREQ= 7.113



\*\*\*\*\*  
 CYCLES PER SEC.  
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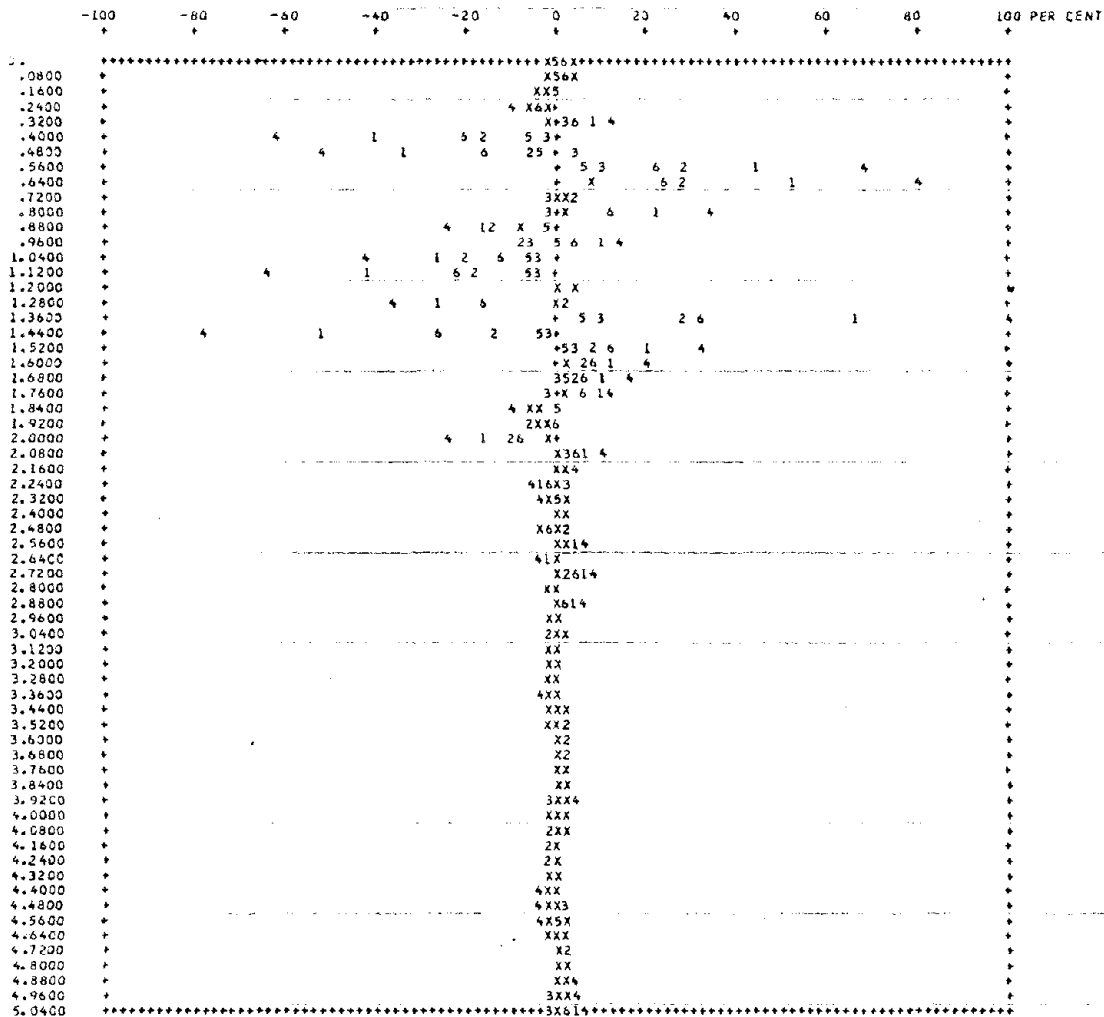




\*\*\* PLOT OF STRESS TIME HISTORY AT ELEMENT NO. 14 \*\*\*

EVERY 2 TH POINTS ARE PLOTTED  
 CURVE NO. 1 CORRESPONDS TO SIG-X  
 \*\* 2 \*\* SIG-Y  
 \*\* 3 \*\* SIG-Z  
 \*\* 4 \*\* SIG-XY  
 \*\* 5 \*\* SIG-YZ  
 \*\* 6 \*\* SIG-ZX

100 PER CENT CORRESPONDS TO 95.3839



\*\*\*\*\* TIME IN SECONDS

TIME FOR OUTPUT OF STRESS HISTORIES = .045 SEC



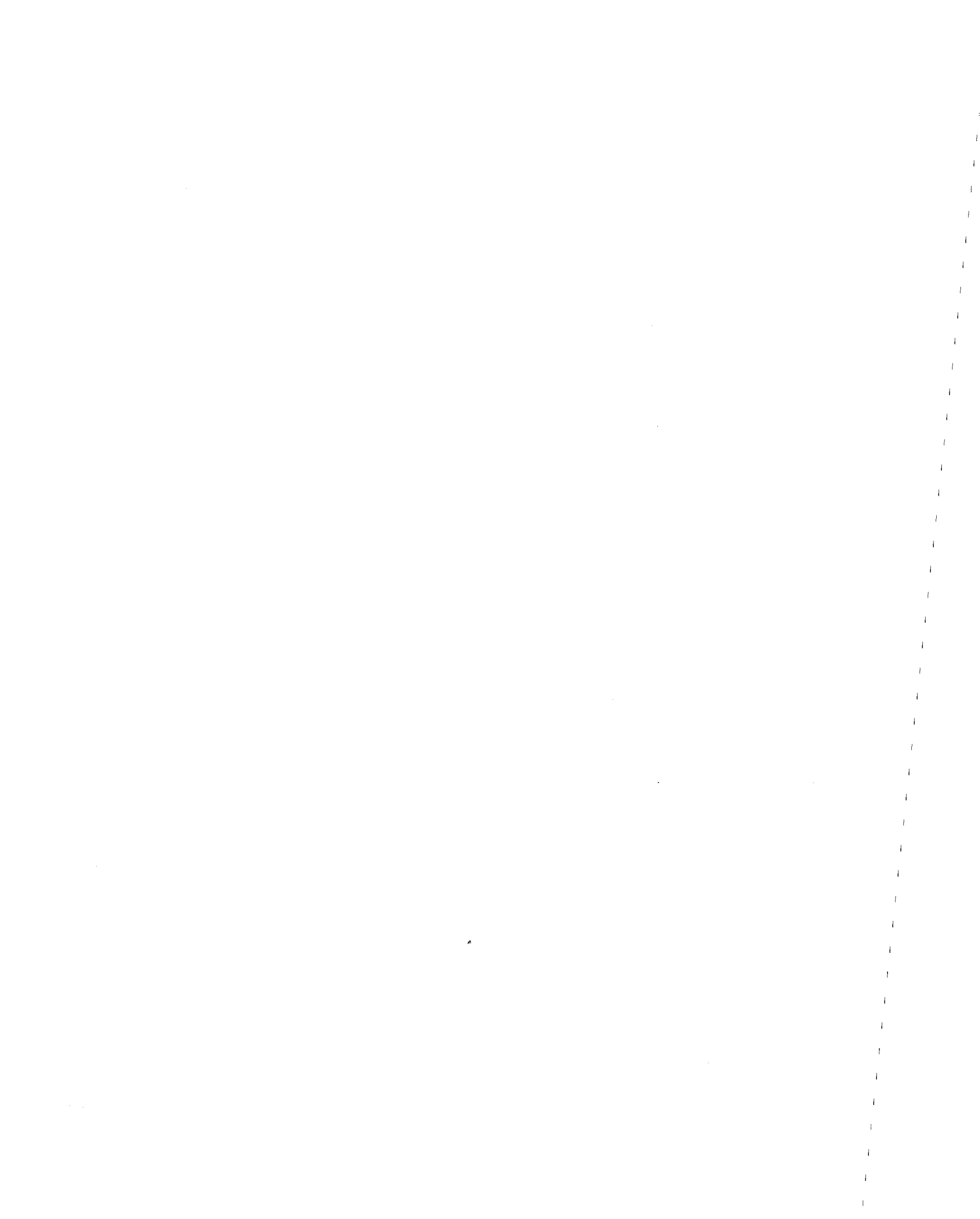
## 7. ACKNOWLEDGEMENTS

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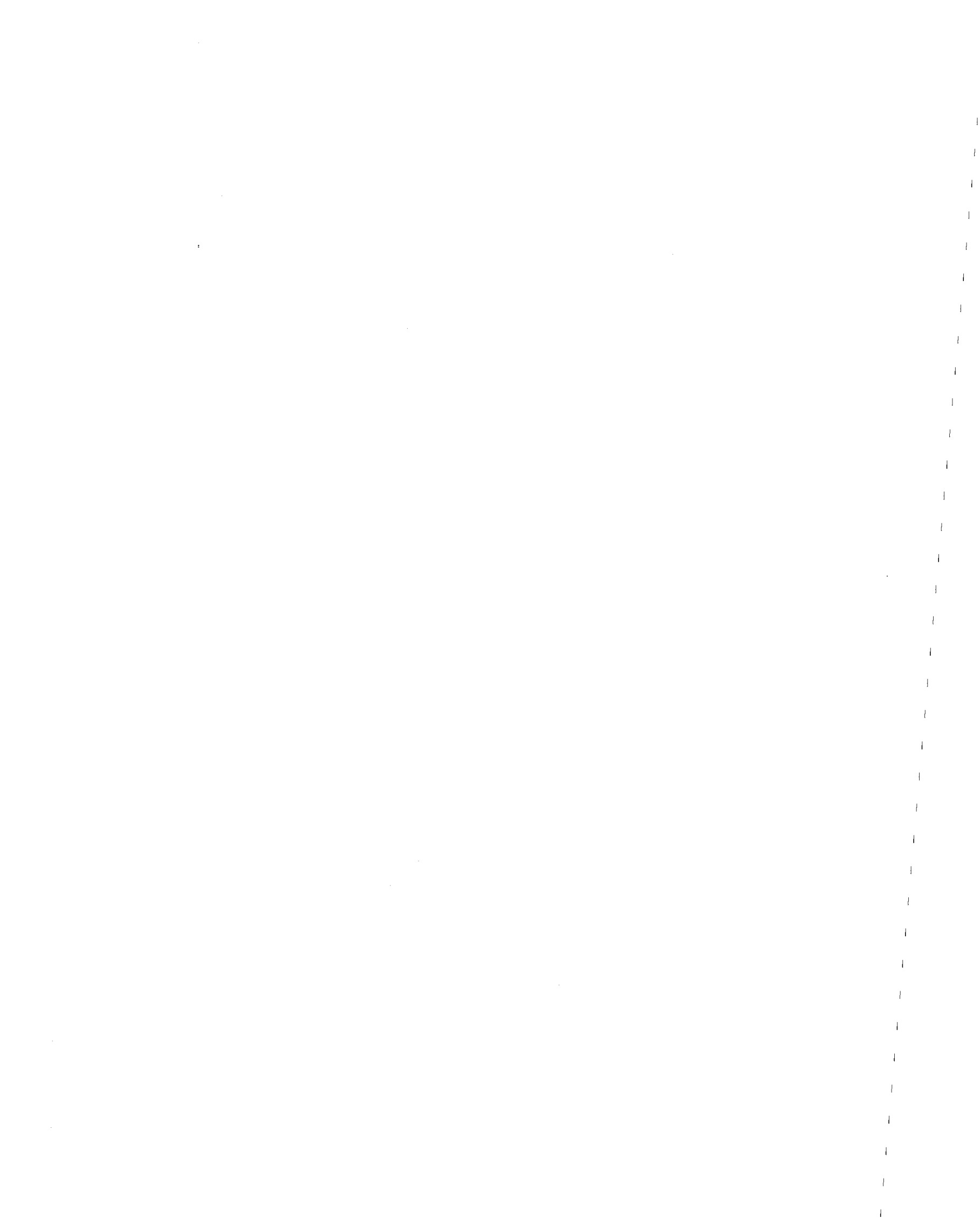




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