

**CIVIL ENGINEERING STUDY
STRUCTURAL SERIES 85-11**

**ODSEWS-2D-II
USER'S MANUAL
A COMPUTER PROGRAM FOR
OPTIMUM DESIGN OF 2-DIMENSIONAL
STEEL STRUCTURES FOR STATIC,
SEISMIC AND WIND FORCES**

**by
Franklin Y. Cheng
Professor**

**Der-Shin Juang
Graduate Assistant**

**Department of Civil Engineering
University of Missouri-Rolla
Rolla, Missouri
1985**



**Report Series
Prepared for the National Science Foundation Under Grant No.
NSF CEE 8213477**

**REPRODUCED BY
U.S. DEPARTMENT OF COMMERCE
NATIONAL TECHNICAL
INFORMATION SERVICE
SPRINGFIELD, VA 22161**

CIVIL ENGINEERING STUDY

STRUCTURAL SERIES 85-11

ODSEWS-2D-II

USER'S MANUAL

A COMPUTER PROGRAM FOR
OPTIMUM DESIGN OF 2-DIMENSIONAL STEEL STRUCTURES
FOR STATIC, SEISMIC, AND WIND FORCES

by

Franklin Y. Cheng

Professor

Der-Shin Juang

Graduate Assistant

Department of Civil Engineering

University of Missouri-Rolla

Rolla, Missouri

1985

Report Series

Prepared for the National Science Foundation Under Grant No.

NSF CEE 8213477

ABSTRACT

This report has been prepared as a user's manual for the computer program, ODSEWS-2D-II, for the optimal design of two-dimensional steel structures subjected to static, earthquake, and wind forces.

The structural systems can be trusses, unbraced and braced frames. The seismic input can be one-dimensional and two-dimensional; one-dimension is horizontal, two-dimension is horizontal coupled with vertical. The dynamic forces may be seismic excitations at the base, dynamic forces applied at structural nodes, and wind forces acting on the structural surfaces. The seismic excitations include actual earthquake records, five groups of response spectra, and three seismic design code provisions of the Uniform Building Code, the Chinese Seismic Design Code, and ATC-3-06. The structural formulation is derived on the basis of the matrix displacement method and consistent mass method associated with the consideration of the second-order P- Δ forces. The constituent members of a system are made of either built-up sections or AISC WF sections. The constraints include stresses, displacements, story drifts, natural frequencies, maximum differences between relative stiffnesses, and lower bound of cross sections. The objective function can be either minimum weight or minimum cost.

The main features of the report include the description of the computer program, instructions for the data preparation, listing of the computer program, preparation of the job control language, sample examples, and a guide to modify the program's capacity.

ACKNOWLEDGMENTS

This report is one of a series for the research program on the aseismic optimum structural design in engineering practice and code provisions. Support of the study was provided by the University of Missouri-Rolla and the National Science Foundation under the Grant No. NSF CEE 8213477. The support is gratefully acknowledged.

TABLE OF CONTENTS

	page
ABSTRACT.....	ii
ACKNOWLEDGEMENTS.....	iii
I. INTRODUCTION.....	1
II. DESCRIPTION OF THE ODSEWS-2D-II COMPUTER PROGRAM.....	2
2.1 Numerical Procedures of the ODSEWS-2D-II Computer Program.....	2
2.2 Description of Subroutines.....	10
2.3 Program Capacity and Guide of Modification.....	19
III. DESCRIPTION OF INPUT DATA.....	24
3.1 Contents of the Input Instructions.....	24
3.2 Input Data Instructions.....	26
3.3 Input Data Stream and Job Control Language.....	49
3.4 Computer Program Listing.....	57
IV. SAMPLE EXAMPLES.....	87
4.1 Description of Input Information for Sample Problems...	87
4.2 Typical Input and Output Solutions.....	94
a. Input Data of Example 1.....	94
b. Output Solution of Example 1.....	94
c. Input Data of Example 2.....	97
d. Output Solution of Example 2.....	99
e. Input Data of Example 3.....	111
f. Output Solution of Example 3.....	113
REFERENCES.....	130

I. INSTRUCTION

This publication is to serve as a user's manual of the computer program, ODSEWS-2D-II (Optimum Design of 2-Dimensional Steel Structures for Static, Earthquake, and Wind Forces - Version II). The numerical procedures of the computer program, the description of subroutines, the program capacity and its modification are included in Chapter II. Chapter III contains the detailed instruction for preparing the input data and the required job control language used to run the program on an AMDAHL 470V/7, AMDAHL 470V/8, or IBM 4341 OS/VS1 system. In Chapter IV, three sample examples are used to illustrate the preparation of input data and the printouts of output solutions.

II. DESCRIPTION OF THE ODSEWS-2D-II COMPUTER PROGRAM

2.1 Numerical Procedures of the ODSEWS-2D-II Computer Program

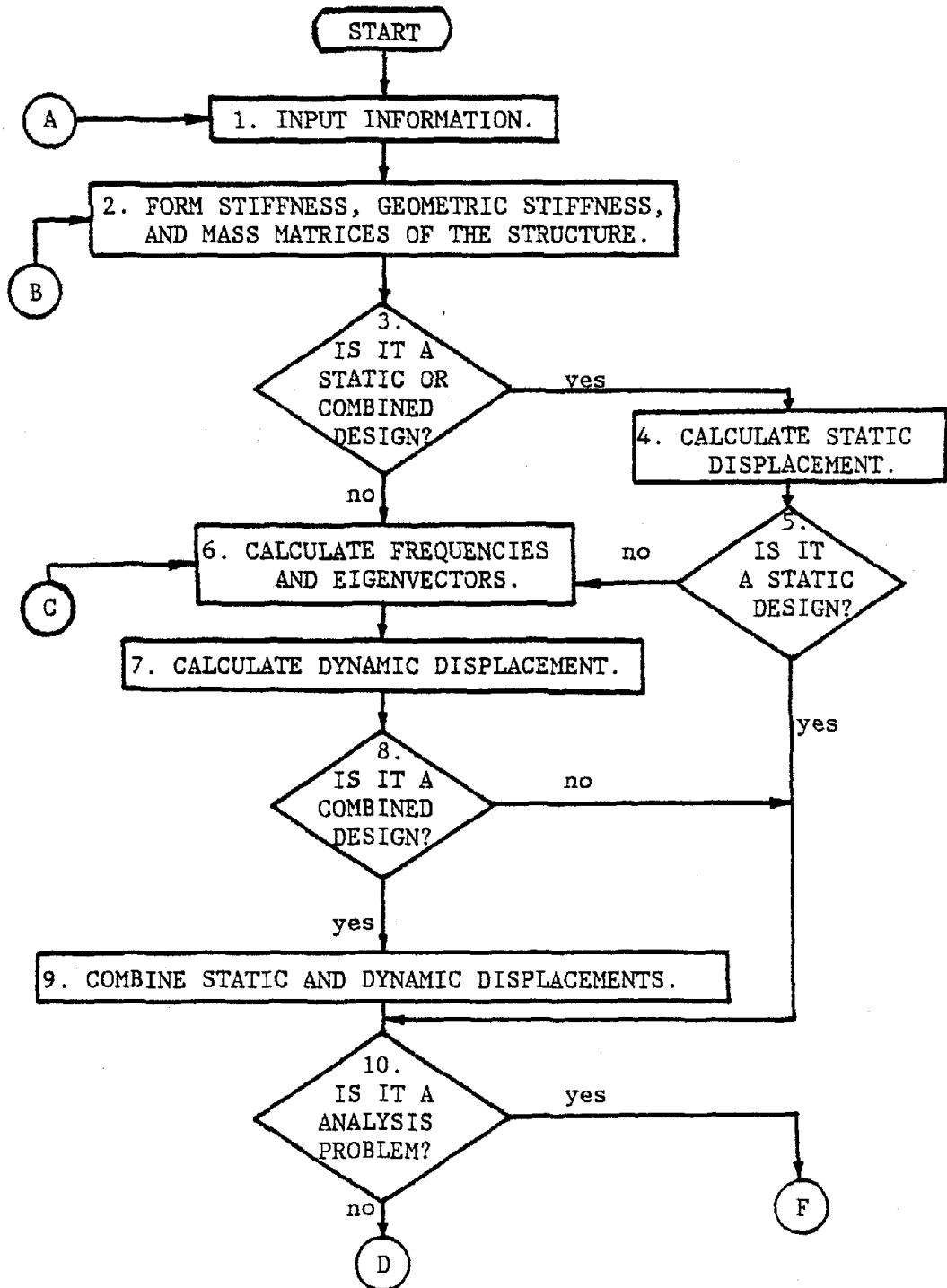
The computer program ODSEWS-2D-II is capable of analyzing and designing two-dimensional steel structures subject to static loads, wind forces, multicomponent earthquake excitations or the combined action of static loads and time-dependent excitations. The numerical procedures are outlined in this section on the basis of the flow chart shown in Figure 2.1.

Step 1. --- Input Data. All analyses or design problems start with reading the input data that are supplied by the user. The input data must be prepared according to the input instructions. The input data are printed out as a user's reference.

Step 2. --- Form Stiffness, Geometric Stiffness and Mass Matrices of the Structure. The initial relative design variables are used to form the stiffness, geometric stiffness, and mass matrices. The initial relative design variables can be supplied by the user in the input data, or else they will be generated automatically as 1.0 for all members. The stiffness and geometric stiffness matrices are formed for all types of loadings. The mass matrix is formed only when the eigen solution is needed.

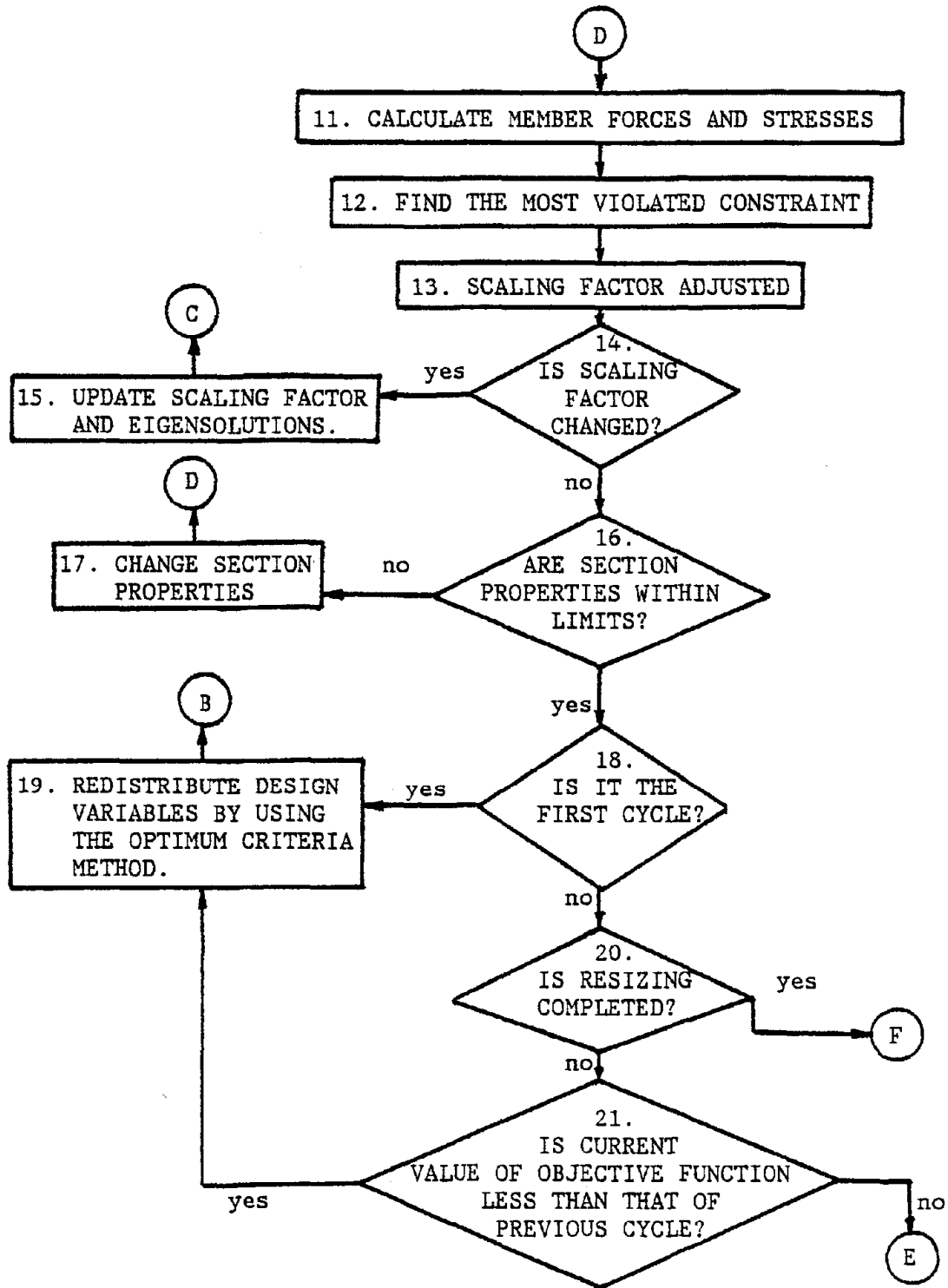
Step 3. --- Is It a Dynamic Design? If it is a dynamic design or an analysis problem, then proceed to Step 6.

Step 4. --- Solve for Static Displacement. The structure is analyzed when it is subjected to static loads. The static loads can be 1) concentrated loads, 2) uniform distributed loads, 3) equivalent



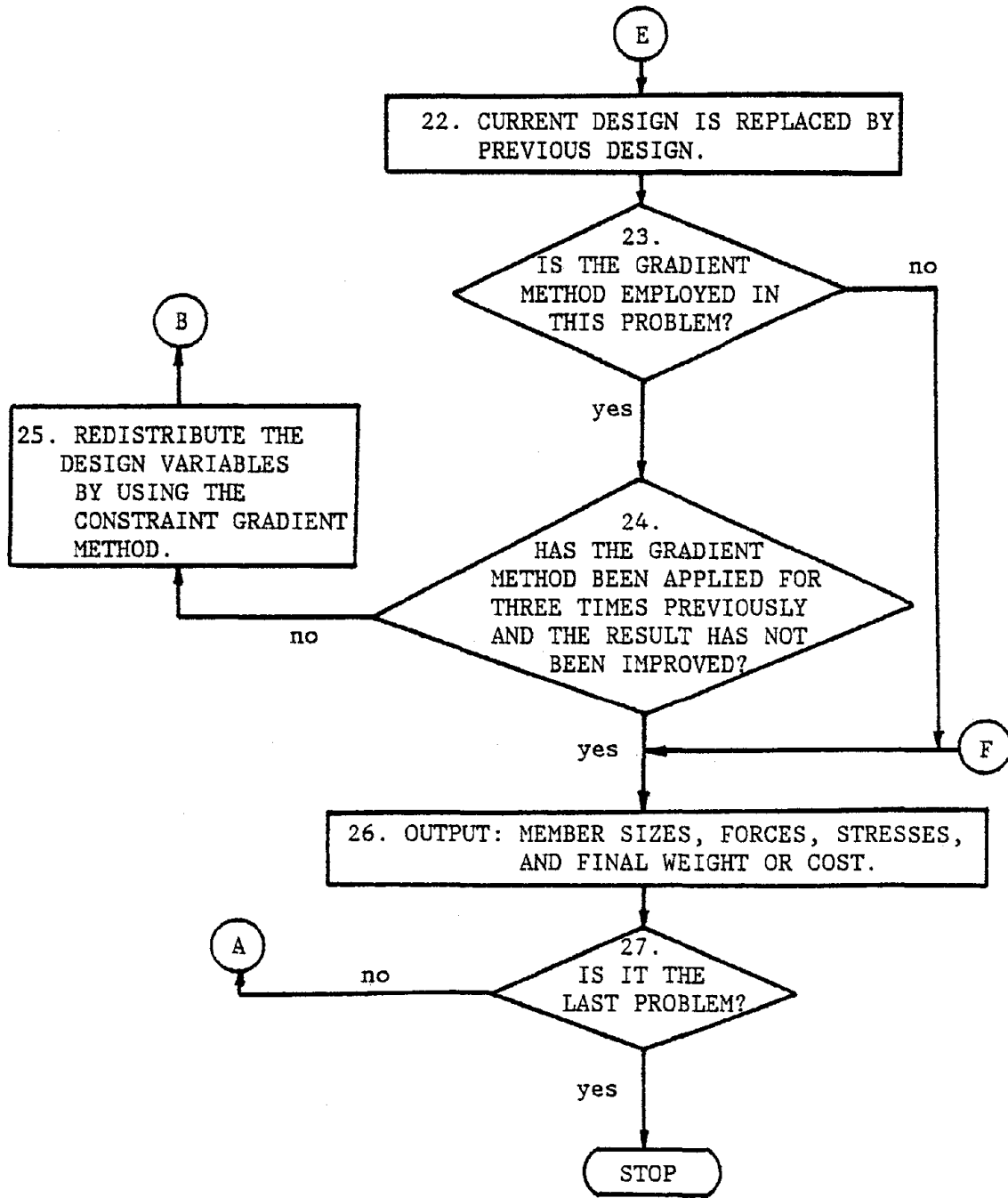
(a) Partial Flow Chart of Figure 2.1

Figure 2.1 Flow Chart of Numerical Procedures



(b) Partial Flow Chart of Figure 2.1

Figure 2.1 Flow Chart of Numerical Procedures (Continued)



(c) Partial Flow Chart of Figure 2.1

Figure 2.1 Flow Chart of Numerical Procedures (Continued)

seismic forces of ATC-3-06, UBC or Chinese Building Code, and
4) equivalent wind forces. In case of 3 and 4, the force vector is
formed first by calling subroutine ATCCD, CODE, CHINA, or WIND1. The
Gauss elimination method is used to calculate static displacements
by calling subroutine GAUSS.

Step 5. --- Is It a Static Design Only? If the structure is
subjected to static loads only, then proceed to Step 10.

Step 6. --- Solve for Eigenvalues and Eigenvectors. If the
structure is subjected to dynamic loads, then the eigen solution must
be found first. The eigenvalues and eigenvectors are determined by
using the Sturm sequence property in conjunction with the bisection
procedure.

Step 7. --- Compute the Dynamic Displacements. The modal response
of each mode is solved first. The total dynamic response is the
combination of modal responses and is obtained by using the method of
root-mean-square, direct or absolute superposition. The dynamic
loads can be multicomponent earthquake acceleration records, time-
dependent forcing function, and five groups of acceleration spectra.
The spectra of Newmark, Housner, and Seed, as well as those available
in the Chinese Building Code and ATC-3-06 are built into the
subroutines CURVE1, CURVE2, CURVE3, and CURVE4. One can obtain
spectral acceleration by calling each of these subroutines on the
basis of natural period of a structure.

Step 8. --- Is It a Combined Design Problem? If the structure is
subjected to dynamic loads only, then proceed to Step 10.

Step 9. --- Combination of Static and Dynamic Displacements. If

the structure is subjected to combined loading, the static and dynamic displacements should be computed as

$$\{r\} = \{r(x)\} \pm \{r(x,t)\} \quad (2.1.1)$$

in which

$\{r\}$ = the vector of combined displacements,

$\{r(x)\}$ = the vector of static displacements,

$\{r(x,t)\}$ = the vector of dynamic displacements.

Step 10. --- Is It an Analysis Problem? If it is an analysis problem, then proceed to Step 26.

Step 11. --- Calculate the Member Forces and Stresses. The member forces and stresses due to static loads and dynamic loads are calculated separately. In case of a combined design, the stresses due to static loads and dynamic loads are combined according to the following equation:

$$\{\sigma\} = \{\sigma(x)\} \pm \{\sigma(x,t)\} \quad (2.1.2)$$

in which

$\{\sigma\}$ = the vector of combined stresses,

$\{\sigma(x)\}$ = the vector of stresses due to static loads,

$\{\sigma(x,t)\}$ = the vector of stresses due to dynamic loads.

Step 12. --- Determine the Active Constraints. The member stresses, story drifts, and displacements should be checked with allowable values to determine whether the constraints have been

violated or not. The violated constraints are to be recorded.

Step 13. --- Adjust the Scaling Factor. The scaling factor is linearly adjusted by using the ratio of the actual value to the allowable value of the most violated constraints.

Step 14. --- Is the Scaling Factor Changed Significantly? The change of scaling factor is insignificant, if it satisfies the following relation:

$$\frac{\Lambda_{\text{new}} - \Lambda_{\text{old}}}{\Lambda_{\text{old}}} \leq \varepsilon \quad (2.1.3)$$

in which Λ_{old} and Λ_{new} are the scaling factors at the beginning and the end of each design cycle, and ε is the prescribed tolerance given by the user in the input data. The procedure is to proceed to Step 16, if the change in scaling factor is insignificant.

Step 15. --- Update the Eigensolutions. If the change in the scaling factor is significant, then the eigen solutions have to be updated. In this case, Steps 6 through 13 should be followed.

Step 16. --- Are the Sectional Properties Within Limits? If the built-up section is used in the design, then the cross sectional properties of all the members have to be checked. When all the sectional properties satisfy the allowable values or the AISC wide-flange sections are used in the design, the procedure is to proceed to Step 18.

Step 17. --- Change Sectional Properties of Built-up Sections. The ratio of flange thickness to depth of the section, t_f/d , is determined by keeping the depth and the ratio of the flange width to

depth as a constant. If the t_f/d is out of limits, then the bound values must be used, depending on whether it exceeds the upper limit or is less than lower bound, to determine the depth of the section. The procedure is to proceed to Step 11 when the sectional properties have been changed.

Step 18. --- Is It the First Cycle? The value of the objective function is determined. If it is not the first cycle, then proceed to Step 20.

Step 19. --- Redistribute Design Variables Based on the Optimum Criterion Recursion Formula. The redistribution of the design variables is based on the method described in the Secs. 2.1 through 2.5 of Ref. 1. The member sizes are checked with the size constraints after redistribution. The bound value is used whenever the member size is out of bounds. After redistribution, the procedure is to proceed to Step 2.

Step 20. --- Is the Resizing Completed? The design is stopped when the number of iterations exceed the maximum allowance.

Step 21. --- Is the Objective Function Value of Current Cycle Less Than the Value of the Previous Cycle? If the value of the objective function is reduced, then proceed to Step 19.

Step 22. --- The Result of Current Cycle Is Replaced by the Previous Result. When the value of the objective function is less than the minimum value obtained in previous cycles, the current design variables are replaced by the design variables of the previous design.

Step 23. --- Is the Method of Redistribution Based on the Constraint Gradient Considered in the Design? If the method of

redistribution based on the constraint gradient is not considered in the design problem, the procedure is to proceed to Step 26.

Step 24. --- Does the Redistribution Method Based on the Constraint Gradient Improve the Design Result? If the method of redistribution based on constraint gradient has been used continuously for more than three times and the design result has not been improved, the procedure is to proceed to Step 26.

Step 25. --- Redistribute the Design Variables Based on Constraint Gradient. The recursion formula and the numerical procedure of the constraint gradient method are given in Sec. 2.6 of Ref. 1. Check the member sizes with the size constraints. For members whose sizes are out of bounds, the bound values are taken and used in the next design cycle. The procedure is to proceed to Step 2 after the design variables have been redistributed.

Step 26. --- Output. Print out the member sizes, forces, stresses, and displacements of the final design.

Step 27. --- Is It the Last Problem for This Job? If it is the last problem to be designed or analyzed, the program is stopped. If it is not, then proceed to Step 1 and read the input data for the next problem.

2.2 Description of Subroutines

The computer program ODSEWS-2D-II includes 54 subroutines. These are depicted in alphabetical order, and the description of each is presented in this section.

(1) Subroutine ADD1 --- ADD1 is called from the main program so

that the static displacements can be combined with the dynamic displacements.

(2) Subroutine ADD2 --- ADD2 is called from the subroutine STRES1 so that the dynamic member forces can be combined with each mode. The combination method is a user's option and is given in the input data.

(3) Subroutine ADD3 --- The MAIN program calls for the subroutine ADD3 so that the energy density of each individual member can be calculated when the structure is subjected to dynamic loadings. The energy density includes the strain energy and kinetic energy.

(4) Subroutine AREA --- AREA is called from the MAIN program, and the subroutine STRSWT is used to calculate the section properties of each member.

(5) Subroutine ASMBL3 --- ASMBL3 is used to assemble the element stiffness, geometric stiffness, and mass matrices in the corresponding matrices of the structural system. This subroutine is called from the MAIN program and subroutine DATA1.

(6) Subroutine ATCCD --- The subroutine ATCCD, which is called from the MAIN program, is used to determine the equivalent seismic forces according to the ATC-3-06 provisions. The subroutines LAT1, LAT2, SOIL1, and MODALA are called from this subroutine. There are three possibilities of calling these subroutines:

i) The subroutine LAT1 and LAT2 are called to compute the seismic forces according to the equivalent lateral force method without considering the effect of the soil-structure interaction.

ii) The subroutine LAT1, LAT2, and SOIL1 are called to compute

the seismic forces according to the equivalent lateral force method with the effect of the soil-structure interaction.

iii) The subroutine MODALA is called to compute the seismic forces according to the modal analysis method. The effect of soil-structure interaction may or may not be considered in the design.

(7) Subroutine ATCIF --- The subroutine ATCIF is called from LOADIN so that the subroutine ATCIF can be used to read and print the required informations for calculating the equivalent seismic forces based on the ATC-3-06 provisions. The subroutine PERFOM is called to determine the seismic performance category.

(8) Subroutine ATCWT --- The MAIN program calls for the subroutine ATCWT to calculate the gravity load of each story, which includes the structural weight and nonstructural weight. The story gravity loads should be used to determine the equivalent seismic forces according to the provisions of ATC-3-06, UBC, or Chinese Building Design Code.

(9) Subroutine BEAMWT --- BEAMWT is called from the subroutine DATA1 to determine the amount of nonstructural weight that is contributed to the columns when the P- Δ effect is considered in the design.

(10) Subroutine BOUND2 --- The subroutine BOUND2 is called from the MAIN program to reduce the stiffness, geometric stiffness, and mass matrices. This is done by taking out the rows and columns that correspond to the fixed degrees of freedom.

(11) Subroutine CHANGE --- CHANGE is called from the MAIN program to combine the modal responses according to the type of combination specified by the user.

(12) Subroutine CHINA --- The MAIN program calls for the subroutine CHINA to compute the equivalent seismic forces according to the provisions of the Chinese Building Design Code.

(13) Subroutine CODE --- CODE is called from the MAIN program to determine the equivalent seismic forces according to the provisions of UBC.

(14) Subroutine COINF --- COINF is called from the subroutine DATA1 to read and print the required information of the minimum cost design.

(15) Subroutine COST1 --- The MAIN program calls for the subroutine COST1 to compute the structural costs.

(16) Subroutine CURVE1 --- The subroutine SPTRUM calls for the subroutine CURVE1 to find the spectral acceleration of Newmark's acceleration spectra which corresponds to the period of the building.

(17) Subroutine CURVE2 --- This subroutine is similar to the subroutine CURVE1. Subroutine CURVE2 is called to find the spectral acceleration of Housner's average response spectrum or Seed's acceleration spectra.

(18) Subroutine CURVE3 --- The response spectra of the Chinese Building Design Code are built into the subroutine CURVE3. In a manner similar to subroutines CURVE1 and CURVE2, the subroutine SPTRUM calls for this subroutine to find the spectral acceleration.

(19) Subroutine CURVE4 --- The spectral acceleration corresponds

to the response design spectra in ATC-3-06. This is found by calling the subroutine CURVE4 from the subroutine SPTRUM.

(20) Subroutine DAMP5 --- DAMP5 is called from the subroutine SOILL1 to find the foundation damping factor when the effect of the soil-structure interaction is considered in using the ATC-3-06 provisions.

(21) Subroutine DATA1 --- The subroutine DATA1 is called from the MAIN program to read and print all the input information.

(22) Subroutine DISP --- The MAIN program calls for the subroutine DISP to calculate the ratios of the displacements to the allowable displacements and the ratios of the story drifts to the allowable story drifts. The active displacement and drift constraints are recorded.

(23) Subroutine EIGEN --- EIGEN is called from the MAIN program to find the eigensolutions. The subroutine STURM is called from this subroutine for using the Sturm sequence property.

(24) Subroutine ELFORC --- The subroutine ELFORC is called to compute the member forces.

(25) Subroutine ELEMENT --- The MAIN program calls for the subroutine ELEMENT to form the stiffness, geometric stiffness, and mass matrices of each individual element.

(26) Subroutine ENERGY --- The subroutine ENERGY is used to perform the multiplication of two matrices.

(27) Subroutine FIXEND --- The subroutine FIXEND is called to calculate the fixed end forces resulting from the uniform distributed loads.

(28) Subroutine GAUSS --- The Gauss elimination method is built

into the subroutine GAUSS. It is called from the MAIN program to solve the static displacements. It is also called from the subroutine EIGEN to solve eigenvectors.

(29) Subroutine INPUT --- The subroutine INPUT is called from the subroutine DATA1 to read and print the member identification data.

(30) Subroutine INPUT3 --- The subroutine INPUT3 is used to read in and print out the initial relative design variables for the design problem or the member information of the analysis problem.

(31) Subroutine LAT1 --- The subroutine LAT1 is called for the following purposes:

- i) to calculate the approximate fundamental period of the building according to the ATC-3-06 provisions,
- ii) to determine the equivalent seismic forces according to the equivalent lateral force method of the ATC-3-06 provisions, and
- iii) to check the overturning moments and determine the final design forces.

This subroutine is called from the subroutine ATCCD, LAT2, SOIL1, or MODALA for the various purposes mentioned above.

(32) Subroutine LAT2 --- The subroutine Lat2 is called from the subroutine ATCCD, SOIL1, or MODALA to check the story drifts and P-Δ effects and to determine the final design forces.

(33) Subroutine LOADIN --- The subroutine LOADIN, which is called from the subroutine DATA1, is used to read the static and dynamic loading information.

(34) Subroutine MLTPLY --- The subroutine MLTPLY is used to

multiply a matrix with a vector and is called from the MAIN program.

(35) Subroutine MODALA --- The subroutine ATCCD calls for the subroutine MODALA to determine the design seismic forces according to the modal analysis method of the ATC-3-06 provisions. The subroutine LAT2 is called from this subroutine to check the story drifts and to determine the final design forces. The subroutine DAMP5 is called to find the foundation damping factor when the effect of soil-structure interaction is considered.

(36) Subroutine PDELTA --- The subroutine PDELTA is used to store the additional axial forces in columns to account for the P- Δ effects in the design. This subroutine is called from the MAIN program.

(37) Subroutine PERFOM --- The subroutine PERFOM is called from the subroutine COINF to determine:

- i) the map area coefficients according to the Table 1-B in the ATC-3-06 provisions,
- ii) the seismic index, the seismic hazard exposure group, and the seismic performance category according to Table 1-A in the ATC-3-06 provisions, and
- iii) the analysis method.

(38) Subroutine POP2 --- The subroutine POP2 is called from the subroutine DATA1 to compute the total number of elements that must be stored in the system stiffness and mass matrices.

(39) Subroutine PRNTDR --- The subroutine PRNTDR is used to print out the displacements of the final design and is called from the MAIN program.

(40) Subroutine REDUCE --- REDUCE is used to reduce the force and

displacement vectors by taking out rows that correspond to the fixed degrees of freedom.

(41) Subroutine RESIZE --- The MAIN program calls for the subroutine RESIZE to prevent the redistributed sizes of the members on the upper floors from being larger than those of the members on the lower floors.

(42) Subroutine RESTOR --- RESTOR is used to restore the force or displacement vector by adding the rows that correspond to the fixed degrees of freedom in the vector.

(43) Subroutine ROOT1 --- ROOT1 is called from the subroutine MODALA to superimpose the base shears of all the required modes. This is done by using the root-mean-square method when the modal analysis method of the ATC-3-06 provisions is considered in the design.

(44) Subroutine ROOT2 --- The subroutine MODALA calls for the subroutine ROOT2 to superimpose story shears, story drifts, and overturning moments of all the required modes. This is done by using the root-mean-square method.

(45) Subroutine SAME --- The subroutine SAME is called from the MAIN program to force the size of those members that have common identification number to be identical.

(46) Subroutine SOIL1 --- The subroutine SOIL1 is called from the subroutine ATCCD to modify the seismic design forces, when the effect of the soil-structure interaction is considered in the equivalent lateral force method of the ATC-3-06 provisions.

(47) Subroutine SPTRUM --- The MAIN program calls for the subroutine SPTRUM to determine spectral acceleration or to perform

Duhamel Integration:

- i) If the design is based on the data taken from earthquake acceleration records, then the maximum earthquake acceleration is found by interpolating according to the natural periods of the the building.
- ii) If the curve fitted function of the acceleration spectrum is considered, then the spectral acceleration is found by calling the subroutines CURVE1, CURVE2, CURVE3, or CURVE4.
- iii) If the input data are the time-history data, then an explicit integration is performed.

(48) Subroutine STRES1 --- The MAIN program calls for the subroutine STRES1 to compute the member forces and stresses. The stress constrains are also checked in this subroutine.

(49) Subroutine STRSW1 --- The subroutine STRSW1 is used to compute the structural weights that contribute to the columns for the analysis problem and is called from the MAIN program.

(50) Subroutine STRSWT --- The subroutine STRSWT is used to compute the structural weights that contribute to the columns for the design problem and is called from the MAIN program.

(51) Subroutine STURM --- The Sturm sequence property of solving eigenvalues and eigenvectors is built into the subroutine STURM which is called from the subroutine EIGEN.

(52) Subroutine TRNSFM --- The subroutine TRNSFM is used to transform the element stiffnesses and mass matrices from a local coordinate to a system coordinate.

(53) Subroutine WFBEAM --- The subroutine WFBEAM is called from

the MAIN program and the subroutine STRES1 to compute member stresses.

(54) Subroutine WIND1 --- The subroutine WIND1 is called from the subroutine LOADIN to determine the wind forces according to the power law.

2.3 Program Capacity and Guide of Modification

The ODSEWS-2D-II computer program is coded in FORTRAN IV language with fixed commons and dimensions. Because of fixed commons and dimensions, the capacity of this program is limited. The program capacity is constrained by the following maximum allowances:

- (1) MAXST --- The maximum number of floor levels of the structure is 30.
- (2) NMMAX --- The maximum number of members in the structure is 200.
- (3) NNMAX --- The maximum number of degrees of freedom is 300.
- (4) MAXSK --- The maximum number of elements in the mass and stiffness matrices of the structure is 3500.
- (5) NJMAX --- The maximum number of nodes of the structure is 300.
- (6) MAXNC --- The maximum number of active constraints is 30.
- (7) LDMAX --- The maximum number of loading conditions is 4.
- (8) MDMAX --- The maximum number of modes that can be considered in the dynamic analysis is 10.
- (9) NGMAX --- The maximum number of members that contribute their weight to columns to account for the P- Δ effect is 200.
- (10) NDMAX --- The maximum number of earthquake acceleration

records and time steps is 200.

- (11) MBAND --- The half-bandwidth of the stiffness and mass matrices is not more than 50.
- (12) NB --- The maximum number of fixed degrees of freedom is 50.
- (13) KCOUNT--- The maximum number of iteration cycles is 20.
- (14) MBAY --- The maximum number of bays of the structure is 10.

In order to modify the program capacity, the numbers for appropriate variables in the following commons and dimensions should be changed.

- (1) Common DATA in subroutines MAIN, DATA1 and LOADIN:
FACTOR(LDMAX), XV(LDMAX), MTYPE(LDMAX), MOTYPE(LDMAX),
NTYPE(LDMAX), NTIME(LDMAX), DEL(LDMAX)
- (2) Common ATCI in subroutines MAIN, ATCCD, ATCIF, DAMP5, DATA1,
LAT1, LAT2, LOADIN, MODALA, PERFORM and SOIL1:
DELA(MAXST)
- (3) Common ACODE in subroutines MAIN, CODE, DATA1 and LOADIN:
Z(LDMAX), CI(LDMAX), CK(LDMAX), TS(LDMAX)
- (4) Common CODE1 in subroutines MAIN, ATCCD, ATCIF, ATCWT, CHINA,
CODE, DATA1, LAT1, LAT2, LOADIN, MODALA, SOIL1, and WIND1:
HT(MAXST), WTT(LDMAX), WWW(MAXST,LDMAX), KNODE(MAXST,LDMAX)
- (5) Common ATC1 in subroutines MAIN, ATCCD, LAT1, LAT2, MODALA and
SOIL1:
V(MAXST), DIS(MAXST), DELT(MAXST), RM(MAXST), CV(MAXST),
TETA(MAXST), PP(MAXST), SUMWF1(MDMAX), SUMWF2(MDMAX),

WBARM(MDMAX), CSM(MDMAX), RMOMNT(MAXST+1), VMODAL(MDMAX),
RMMODE(MAXST+1,MDMAX), DRIFMO(MAXST,MDMAX), U(MAXST,MDMAX),
CVXM(MAXST,MDMAX), FMODAL(MAXST,MDMAX), DELXEM(MAXST,MDMAX),
VMODE(MAXST,MDMAX)

(6) Common ATC2 in subroutines MAIN, ATCCD, LAT1, LAT2, MODALA and
SOIL1:

NBEAM(MAXST), NCONT(MAXST), NOCONT(MAXST,2*MBAY+2),
NOBEAM(MAXST,MBAY)

(7) The following dimensions need to be changed in the MAIN
program:

TTT(LDMAX), CC(LDMAX), SS(LDMAX), TOTAL(LDMAX), FT(LDMAX),
STRENG(MAXNC), ENG2(MAXNC-2*LDMAX), IBND(NB), ICOL(NNMAX),
RATIO1(2*LDMAX), CONST(MAXNC), ENG1(MAXNC-2*LDMAX), UMF(
LDMAX), NDEFEQ(LDMAX), MDEFEQ(LDMAX), FQ(LDMAX), SUM1(LDMAX),
SUM2(LDMAX), RATIO(MAXNC), MA(NNMAX), MB(NNMAX), AE(NNMAX),
DEP(NNMAX), TFD(NNMAX), ELENTH(NNMAX), JTYPE(NNMAX), DDD1(
NNMAX), FINDMI(KCOUNT), DDDD(NNMAX), BD(NNMAX), FRR(NNMAX),
STRES(NNMAX), ENGLTA(NNMAX), NMB(NNMAX), RKAPA(MAXST), ICOLS(
NNMAX), IDIAGS(NNMAX), DEFLMT(NNMAX), GN(MAXSK), FRS(NNMAX),
VAL(NNMAX), GM(MAXSK), MBC(NNMAX), WWWW(NNMAX), FFF(MAXST),
MSOIL(LDMAX), AREA1(NNMAX), SX(NNMAX), NDUCTY(LDMAX), KACTIV(
NNMAX), AHMX(LDMAX), AVMX(LDMAX), IDIAG(NNMAX), X(NJMAX),
Y(NJMAX), SK(MAXSK), WW(MAXST), PHI1(NNMAX), VALMIN(MDMAX),
TT(MDMAX), VWORK(MDMAX), DAMP(MDMAX), AM(NNMAX), AN(NNMAX),
FACTDL(LDMAX), FACTLL(LDMAX), FACTSL(LDMAX), FACTEL(LDMAX),

SK1(MAXSK), NLINE(NMMAX), FI(NNMAX,LDMAX), DI(NNMAX,LDMAX),
 FR(NNMAX,LDMAX), DR(NNMAX,LDMAX), XH(NNMAX,LDMAX), JDEFEQ(
 MDMAX,LDMAX), FS(NNMAX,LDMAX), DLF1(MDMAX,LDMAX), DLF2(MDMAX,
 LDMAX), HH(MDMAX,LDMAX), VV(MDMAX,LDMAX), PNT(NDMAX,LDMAX),
 DS(NNMAX,LDMAX), ENG(NMMAX,MAXNC), H1(NDMAX,LDMAX), FV(NNMAX,
 LDMAX), FD(NNMAX,LDMAX), S1(6,MAXNC-2*LDMAX), S2(6,MAXNC
 -2*LDMAX), SS1(6,MAXNC-2*LDMAX), SS2(6,MAXNC-2*LDMAX), DM(
 NNMAX,MDMAX), EDF(6,MAXNC-2*LDMAX), EFF(6,MAXNC-2*LDMAX),
 FF(NNMAX,MAXNC-2*LDMAX), DF(NNMAX,MAXNC-2*LDMAX), H2(NDMAX,
 LDMAX), WT(NMMAX,3), ISGN(NNMAX,MAXNC-2*LDMAX), ISTR1(NNMAX,
 LDMAX), FSATC(NNMAX,LDMAX), WS(MAXST,LDMAX), XS(NNMAX,LDMAX),
 MNO(NMMAX,NGMAX), DELTA0(NNMAX,LDMAX), AERECD(NMMAX,KCOUNT),
 SK1U(NNMAX,1), DELTAR(NNMAX,NMMAX), SK12(NMMAX,1), DSTMAX(
 NMMAX,NMMAX), ASTRE(NMMAX,LDMAX), SK2U(NNMAX,1)

- (8) The following dimension needs to be changed in the subroutine ATCCD:

TTT(MDMAX)

- (9) The following dimensions need to be changed in the subroutine CODE:

FX(NNMAX,1), DX(NNMAX,1)

- (10) The following dimensions need to be changed in the subroutine DATA1:

UMF(LDMAX), WMASS(NJMAX), AML(6,LDMAX)

- (11) The following dimensions need to be changed in the subroutine EIGEN:

VALUP(NNMAX), SKK(MAXSK), FI(NNMAX,1), DI(NNMAX,1),
DRP(NNMAX,1)

(12) The following dimension needs to be changed in the subroutine

LOADIN:

XS(NNMAX,1)

(13) The following dimensions need to be changed in the subroutine

RESTOR:

TDR1(LDMAX), TDR2(LDMAX)

(14) The following dimension needs to be changed in the subroutine

STRES1:

UMF(LDMAX)

(15) The following dimensions need to be changed in the subroutine

STURM:

IREC(NNMAX), ICHNG(NNMAX), D(2*MBAND), C(MBAND,2*MBAND)

(16) The following dimensions need to be changed in the subroutine

WIND1:

WF(MAXST+1), FFF(MAXST), HEIGHT(MAXST+1)

III. DESCRIPTION OF INPUT DATA

3.1 Contents of the Input Instructions

In this section, 12 input codes included in the input instruction are outlined. The detailed input data instructions of each code are shown in the next section.

- (I) TOTAL NUMBER OF PROBLEMS
- (II) CONTROL CARDS
- (III) STRUCTURAL COSTS INFORMATION CARDS
- (IV) MEMBER INFORMATION CARDS
 - a. Girder-Column Property Card
 - (i) Built-up section
 - (ii) AISC wide flange section
 - b. Bracing Property Card
 - c. Bracing Identification Cards
 - d. Girder Identification Cards
 - e. Column Identification Cards
- (V) JOINT INFORMATION CARDS
- (VI) INITIAL RELATIVE DESIGN VARIABLE CARDS
- (VII) BOUNDARY CONDITION CARDS
- (VIII) STATIC LOADING INFORMATION CARDS
 - a. Control Cards
 - b. Loading Cards
 - (i) Loading parameter > 0
 - (ii) Loading parameter < 0

- (1) Story weight information cards
- (2) UBC information card
- (3) ATC information cards
- (4) Chinese Building Design Code information card
- (5) Story member information cards
- (6) Wind force information cards

c. Load Combination Factor Cards

(IX) DYNAMIC PROPERTY CARDS

a. Control Card

b. Damping Factor Card

c. Additional Nodal Mass Cards

d. Dynamic Loading Information Cards

(i) Control card

(ii) Loading cards

(iii) Acceleration data cards

(1) Periods or time cards

(2) Horizontal acceleration data cards

(3) Vertical acceleration data cards

(4) Curve fitting acceleration card

e. Story Drift Information Cards

f. Story Member Cards

(X) MEMBER INFORMATION CARDS OF ANALYSIS PROBLEM

a. Relative Size Cards

b. Sectional Area Cards

c. Sectional Modulus Cards

d. Member Property Card

(XI) DISPLACEMENT CONSTRAINT CARDS

a. Equal Value for All Displacement Constraints

b. Different Displacement Constraints

(i) Control card

(ii) Allowable values

(XII) FREQUENCY CONSTRAINT CARDS

3.2 Input Data Instructions

The detailed instructions for each of the 12 input codes are as follows:

(I) TOTAL NUMBER OF PROBLEMS (2I5)

Columns	Variable	Entry
1-5	NSTRS	Total number of problems to be designed or analyzed in this job run.
6-10	IOUT1	The logical unit number of the disc or tape on which the plotting data of the first problem will be stored.

(II) CONTROL CARDS (11I5,/,8F10.0,/,8I5,10A4)

Columns	Variable	Entry
1-5	ICODE	EQ. 1; Uniform Building Code. EQ. 2; ATC-3-06. EQ. 3; Chinese Building Design Code. EQ. 4; Wind force. EQ. 5; No code provisions will be considered in

in the design.

6-10	IDESGT	EQ.-1; Analysis. EQ. 0; Minimum weight design. EQ. 1; Minimum cost design.
11-15	IRELT	EQ. 0; The initial design variables will be equal to 1.0 for all members. EQ. 1; The initial design variables will be read in from the input data.
16-20	IPLOT	EQ. 0; No plot. EQ. 1; The results of design will be plotted.
21-25	NSTORY	Total number of floor levels.
26-30	IGRADT	EQ. 0; The constraint gradient method will not be considered in the design. GE. 1; Total number of times that the method based on constraint gradient will be used.
31-35	LGRADT	A positive number, which is used to determine whether the constraint gradient method has improved the design result or not. If the number of cycles that the method has been used continuously for more than this number and the result has not been improved, then the program will be stopped.
36-40	IMAX	The maximum number of cycles for scaling without changing eigen solutions.
41-45	IIMAX	The maximum number of cycles for scaling by

reformulating stiffness and mass matrices.

46-50 INSIST EQ. 1; The energy distribution method will be used until the number of iterations is equal to the maximum allowance.

EQ. 0; The program will be stopped when the design result is no longer improved.

51-55 IUNILD EQ. 1; The uniform distributed loads will be included in the force vector when ATC-3-06, UBC or the Chinese Building Design Code is considered.

EQ. 0; The uniform loads will not be included in the force vector.

1-10 EPSI A prescribed tolerance, which is used to determine the active displacement constraints.

11-20 EPS1 A prescribed tolerance, which is used to determine the convergence of scaling without changing the eigen solutions.

21-30 EPS2 A prescribed tolerance, which is used to determine the convergence of scaling.

31-40 EPS A prescribed tolerance, which is used to determine whether the constraint gradient method will be used in the next cycle or not.

41-50 URANGE The upper bound of the change in the relative design variable for the constraint gradient method.

51-60	DRANGE	The lower bound of the change in the relative design variable for the constraint gradient method.
61-70	REFATR	The percentage of reducing the member sizes to compute constraint gradients and have a value of 1%~2%.
71-80	FINCRT	The coefficient that is used to change the design variables for computing the constraint gradient in sensitive analysis with the value ranging from 1.01 to 1.10
1-5	MEMBS	Total number of members.
6-10	JOINTS	Total number of nodes.
11-15	NBNDRY	Total number of fixed degrees of freedom.
16-20	LOADS	Total number of loading conditions.
21-25	LMTDSP	Displacement constraint code: EQ. 0; No displacement constraint. EQ. 1; All degrees of freedom have the same constraint value. EQ. 2; Displacement constraints can be varied per degree of freedom. EQ. 3; Displacement constraints will be set up according to the allowable story drift and the story drifts will be checked.
26-30	INDEX	Type of design: EQ.-1; Static design or analysis. EQ. 0; Static + dynamic design or analysis.

EQ. 1; Dynamic design or analysis.

31-35 NCOUNT Maximum number of iterations.

36-40 ISEC Type of section:
EQ. 0; Built-up section.
GT. 0; AISC wide flange section.
LT. 0; Truss member.

41-80 TITLE User's option.

(III) STRUCTURAL COSTS INFORMATION CARDS (5F10.0,/,I5,4F10.0,F25.0)

Skip these two cards if it is not a minimum cost design.

Columns	Variable	Entry
1-10	ASTAR	The ground acceleration of the earthquake normalized by the gravity acceleration g.
11-20	CS8	The unit price of steel (\$/lb).
21-30	CPT	The unit price of painting (\$/in ²).
31-40	CSC	The unit price of steel at connections (\$/lb).
41-50	CSW	The unit price of welding metal (\$/lb).
1-5	NLIFE	Total life of the structure (years).
6-15	CONONS	The ratio of the nonstructural costs to the construction costs.
16-25	GAMMA6	The coefficient of the power in the equation for computing the seismic frequency; its value is 14.6118 for southern California.
26-35	DELTA6	The coefficient in the equation for computing the seismic frequency; its value is 2.9335 for southern California.

36-45 GACCMX The maximum expected intensity of an earthquake during the lifetime of the structure.

46-70 RINIST The initial stiffness of the connection.

(IV) MEMBER INFORMATION CARDS

Omit cards a. and b., when it is a analysis problem.

a. Girder-Column Property Card

(i) Built-up section (2F10.0,10F5.0)

Columns	Variable	Entry
1-10	EE	Modulus of elasticity (psi).
11-20	RHO	Specific weight.
21-25	BDI	Initial value of b_f/d .
26-30	DEPMAX	Maximum depth of the section.
31-35	DEPMIN	Minimum depth of the section.
36-40	TFDMAX	Maximum t_f/d .
41-45	TFDMIN	Minimum t_f/d .
46-50	BTFMAX	Maximum b_f/t_f .
51-55	BTFMIN	Minimum b_f/t_f .
56-60	BDMAX	Maximum b_f/d .
61-65	BDMIN	Minimum b_f/d .
66-70	AENM	The ratio of the minimum moment of inertia to the maximum moment of inertia.

(ii) AISC wide flange section (6F10.0)

Columns	Variable	Entry
1-10	EE	Modulus of elasticity (psi).
11-20	RHO	Specific weight.

21-30	BASEA	Initial moment of inertia.
31-40	AEMNMX	Maximum moment of inertia.
41-50	AEMNMM	Minimum moment of inertia.
66-70	AENM	The ratio of the minimum moment of inertia to the maximum moment of inertia.

b. Bracing Property Card (5F10.0)

Columns	Variable	Entry
1-10	E1	Modulus of elasticity (psi).
11-20	RHO1	Specific weight.
21-30	ABRACE	Initial sectional area.
31-40	ABRMAX	Maximum area.
41-50	ABRMIN	Minimum area.

c. Bracing Identification Cards (4I5,F10.0,30X,2I5)

Columns	Variable	Entry
1-5	ID	Identification number of the member. (Number the independent variables first, the subsequent members are then numbered in the order of common design variables.)
6-10	MA	First node number of the member (the smaller node number of the member).
11-15	MB	Second node number of the member (the larger node number of the member).
16-20	JT	Member type code, 0 for bracings.
21-30	ST	Allowable stress of the member.
31-60		Blank.

- 61-65 NT Design variable number. (Use the number of independent variable if that is the common design variable for this member.)
- 66-70 NLINE The number defines the bracing line to which this member belongs. (This number is used to redistribute the member sizes if the member sizes on the upper stories are larger than those of lower stories because of the redistribution based on energy.)

Repeat the directions on this card through all of the bracings.

d. Girder Identification Cards (4I5,4F10.0,2I5)

Columns	Variable	Entry
1-5	ID	Identification number of the member. (See bracing identification cards.)
6-10	MA	First node number of the member.
11-15	MB	Second node number of the member.
16-20	JT	Member type code, 2 for girter.
21-30	ST	Allowable stress of the member.
31-40	WTD	Uniform dead load, lb/in.
41-50	WTL	Uniform live load, lb/in.
51-60	WTS	Uniform snow load, lb/in.
61-65	NT	Design variable number. (See bracing identification cards.)
66-70	NLINE	The number defines the girder line to which this member belongs. (See bracing identification

cards.)

Repeat the directions on this card through all of the girders.

e. Column Identification Cards (4I5,2F10.0,20X,3I5,/(16I5))

Columns	Variable	Entry
1-5	ID	Identification number of the member. (See bracing identification cards.)
6-10	MA	First node number of the member.
11-15	MB	Second node number of the member.
16-20	JT	Member type code, 1 for column.
21-30	ST	Allowable stress of the member.
31-40	WTD	Additional axial force (P-Δ force).
41-60		Blank.
61-65	NT	Design variable number. (see bracing identification cards.)
66-70	NLINE	The number defines the column line to which this member belongs. (See bracing identification cards.)
71-75	NB	Total number of members that contribute their weights to this column.

If NB=0, then skip the following card.

1-5	MNO	Identification number of the member that contributes its weight to the column.
6-10		Repeat for each additional member.

Repeat the directions on these cards through all the columns.

(V) JOINT INFORMATION CARDS (I5,2F10.0)

Columns	Variable	Entry
1-5	JT	Joint identification number.
5-15	X	X-coordinate.
16-25	Y	Y-coordinate.

(VI) INITIAL RELATIVE DESIGN VARIABLE CARDS ((8F10.0))

If IREL_T=0, then skip these cards.

Columns	Variable	Entry
1-10	AE	Relative design variable of the first member.
11-20		Repeat through all members.

(VII) BOUNDARY CONDITION CARDS (16I5)

Columns	Variable	Entry
1-5	IBND	First degree of freedom that is fixed.
6-10		Repeat for each additional degree of freedom that that is fixed.

(VIII) STATIC LOADING INFORMATION CARDS

The following data set is prepared for one loading case. It is necessary to repeat this data set through all the loading cases. If the problem is one of dynamic design, then skip this data set.

a. Control Cards (I5)

Columns	Variable	Entry
1-5	NJ	<p>Loading parameter:</p> <p>GE. 0; Number of load components for this loading case.</p> <p>LT. 0; The lateral forces are determined by using the provisions of UBC, ATC-3-06, or the</p>

Chinese Building Design Code, or use the power law to find the wind forces.

b. Loading Cards

(i) Loading parameter > 0 (3(F10.0,2I5))

Columns	Variable	Entry
1-10	TFR	Magnitude of the force.
11-15	IM	Direction of the force. EQ. 1; X-direction EQ. 2; Y-direction EQ. 3; Z-direction
16-20	JM	Number of the node to which the load is applied.
21-30		Repeat for each additional load. Three sets of data can be prepared on one card.

(ii) Loading parameter < 0

(1) Story weight information cards (2I5,2F10.0)

These cards are needed for the following cases: 1) for checking the story drifts, 2) for using the code provisions to compute the equivalent seismic forces, or 3) using the power law to determine the wind forces.

Columns	Variable	Entry
1-5	II	Floor level.
6-10	KNODE	Number of the node to which the lateral force is applied. It is also the node number at which the the story drifts will be calculated.
11-20	HT	Height of the floor level from the base.

21-30 WS Total gravity load on this floor level.
 (2) UBC information card (4F5.0)

If UBC is not considered, then skip this card.

Columns	Variable	Entry
1-5	Z	The seismic zone coefficient.
6-10	CI	The importance factor of the structure.
11-15	CK	The structural type coefficient.
16-20	TS	Site period.

Note that it is necessary to repeat cards (1) and (2) for each loading case when UBC is considered.

(3) ATC information cards (12I5/3F10.0/3F10.0,2F20.0,2I5)

If ATC is not considered, then skip these cards.

Columns	Variable	Entry
1-5	NNNNN	Map area number for effective peak ground acceleration.
6-10	MMMMM	Map area number for effective peak ground velocity.
11-15	ISH	Seismic hazard exposure group.
16-20	NBRITL	EQ. 1; Brittle type structure. EQ. 0; Non-brittle type structure.
21-25	IACTIV	EQ. 1; Active fault. EQ. 0; Inactive fault.
26-30	ISOIL	Soil profile type.
31-35	NPREGR	EQ. 1; Plan configuration is regular. EQ. 0; Plan configuration is irregular.

36-40	NVREGR	EQ. 1; Vertical configuration is regular. EQ. 0; Vertical configuration is irregular.
41-45	MR	EQ. 1; Moment resisting frame. EQ. 0; All other kinds of structure.
46-50	ISF	EQ. 1; Steel frame. EQ. 0; Concrete frame.
51-55	IFIX	EQ. 1; Fixed base. EQ. 0; Consider the effect of soil-structure interaction.
56-60	NANALS	EQ. 0; The method of analysis will be automatically determined by the computer program. EQ. 1; Equivalent lateral force method. EQ. 2; Modal analysis method.
1-10	R	The response modification factor.
11-20	CD	The deflection modification factor.
21-30	RL	The overall length at the base in the direction of analysis.

The following card is needed only when the effect of soil-structure interaction is considered, i.e., IFIX=0.

1-10	RR	The characteristic foundation length, in.
11-20	VS	The shear wave velocity, in./sec.
21-30	DSSSS	The depth of stratum, in.
31-50	RKY	The lateral stiffness of the foundation, lb/in.
51-70	RKTETA	The rocking stiffness of the foundation,

1b-in./rad.

71-75	ISLGL	EQ. 1; Single gravity load. EQ. 0; Non-single gravity load.
76-80	IPILE	EQ. 1; For buildings supported on point bearing piles or in other cases where the foundation soil consists of a soft stratum of reasonably uniform properties underlain by a much stiffer rock-like deposit with an abrupt increase in stiffness. EQ. 0; For all other cases.

(4) Chinese Building Design Code information card (2F10.0,I5)

This card is needed only when the Chinese Building Design Code is considered.

Columns	Variable	Entry
1-10	CII	The structural influence coefficient.
11-20	ALPHMX	The maximum seismic influence coefficient: Earthquake magnitude 7, $\alpha_{\max}=0.23$ Earthquake magnitude 8, $\alpha_{\max}=0.45$ Earthquake magnitude 9, $\alpha_{\max}=0.90$
21-25	NSOIL	Soil profile type.

(5) Story member information cards (16I5)

Skip this input for the following cases: 1) INC=0 and ICODE=5, 2) ICODE=5 and IDESGT=-1, 3) ICODE=4, or 4) NSTORY=0.

Columns	Variable	Entry
1-5	NBBBB	Total number of girders that contribute their

weights to floor level i.

6-10	NNCON	Total number of columns and bracings that contribute their weights to floor level i.
11-15	NOBEAM	Identification number of girders. Repeat through total number of girders, NBBBB.
16-20	NOCONT	Identification number of columns. Repeat through total number of columns, NNCON.

Repeat these data from the bottom to top level.

(6) Wind force information cards (8F10.0,/,F10.0)

Skip these cards when the wind forces determined by the power law are not considered.

Columns	Variable	Entry
1-10	BAYWTH	The width of the structural bay in the direction of analysis.
11-20	HGRADT	The gradient height of the wind, in.
21-30	VGRADT	The gradient velocity of the wind, in./sec.
31-40	POWER	The power law coefficient for the gradient height.
41-50	CPEA	External pressure coefficient on the windward wall.
51-60	CPEB	External pressure coefficient on the leeward wall.
61-70	CPIA	Internal pressure coefficient on the windward wall.
71-80	CPIB	Internal pressure coefficient on the leeward

wall.

1-10 GUSTF Gust factor.

c. Load Combination Factor Cards (4F5.0)

Columns	Variable	Entry
1-5	FACTDL	Load combination factor for the dead load.
6-10	FACTLL	Load combination factor for the live load.
11-15	FACTSL	Load combination factor for the snow load.
16-20	FACTEL	Load combination factor for the equivalent seismic load.

(IX) DYNAMIC PROPERTY CARDS

For the static design, card a. only is needed.

a. Control Card (4I5,F10.0)

Columns	Variable	Entry
1-5	KOPTON	Option code: EQ. 1; Use only the significant modes in the design. EQ. 2; Use all specified modes in the design. EQ. 0; Static design.
6-10	NSIGN	Type of mode superposition: EQ. -1; Direct superposition. EQ. 0; Root-mean-square superposition. EQ. 1; Absolute superposition.
11-15	NBEGIN	The first of the desired modes.
16-20	NDESIR	The last of the desired modes.
21-30	XG	The gravity acceleration.

b. Damping Factor Card (8F10.0)

Skip this card when INDEX=-1.

Columns	Variable	Entry
1-10	DAMP	Damping factor of the first mode.
11-20		Repeat through all the desired modes.

c. Additional Nodal Mass Cards (8F10.0)

Skip this card for 1) static loads, 2) UBC, or 3) wind forces determined by using the power law.

Columns	Variable	Entry
1-10	WMASS	Mass at first node.
11-20		Repeat for each node.

d. Dynamic Loading Information Cards

The following data set is prepared for one loading case only. Repeat this data set for each additional loading case.

(i) Control card (4I5,3F5.0,I5,10A4)

Columns	Variable	Entry
1-5	NJ	Loading parameter: GE. 0; Number of load components. LT. 0; Seismic excitations.
6-10	MO	Direction of dynamic motion: LT. 0; Horizontal + vertical. EQ. 1; Horizontal only. EQ. 2; Vertical only.
11-15	NZ	Type of input acceleration: EQ. 1; Time-history data.

EQ. 2; Spectrum data.

EQ. 3; Curve fitted spectrum data.

16-20	NNM	Number of acceleration data or spectrum data.
21-25	F	Load multiplier or scale factor of horizontal motion.
26-30	XV	Scale factor of vertical motion.
31-35	DEL	Time increment for explicit integration.
36-40	NT	Number of time steps for integration purposes.
41-80	TITLE	User's option.

(ii) Loading cards (3(F10.0,2I5))

This data set is needed when the loading parameter is greater than zero.

Columns	Variable	Entry
1-10	TFR	Magnitude of load.
11-15	IM	Direction of load (X=1, Y=2, Z=3).
16-20	JM	Number of node to which the load is applied.
21-30		Repeat for each additional load.

(iii) Acceleration Data Cards

Data sets (1), (2), and (3) are needed when the type of acceleration data is time-history data or spectrum data.

(1) Periods or time cards (8F10.0)

Columns	Variable	Entry
1-10	PT	First datum of period or time.
11-20		Repeat for each additional period or time.

(2) Horizontal acceleration data cards (8F10.0)

Columns	Variable	Entry
1-10	H1	First datum of spectral acceleration or ground acceleration in the horizontal direction.
11-20		Repeat for each additional datum.

(3) Vertical acceleration data cards (8F10.0)

This data set is needed for multicomponent ground motions only.

Columns	Variable	Entry
1-10	H2	First datum of spectral acceleration or ground acceleration in the vertical direction.
11-20		Repeat for each additional datum.

(4) Curve fitting acceleration card (2I5,2F10.0)

Columns	Variable	Entry
1-5	NDUCTY	EQ. 1; Newmark's spectrum (5% damping, 50 percentile alluvium). EQ. 2; Newmark's spectrum (5% damping, 90 percentile alluvium). EQ. 3; Newmark's spectrum (5% damping, and ductility is 3). EQ. 4; Newmark's spectrum (5% damping, and ductility is 4). EQ. 0; All other response spectra.
6-10	MSOIL	EQ. 0; If NDUCTY is also equal to zero, then the Housner's average response spectrum with 5% damping is selected. When NDUCTY is not zero, then it is required to let MSOIL

be equal to zero.

- EQ. 1; Seed's spectrum (rock).
- EQ. 2; Seed's spectrum (stiff site condition).
- EQ. 3; Seed's spectrum (deep cohesionless soil).
less soil)
- EQ. 4; Seed's spectrum (soft to medium clay and sand).
- EQ. 5; Chinese building design spectrum (soil type 1).
- EQ. 6; Chinese building design spectrum (soil type 2).
- EQ. 7; Chinese building design spectrum (soil type 3).
- EQ. 8; ATC-3-06 recommended building design spectrum (soil type 1).
- EQ. 9; ATC-3-06 recommended building design spectrum (soil type 2).
- EQ.10; ATC-3-06 recommended building design spectrum (soil type 3 and the effective peak ground acceleration is less than 0.3g).
- EQ.11; ATC-3-06 recommended building design spectrum (soil type 3 and the effective peak ground acceleration is greater than or equal to 0.3g).

11-20 AHMX The maximum seismic influence coefficient of horizontal motion for the Chinese building design spectrum:

Earthquake magnitude 7, $\alpha_{hm} = 0.23$.

Earthquake magnitude 8, $\alpha_{hm} = 0.45$.

Earthquake magnitude 9, $\alpha_{hm} = 0.90$.

21-30 AVMX The maximum seismic influence coefficient of vertical motion for the Chinese building design spectrum:

Earthquake magnitude 7, $\alpha_{vm} = 0.115$.

Earthquake magnitude 8, $\alpha_{vm} = 0.23$.

Earthquake magnitude 9, $\alpha_{vm} = 0.46$.

e. Story Drift Information Cards (2I5)

These cards are needed only when one wants to check story drifts.

Columns	Variable	Entry
1-5	II	Floor level.
6-10	KNODE	Number of the node that will be used to compute story drifts.

f. Story Member Cards (16I5)

Skip these cards for the analysis problem or NSTORY=0.

Columns	Variable	Entry
1-5	NBBBB	Total number of girders on floor level i.
6-10	NNCON	Total number of columns and bracings on floor level i.
11-15	NOBEAM	Identification number of girders.

Repeat through total number of girders, NBBBB.

16-20 NOCONT Identification number of columns.

Repeat through total number of columns, NNCON.

Repeat these data from the bottom to top level.

(X) MEMBER INFORMATION CARDS OF ANALYSIS PROBLEM

Skip this data set for the design problem.

a. Relative Size Cards (8F10.0)

Columns	Variable	Entry
1-10	AE	Relative size of the first member. For girders and columns, this value is I/I_{\max} . For bracings, it is A/I_{\max} . For members of truss, it is A/A_{\max} .
11-20		Repeat for each member.

b. Sectional Area Cards (8F10.0)

Columns	Variable	Entry
1-10	AREA1	Sectional area of the first member.
11-20		Repeat for each member.

c. Sectional Modulus Cards (8F10.0)

Columns	Variable	Entry
1-10	SX	Sectional modulus of the first member. It could be zero if the member stresses are not computed.
11-20		Repeat for each member.

d. Member Property Card (F10.0,2(F15.0,F10.0))

Columns	Variable	Entry
1-10	BASEA	The maximum moment of inertia among all members

of the frame or the maximum sectional area among all members of truss.

11-25	EE	Modulus of elasticity of girders and columns.
26-35	RHO	Specific weight of girders and columns.
36-50	E1	Modulus of elasticity of bracings.
51-60	RHO1	Specific weight of bracings.

(XI) DISPLACEMENT CONSTRAINT CARDS

Skip this data set for the analysis problem. If the displacement constraint is not considered in the design, then this data set is also skipped. Either a. or b. has to be prepared in accordance with the displacement constraint code.

a. Equal Value for All Displacement Constraints (3F10.0)

This card is needed when LMTDSP=1.

Columns	Variable	Entry
1-10	DEFMAX(1)	The allowable displacement in X-direction.
11-20	DEFMAX(2)	The allowable displacement in Y-direction.
21-30	DEFMAX(3)	The allowable displacement in Z-direction.

b. Displacement Constraints Vary

These cards are needed when LMTDSP=2 or 3.

(i) Control card (I5)

Columns	Variable	Entry
1-5	NLTDEF	Total number of displacement constraints.

(ii) Allowable values (3(F10.0,2I5))

Columns	Variable	Entry
1-10	TFR	The allowable value of the first constraint.

11-15	IM	Direction code (X=1, Y=2, Z=3).
16-20	JM	Node number.
21-30		Repeat for each additional displacement constraint.

(XII) FREQUENCY CONSTRAINT CARDS (8F10.0)

Skip this card if it is a analysis problem or is a static design problem.

Columns	Variable	Entry
1-10	VALMIN	The allowable frequency of the first desired mode.
11-20		Repeat for each additional mode.

3.3 Input Data Stream and Job Control Language

The input data stream for a job is shown in Figure 3.1. Several problems may be run at one time. Figure 3.2 shows the detailed stream of input codes. Code I is required for each job, and codes II through XII are then repeated for each problem. The detailed input information has already been presented in Sec. 3.2.

In order to run the program on an AMDAHL 470V/7, 470V/8, or IBM 4341 OS/VS1 system, the following sets of JCL are required:

- (1) To run card decks on an AMDAHL 470V/7 or 470V/8:

```
//jobname JOB (xxxxRL,pswd),'name',MSGLEVEL=(1,1)
/*JOBPARM L=20,T=10,R=1280
// EXEC FRTVCLG,PARM.FORT='SOURCE,MAP',PARM.LKED='SIZE=2048K',
// FVLNSPC='CYL,(1,1)'
```

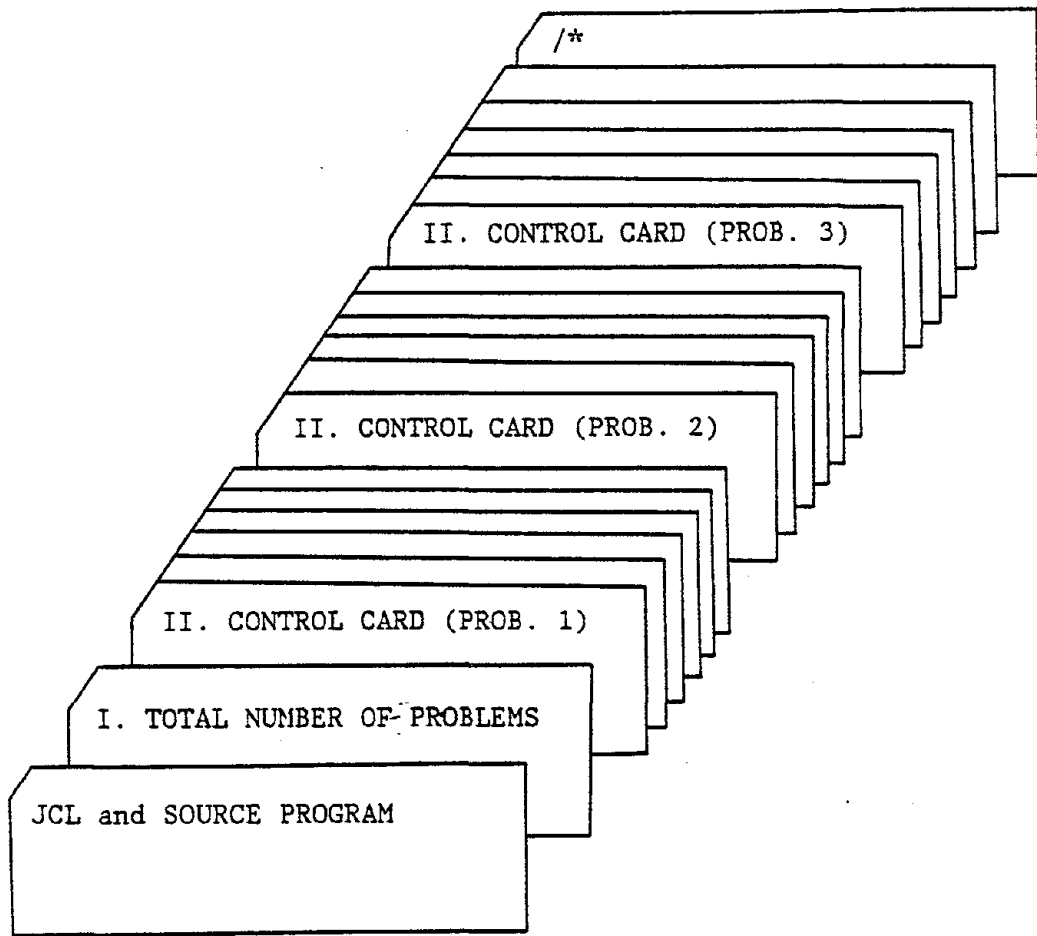



Figure 3.1 Input Data Stream

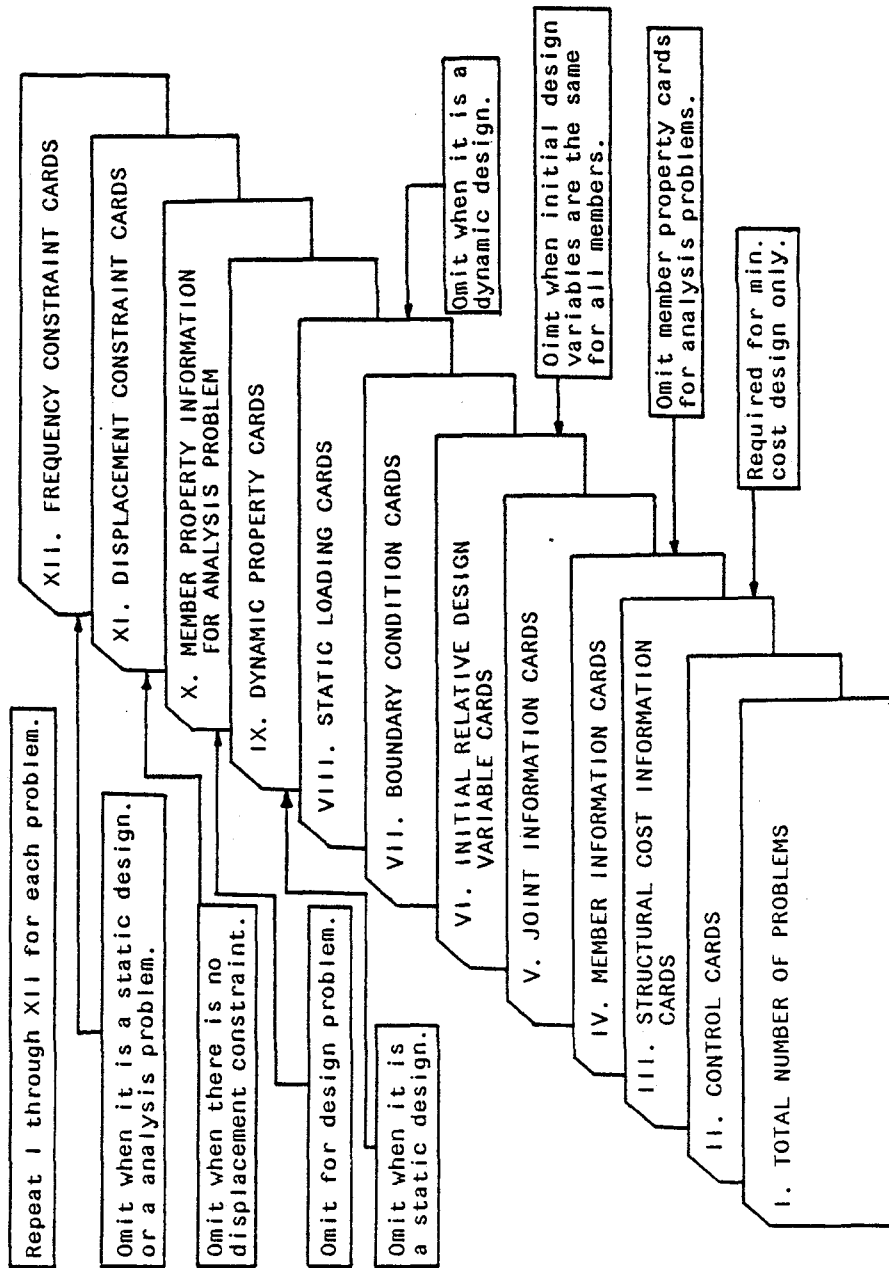


Figure 3.2 Detailed Input Data Stream

```

//FORT.SYSIN DD *
:
:
Source program
:
:
//GO.FT01F001 DD DSN=&&FT01,DISP=(NEW,DELETE),UNIT=SYSDA,
// DCB=(RECFM=VBS,LRECL=84,BLKSIZE=8440),SPACE=(TRK,(45,3))
//GO.FT02F001 DD DSN=&&FT02,DISP=(NEW,DELETE),UNIT=SYSDA,
// DCB=(RECFM=VBS,LRECL=84,BLKSIZE=8440),SPACE=(TRK,(45,3))
//GO.FT03F001 DD DSN=&&FT03,DISP=(NEW,DELETE),UNIT=SYSDA,
// DCB=(RECFM=VBS,LRECL=84,BLKSIZE=8440),SPACE=(TRK,(45,3))
//GO.FT15F001 DD DSN=filename of plotting data,
// DISP=(NEW,CATLG),SPACE=(TRK,(10,10),RLSE),
// UNIT=UMRDA,DCB=(RECFM=FB,LRECL=80,BLKSIZE=6160)
//GO.SYSIN DD *
:
:
Data
:
:
/*
//

```

(2) To run a program by executing a load module on an AMDAHL 470V/7

or 470V/8:

```
//jobname JOB (xxxxRL,pswd),'name',MSGLEVEL=(1,1)
/*JOBPARM L=20,T=10,R=1280
// EXEC PGM=name of the compiled program
//STEPLIB DD DSN=ddname of the compiled program,DISP=SHR
//FT01F001 DD DSN=&&FT01,DISP=(NEW,DELETE),UNIT=SYSDA,
// DCB=(RECFM=VBS,LRECL=84,BLKSIZE=8440),SPACE=(TRK,(45,3))
//FT02F001 DD DSN=&&FT02,DISP=(NEW,DELETE),UNIT=SYSDA,
// DCB=(RECFM=VBS,LRECL=84,BLKSIZE=8440),SPACE=(TRK,(45,3))
//FT03F001 DD DSN=&&FT03,DISP=(NEW,DELETE),UNIT=SYSDA,
// DCB=(RECFM=VBS,LRECL=84,BLKSIZE=8440),SPACE=(TRK,(45,3))
//FT15F001 DD DSN=filename of plotting data,
//      DISP=(NEW,CATLG),SPACE=(TRK,(10,10),RLSE),
//      UNIT=UMRDA,DCB=(RECFM=FB,LRECL=80,BLKSIZE=6160)
//FT05F001 DD *
          :
          :
          Data
          :
          :
/*
//
```

(3) To run card decks on an IBM 4341 OS/VS1 system:

```
//jobname JOB (xxxxVS1C,pswd),'name',MSGLEVEL=(1,1),
```

```

// MSGCLASS=A,TIME=20
//S1 EXEC FORTVCLG,PARM.FORT='SOURCE,MAP',
//  PARM.LKED='SIZE=2048K',FVLNSPC='CYL,(1,1)'
//FORT.SYSPRINT DD SYSOUT=A,OUTLIM=10000
//FORT.SYSIN DD *
      :
      :
      Source program
      :
      :
//LKED.SYSUT1 DD SPACE=(2048,(20,10,1),RLSE),
//  DCB=BLKSIZE=2048
//LKED.SYSLMOD DD SPACE=(2048,(20,10,1),RLSE),
//  DCB=BLKSIZE=2048
//LKED.SYSPRINT DD SYSOUT=A
//GO.FT06F001 DD SYSOUT=A,OUTLIM=10000
//GO.FT07F001 DD DUMMY
//GO.FT01F001 DD DSN=&&FT01,DISP=(NEW,DELETE),UNIT=SYSDA,
//  DCB=(RECFM=VBS,LRECL=84,BLKSIZE=8440),SPACE=(TRK,(45,3))
//GO.FT02F001 DD DSN=&&FT02,DISP=(NEW,DELETE),UNIT=SYSDA,
//  DCB=(RECFM=VBS,LRECL=84,BLKSIZE=8440),SPACE=(TRK,(45,3))
//GO.FT03F001 DD DSN=&&FT03,DISP=(NEW,DELETE),UNIT=SYSDA,
//  DCB=(RECFM=VBS,LRECL=84,BLKSIZE=8440),SPACE=(TRK,(45,3))
//GO.FT15F001 DD DSN=filename of plotting data,
//  DISP=(NEW,CATLG),SPACE=(TRK,(10,10),RLSE),

```

```

//          UNIT=UMRDA,DCB=(RECFM=FB,LRECL=80,BLKSIZE=6160)
//GO.SYSIN DD *
           :
           :
           Data
           :
           :
/*
//

```

- (4) To run a program by executing a load module on an IBM 4341 OS/VS1 system:

```

//jobname JOB (xxxxVS1C,pswd),'name',MSGLEVEL=(1,1)
// MSGCLASS=A,TIME=20
// EXEC PGM=name of the compiled program
//STEPLIB DD DSN=ddname of the compiled program,DISP=SHR
//FT06F001 DD SYSOUT=A,OUTLIM=10000
//FT07F001 DD DUMMY
//FT01F001 DD DSN=&&FT01,DISP=(NEW,DELETE),UNIT=SYSDA,
// DCB=(RECFM=VBS,LRECL=84,BLKSIZE=8440),SPACE=(TRK,(45,3))
//FT02F001 DD DSN=&&FT02,DISP=(NEW,DELETE),UNIT=SYSDA,
// DCB=(RECFM=VBS,LRECL=84,BLKSIZE=8440),SPACE=(TRK,(45,3))
//FT03F001 DD DSN=&&FT03,DISP=(NEW,DELETE),UNIT=SYSDA,
// DCB=(RECFM=VBS,LRECL=84,BLKSIZE=8440),SPACE=(TRK,(45,3))
//FT15F001 DD DSN=filename of plotting data,

```

```
//      DISP=(NEW,CATLG),SPACE=(TRK,(10,10),RLSE),  
//      UNIT=UMRDA,DCB=(RECFM=FB,LRECL=80,BLKSIZE=6160)
```

```
//FT05F001 DD *
```

```
      :
```

```
      :
```

```
      Data
```

```
      :
```

```
      :
```

```
/*
```

```
//
```



```

C----- CALCULATE THE DISPLACEMENTS AND STRESSES -----
IAEDUC=1
GO TO 15000
C----- CALCULATE THE ACTUAL CHANGE IN SIZE OF ELEMENT I -----
20089 CONTINUE I=1, MEMBS
DO 20091 I=1, MEMBS
STRESS=STRESS(I)
IF (STRESS(I).GT.0) STRESS=0.65*STRESS
IF (STRESS(I).LT.-0.65*STRESS) STRESS=-0.65*STRESS
IF (STRESS(I).GT.0) STRESS=0.65*STRESS
IF (STRESS(I).LT.-0.65*STRESS) STRESS=-0.65*STRESS
CONTINUE
20401 IF (ACTIV=0) GO TO 20113
SUMSTRESS=0
DO 20091 I=1, MEMBS
SUMSTRESS=SUMSTRESS+STRESS(I)
CONTINUE
20091 IF (SUMSTRESS.GT.0) GO TO 20403
IF (SUMSTRESS.LT.0) GO TO 20403
SUMSTRESS=ABS(SUMSTRESS)
DO 20091 I=1, MEMBS
STRESS(I)=STRESS(I)/SUMSTRESS
CONTINUE
20403 GO TO 20116
C----- CALCULATE THE ACTUAL CHANGE IN SIZE OF ELEMENT I -----
20100 RATIO=0
DEFB=DEFB*(1+I)/MEMBS
ANAX=ANAX*(1+I)/MEMBS
ANAY=ANAY*(1+I)/MEMBS
ANAZ=ANAZ*(1+I)/MEMBS
RATIO=RATIO+ANAX**2+ANAY**2+ANAZ**2
CONTINUE
20101 CONTINUE ANSDR(1:3, I)=ANAX, ANAY, ANAZ
SUMDR=SUMDR+ANSDR(1:3, I)
DO 20111 I=1, MEMBS
DELTA=DELTA*(1+I)/MEMBS
SUMDR=SUMDR+DELTA*(1+I)/MEMBS
IF (SUMDR.GT.0) GO TO 20114
IF (SUMDR.LT.0) GO TO 20114
CONTINUE
20114 WRITE(OUT, 20121) ALL INCREMENTS ARE NEGATIVE *****
20115 CONTINUE
20115 DO 20117 I=1, MEMBS
SUMD(1:3, I)=DELTA*(1+I)/MEMBS
CONTINUE
20116 DEFIX=0
IF (DEFIX.GT.0) GO TO 20117
IF (DEFIX.LT.0) GO TO 20117
CONTINUE
20117 RATIOA=DEFIX
IF (RATIOA.GT.0) URANGE=0
IF (RATIOA.LT.0) URANGE=1
20127 RATIOA=URANGE
20128 GO TO 20129
20129 RATIOA=URANGE
20130 RATIOA=URANGE
20131 DO 20133 I=1, MEMBS
DO 20133 I=1, MEMBS
DO 20133 I=1, MEMBS
DO 20133 I=1, MEMBS
20136 ANAX=ANAX*(1+I)/MEMBS

```

```

20140 DO 20142 L=1, MEMBS
ANAX=ANAX*(1+L)/MEMBS
ANAY=ANAY*(1+L)/MEMBS
ANAZ=ANAZ*(1+L)/MEMBS
CONTINUE
20142 CONTINUE
500 CONTINUE
505 IF (INDEX.EQ.1) GO TO 102
506 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
507 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
508 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
509 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
510 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
511 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
512 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
513 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
514 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
515 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
516 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
517 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
518 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
519 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
520 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
521 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
522 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
523 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)
524 CALL RESFOR(DM, I, MD, NH, NBD, NDY, NDESR, NMMAX)

```



```

CUR10400 CUR20090
CUR10410 CUR20100
CUR10420 CUR20110
CUR10430 CUR20120
CUR10440 CUR20130
CUR10450 CUR20140
CUR10460 CUR20150
CUR10470 CUR20160
CUR10480 CUR20170
CUR10490 CUR20180
CUR10500 CUR20190
CUR10510 CUR20200
CUR10520 CUR20210
CUR10530 CUR20220
CUR10540 CUR20230
CUR10550 CUR20240
CUR10560 CUR20250
CUR10570 CUR20260
CUR10580 CUR20270
CUR10590 CUR20280
CUR10600 CUR20290
CUR10610 CUR20300
CUR10620 CUR20310
CUR10630 CUR20320
CUR10640 CUR20330
CUR10650 CUR20340
CUR10660 CUR20350
CUR10670 CUR20360
CUR10680 CUR20370
CUR10690 CUR20380
CUR10700 CUR20390
CUR10710 CUR20400
CUR10720 CUR20410
CUR10730 CUR20420
CUR10740 CUR20430
CUR10750 CUR20440
CUR10760 CUR20450
CUR10770 CUR20460
CUR10780 CUR20470
CUR10790 CUR20480
CUR10800 CUR20490
CUR10810 CUR20500
CUR10820 CUR20510
CUR10830 CUR20520
CUR10840 CUR20530
CUR10850 CUR20540
CUR10860 CUR20550
CUR10870 CUR20560
CUR20010 CUR30010
CUR20020 CUR30020
CUR20030 CUR30030
CUR20040 CUR30040
CUR20050 CUR30050
CUR20060 CUR30060
CUR20070 CUR30070
CUR20080 CUR30080
CUR20090 CUR30090
CUR20100 CUR30100
CUR20110 CUR30110
CUR20120 CUR30120
CUR20130 CUR30130
CUR20140 CUR30140
CUR20150 CUR30150
CUR20160 CUR30160
CUR20170 CUR30170
CUR20180 CUR30180
CUR20190 CUR30190
CUR20200 CUR30200
CUR20210 CUR30210
CUR20220 CUR30220
CUR20230 CUR30230
CUR20240 CUR30240
CUR20250 CUR30250
CUR20260 CUR30260
CUR20270 CUR30270
CUR20280 CUR30280
CUR20290 CUR30290
CUR20300 CUR30300
CUR20310 CUR30310
CUR20320 CUR30320
CUR20330 CUR30330
CUR20340 CUR30340
CUR20350 CUR30350
CUR20360 CUR30360
CUR20370 CUR30370
CUR20380 CUR30380
CUR20390 CUR30390
CUR20400 CUR30400
CUR20410 CUR30410
CUR20420 CUR30420
CUR20430 CUR30430
CUR20440 CUR30440
CUR20450 CUR30450
CUR20460 CUR30460
CUR20470 CUR30470
CUR20480 CUR30480
CUR20490 CUR30490
CUR20500 CUR30500
CUR20510 CUR30510
CUR20520 CUR30520
CUR20530 CUR30530
CUR20540 CUR30540
CUR20550 CUR30550
CUR20560 CUR30560
CUR20570 CUR30570
CUR20580 CUR30580
CUR20590 CUR30590
CUR20600 CUR30600
CUR20610 CUR30610
CUR20620 CUR30620
CUR20630 CUR30630
CUR20640 CUR30640
CUR20650 CUR30650
CUR20660 CUR30660
CUR20670 CUR30670
CUR20680 CUR30680
CUR20690 CUR30690
CUR20700 CUR30700
CUR20710 CUR30710
CUR20720 CUR30720
CUR20730 CUR30730
CUR20740 CUR30740
CUR20750 CUR30750
CUR20760 CUR30760
CUR20770 CUR30770
CUR20780 CUR30780
CUR20790 CUR30790
CUR20800 CUR30800
CUR20810 CUR30810
CUR20820 CUR30820
CUR20830 CUR30830
CUR20840 CUR30840
CUR20850 CUR30850
CUR20860 CUR30860
CUR20870 CUR30870
CUR20880 CUR30880
CUR20890 CUR30890
CUR20900 CUR30900
CUR20910 CUR30910
CUR20920 CUR30920
CUR20930 CUR30930
CUR20940 CUR30940
CUR20950 CUR30950
CUR20960 CUR30960
CUR20970 CUR30970
CUR20980 CUR30980
CUR20990 CUR30990
CUR21000 CUR31000

```



```

100020
100030
100040
100050
100060
100070
100080
100090
100100
100110
100120
100130
100140
100150
100160
100170
100180
100190
100200
100210
100220
100230
100240
100250
100260
100270
100280
100290
100300
100310
100320
100330
100340
100350
100360
100370
100380
100390

```

```

100020
100030
100040
100050
100060
100070
100080
100090
100100
100110
100120
100130
100140
100150
100160
100170
100180
100190
100200
100210
100220
100230
100240
100250
100260
100270
100280
100290
100300
100310
100320
100330
100340
100350
100360
100370
100380
100390

```

```

100020
100030
100040
100050
100060
100070
100080
100090
100100
100110
100120
100130
100140
100150
100160
100170
100180
100190
100200
100210
100220
100230
100240
100250
100260
100270
100280
100290
100300
100310
100320
100330
100340
100350
100360
100370
100380
100390

```

```

100020
100030
100040
100050
100060
100070
100080
100090
100100
100110
100120
100130
100140
100150
100160
100170
100180
100190
100200
100210
100220
100230
100240
100250
100260
100270
100280
100290
100300
100310
100320
100330
100340
100350
100360
100370
100380
100390

```

```

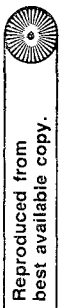
100020
100030
100040
100050
100060
100070
100080
100090
100100
100110
100120
100130
100140
100150
100160
100170
100180
100190
100200
100210
100220
100230
100240
100250
100260
100270
100280
100290
100300
100310
100320
100330
100340
100350
100360
100370
100380
100390

```

```

100020
100030
100040
100050
100060
100070
100080
100090
100100
100110
100120
100130
100140
100150
100160
100170
100180
100190
100200
100210
100220
100230
100240
100250
100260
100270
100280
100290
100300
100310
100320
100330
100340
100350
100360
100370
100380
100390

```



Reproduced from
best available copy.

IV. SAMPLE EXAMPLES

There are three examples illustrated in this chapter. The geometry and dimensions of each are shown in Figures 4.1 through 4.3. The description of the design loading and other input information is given in Sec. 4.1. The input and output listings of each example are shown in Sec. 4.2 for which the sign convention of the member forces and deformations are illustrated in Figure 4.4.

4.1 Description of Input Information for Sample Problems

The loading and other input information for these three examples are given as follows:

a. Static design of ten-bar truss. The geometry and dimensions of the ten-bar truss is shown in Figure 4.1. There are 100 kips (444.8 kN) of static concentrated load acting downward on nodes 2 and 4 simultaneously. The material properties are the modulus of elasticity, 1.0×10^7 psi (68950 N/mm²), and the specific weight, 0.1 lbs/in³ (0.0271 N/cm³). The lower bound of the sectional area is 0.1 in² (64.52 mm²), and the allowable stress is 25,000 psi (172.375 N/mm²) for all members. The allowable displacement is ± 2.0 in. (5.08 cm) in the y-direction at nodes 1, 2, 3, and 4. The maximum number of iterations is 20. The incremental and reduction factors that are used to calculate the constraint gradients are 1 and 10% respectively. The upper bound of the change of member size, δ , for the constraint gradient method is 0.5, and the lower bound is 0.2.

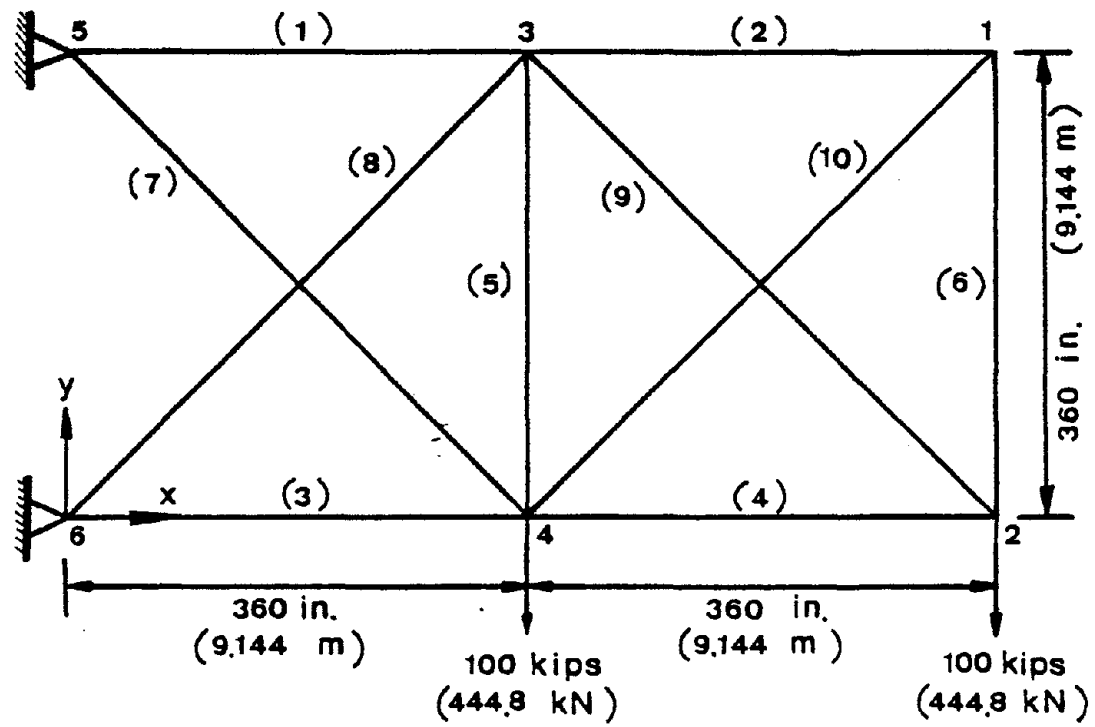


Figure 4.1 Ten-Bar Truss of Sample Example 1

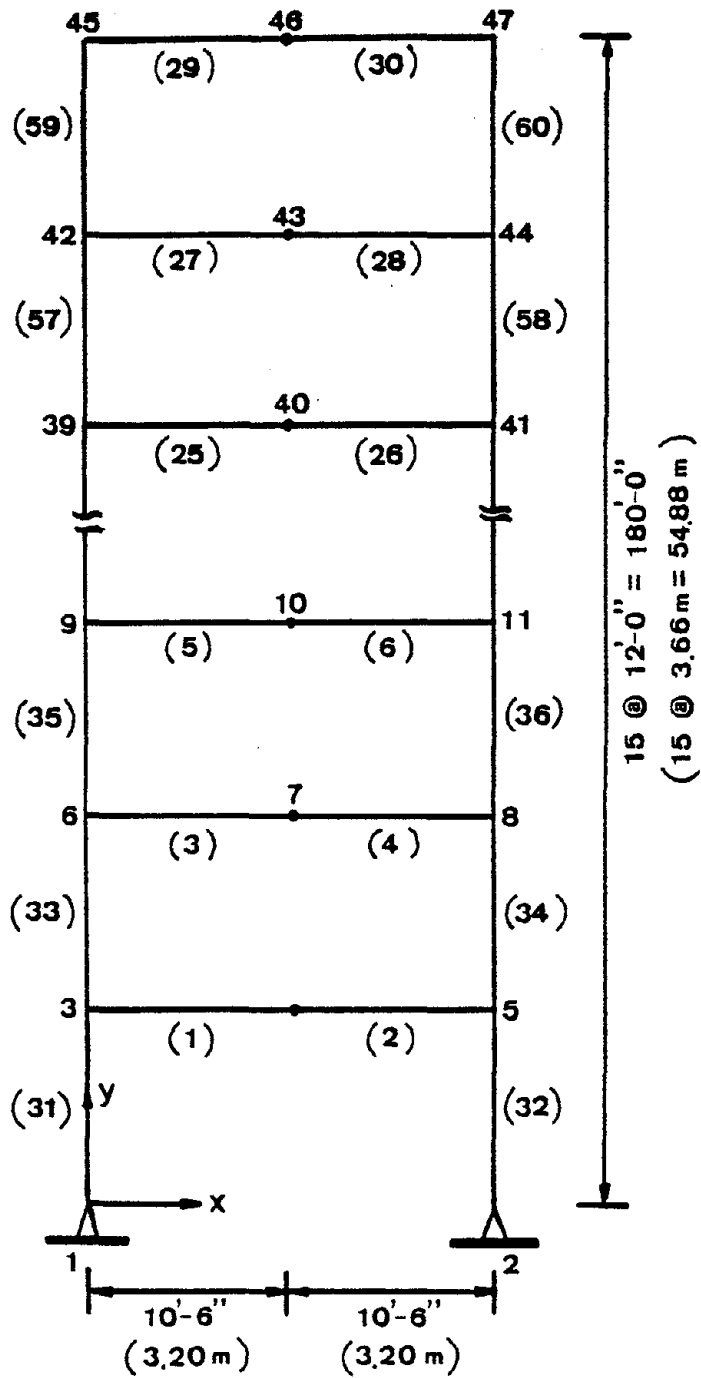


Figure 4.2 Fifteen-Story One-Bay Unbraced Frame of Sample Example 2.

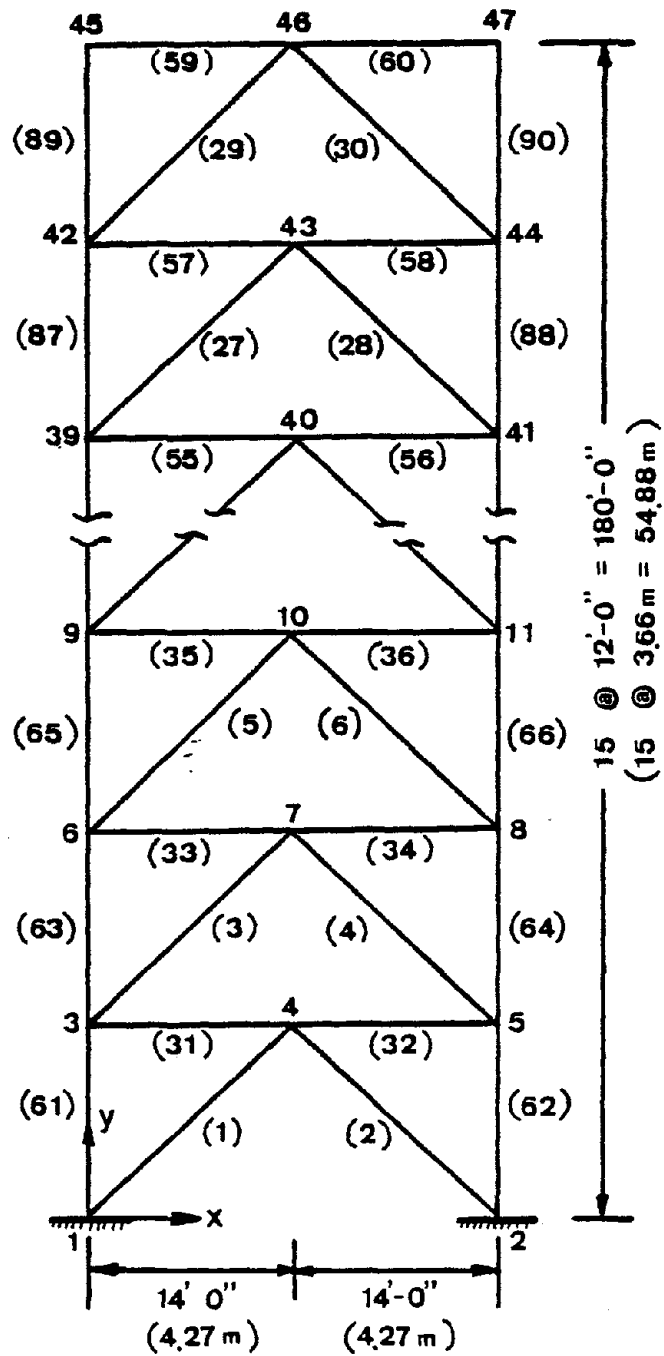
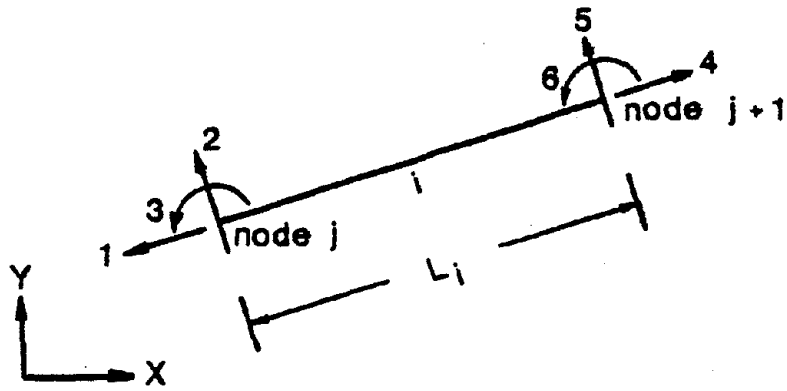
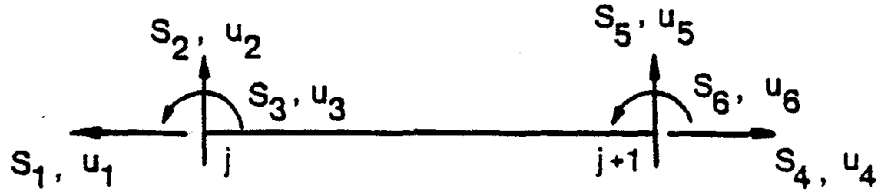


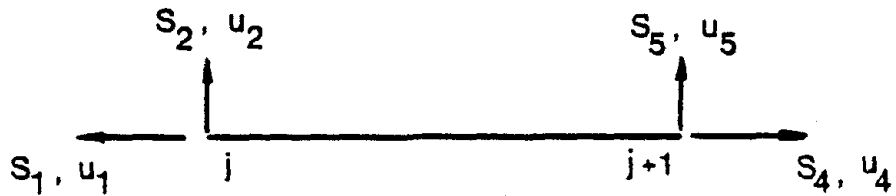
Figure 4.3 Fifteen-Story One-Bay K-Braced Frame of Sample Example 3.



(a) Coordinates of Planar Members



(b) Sign Convention of the Forces and Deformations for Girders and Columns.



(c) Sign Convention of Forces and Deformations for Bracings

Figure 4.4 Positive Forces and Deformations of Typical Members

b. Fifteen-story, one-bay, unbraced frame dynamic design based on Newmark's design spectra. The fifteen-story, one-bay, unbraced frame shown in Figure 4.2 is composed of AISC wide-flange sections. The modulus of elasticity, E , is 29×10^6 psi (199.955 kN/mm^2), and the specific weight, ρ , is 0.283 lbs/in^3 (0.0768 N/cm^3). The allowable stress for bending combined with the axial force is assumed to be $29,000$ psi (199.955 N/mm^2), and the allowable displacement is $0.00375h_x$ for which h_x is the height of floor level, x , measured from the ground surface. The spectral acceleration is obtained from the Newmark's design spectra with 5% damping and 90 percentile alluvium. The maximum ground acceleration is $0.0893g$ for the horizontal motion and a scale factor of 0.67 for the vertical motion. The P- Δ effect is also considered in the design. The uniform nonstructural mass on each floor level is 180 lbs/in. (31.52 kN/m). The dynamic analysis, however, does not include the effect of the static loads.

c. Seismic-resistant optimum weight design of a fifteen-story, one-bay, K-braced frame based on ATC-3-06 equivalent lateral force method. Figure 4.3 shows the schematic diagram and dimensions of the K-braced frame. The girders and columns are designed on the basis of AISC wide-flange sections, and the sections of the braces can be either rectangular or circular. The modulus of elasticity, E , is 29×10^6 psi (199.955 kN/mm^2), and the specific weight, ρ , is 0.283 lbs/in^3 (0.0768 N/cm^3). The lower bound of the moment of inertia of the wide-flange section is 10 in^4 (416.23 cm^4). The sectional area of the braces can not be larger than 6 in^2 (38.71 cm^2) nor smaller than 0.5

in² (3.23 cm²). The frame is subjected to a gravity load of 50,000 lbs (222.4 kN) on each floor level, and the seismic lateral forces are determined in accordance with the requirements of the ATC-3-06 equivalent lateral force method. The map area numbers of A_a and A_v for the location of structure are both 7. The site condition is an S_3 soil profile type. The response modification factor, R , and the deflection amplification factor, C_d , are 6.0 and 5.0 respectively. The frame is classified as in seismic hazard exposure group II for which the allowable drift based on the elastic analysis is $0.015h_{sx}/C_d$, for which h_{sx} is the story height below level x . The incremental factor and the reduction factor used in the constraint gradient method are both 1%. The upper bound and the lower bound of δ are 0.8 and 0.3 respectively.

All of the input data for these three examples are respectively shown in Sections 4.2(a), 4.2(c), and 4.2(e). The design results of the first cycle and the optimum design cycle including the member forces, combined stresses, and nodal displacements of the optimum design cycle are given in the rest of the sections in this chapter.

4.2 Typical Input and Output Solutions

Reproduced from
best available copy.



a. Input Data of Example 1.

```

1 0
5 0 0 0 1 10 4 1 10 0 0
0.02 0.01 0.01 0.0001 0.5 0.2 0.10 1.01
10 6 10 1 2 -1 20 -1TEN BAR TRUSS, WT, GRADIENT, EXAMPLE 1
0.
10000000. 0.1 100. 1000. 0.1
1 3 5 0 25000. 0. 0. 0. 1
2 1 3 0 25000. 0. 0. 0. 2
3 4 6 0 25000. 0. 0. 0. 3
4 2 4 0 25000. 0. 0. 0. 4
5 3 4 0 25000. 0. 0. 0. 5
6 1 2 0 25000. 0. 0. 0. 6
7 4 5 0 25000. 0. 0. 0. 7
8 3 6 0 25000. 0. 0. 0. 8
9 2 3 0 25000. 0. 0. 0. 9
10 1 4 0 25000. 0. 0. 0. 10
1 720. 360.
2 720. 0.
3 360. 360.
4 360. 0.
5 0. 360.
6 0. 0.
3 6 9 12 13 14 15 16 17 18
2
-100000. 2 2-100000. 2 4
0. 0. 0. 0.
0 0 0 3366.4
4
2. 2 1 2. 2 2 2. 2 3
2. 2 4

```

b. Output Solution of Example 1.

```

STRUCTURE NO. 1: TEN BAR TRUSS, WT, GRADIENT, EXAMPLE 1
TOTAL NUMBER OF STORIES----- 1
TOTAL NUMBER OF MEMBERS----- 10
TOTAL NUMBER OF NODES----- 6
TOTAL NUMBER OF RESTRAINED D.O.F.----- 10
TOTAL NUMBER OF LOADING CONDITIONS----- 1
DISPLACEMENT CONSTRAINT CODE----- 2
EQ. 0: NO DISPLACEMENT CONSTRAINTS
EQ. 1: DISPLACEMENT CONSTRAINT IS THE SAME FOR ALL NODES
EQ. 2: DISPLACEMENT CONSTRAINT CAN BE VARIED PER NODE
EQ. 3: DRIFT CONSTRAINT ON EACH STORY
TYPE OF DESIGN----- -1
EQ. -1: STATIC DESIGN
EQ. 0: STATIC AND DYNAMIC DESIGN
EQ. 1: DYNAMIC DESIGN
MAXIMUM NUMBER OF ITERATIONS----- 20
TYPE OF SECTIONS----- -1
EQ. 0: BUILT-UP SECTIONS
EQ. 1: WF SECTIONS
EQ. 2: TRUSS MEMBERS
BRACING PROPERTIES:
MODULUS OF ELASTICITY----- 10000000.0
SPECIFIC WEIGHT----- 0.100
MAXIMUM BRACING AREA----- 1000.000
MINIMUM BRACING AREA----- 0.100
GRADIENT PARAMETERS:
GRADIENT ITERATION----- 10
UPPER RANGE FOR DI----- 0.50000
LOWER RANGE FOR DI----- 0.20000
REDUCTION FACTOR----- 0.10000
INCREMENTAL FACTOR----- 1.01000

```

MEMBER IDENTIFICATION DATA:

NO	J-END	K-END	TYPE	STRESS LIMIT	DEAD LOAD	LIVE LOAD	SNOW LOAD	OVN	NO	GIRDER IDENTIFICATION NUMBERS:
1	3	5	0	25000.000	0.0	0.0	0.0	1	0	0
2	1	3	0	25000.000	0.0	0.0	0.0	2	0	0
3	4	6	0	25000.000	0.0	0.0	0.0	3	0	0
4	2	4	0	25000.000	0.0	0.0	0.0	4	0	0
5	3	4	0	25000.000	0.0	0.0	0.0	5	0	0
6	1	2	0	25000.000	0.0	0.0	0.0	6	0	0
7	4	5	0	25000.000	0.0	0.0	0.0	7	0	0
8	3	6	0	25000.000	0.0	0.0	0.0	8	0	0
9	2	3	0	25000.000	0.0	0.0	0.0	9	0	0
10	1	4	0	25000.000	0.0	0.0	0.0	10	0	0

HBAND WIDTH = 7

RESTRAINED DEGREES OF FREEDOM:

3 6 9 12 13 14 15 16 17 18

TOTAL ELEMENTS NONZERO ELEMENTS STORED ELEMENTS

171 126 135

STARTING ROW NUMBERS FOR EACH COLUMN

1 1 1 1 1 1 1 1 1 1
1 1 7 7 7 7 7 7 1 1

NUMBERS OF DIAGONAL ELEMENTS IN SINGLE ARRAY STIFFNESS MATRIX

1 3 6 10 15 21 28 36 45 55
66 78 85 93 102 112 123 135

TOTAL ELEMENTS MEAN THE TOTAL NUMBER OF ELEMENTS IN THE STIFFNESS MATRIX. NONZERO ELEMENTS ARE THE TOTAL NUMBER OF NON-ZERO ELEMENTS IN THE STIFFNESS MATRIX, AND STORED ELEMENTS ARE THE ELEMENTS IN THE STIFFNESS MATRIX NEEDED TO BE STORED BY USING THIS NUMBERING SCHEME. WHEN THE NUMBERING SCHEME IS AN EFFICIENT ONE, THE NUMBER OF NONZERO ELEMENTS IS CLOSE TO THE NUMBER OF STORED ELEMENTS.

LOADING INFORMATION :

STATIC LOADING INFORMATION:

JOINT LOADS FOR LOADING CONDITION 1 ...

JOINT DIRECTION VALUE
2 2 -100000.000
4 2 -100000.000

LOAD COMBINATION FACTORS FOR LOADING CONDITION 1

DEAD LOAD ----- 0.0
LIVE LOAD ----- 0.0
SNOW LOAD ----- 0.0
EARTHQUAKE LOAD ----- 0.0

DYNAMIC PROPERTIES:

FIRST MODE NUMBER----- 0
LAST MODE NUMBER----- 0
GRAVITY ACCELERATION----- 386.40
TYPE OF MODE SUPERPOSITION----- 0
EQ. -1: DIRECT SUPERPOSITION OF MODES
EQ. 0: ROOT-MEAN-SQUARE SUPERPOSITION OF MODES
EQ. 1: ABSOLUTE SUPERPOSITION OF MODES

DISPLACEMENT CONSTRAINTS:

NODE DIRECTION VALUE
1 2 2.000
2 2 2.000
3 2 2.000
4 2 2.000

RELATIVE SIZES OF MEMBERS:
 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000

OLD SCALING FACTOR----- 0.19698E+02
 NEW SCALING FACTOR----- 0.19698E+02
 ACTIVE STRESS IN LOADING CONDITION 1----- 3
 ACTIVE DISPLACEMENT IN LOADING CONDITION 1----- 5
 MOST CRITICAL CONSTRAINT----- 2
 NUMBER OF ANALYSIS----- 2

***** TOTAL WEIGHT OF STRUCTURE = 8.266156E+03 *****
 ***** BASE AE = 1.969792E+08; CYCLE NO. = 1 *****

NO CONSTRAINT ENERGY DENSITY RATIO
 1 2 0.962437E+00
 2 2 0.887448E-01
 3 2 0.100000E+01
 4 2 0.100189E+00
 5 1 0.519373E-02
 6 2 0.587493E-01
 7 2 0.338123E+00
 8 2 0.315723E+00
 9 2 0.216378E+00
 10 2 0.117490E+00

.
.
.
.
.
.
.
.
.
.

RELATIVE SIZES OF MEMBERS:
 1.00000000 0.0032081190 0.726609536 0.492967963 0.0033042870 0.0172402486 0.240490201 0.700974107 0.697161853 0.0032081190

OLD SCALING FACTOR----- 0.30718E+02
 NEW SCALING FACTOR----- 0.30718E+02
 ACTIVE STRESS IN LOADING CONDITION 1----- 5
 ACTIVE DISPLACEMENT IN LOADING CONDITION 1----- 2 5
 MOST CRITICAL CONSTRAINT----- 2
 NUMBER OF ANALYSIS----- 1

***** TOTAL WEIGHT OF STRUCTURE = 5.065000E+03 *****
 ***** BASE AE = 3.071770E+08; CYCLE NO. = 11 *****

MEMBER	LENGTH	AE	I	AREA	MA	MB	AXIAL FORCE	SHEAR	MOMENT	AXIAL FORCE	SHEAR	MOMENT
1	360.00	1.00000		30.7177	3	5	-2.02680E+05 -6.59814E+03					
2	360.00	0.00321		0.0985	1	3	1.18091E+02 1.19834E+03					
3	360.00	0.72681		22.3874	4	6	1.97319E+05 8.81386E+03					
4	360.00	0.49297		15.1428	2	4	1.00118E+05 6.61156E+03					
5	360.00	0.00338		0.1040	3	4	-2.56207E+03 -2.46454E+04					
6	360.00	0.01724		0.5296	1	2	1.18062E+02 2.22936E+02					

7	509.12	0.24949	7.6638	4	5	-1.37631E+05	-1.79567E+04
8	509.12	0.70097	21.5323	3	6	1.45211E+05	6.74368E+03
9	509.12	0.69716	21.4152	2	3	-1.41588E+05	-6.61157E+03
10	509.12	0.00321	0.0985	1	4	-1.67006E+02	-1.69470E+03

JOINT	-X	-Y	DISPL-X	DISPL-Y	ROTATION
1	720.000	360.000	1.96392979E-01	-2.00000000E+00	0.0
2	720.000	0.0	-5.55315733E-01	-1.99197483E+00	0.0
3	360.000	360.000	2.37533092E-01	-7.23092675E-01	0.0
4	360.000	0.0	-3.17299247E-01	-1.61032677E+00	0.0
5	0.0	360.000	0.0	0.0	0.0
6	0.0	0.0	0.0	0.0	0.0

c. Input Data of Example 2.

```

1 0
5 0 0 0 15 5 2 1 10 0
0.02 0.01 0.01 0.0001 0.6 0.3 0.01 1.01
40 47 6 1 2 1 8 115-ST. 1 BAY, MEMARK SPECTRA, EXAMPLE 2.
29000000. 0.283 700. 20300. 10. 0.1
0.
1 3 4 2 29000. 180. 0. 0. 1 1
2 4 5 2 29000. 180. 0. 0. 1 2
3 6 7 2 29000. 180. 0. 0. 3 1
4 7 8 2 29000. 180. 0. 0. 3 2
5 9 10 2 29000. 180. 0. 0. 5 1
6 10 11 2 29000. 180. 0. 0. 5 2
7 12 13 2 29000. 180. 0. 0. 7 1
8 13 14 2 29000. 180. 0. 0. 7 2
9 15 16 2 29000. 180. 0. 0. 9 1
10 16 17 2 29000. 180. 0. 0. 9 2
11 18 19 2 29000. 180. 0. 0. 11 1
12 19 20 2 29000. 180. 0. 0. 11 2
13 21 22 2 29000. 180. 0. 0. 13 1
14 22 23 2 29000. 180. 0. 0. 13 2
15 24 25 2 29000. 180. 0. 0. 15 1
16 25 26 2 29000. 180. 0. 0. 15 2
17 27 28 2 29000. 180. 0. 0. 17 1
18 28 29 2 29000. 180. 0. 0. 17 2
19 30 31 2 29000. 180. 0. 0. 19 1
20 31 32 2 29000. 180. 0. 0. 19 2
21 33 34 2 29000. 180. 0. 0. 21 1
22 34 35 2 29000. 180. 0. 0. 21 2
23 36 37 2 29000. 180. 0. 0. 23 1
24 37 38 2 29000. 180. 0. 0. 23 2
25 39 40 2 29000. 180. 0. 0. 25 1
26 40 41 2 29000. 180. 0. 0. 25 2
27 42 43 2 29000. 180. 0. 0. 27 1
28 43 44 2 29000. 180. 0. 0. 27 2
29 45 46 2 29000. 180. 0. 0. 29 1
30 46 47 2 29000. 180. 0. 0. 29 2
31 1 3 1 29000. 0. 0. 0. 31 1 59
1 1 3 3 5 5 7 7 9 9 11 11 13 13 15 15
17 17 19 19 21 21 23 23 25 25 27 27 29 29 33 33
35 35 37 37 39 39 41 41 43 43 45 45 47 47 49 49
51 51 53 53 55 55 57 57 59 59 31
32 2 5 1 29000. 0. 0. 0. 31 2 59
2 2 4 4 6 6 8 8 10 10 12 12 14 14 16 16

```


18	18	20	20	22	22	24	24	26	26	28	28	30	30	34	34
36	36	38	38	40	40	42	42	44	44	46	46	48	48	50	50
52	52	54	54	56	56	58	58	60	60	32					
33	3	6	1	29000.								0.	33	1	55
3	3	5	5	7	7	9	9	11	11	13	13	15	15	17	17
19	19	21	21	23	23	25	25	27	27	29	29	31	31	33	33
39	39	41	41	43	43	45	45	47	47	49	49	51	51	53	53
55	55	57	57	59	59	33									
34	5	8	1	29000.								0.	33	2	55
4	4	6	6	8	8	10	10	12	12	14	14	16	16	18	18
20	20	22	22	24	24	26	26	28	28	30	30	32	32	34	34
40	40	42	42	44	44	46	46	48	48	50	50	52	52	54	54
56	56	58	58	60	60	34									
35	6	9	1	29000.								0.	35	1	51
5	5	7	7	9	9	11	11	13	13	15	15	17	17	19	19
21	21	23	23	25	25	27	27	29	29	31	31	33	33	35	35
43	43	45	45	47	47	49	49	51	51	53	53	55	55	57	57
59	59	35													
36	8	11	1	29000.								0.	35	2	51
6	6	8	8	10	10	12	12	14	14	16	16	18	18	20	20
22	22	24	24	26	26	28	28	30	30	32	32	34	34	36	36
44	44	46	46	48	48	50	50	52	52	54	54	56	56	58	58
60	60	36													
37	9	12	1	29000.								0.	37	1	47
7	7	9	9	11	11	13	13	15	15	17	17	19	19	21	21
23	23	25	25	27	27	29	29	31	31	33	33	35	35	37	37
47	47	49	49	51	51	53	53	55	55	57	57	59	59	61	61
38	11	14	1	29000.								0.	37	2	47
8	8	10	10	12	12	14	14	16	16	18	18	20	20	22	22
24	24	26	26	28	28	30	30	32	32	34	34	36	36	38	38
48	48	50	50	52	52	54	54	56	56	58	58	60	60	62	62
39	12	15	1	29000.								0.	39	1	43
9	9	11	11	13	13	15	15	17	17	19	19	21	21	23	23
25	25	27	27	29	29	31	31	33	33	35	35	37	37	39	39
51	51	53	53	55	55	57	57	59	59	39					
40	14	17	1	29000.								0.	39	2	43
10	10	12	12	14	14	16	16	18	18	20	20	22	22	24	24
26	26	28	28	30	30	32	32	34	34	36	36	38	38	40	40
52	52	54	54	56	56	58	58	60	60	40					
41	15	18	1	29000.								0.	41	1	39
11	11	13	13	15	15	17	17	19	19	21	21	23	23	25	25
27	27	29	29	31	31	33	33	35	35	37	37	39	39	41	41
53	53	55	55	57	57	59	59	41							
42	17	20	1	29000.								0.	41	2	39
12	12	14	14	16	16	18	18	20	20	22	22	24	24	26	26
28	28	30	30	32	32	34	34	36	36	38	38	40	40	42	42
54	54	56	56	58	58	60	60	42							
43	18	21	1	29000.								0.	43	1	35
13	13	15	15	17	17	19	19	21	21	23	23	25	25	27	27
29	29	31	31	33	33	35	35	37	37	39	39	41	41	43	43
59	59	43													
44	20	23	1	29000.								0.	43	2	35
14	14	16	16	18	18	20	20	22	22	24	24	26	26	28	28
30	30	32	32	34	34	36	36	38	38	40	40	42	42	44	44
60	60	44													
45	21	24	1	29000.								0.	45	1	31
15	15	17	17	19	19	21	21	23	23	25	25	27	27	29	29
47	47	49	49	51	51	53	53	55	55	57	57	59	59	61	61
46	23	26	1	29000.								0.	45	2	31
16	16	18	18	20	20	22	22	24	24	26	26	28	28	30	30
48	48	50	50	52	52	54	54	56	56	58	58	60	60	62	62
47	24	27	1	29000.								0.	47	1	27
17	17	19	19	21	21	23	23	25	25	27	27	29	29	31	31
51	51	53	53	55	55	57	57	59	59	47					
48	26	29	1	29000.								0.	47	2	27
18	18	20	20	22	22	24	24	26	26	28	28	30	30	32	32
52	52	54	54	56	56	58	58	60	60	48					
49	27	30	1	29000.								0.	49	1	23
19	19	21	21	23	23	25	25	27	27	29	29	31	31	33	33
53	53	55	55	57	57	59	59	49							
50	29	32	1	29000.								0.	49	2	23
20	20	22	22	24	24	26	26	28	28	30	30	32	32	34	34
56	56	58	58	60	60	50									
51	30	33	1	29000.								0.	51	1	19
21	21	23	23	25	25	27	27	29	29	31	31	33	33	35	35
59	59	51													
52	32	35	1	29000.								0.	51	2	19
22	22	24	24	26	26	28	28	30	30	32	32	34	34	36	36
60	60	52													
53	33	36	1	29000.								0.	53	1	15
23	23	25	25	27	27	29	29	31	31	33	33	35	35	37	37
54	35	38	1	29000.								0.	53	2	15
24	24	26	26	28	28	30	30	32	32	34	34	36	36	38	38
55	36	39	1	29000.								0.	55	1	11
25	25	27	27	29	29	31	31	33	33	35	35	37	37	39	39
56	38	41	1	29000.								0.	55	2	11
26	26	28	28	30	30	32	32	34	34	36	36	38	38	40	40
57	39	42	1	29000.								0.	57	1	7
27	27	29	29	31	31	33	33	35	35	37	37	39	39	41	41
58	41	44	1	29000.								0.	57	2	7
28	28	30	30	32	32	34	34	36	36	38	38	40	40	42	42
59	42	45	1	29000.								0.	59	1	3
29	29	31	31	33	33	35	35	37	37	39	39	41	41	43	43
60	44	47	1	29000.								0.	59	2	3
30	30	32													
1															
2	252.														
3		144.													
4	126.		144.												
5	252.		144.												
6			288.												
7	126.		288.												
8	252.		288.												
9			432.												
10	126.		432.												
11	252.		432.												
12			576.												

13	126.	576.
14	252.	576.
15	0.	720.
16	126.	720.
17	252.	720.
18	0.	864.
19	126.	864.
20	252.	864.
21	0.	1008.
22	126.	1008.
23	252.	1008.
24	0.	1152.
25	126.	1152.
26	252.	1152.
27	0.	1296.
28	126.	1296.
29	252.	1296.
30	0.	1440.
31	126.	1440.
32	252.	1440.
33	0.	1584.
34	126.	1584.
35	252.	1584.
36	0.	1728.
37	126.	1728.
38	252.	1728.
39	0.	1872.
40	126.	1872.
41	252.	1872.
42	0.	2016.
43	126.	2016.
44	252.	2016.
45	0.	2160.
46	126.	2160.
47	252.	2160.

1	2	3	4	5	6
2	0	1	5	386.4	
	0.05	0.05	0.05	0.05	0.05
	0.	0.			

-1	-1	3	00.089	0.67	NENMARK SPECTRUM, 90 PERCENTILE ALLUVIUM				
2	0								
2	4	1	2	31	32	33	34		
2	4	3	4	33	34	35	36		
2	4	5	6	35	36	37	38		
2	4	7	8	37	38	39	40		
2	4	9	10	39	40	41	42		
2	4	11	12	41	42	43	44		
2	4	13	14	43	44	45	46		
2	4	15	16	45	46	47	48		
2	4	17	18	47	48	49	50		
2	4	19	20	49	50	51	52		
2	4	21	22	51	52	53	54		
2	4	23	24	53	54	55	56		
2	4	25	26	55	56	57	58		
2	4	27	28	57	58	59	60		
2	2	29	30	59	60				

15									
0.54	1	31.08		1	61.62		1	9	
2.16	1	122.7		1	153.24		1	18	
3.78	1	214.32		1	244.86		1	27	
5.4	1	305.94		1	336.48		1	36	
7.02	1	397.56		1	428.1		1	45	
0.									

d. Output Solution of Example 2.

STRUCTURE NO. 1: 15-ST. 1 BAY. NENMARK SPECTRA. EXAMPLE A

TOTAL NUMBER OF STORIES----- 15
TOTAL NUMBER OF MEMBERS----- 60
TOTAL NUMBER OF NODES----- 47
TOTAL NUMBER OF RESTRAINED D.O.F.----- 6
TOTAL NUMBER OF LOADING CONDITIONS----- 1
DISPLACEMENT CONSTRAINT CODE----- 2
EQ. 0: NO DISPLACEMENT CONSTRAINTS
EQ. 1: DISPLACEMENT CONSTRAINT IS THE SAME FOR ALL NODES
EQ. 2: DISPLACEMENT CONSTRAINT CAN BE VARIED PER NODE
EQ. 3: DRIFT CONSTRAINT ON EACH STORY

TYPE OF DESIGN----- 1
EQ.-1: STATIC DESIGN
EQ. 0: STATIC AND DYNAMIC DESIGN
EQ. 1: DYNAMIC DESIGN

MAXIMUM NUMBER OF ITERATIONS----- 8
TYPE OF SECTIONS----- 1
EQ. 0: BUILT-UP SECTIONS
GT. 0: WF SECTIONS
LT. 0: TRUSS MEMBERS

36	8	11	1	29000.000	0.0	0.0	0.0	35	51	6	6	8	8	10	10	12
										12	14	14	16	16	18	18
										20	20	22	22	24	24	26
										26	28	28	30	30	38	38
										40	40	42	42	44	44	46
										46	48	48	50	50	52	52
										54	54	56	56	58	58	60
										60	36					
37	9	12	1	29000.000	0.0	0.0	0.0	37	47	7	7	9	9	11	11	13
										13	15	15	17	17	19	19
										21	21	23	23	25	25	27
										27	29	29	30	30	41	41
										43	43	45	45	47	47	49
										49	51	51	53	53	55	55
										57	57	59	59	37		
										8	8	10	10	12	12	14
										14	16	16	18	18	20	20
										22	22	24	24	26	26	28
										28	30	30	40	40	42	42
										44	44	46	46	48	48	50
										50	52	52	54	54	56	56
										58	58	60	60	38		
39	12	15	1	29000.000	0.0	0.0	0.0	39	43	9	9	11	11	13	13	15
										15	17	17	19	19	21	21
										23	23	25	25	27	27	29
										29	41	41	43	43	45	45
										47	47	49	49	51	51	53
										53	55	55	57	57	59	59
										39						
										10	18	12	12	14	14	16
										16	18	18	20	20	22	22
										24	24	26	26	28	28	30
										30	42	42	44	44	46	46
										48	48	50	50	52	52	54
										54	56	56	58	58	60	60
										40						
										11	11	13	13	15	15	17
										17	19	19	21	21	23	23
										25	25	27	27	29	29	43
										43	45	45	47	47	49	49
										51	51	53	53	55	55	57
										57	59	59	41			
42	17	20	1	29000.000	0.0	0.0	0.0	41	39	12	12	14	14	16	16	18
										18	20	20	22	22	24	24
										26	26	28	28	30	30	44
										44	46	46	48	48	50	50
										52	52	54	54	56	56	58
										58	60	60	42			
										13	13	15	15	17	17	19
										19	21	21	23	23	25	25
										27	27	29	29	45	45	47
										47	49	49	51	51	53	53
										55	55	57	57	59	59	43
										14	14	16	16	18	18	20
										20	22	22	24	24	26	26
										28	28	30	30	46	46	48
										48	50	50	52	52	54	54
										56	56	58	58	60	60	44
45	21	24	1	29000.000	0.0	0.0	0.0	45	31	15	15	17	17	19	19	21
										21	23	23	25	25	27	27
										29	29	47	47	49	49	51
										51	53	53	55	55	57	57
										59	59	45				
										16	18	18	18	20	20	22
										22	24	24	26	26	28	28
										30	30	48	48	50	50	52
										52	54	54	56	56	58	58
										60	60	46				
										17	17	19	19	21	21	23
										23	25	25	27	27	29	29
										49	49	51	51	53	53	55
										55	57	57	59	59	47	
48	26	29	1	29000.000	0.0	0.0	0.0	47	27	18	18	20	20	22	22	24
										24	26	26	28	28	30	30
										50	50	52	52	54	54	56
										56	58	58	60	60	48	
										19	19	21	21	23	23	25
										25	27	27	29	29	51	51
										53	53	55	55	57	57	59
										59	49					
50	29	32	1	29000.000	0.0	0.0	0.0	49	23	20	20	22	22	24	24	26
										26	28	28	30	30	52	52
										54	54	56	56	58	58	60
										60	50					
										21	21	23	23	25	25	27
										27	29	29	53	53	55	55
										57	57	59	59	51		
										22	22	24	24	26	26	28
										28	30	30	54	54	56	56
										58	58	60	60	52		
53	33	36	1	29000.000	0.0	0.0	0.0	53	15	23	23	25	25	27	27	29
										29	55	55	57	57	59	59
										53						
										24	24	26	26	28	28	30
										30	56	56	58	58	60	60
										54						
										25	25	27	27	29	29	57
										57	59	59	55			
56	38	41	1	29000.000	0.0	0.0	0.0	55	11	26	26	28	28	30	30	58
										58	60	60	56			
										27	27	29	29	59	59	57
58	41	44	1	29000.000	0.0	0.0	0.0	57	7	28	28	30	30	60	60	58
										29	29	59				
59	42	45	1	29000.000	0.0	0.0	0.0	59	3	30	30	60				
60	44	47	1	29000.000	0.0	0.0	0.0	59	3							

MBAND WIDTH = 11

RESTRAINED DEGREES OF FREEDOM:

1 2 3 4 5 6

TOTAL ELEMENTS	NONZERO ELEMENTS	STORED ELEMENTS
10011	822	1218

STARTING ROW NUMBERS FOR EACH COLUMN

1	1	1	4	4	4	1	1	1	7
7	7	4	4	4	7	7	7	16	16
16	13	13	13	16	16	16	25	25	25
22	22	22	25	25	25	34	34	34	31
31	31	34	34	34	43	43	43	40	40
40	43	43	43	52	52	52	49	49	49
52	52	52	61	61	61	58	58	58	61
61	61	70	70	70	67	67	70	70	70
70	79	79	79	76	76	76	79	79	79
88	88	88	85	85	85	88	88	88	97
97	97	94	94	94	97	97	97	106	106
106	103	103	103	106	106	106	115	115	115
112	112	112	115	115	115	124	124	124	121
121	121	124	124	124	133	133	133	130	130
130									

NUMBERS OF DIAGONAL ELEMENTS IN SINGLE ARRAY STIFFNESS MATRIX

1	3	6	7	9	12	19	27	36	40
45	51	61	72	84	94	105	117	121	126
132	142	153	165	175	186	198	202	207	213
223	234	246	256	267	279	283	288	294	304
315	327	337	348	360	364	369	375	385	396
408	418	429	441	445	450	456	466	477	489
499	510	522	524	531	537	547	558	570	580
591	603	607	612	618	628	639	651	661	672
684	688	693	699	709	720	732	742	753	765
769	774	780	790	801	813	823	834	844	850
855	861	871	882	894	904	915	927	931	936
942	952	963	975	985	996	1008	1012	1017	1023
1033	1044	1056	1066	1077	1089	1093	1098	1104	1114
1125	1137	1147	1158	1170	1174	1179	1185	1195	1206
1218									

TOTAL ELEMENTS MEAN THE TOTAL NUMBER OF ELEMENTS IN THE STIFFNESS MATRIX, NONZERO ELEMENTS ARE THE TOTAL NUMBER OF NON-ZERO ELEMENTS IN THE STIFFNESS MATRIX, AND STORED ELEMENTS ARE THE ELEMENTS IN THE STIFFNESS MATRIX NEEDED TO BE STORED BY USING THIS NUMBERING SCHEME. WHEN THE NUMBERING SCHEME IS AN EFFICIENT ONE, THE NUMBER OF NONZERO ELEMENTS IS CLOSE TO THE NUMBER OF STORED ELEMENTS.

LOADING INFORMATION :

DYNAMIC PROPERTIES:

FIRST MODE NUMBER----- 1
 LAST MODE NUMBER----- 5
 GRAVITY ACCELERATION----- 386.40
 TYPE OF MODE SUPERPOSITION----- 0
 EQ. 1: DIRECT SUPERPOSITION OF MODES
 EQ. 0: ROOT-MEAN-SQUARE SUPERPOSITION OF MODES
 EQ. 1: ABSOLUTE SUPERPOSITION OF MODES

DAMPING RATIO FOR EACH MODE:

0.050 0.050 0.050 0.050 0.050

ADDITIONAL NODAL MASSES:

0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

DYNAMIC LOADING INFORMATION:

DATA FOR LOADING CONDITION 1 ... NEHRM SPECTRUM, 90 PERCENTILE ALLUVIUM

MOTION TYPE CODE----- -1
LT, 0: HORIZONTAL + VERTICAL MOTIONS
GT, 0: HORIZONTAL = 1 | VERTICAL = 2
NUMBER OF ACCELERATION DATA----- 0
LOAD MULTIPLIER----- 0.089
SCALE FACTOR FOR VERTICAL MOTION----- 0.670
TYPE OF INPUT ACCELERATION DATA----- 3
EQ. 1: TIME-HISTORY DATA ; EQ. 2: SPECTRUM DATA ; EQ. 3: CURVE FITTING SPECTRUM DATA

USE NEHRM SPECTRUM TO FIND MAXIMUM ACCELERATION DATA

THE CURVE IS CORRESPONDING TO DESIGN SPECTRA : 2
EQ. 1: 50 PERCENTILE, 5% DAMPING, ALLUVIUM ;
EQ. 2: 90 PERCENTILE, 5% DAMPING, ALLUVIUM ;
EQ. 3: DUCTILITY 3, 5% DAMPING ;
EQ. 4: DUCTILITY 4, 10% DAMPING .

DISPLACEMENT CONSTRAINTS:

NODE	DIRECTION	VALUE
3	1	0.540
6	1	1.080
9	1	1.620
12	1	2.160
15	1	2.700
18	1	3.240
21	1	3.780
24	1	4.320
27	1	4.860
30	1	5.400
33	1	5.940
36	1	6.480
39	1	7.020
42	1	7.560
45	1	8.100

FREQUENCY CONSTRAINTS:

MODE	MINIMUM
1	0.0
2	0.0
3	0.0
4	0.0
5	0.0

RELATIVE SIZES OF MEMBERS:

1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000
1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000
1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000
1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000
1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000
1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000	1.00000000

NATURAL FREQUENCIES:

0.370261E+01 0.425228E+02 0.158026E+03 0.343234E+03 0.556162E+03

MODE EIGENVECTORS:

MODE	EIGENVECTORS:
1	0.315075E-01 0.618247E-02 0.277675E-03 0.315004E-01 0.114644E-06 0.652414E-04 0.315075E-01 0.618220E-02 0.277674E-03 0.897400E-01 0.118581E-01 0.367038E-03 0.897422E-01 0.188656E-06 0.423552E-04 0.897397E-01 0.118575E-01 0.367036E-03 0.157598E+00 0.169283E-01 0.412189E-03 0.157602E+00 0.492688E-06 0.456969E-05 0.157597E+00 0.169275E-01 0.412191E-03 0.230635E+00 0.213823E-01 0.442113E-03 0.230641E+00 0.632005E-06 0.334938E-04 0.230635E+00 0.213812E-01 0.442114E-03 0.306949E+00 0.252317E-01 0.462802E-03 0.306957E+00 0.745926E-06 0.689771E-04 0.306949E+00 0.252303E-01 0.462803E-03 0.385192E+00 0.284983E-01 0.475567E-03 0.385203E+00 0.105286E-05 0.101484E-03 0.385192E+00 0.284968E-01 0.475573E-03 0.464119E+00 0.312113E-01 0.480915E-03 0.464131E+00 0.910639E-06 0.131110E-03 0.464119E+00 0.312095E-01 0.480914E-03 0.542584E+00 0.334063E-01 0.479282E-03 0.542599E+00 0.727786E-06 0.158060E-03 0.542584E+00 0.334044E-01 0.479274E-03 0.619485E+00 0.351255E-01 0.471042E-03 0.619501E+00 0.637810E-06 0.182649E-03 0.619485E+00 0.351239E-01 0.471029E-03 0.693808E+00 0.364175E-01 0.456991E-03 0.693825E+00 0.127054E-05 0.205071E-03 0.693808E+00 0.364154E-01 0.456995E-03 0.764678E+00 0.373362E-01 0.437761E-03 0.764697E+00 0.130682E-05 0.225605E-03 0.764678E+00 0.373339E-01 0.437764E-03 0.831313E+00 0.379401E-01 0.413930E-03 0.831334E+00 0.154264E-05 0.244710E-03 0.831313E+00 0.379377E-01 0.413936E-03 0.892993E+00 0.382928E-01 0.386135E-03 0.893017E+00 0.137648E-05 0.262806E-03 0.892993E+00 0.382902E-01 0.386137E-03 0.949195E+00 0.384619E-01 0.356291E-03 0.949219E+00 0.809054E-06 0.279747E-03 0.949195E+00 0.384592E-01 0.356274E-03 0.999975E+00 0.385167E-01 0.329908E-03 0.100000E+01 0.166019E-05 0.293586E-03 0.999973E+00 0.385140E-01 0.329915E-03 -0.156824E+00 0.778102E-03 0.125676E-02 0.156874E+00 0.109199E-06 0.638115E-03 0.156824E+00 0.778507E-03 0.125675E-02 -0.410572E+00 0.434618E-02 0.136427E-02 0.410701E+00 0.372599E-06 0.734429E-03 0.410571E+00 0.434699E-02 0.136427E-02 -0.649206E+00 0.110032E-01 0.114389E-02 0.649411E+00 0.416941E-06 0.703484E-03 0.649205E+00 0.110044E-01 0.114388E-02 -0.834516E+00 0.203770E-01 0.779318E-03 0.834779E+00 0.975509E-06 0.632766E-03 0.834514E+00 0.203785E-01 0.779324E-03 -0.947290E+00 0.318234E-01 0.334368E-03 0.947589E+00 0.100529E-05 0.546522E-03 0.947289E+00 0.318253E-01 0.334367E-03 -0.977364E+00 0.445545E-01 0.147065E-03 0.977673E+00 0.100132E-05 0.457356E-03 0.977363E+00 0.445509E-01 0.147072E-03 -0.922118E+00 0.577322E-01 0.623606E-03 0.922409E+00 0.126009E-05 0.375911E-03 0.922117E+00 0.577349E-01 0.623612E-03 -0.786180E+00 0.705242E-01 0.105600E-02 0.786429E+00 0.172005E-05 0.311989E-03 0.786179E+00 0.705273E-01 0.105599E-02 -0.580803E+00 0.821978E-01 0.140980E-02 0.580986E+00 0.174586E-05 0.274061E-03 0.580802E+00 0.822011E-01 0.140980E-02 -0.322762E+00 0.921724E-01 0.165789E-02 0.322865E+00 0.17541E-05 0.268778E-03 0.322762E+00 0.921758E-01 0.165789E-02 -0.328752E-01 0.100071E+00 0.178254E-02 0.328856E-01 0.169423E-05 0.300532E-03 0.328751E-01 0.100075E+00 0.178255E-02 0.265832E+00 0.105756E+00 0.177681E-02 0.265916E+00 0.288020E-05 0.371147E-03 0.265832E+00 0.105760E+00 0.177678E-02 0.550132E+00 0.109336E+00 0.184595E-02 0.550311E+00 0.206816E-05 0.479279E-03 0.550132E+00 0.109340E+00 0.184595E-02 0.799183E+00 0.111433E+00 0.181339E-02 0.799344E+00 0.214830E-05 0.617417E-03 0.799183E+00 0.111477E+00 0.181338E-02 0.999685E+00 0.111782E+00 0.115206E-02 0.100000E+01 0.209662E-05 0.755553E-03 0.999683E+00 0.111786E+00 0.115206E-02

```

3  0.286136E+00 0.555393E-02 0.221866E-02 0.286472E+00 0.413532E-06 0.104601E-02 0.286136E+00 0.555399E-02 0.221866E-02
0.608726E+00 0.634323E-02 0.197028E-02 0.699548E+00 0.281860E-06 0.912052E-03 0.698728E+00 0.634335E-02 0.197029E-02
0.969545E+00 0.292415E-02 0.852429E-03 0.970686E+00 0.191336E-06 0.392444E-03 0.969547E+00 0.292435E-02 0.852433E-03
0.998824E+00 0.231274E-02 0.547909E-03 0.100000E+01 0.703622E-06 0.247074E-03 0.998820E+00 0.231247E-02 0.547893E-03
0.775075E+00 0.638824E-02 0.180666E-02 0.775987E+00 0.174877E-06 0.829485E-03 0.775077E+00 0.638791E-02 0.180667E-02
0.357336E+00 0.661410E-02 0.258539E-02 0.357756E+00 0.193393E-06 0.121721E-02 0.357336E+00 0.661370E-02 0.258539E-02
-0.144813E+00 0.129004E-02 0.268265E-02 0.144983E+00 0.432049E-06 0.132958E-02 0.144813E+00 0.128958E-02 0.268266E-02
-0.808945E+00 0.988205E-02 0.207690E-02 0.808653E+00 0.113567E-05 0.115935E-02 0.808945E+00 0.988359E-02 0.207693E-02
-0.893767E+00 0.257602E-01 0.927590E-03 0.894820E+00 0.553434E-06 0.772799E-03 0.893771E+00 0.257618E-01 0.927592E-03
-0.949006E+00 0.440859E-01 0.469248E-03 0.950123E+00 0.106983E-05 0.291387E-03 0.949009E+00 0.440866E-01 0.469233E-03
-0.753279E+00 0.620988E-01 0.175917E-02 0.75417E+00 0.388844E-06 0.140160E-03 0.753272E+00 0.620994E-01 0.175918E-02
-0.358625E+00 0.772690E-01 0.262115E-02 0.359063E+00 0.489417E-06 0.391100E-03 0.358642E+00 0.772699E-01 0.262118E-02
0.136055E+00 0.879563E-01 0.285162E-02 0.136215E+00 0.125500E-06 0.378983E-03 0.136056E+00 0.879571E-01 0.285164E-02
0.608767E+00 0.938296E-01 0.243444E-02 0.609478E+00 0.320950E-06 0.996473E-04 0.608764E+00 0.938305E-01 0.243444E-02
0.959099E+00 0.958969E-01 0.165696E-02 0.960226E+00 0.625442E-06 0.314839E-03 0.959100E+00 0.958978E-01 0.165696E-02
-0.409139E+00 0.492851E-02 0.293165E-02 0.410166E+00 0.482838E-06 0.153364E-02 0.409139E+00 0.492785E-02 0.293165E-02
-0.882321E+00 0.162434E-01 0.161176E-02 0.884579E+00 0.863225E-06 0.100549E-02 0.882320E+00 0.162421E-01 0.161175E-02
-0.952650E+00 0.312666E-01 0.101400E-02 0.952088E+00 0.982748E-06 0.135401E-03 0.952649E+00 0.312646E-01 0.101479E-02
0.547001E+00 0.445597E-01 0.322508E-02 0.548401E+00 0.156266E-05 0.108767E-02 0.547001E+00 0.445571E-01 0.322509E-02
0.135319E+00 0.515556E-01 0.384896E-02 0.135666E+00 0.111998E-05 0.131755E-02 0.135319E+00 0.515524E-01 0.384894E-02
0.752541E+00 0.510127E-01 0.258039E-02 0.754467E+00 0.187779E-05 0.686015E-03 0.752540E+00 0.510089E-01 0.258038E-02
0.997447E+00 0.456533E-01 0.704599E-04 0.100000E+01 0.232407E-05 0.512140E-03 0.997446E+00 0.456509E-01 0.704630E-04
0.750671E+00 0.408115E-01 0.241853E-02 0.752592E+00 0.289070E-05 0.170593E-02 0.750671E+00 0.408066E-01 0.241852E-02
0.137810E+00 0.417719E-01 0.364886E-02 0.138162E+00 0.204075E-05 0.233627E-02 0.137810E+00 0.417648E-01 0.364888E-02
-0.533971E+00 0.812004E-01 0.301832E-02 0.536942E+00 0.336879E-05 0.213220E-02 0.533971E+00 0.811948E-01 0.301831E-02
-0.936668E+00 0.678762E-01 0.844633E-03 0.939063E+00 0.350160E-05 0.123992E-02 0.936668E+00 0.678700E-01 0.844628E-03
-0.869108E+00 0.873630E-01 0.179639E-02 0.871330E+00 0.270477E-05 0.144325E-03 0.869108E+00 0.873538E-01 0.179636E-02
-0.368826E+00 0.104258E+00 0.363149E-02 0.367774E+00 0.761468E-05 0.576095E-03 0.368833E+00 0.104251E+00 0.363160E-02
0.328945E+00 0.114844E+00 0.383397E-02 0.329991E+00 0.398412E-06 0.551419E-03 0.328953E+00 0.114841E+00 0.383386E-02
0.912379E+00 0.118851E+00 0.264794E-02 0.916713E+00 0.424972E-06 0.836280E-04 0.912377E+00 0.118844E+00 0.264785E-02
0.198579E-03 0.891428E-01 0.344478E-06 0.387527E-05 0.977525E-01 0.686177E-08 0.206297E-03 0.891436E-01 0.343941E-04
0.281321E-03 0.177372E+00 0.707884E-04 0.746412E-05 0.196656E+08 0.403248E-08 0.216550E-03 0.177374E+00 0.707688E-04
0.194802E-03 0.263782E+00 0.104663E-03 0.661779E-05 0.289462E+00 0.227742E-07 0.207224E-03 0.263785E+00 0.104697E-03
0.192512E-03 0.347487E+00 0.138164E-03 0.127588E-05 0.381318E+00 0.335397E-07 0.195042E-03 0.347491E+00 0.138226E-03
0.189044E-03 0.427629E+00 0.169998E-03 0.477399E-05 0.469258E+00 0.374231E-07 0.179537E-03 0.427633E+00 0.170043E-03
0.179906E-03 0.503385E+00 0.200117E-03 0.689718E-05 0.552387E+00 0.268506E-07 0.166160E-03 0.503391E+00 0.200095E-03
0.162814E-03 0.573979E+00 0.228168E-03 0.286042E-05 0.629851E+00 0.199813E-07 0.157111E-03 0.573986E+00 0.228088E-03
0.139842E-03 0.638687E+00 0.253855E-03 0.545901E-05 0.700855E+00 0.151778E-07 0.150498E-03 0.638695E+00 0.253733E-03
0.116089E-03 0.696845E+00 0.276897E-03 0.128175E-04 0.764671E+00 0.351873E-07 0.141609E-03 0.696853E+00 0.276845E-03
0.101447E-03 0.747837E+00 0.296939E-03 0.114524E-04 0.828637E+00 0.576550E-07 0.124268E-03 0.747840E+00 0.296983E-03
0.828308E-04 0.791200E+00 0.314770E-03 0.301334E-05 0.868241E+00 0.787176E-07 0.885504E-04 0.791208E+00 0.314857E-03
0.120876E-03 0.826429E+00 0.324757E-03 0.694331E-05 0.906629E+00 0.764050E-07 0.108837E-03 0.826440E+00 0.324806E-03
-0.170848E-03 0.853185E-03 0.357754E-03 0.937867E-05 0.937483E+00 0.575178E-07 0.189523E-03 0.853197E+00 0.357726E-03
0.125988E-02 0.871184E+00 0.244948E-03 0.513535E-05 0.948897E+00 0.389490E-07 0.124888E-02 0.871197E+00 0.244980E-03
-0.309627E-02 0.888294E+00 0.858669E-03 0.442903E-05 0.108000E+01 0.412177E-07 0.308747E-02 0.888306E+00 0.858538E-03

```

MODE PERIOD DYNAMIC LOAD FACTOR FOR EACH LOADING CONDITION:

1	3.26532	0.59997
2	0.96354	2.03324
3	0.49982	2.82000
4	0.33914	2.82000
5	0.26643	2.82000

MODE PERIOD VERTICAL DYNAMIC LOAD FACTOR FOR EACH LOADING CONDITION:

1	3.26532	0.50986
2	0.96354	1.72785
3	0.49982	3.04000
4	0.33914	3.04000
5	0.26643	3.04000

TOTAL WORKDONE VS TOTAL NUMBER OF MODES INCLUDED:

NUMBER	WORKDONE/TOTAL WORKDONE
1	0.753666
2	0.910669
3	0.930658
4	0.933939
5	1.000000

```

OLD SCALING FACTOR----- 0.14329E+04
NEW SCALING FACTOR----- 0.19413E+04
ACTIVE STRESS IN LOADING CONDITION 1----- 31
ACTIVE DISPLACEMENT IN LOADING CONDITION 1----- 61 70 79 88 97 106 115 124
MOST CRITICAL CONSTRAINT----- 6
NUMBER OF ANALYSIS----- 8

```

```

***** TOTAL WEIGHT OF STRUCTURE = 4.043053E+04 *****
***** BASE AE = 4.179889E+10; CYCLE NO. = 1 *****

```

RAYLEIGH'S QUOTIENT APPROXIMATION FOR EACH LOADING:
0.484428E+01

NO CONSTRAINT ENERGY DENSITY RATIO

1	1	0.349397E+00
2	1	0.349396E+00
3	1	0.468393E+00
4	1	0.468390E+00
5	1	0.444522E+00
6	1	0.444525E+00
7	1	0.390798E+00
8	1	0.390799E+00
9	2	0.391845E+00
10	2	0.377076E+00

11	2	0.370687E+00
12	2	0.351135E+00
13	3	0.342573E+00
14	3	0.322046E+00
15	4	0.320396E+00
16	4	0.308688E+00
17	5	0.327106E+00
18	6	0.307712E+00
19	6	0.328976E+00
20	7	0.310648E+00
21	7	0.318634E+00
22	8	0.300654E+00
23	8	0.281690E+00
24	9	0.264182E+00
25	9	0.212027E+00
26	9	0.196476E+00
27	9	0.164785E-01
28	9	0.465033E-01
29	1	0.126792E-01
30	1	0.126827E-01
31	6	0.100000E+01
32	1	0.044511E+00
33	7	0.066258E+00
34	1	0.054315E+00
35	8	0.763098E+00
36	1	0.649272E+00
37	8	0.666122E+00
38	1	0.456248E+00
39	9	0.573720E+00
40	1	0.374474E+00
41	9	0.485138E+00
42	1	0.303552E+00
43	9	0.401539E+00
44	1	0.241777E+00
45	9	0.325643E+00
46	1	0.180581E+00
47	9	0.259695E+00
48	1	0.142564E+00
49	9	0.202718E+00
50	9	0.112271E+00
51	9	0.154018E+00
52	9	0.498352E-01
53	9	0.111227E+00
54	8	0.677202E-01
55	8	0.726025E-01
56	8	0.462842E-01
57	9	0.403748E-01
58	1	0.227701E-01
59	9	0.106282E-01
60	1	0.101667E-01

RELATIVE SIZES OF MEMBERS:

0.224269271	0.224269271	0.463381350	0.463381350	0.455493000	0.455493000	0.413136264	0.413136264	0.382830560	0.382830560
0.350668159	0.350668159	0.32263226	0.32263226	0.297408700	0.297408700	0.271259606	0.271259606	0.232683778	0.232683778
0.219056129	0.219056129	0.223232388	0.223232388	0.209799409	0.209799409	0.145036101	0.145036101	0.0655871034	0.0655871034
1.000000000	1.000000000	0.685976207	0.685976207	0.575019419	0.575019419	0.478206846	0.478206846	0.395420969	0.395420969
0.326416850	0.326416850	0.269402623	0.269402623	0.221590579	0.221590579	0.187514961	0.187514961	0.157270491	0.157270491
0.131040156	0.131040156	0.113263428	0.113263428	0.110837042	0.110837042	0.101707220	0.101707220	0.100000024	0.100000024

NATURAL FREQUENCIES:

0.371933E+01	0.366449E+02	0.109773E+03	0.222709E+03	0.400462E+03
--------------	--------------	--------------	--------------	--------------

MODES:

1	EIGENVECTORS:	0.187546E-01	0.335324E-02	0.208280E-03	0.187558E-01	0.641655E-07	0.642254E-04	0.187546E-01	0.335331E-02	0.208279E-03
		0.575623E-01	0.722307E-02	0.252827E-03	0.575635E-01	0.649576E-07	0.404230E-04	0.575621E-01	0.722323E-02	0.252828E-03
		0.103333E+00	0.110154E-01	0.290235E-03	0.103336E+00	0.683244E-07	0.139795E-04	0.103333E+00	0.110156E-01	0.290236E-03
		0.156204E+00	0.146881E-01	0.333854E-03	0.156208E+00	0.214479E-06	0.793698E-05	0.156204E+00	0.146880E-01	0.333851E-03
		0.126597E+00	0.182076E-01	0.374153E-03	0.126603E+00	0.253264E-06	0.296888E-04	0.126596E+00	0.182081E-01	0.374154E-03
		0.204269E+00	0.215215E-01	0.412084E-03	0.204277E+00	0.216063E-06	0.501863E-04	0.204269E+00	0.215220E-01	0.412086E-03
		0.398741E+00	0.245759E-01	0.444078E-03	0.398752E+00	0.349844E-06	0.705472E-04	0.398741E+00	0.245766E-01	0.444075E-03
		0.439146E+00	0.273227E-01	0.483357E-03	0.439160E+00	0.287782E-06	0.906094E-04	0.439146E+00	0.273234E-01	0.483360E-03
		0.524164E+00	0.294773E-01	0.489482E-03	0.524182E+00	0.350113E-06	0.108582E-03	0.524164E+00	0.294781E-01	0.489484E-03
		0.413253E+00	0.316158E-01	0.509010E-03	0.413275E+00	0.592343E-06	0.121902E-03	0.413253E+00	0.316167E-01	0.509006E-03
		0.703758E+00	0.331352E-01	0.497994E-03	0.703784E+00	0.748399E-06	0.145504E-03	0.703758E+00	0.331362E-01	0.497984E-03
		0.790030E+00	0.342121E-01	0.460262E-03	0.790059E+00	0.119023E-06	0.177187E-03	0.790030E+00	0.342131E-01	0.460283E-03
		0.867446E+00	0.348391E-01	0.425933E-03	0.867481E+00	0.404599E-06	0.201827E-03	0.867446E+00	0.348402E-01	0.425936E-03
		0.937609E+00	0.351347E-01	0.407780E-03	0.937653E+00	0.200822E-07	0.214437E-03	0.937613E+00	0.351358E-01	0.407792E-03
		0.999933E+00	0.352162E-01	0.385386E-03	0.100000E+01	0.820685E-06	0.226674E-03	0.999937E+00	0.352173E-01	0.385387E-03
		0.824038E-01	0.188118E-02	0.888779E-03	0.824311E-01	0.718088E-08	0.422406E-03	0.824039E-01	0.188120E-02	0.888778E-03
		0.239735E+00	0.329593E-02	0.924344E-03	0.239791E+00	0.646279E-07	0.423132E-03	0.239735E+00	0.329599E-02	0.924342E-03
		0.399164E+00	0.284963E-02	0.875499E-03	0.399258E+00	0.181916E-06	0.444016E-03	0.399164E+00	0.284972E-02	0.875495E-03
		0.550849E+00	0.321962E-03	0.791828E-03	0.550983E+00	0.148405E-06	0.392294E-03	0.550848E+00	0.322000E-03	0.791838E-03
		0.684922E+00	0.433818E-02	0.626850E-03	0.684957E+00	0.393429E-06	0.365293E-03	0.684941E+00	0.433822E-02	0.626855E-03
		0.784007E+00	0.110726E-01	0.380355E-03	0.784217E+00	0.213701E-08	0.323045E-03	0.784010E+00	0.110724E-01	0.380345E-03
		0.613477E+00	0.196330E-01	0.553181E-04	0.613498E+00	0.287441E-06	0.241612E-03	0.613479E+00	0.196327E-01	0.553291E-04
		0.816376E+00	0.295897E-01	0.338913E-03	0.816411E+00	0.201065E-06	0.183809E-03	0.816377E+00	0.295893E-01	0.338902E-03
		0.718223E+00	0.401440E-01	0.772367E-03	0.718440E+00	0.315959E-06	0.918840E-04	0.718225E+00	0.401435E-01	0.772391E-03
		0.529663E+00	0.505290E-01	0.122918E-02	0.529835E+00	0.725296E-06	0.129187E-04	0.529662E+00	0.505285E-01	0.122919E-02
		0.255589E+00	0.599583E-01	0.156080E-02	0.255674E+00	0.232182E-06	0.644852E-04	0.255589E+00	0.599579E-01	0.156079E-02
		-0.717640E-01	0.675187E-01	0.146641E-02	0.717877E-01	0.210639E-06	0.280995E-04	-0.717639E-01	0.675182E-01	0.146641E-02
		-0.403562E+00	0.723931E-01	0.165124E-02	0.403700E+00	0.335108E-06	0.364823E-04	-0.403562E+00	0.723926E-01	0.165124E-02
		-0.722959E+00	0.748855E-01	0.168504E-02	0.723255E+00	0.421279E-06	0.444303E-04	-0.722958E+00	0.748850E-01	0.168504E-02
		-0.999392E+00	0.756079E-01	0.152694E-02	0.100000E+01	0.612769E-06	0.137882E-03	-0.999393E+00	0.756074E-01	0.152695E-02

Reproduced from
best available copy.



3 0.130452E+00 0.246703E-02 0.130789E-02 0.130509E+00 0.125127E-06 0.666367E-03 0.130452E+00 0.246702E-02 0.130789E-02
0.367331E+00 0.416160E-02 0.131422E-02 0.367601E+00 0.113806E-06 0.608474E-03 0.367332E+00 0.416158E-02 0.131422E-02
0.572920E+00 0.316227E-02 0.982047E-03 0.573353E+00 0.120361E-06 0.454066E-03 0.572929E+00 0.316225E-02 0.982050E-03
0.706761E+00 0.210429E-03 0.470501E-03 0.707310E+00 0.345590E-07 0.238162E-03 0.706762E+00 0.210447E-03 0.470502E-03
0.71801E+00 0.503781E-02 0.218190E-03 0.723391E+00 0.201395E-06 0.491811E-04 0.71802E+00 0.503785E-02 0.218188E-03
0.61834E+00 0.391435E-02 0.978052E-03 0.618666E+00 0.63753E-07 0.371055E-03 0.618347E+00 0.391431E-02 0.978055E-03
0.36027E+00 0.130892E-01 0.163773E-02 0.36083E+00 0.104869E-06 0.644397E-03 0.36052E+00 0.130893E-01 0.163774E-02
-0.108342E-01 0.127924E-01 0.198647E-02 0.108436E-01 0.324139E-06 0.842831E-03 0.108342E-01 0.127925E-01 0.198646E-02
-0.415974E+00 0.780235E-02 0.184499E-02 0.415970E+00 0.744805E-07 0.832693E-03 0.415575E+00 0.780250E-02 0.184499E-02
-0.743211E+00 0.203961E-02 0.114613E-02 0.743974E+00 0.572011E-07 0.598662E-03 0.743211E+00 0.203960E-02 0.114613E-02
-0.859651E+00 0.154941E-01 0.167196E-03 0.860560E+00 0.136448E-06 0.101337E-03 0.859651E+00 0.154940E-01 0.167192E-03
-0.673481E+00 0.278410E-01 0.152660E-02 0.674187E+00 0.381340E-07 0.409076E-03 0.673482E+00 0.278410E-01 0.152669E-02
-0.225400E+00 0.412048E-01 0.250043E-02 0.225724E+00 0.850116E-07 0.761779E-03 0.225481E+00 0.412047E-01 0.250043E-02
0.990809E+00 0.499467E-01 0.316640E-02 0.100000E+01 0.953351E-06 0.997440E-03 0.990809E+00 0.499466E-01 0.316640E-02
0.200844E+00 0.121804E-02 0.200903E-02 0.201290E+00 0.351017E-07 0.106616E-02 0.200844E+00 0.121796E-02 0.200903E-02
0.537341E+00 0.469114E-02 0.167663E-02 0.538141E+00 0.296171E-07 0.896971E-03 0.537341E+00 0.469096E-02 0.167662E-02
0.754289E+00 0.122293E-01 0.676031E-03 0.754281E+00 0.138761E-06 0.485244E-03 0.754289E+00 0.122290E-01 0.676031E-03
0.762293E+00 0.223000E-01 0.649495E-03 0.763493E+00 0.425608E-06 0.592242E-04 0.762293E+00 0.223004E-01 0.649497E-03
0.518935E+00 0.322312E-01 0.196865E-02 0.519783E+00 0.456322E-06 0.602475E-03 0.518936E+00 0.322307E-01 0.196862E-02
0.597987E-01 0.390070E-01 0.277464E-02 0.599007E-01 0.449302E-05 0.926244E-03 0.597987E-01 0.390063E-01 0.277477E-02
-0.473903E+00 0.404544E-01 0.255950E-02 0.474744E+00 0.149102E-06 0.801111E-03 0.473903E+00 0.404536E-01 0.255948E-02
-0.853827E+00 0.365973E-01 0.113967E-02 0.85405E+00 0.513362E-06 0.135322E-03 0.853827E+00 0.365965E-01 0.113967E-02
-0.849249E+00 0.300811E-01 0.105436E-02 0.85080E+00 0.539689E-06 0.890196E-03 0.849249E+00 0.300802E-01 0.105436E-02
-0.368841E+00 0.280830E-01 0.306441E-02 0.369610E+00 0.184284E-06 0.185550E-02 0.368841E+00 0.280821E-01 0.306442E-02
0.392880E+00 0.299808E-01 0.322659E-02 0.393523E+00 0.402762E-06 0.197201E-02 0.392880E+00 0.299799E-01 0.322658E-02
0.944740E+00 0.401492E-01 0.101440E-02 0.944749E+00 0.394894E-06 0.992253E-03 0.944740E+00 0.401483E-01 0.101440E-02
0.855175E+00 0.543729E-01 0.206665E-02 0.857050E+00 0.728360E-06 0.367479E-03 0.855175E+00 0.543720E-01 0.206665E-02
0.760657E+00 0.632746E-01 0.49575E-02 0.762658E-02 0.298365E-06 0.171631E-02 0.760660E-01 0.632736E-01 0.495752E-02
-0.996127E+00 0.690171E-01 0.578337E-02 0.100000E+01 0.421220E-06 0.211000E-02 0.996125E+00 0.690160E-01 0.578336E-02
0.265389E+00 0.465505E-03 0.268797E-02 0.266407E+00 0.122227E-06 0.136505E-02 0.265388E+00 0.464988E-03 0.268798E-02
0.665281E+00 0.333039E-02 0.170090E-02 0.667073E+00 0.242004E-06 0.897533E-03 0.665284E+00 0.332925E-02 0.170091E-02
0.801763E+00 0.106072E-01 0.256355E-03 0.803940E+00 0.857895E-06 0.173260E-05 0.801765E+00 0.106054E-01 0.256357E-03
0.555121E+00 0.179384E-01 0.235390E-02 0.556701E+00 0.144136E-05 0.969077E-03 0.555121E+00 0.179358E-01 0.235390E-02
-0.153663E-01 0.208253E-01 0.345047E-02 0.154116E-01 0.694463E-06 0.148607E-02 0.153662E-01 0.208220E-01 0.345048E-02
-0.634074E+00 0.141938E-01 0.241288E-02 0.634029E+00 0.292863E-05 0.112152E-02 0.634075E+00 0.141959E-01 0.241290E-02
-0.898138E+00 0.515315E-02 0.125107E-03 0.901025E+00 0.249989E-03 0.124995E-03 0.898142E+00 0.514799E-02 0.125114E-03
-0.531379E+00 0.666216E-02 0.316787E-02 0.531355E+00 0.348232E-05 0.151770E-02 0.531379E+00 0.666428E-02 0.316786E-02
0.302161E+00 0.119830E-01 0.403177E-02 0.303217E+00 0.294959E-05 0.189064E-02 0.302160E+00 0.119900E-01 0.403181E-02
0.958107E+00 0.641860E-02 0.138752E-02 0.961719E+00 0.433899E-05 0.599764E-03 0.958110E+00 0.64263E-02 0.138751E-02
0.737475E+00 0.135891E-02 0.312139E-02 0.740379E+00 0.408633E-05 0.159784E-02 0.737477E+00 0.135988E-02 0.312137E-02
-0.277713E+00 0.115354E-02 0.436268E-02 0.278781E+00 0.122504E-04 0.222055E-02 0.277713E+00 0.116345E-02 0.436289E-02
-0.996050E+00 0.108142E-01 0.844977E-03 0.100000E+01 0.678936E-05 0.558499E-03 0.996052E+00 0.108248E-01 0.845027E-03
-0.527725E+00 0.229360E-01 0.497659E-02 0.530234E+00 0.258291E-06 0.225221E-02 0.527724E+00 0.229472E-01 0.497678E-02
0.853510E+00 0.277232E-01 0.779642E-02 0.859522E+00 0.206478E-04 0.370490E-02 0.853509E+00 0.277348E-01 0.779610E-02

MODE	PERIOD	DYNAMIC LOAD FACTOR FOR EACH LOADING CONDITION:
1	3.25776	0.60137
2	1.06748	1.83526
3	0.59970	2.82000
4	0.42103	2.82000
5	0.31398	2.82000

MODE	PERIOD	VERTICAL DYNAMIC LOAD FACTOR FOR EACH LOADING CONDITION:
1	3.25776	0.51104
2	1.06748	1.55960
3	0.59970	2.77614
4	0.42103	3.04000
5	0.31398	3.04000

TOTAL WORKDONE VS TOTAL NUMBER OF MODES INCLUDED:	
NUMBER	WORKDONE/TOTAL WORKDONE
1	0.750466
2	0.945017
3	0.983232
4	0.996634
5	1.000000

OLD SCALING FACTOR----- 0.38530E+04
NEW SCALING FACTOR----- 0.38861E+04
ACTIVE STRESS IN LOADING CONDITION 1----- 53
ACTIVE DISPLACEMENT IN LOADING CONDITION 1----- 115 124 133
MOST CRITICAL CONSTRAINT----- 4
NUMBER OF ANALYSIS----- 1

***** TOTAL HEIGHT OF STRUCTURE = 3.490082E+04 *****
***** BASE AE = 1.126965E+111 CYCLE NO. = 7 *****

MEMBER	LENGTH	AE	I	AREA	MA	MB	AXIAL FORCE	SHEAR	MOMENT	AXIAL FORCE	SHEAR	MOMENT
1	126.00	0.22427	871.5298	13.7150	3	4	2.77136E+02	9.00384E+03	1.13499E+06	2.77136E+02	9.00384E+03	1.07819E+03
							-1.41637E+04	0.0	1.42041E+04	-6.73267E+00	9.0	-3.36800E+01
2	126.00	0.22427	871.5298	13.7150	4	5	2.76911E+02	9.00385E+03	1.07836E+03	2.76911E+02	9.00385E+03	1.13499E+06
							6.71407E+00	0.0	3.36685E+01	1.41637E+04	0.0	-1.42041E+04
3	126.00	0.46338	1800.7400	19.7143	6	7	7.56026E+02	1.94960E+04	2.45691E+06	7.56026E+02	1.94960E+04	9.13397E+02
							-1.63878E+04	0.0	1.64464E+04	-3.22425E+01	0.0	-4.44558E+01

4	126.00	0.46336	1800.7400	19.7143	7	8	7.56002E+02 3.22378E+01	1.94960E+04 0.0	9.13946E+02 4.44582E+01	7.56002E+02 1.63878E+04	1.94960E+04 0.0	2.45692E+06 -1.64645E+04
5	126.00	0.45549	1770.0930	19.5458	9	10	1.10178E+03 -1.62123E+04	1.90369E+04 0.0	2.39892E+06 1.63251E+04	1.10178E+03 -5.21002E+01	1.90369E+04 0.0	6.17706E+02 -6.85504E+01
6	126.00	0.45549	1770.0930	19.5458	10	11	1.10120E+03 5.21599E+01	1.90369E+04 0.0	6.16307E+02 6.05191E+01	1.10120E+03 1.62124E+04	1.90369E+04 0.0	2.39892E+06 -1.63251E+04
7	126.00	0.41314	1605.4032	18.6148	12	13	1.25548E+03 -1.62642E+04	1.76136E+04 0.0	2.21950E+06 1.63991E+04	1.25548E+03 -6.31472E+01	1.76136E+04 0.0	5.85587E+02 -7.17450E+01
8	126.00	0.41314	1605.4032	18.6148	13	14	1.25797E+03 6.32746E+01	1.76135E+04 0.0	5.84992E+02 7.16837E+01	1.25797E+03 1.62641E+04	1.76135E+04 0.0	2.21950E+06 -1.63992E+04
9	126.00	0.38283	1467.7126	17.9191	15	16	1.28591E+03 -1.62546E+04	1.65115E+04 0.0	2.08061E+06 1.63981E+04	1.28591E+03 -6.57543E+01	1.65115E+04 0.0	7.65593E+02 -7.77694E+01
10	126.00	0.38283	1467.7126	17.9191	16	17	1.27885E+03 6.53507E+01	1.65114E+04 0.0	7.66803E+02 7.73860E+01	1.27885E+03 1.62550E+04	1.65114E+04 0.0	2.08061E+06 -1.63977E+04
11	126.00	0.35069	1362.8044	17.1503	18	19	1.31289E+03 -1.63038E+04	1.53748E+04 0.0	1.93734E+06 1.64569E+04	1.31289E+03 -8.99063E+01	1.53748E+04 0.0	7.85953E+02 -8.31969E+01
12	126.00	0.35069	1362.8044	17.1503	19	20	1.29937E+03 6.90803E+01	1.53748E+04 0.0	7.90467E+02 8.24472E+01	1.29937E+03 1.63046E+04	1.53748E+04 0.0	1.93734E+06 -1.64562E+04
13	126.00	0.32260	1253.6641	16.4493	21	22	1.34724E+03 -1.63441E+04	1.43510E+04 0.0	1.88834E+06 1.65079E+04	1.34724E+03 -7.52557E+01	1.43510E+04 0.0	7.31737E+02 -8.85491E+01
14	126.00	0.32260	1253.6641	16.4493	22	23	1.34050E+03 7.48554E+01	1.43510E+04 0.0	7.30751E+02 8.81309E+01	1.34050E+03 1.63445E+04	1.43510E+04 0.0	1.88834E+06 -1.65075E+04
15	126.00	0.29741	1155.7559	15.7939	24	25	1.32794E+03 -1.64708E+04	1.34802E+04 0.0	1.69862E+06 1.66390E+04	1.32794E+03 -7.45649E+01	1.34802E+04 0.0	9.76254E+02 -9.35942E+01
16	126.00	0.29741	1155.7559	15.7939	25	26	1.32307E+03 7.42593E+01	1.34802E+04 0.0	9.75935E+02 9.32824E+01	1.32307E+03 1.64712E+04	1.34802E+04 0.0	1.69862E+06 -1.66387E+04

MEMBER	LENGTH	AE	I	AREA	MA	MB	AXIAL FORCE	SHEAR	MOMENT	AXIAL FORCE	SHEAR	MOMENT
17	126.00	0.27126	1054.1382	15.0836	27	28	1.31518E+03 -1.68542E+04	1.27297E+04 0.0	1.60406E+06 1.70286E+04	1.31518E+03 -7.44856E+01	1.27297E+04 0.0	1.20294E+03 -9.98957E+01
18	126.00	0.27126	1054.1382	15.0836	28	29	1.30506E+03 7.38206E+01	1.27297E+04 0.0	1.20260E+06 9.92233E+01	1.30506E+03 1.68550E+04	1.27297E+04 0.0	1.60407E+06 -1.70287E+04
19	126.00	0.23268	904.2292	13.9700	30	31	1.35240E+03 -1.77620E+04	1.17194E+04 0.0	1.47671E+06 1.79556E+04	1.35240E+03 -8.18865E+01	1.17194E+04 0.0	1.23382E+03 -1.11729E+02
20	126.00	0.23268	904.2292	13.9700	31	32	1.35109E+03 8.17786E+01	1.17195E+04 0.0	1.23495E+06 1.11649E+02	1.35109E+03 1.77621E+04	1.17195E+04 0.0	1.47671E+06 -1.79556E+04
21	126.00	0.21906	851.2710	13.5547	33	34	1.30819E+03 -1.74869E+04	1.09342E+04 0.0	1.37774E+06 1.76787E+04	1.30819E+03 -7.86647E+01	1.09342E+04 0.0	1.35223E+03 -1.13179E+02
22	126.00	0.21906	851.2710	13.5547	34	35	1.30077E+03 7.84904E+01	1.09342E+04 0.0	1.35354E+06 1.13238E+02	1.30077E+03 1.74846E+04	1.09342E+04 0.0	1.37772E+06 -1.76785E+04
23	126.00	0.22323	867.5002	13.6833	36	37	1.26442E+03 -1.58062E+04	1.00559E+04 0.0	1.26696E+06 1.59910E+04	1.26442E+03 -7.75004E+01	1.00559E+04 0.0	1.18784E+03 -1.07312E+02
24	126.00	0.22323	867.5002	13.6833	37	38	1.26394E+03 7.75026E+01	1.00559E+04 0.0	1.18488E+06 1.07240E+02	1.26394E+03 1.58066E+04	1.00559E+04 0.0	1.26698E+06 -1.59913E+04
25	126.00	0.20980	815.2986	13.2652	39	40	1.32611E+03 -1.44026E+04	8.67771E+03 0.0	1.09329E+06 1.46025E+04	1.32611E+03 -9.07964E+01	8.67771E+03 0.0	6.91486E+02 -1.09142E+02
26	126.00	0.20980	815.2986	13.2652	40	41	1.24911E+03 8.50435E+01	8.67765E+03 0.0	6.87604E+02 1.03286E+02	1.24911E+03 1.44084E+04	8.67765E+03 0.0	1.09329E+06 -1.45967E+04
27	126.00	0.14504	563.6228	11.0294	42	43	1.39247E+03 -1.42904E+04	6.16245E+03 0.0	7.76812E+05 1.45429E+04	1.39247E+03 -9.85608E+01	6.16245E+03 0.0	1.49207E+03 -1.53942E+02
28	126.00	0.14504	563.6228	11.0294	43	44	1.34501E+03 9.43232E+01	6.16233E+03 0.0	1.48854E+06 1.49574E+02	1.34501E+03 1.42947E+04	6.16233E+03 0.0	7.76812E+05 -1.45386E+04
29	126.00	0.06559	254.8772	7.4169	45	46	2.10705E+03 -1.23486E+04	2.55248E+03 0.0	3.22237E+05 1.29167E+04	2.10705E+03 -2.06198E+02	2.55248E+03 0.0	1.98604E+03 -3.61977E+02
30	126.00	0.06559	254.8772	7.4169	46	47	2.08747E+03 2.03574E+02	2.55258E+03 0.0	1.98646E+03 3.59324E+02	2.08747E+03 1.23515E+04	2.55258E+03 0.0	3.22244E+05 -1.29144E+04
31	144.00	1.00000	3886.0867	28.9609	1	3	1.57084E+05 -5.07977E+03	1.72637E+04 0.0	2.89199E+06 1.59278E+04	1.57084E+05 -4.20461E+03	1.72637E+04 0.0	3.35735E+05 -6.64340E+03
32	144.00	1.00000	3886.0867	28.9609	2	5	1.57087E+05 -5.07968E+03	1.72639E+04 0.0	2.89200E+06 1.59279E+04	1.57087E+05 -4.20479E+03	1.72639E+04 0.0	3.35718E+05 -6.64345E+03

MEMBER	LENGTH	AE	I	AREA	NA	NB	AXIAL FORCE	SHEAR	MOMENT	AXIAL FORCE	SHEAR	MOMENT
33	144.00	0.68590	2665.7629	23.9665	3	6	1.49787E+05 -0.25945E+02	1.67977E+04 0.0	1.45390E+06 1.33152E+04	1.49787E+05 -6.41039E+02	1.67977E+04 0.0	1.15225E+06 -1.18482E+04
34	144.00	0.68590	2665.7629	23.9665	5	8	1.49790E+05 -8.25734E+02	1.67974E+04 0.0	1.45388E+06 1.33153E+04	1.49790E+05 -6.41281E+02	1.67974E+04 0.0	1.15223E+06 -1.18483E+04
35	144.00	0.57502	2234.5752	21.9611	6	9	1.34233E+05 -1.32551E+03	1.60237E+04 0.0	1.32784E+06 1.35501E+04	1.34233E+05 4.68910E+02	1.60237E+04 0.0	1.17491E+06 -1.26935E+04
36	144.00	0.57502	2234.5752	21.9611	8	11	1.34236E+05 -1.32548E+03	1.60239E+04 0.0	1.32785E+06 1.35504E+04	1.34236E+05 4.68832E+02	1.60239E+04 0.0	1.17492E+06 -1.26937E+04
37	144.00	0.47820	1858.3455	20.0271	9	12	1.19319E+05 -2.24648E+03	1.50034E+04 0.0	1.25959E+06 1.41622E+04	1.19319E+05 1.19456E+03	1.50034E+04 0.0	1.09809E+06 -1.31103E+04
38	144.00	0.47820	1858.3455	20.0271	11	14	1.19323E+05 -2.24621E+03	1.50032E+04 0.0	1.25957E+06 1.41623E+04	1.19323E+05 1.19434E+03	1.50032E+04 0.0	1.09808E+06 -1.31104E+04
39	144.00	0.39542	1536.6401	18.2113	12	15	1.05622E+05 -3.08327E+03	1.39568E+04 0.0	1.16347E+06 1.46829E+04	1.05622E+05 2.15329E+03	1.39568E+04 0.0	1.04166E+06 -1.37529E+04
40	144.00	0.39542	1536.6401	18.2113	14	17	1.05625E+05 -3.08302E+03	1.39566E+04 0.0	1.16346E+06 1.46830E+04	1.05625E+05 2.15293E+03	1.39566E+04 0.0	1.04164E+06 -1.37529E+04
41	144.00	0.32642	1268.4841	16.5462	15	18	9.27637E+04 -4.11838E+03	1.29717E+04 0.0	1.08061E+06 1.53231E+04	9.27637E+04 3.18299E+03	1.29717E+04 0.0	9.77470E+05 -1.43957E+04
42	144.00	0.32642	1268.4841	16.5462	17	20	9.27657E+04 -4.11057E+03	1.29719E+04 0.0	1.08064E+06 1.53235E+04	9.27657E+04 3.18310E+03	1.29719E+04 0.0	9.77496E+05 -1.43960E+04
43	144.00	0.26940	1046.9219	15.0319	18	21	8.06660E+04 -5.26634E+03	1.20612E+04 0.0	1.00068E+06 1.59990E+04	8.06660E+04 4.39909E+03	1.20612E+04 0.0	9.19860E+05 -1.51317E+04
44	144.00	0.26940	1046.9219	15.0319	20	23	8.06679E+04 -5.26573E+03	1.20608E+04 0.0	1.00064E+06 1.59986E+04	8.06679E+04 4.39867E+03	1.20608E+04 0.0	9.19033E+05 -1.51316E+04
45	144.00	0.22159	861.1201	13.6329	21	24	6.91502E+04 -6.67291E+03	1.12375E+04 0.0	9.29828E+05 1.68175E+04	6.91502E+04 5.80432E+03	1.12375E+04 0.0	8.61065E+05 -1.59497E+04
46	144.00	0.22159	861.1201	13.6329	23	26	6.91517E+04 -6.67312E+03	1.12375E+04 0.0	9.29854E+05 1.68180E+04	6.91517E+04 5.80443E+03	1.12375E+04 0.0	8.61082E+05 -1.59493E+04
47	144.00	0.18751	728.6992	12.5409	24	27	5.80341E+04 -8.22452E+03	1.06006E+04 0.0	8.75649E+05 1.74796E+04	5.80341E+04 7.26494E+03	1.06006E+04 0.0	8.10270E+05 -1.65201E+04
48	144.00	0.18751	728.6992	12.5409	26	29	5.80354E+04 -8.22456E+03	1.06005E+04 0.0	8.75660E+05 1.74798E+04	5.80354E+04 7.26474E+03	1.06005E+04 0.0	8.10264E+05 -1.65201E+04

MEMBER	LENGTH	AE	I	AREA	NA	NB	AXIAL FORCE	SHEAR	MOMENT	AXIAL FORCE	SHEAR	MOMENT
49	144.00	0.15727	611.1667	11.4851	27	30	4.71880E+04 -1.01910E+04	1.00210E+04 0.0	8.30048E+05 1.64082E+04	4.71880E+04 8.94710E+03	1.00210E+04 0.0	7.57844E+05 -1.71643E+04
50	144.00	0.15727	611.1667	11.4851	29	32	4.71890E+04 -1.01911E+04	1.00212E+04 0.0	8.30061E+05 1.64085E+04	4.71890E+04 8.94731E+03	1.00212E+04 0.0	7.57861E+05 -1.71647E+04
51	144.00	0.13104	509.2334	10.4837	30	33	3.68267E+04 -1.18782E+04	9.46760E+03 0.0	7.55033E+05 1.89037E+04	3.68267E+04 1.14325E+04	9.46760E+03 0.0	7.33167E+05 -1.84580E+04
52	144.00	0.13104	509.2334	10.4837	32	35	3.68274E+04 -1.18786E+04	9.46788E+03 0.0	7.55055E+05 1.89042E+04	3.68274E+04 1.14332E+04	9.46788E+03 0.0	7.33205E+05 -1.84588E+04
53	144.00	0.11326	440.1514	9.7467	33	36	2.67993E+04 -1.30490E+04	8.66754E+03 0.0	6.76590E+05 1.85482E+04	2.67993E+04 1.35301E+04	8.66754E+03 0.0	6.97192E+05 -1.90292E+04
54	144.00	0.11326	440.1514	9.7467	35	38	2.67998E+04 -1.30490E+04	8.66731E+03 0.0	6.76591E+05 1.85482E+04	2.67998E+04 1.35293E+04	8.66731E+03 0.0	6.97159E+05 -1.90285E+04
55	144.00	0.11084	430.7222	9.6417	36	39	1.72379E+04 -1.24853E+04	7.97316E+03 0.0	5.98995E+05 1.60610E+04	1.72379E+04 1.28782E+04	7.97316E+03 0.0	6.15482E+05 -1.64539E+04
56	144.00	0.11084	430.7222	9.6417	38	41	1.72385E+04 -1.24866E+04	7.97407E+03 0.0	5.99051E+05 1.60624E+04	1.72385E+04 1.28797E+04	7.97407E+03 0.0	6.15551E+05 -1.64555E+04
57	144.00	0.10171	395.2429	9.2361	39	42	8.71030E+03 -1.21364E+04	6.63042E+03 0.0	5.06335E+05 1.40226E+04	8.71030E+03 1.16721E+04	6.63042E+03 0.0	4.88362E+05 -1.35583E+04
58	144.00	0.10171	395.2429	9.2361	41	44	8.71055E+03 -1.21351E+04	6.62987E+03 0.0	5.06284E+05 1.40213E+04	8.71055E+03 1.16704E+04	6.62987E+03 0.0	4.88295E+05 -1.35566E+04

59	144.00	0.10000	386.6086	9.1582	42	45	2.55603E+03	4.23690E+03	3.03550E+05	2.55603E+03	4.23690E+03	3.22657E+05
							-7.68797E+03	0.0	8.24634E+03	8.18948E+03	0.0	-8.74785E+03
60	144.00	0.10000	386.6086	9.1582	44	47	2.55695E+03	4.23667E+03	3.03525E+05	2.55695E+03	4.23667E+03	3.22636E+05
							-7.68732E+03	0.0	8.24572E+03	8.18892E+03	0.0	-8.74732E+03

GIRDER-COLUMN SECTION PROPERTIES:

MEMBER	TYPE	SECTION MODULUS	TF/D	DEPTH	TM	TF
1	2	80.020	0.0	0.0	0.0	0.0
2	2	80.020	0.0	0.0	0.0	0.0
3	2	149.573	0.0	0.0	0.0	0.0
4	2	149.573	0.0	0.0	0.0	0.0
5	2	147.456	0.0	0.0	0.0	0.0
6	2	147.456	0.0	0.0	0.0	0.0
7	2	138.902	0.0	0.0	0.0	0.0
8	2	138.902	0.0	0.0	0.0	0.0
9	2	127.439	0.0	0.0	0.0	0.0
10	2	127.439	0.0	0.0	0.0	0.0
11	2	118.272	0.0	0.0	0.0	0.0
12	2	118.272	0.0	0.0	0.0	0.0
13	2	110.090	0.0	0.0	0.0	0.0
14	2	110.090	0.0	0.0	0.0	0.0
15	2	102.605	0.0	0.0	0.0	0.0
16	2	102.605	0.0	0.0	0.0	0.0
17	2	94.683	0.0	0.0	0.0	0.0
18	2	94.683	0.0	0.0	0.0	0.0
19	2	82.668	0.0	0.0	0.0	0.0
20	2	82.668	0.0	0.0	0.0	0.0
21	2	78.357	0.0	0.0	0.0	0.0
22	2	78.357	0.0	0.0	0.0	0.0
23	2	79.690	0.0	0.0	0.0	0.0
24	2	79.690	0.0	0.0	0.0	0.0
25	2	75.386	0.0	0.0	0.0	0.0
26	2	75.386	0.0	0.0	0.0	0.0
27	2	53.883	0.0	0.0	0.0	0.0
28	2	53.883	0.0	0.0	0.0	0.0
29	2	25.506	0.0	0.0	0.0	0.0
30	2	25.506	0.0	0.0	0.0	0.0
31	1	275.329	0.0	0.0	0.0	0.0
32	1	275.329	0.0	0.0	0.0	0.0
33	1	205.626	0.0	0.0	0.0	0.0
34	1	205.626	0.0	0.0	0.0	0.0
35	1	178.525	0.0	0.0	0.0	0.0
36	1	178.525	0.0	0.0	0.0	0.0
37	1	153.526	0.0	0.0	0.0	0.0
38	1	153.526	0.0	0.0	0.0	0.0
39	1	130.976	0.0	0.0	0.0	0.0
40	1	130.976	0.0	0.0	0.0	0.0
41	1	111.211	0.0	0.0	0.0	0.0
42	1	111.211	0.0	0.0	0.0	0.0
43	1	94.114	0.0	0.0	0.0	0.0

44	1	94.114	0.0	0.0	0.0	0.0
45	1	79.167	0.0	0.0	0.0	0.0
46	1	79.167	0.0	0.0	0.0	0.0
47	1	68.133	0.0	0.0	0.0	0.0
48	1	68.133	0.0	0.0	0.0	0.0
49	1	58.047	0.0	0.0	0.0	0.0
50	1	58.047	0.0	0.0	0.0	0.0
51	1	49.057	0.0	0.0	0.0	0.0
52	1	49.057	0.0	0.0	0.0	0.0
53	1	42.826	0.0	0.0	0.0	0.0
54	1	42.826	0.0	0.0	0.0	0.0
55	1	41.967	0.0	0.0	0.0	0.0
56	1	41.967	0.0	0.0	0.0	0.0
57	1	38.712	0.0	0.0	0.0	0.0
58	1	38.712	0.0	0.0	0.0	0.0
59	1	38.100	0.0	0.0	0.0	0.0
60	1	38.100	0.0	0.0	0.0	0.0

JOINT	-X	-Y	DISPL-X	DISPL-Y	ROTATION
1	0.0	0.0	0.0	0.0	0.0
2	252.000	0.0	0.0	0.0	0.0
3	0.0	144.000	1.88266575E-01	2.69330218E-02	2.06606323E-03
4	126.000	144.000	1.88308358E-01	5.15056115E-07	7.77550275E-04
5	252.000	144.000	1.88266575E-01	2.69335806E-02	2.06605811E-03
6	0.0	288.000	5.65244853E-01	5.79326898E-02	2.37279106E-03
7	126.000	288.000	5.65322816E-01	5.27161717E-07	6.56869030E-04
8	252.000	288.000	5.65243781E-01	5.79339229E-02	2.37279339E-03
9	0.0	432.000	9.85852599E-01	8.81890059E-02	2.58336333E-03
10	126.000	432.000	9.85970088E-01	5.93775667E-07	5.36622712E-04
11	252.000	432.000	9.85852599E-01	8.81908536E-02	2.58337241E-03
12	0.0	576.000	1.44517612E+00	1.17503703E-01	2.85106082E-03
13	126.000	576.000	1.44532108E+00	1.72712225E-06	4.97363042E-04
14	252.000	576.000	1.44517517E+00	1.17506325E-01	2.85104732E-03
15	0.0	720.000	1.94414234E+00	1.45778418E-01	3.10354796E-03
16	126.000	720.000	1.94430447E+00	1.31737397E-06	5.19002322E-04
17	252.000	720.000	1.94413948E+00	1.45781875E-01	3.10356379E-03
18	0.0	864.000	2.48023033E+00	1.72757447E-01	3.36169801E-03
19	126.000	864.000	2.48040581E+00	1.82163421E-06	5.90927200E-04
20	252.000	864.000	2.48023129E+00	1.72761559E-01	3.36171384E-03
21	0.0	1008.000	3.05155182E+00	1.98146820E-01	3.60625726E-03
22	126.000	1008.000	3.05173683E+00	2.81493885E-06	6.93573384E-04
23	252.000	1008.000	3.05155087E+00	1.98151708E-01	3.60624885E-03
24	0.0	1152.000	3.65544319E+00	2.21639693E-01	3.84128466E-03
25	126.000	1152.000	3.65563679E+00	2.16535227E-06	8.14049505E-04

JOINT	-X	-Y	DISPL-X	DISPL-Y	ROTATION
26	257.000	1152.000	3.65344415E+00	2.21645296E-01	3.84130282E-03
27	0.0	1296.000	4.28842258E+00	2.42519081E-01	4.08346378E-03
28	126.000	1296.000	4.28842000E+00	2.83147347E-06	9.29287169E-04
29	252.000	1296.000	4.28842545E+00	2.42525260E-01	4.08348069E-03
30	0.0	1440.000	4.95495319E+00	2.60467350E-01	4.37562168E-03
31	126.000	1440.000	4.95516777E+00	4.82573159E-06	1.01691484E-03
32	252.000	1440.000	4.95495415E+00	2.60473967E-01	4.37559932E-03
33	0.0	1584.000	5.64523829E+00	2.73236189E-01	4.44186479E-03
34	126.000	1584.000	5.64545631E+00	5.99605755E-06	1.18938112E-03
35	252.000	1584.000	5.64523029E+00	2.73243223E-01	4.44179401E-03
36	0.0	1728.000	6.32421303E+00	2.86266029E-01	4.25393507E-03
37	126.000	1728.000	6.32445335E+00	1.13983479E-06	1.43214758E-03
38	252.000	1728.000	6.32421494E+00	2.86273360E-01	4.25407663E-03
39	0.0	1872.000	6.9568343E+00	2.93031037E-01	4.06607613E-03
40	126.000	1872.000	6.95717812E+00	3.27853286E-06	1.6360827E-03
41	252.000	1872.000	6.95692825E+00	2.93038666E-01	4.06609476E-03
42	0.0	2016.000	7.55313683E+00	2.96369612E-01	4.84322147E-03
43	126.000	2016.000	7.55353165E+00	6.09043980E-07	1.76585838E-03
44	252.000	2016.000	7.55316925E+00	2.96377480E-01	4.84330343E-03
45	0.0	2160.000	8.1000610E+00	2.97316372E-01	3.83905857E-03
46	126.000	2160.000	8.10067940E+00	6.47753739E-06	1.87419890E-03
47	252.000	2160.000	8.10003185E+00	2.97324479E-01	3.83906765E-03

e. Input Data of Example 3.

```

1 0
2 0 0 0 15 10 5 1 10 0 0 0.01 0.01 0.0001 0.8 0.3 0.01 1.01
90 47 6 2 3 -1 5 1 15-ST, 1-BAY K-BRACED FRAME, EXAMPLE 3
29000000. 0.283 700. 20300. 10.
29000000. 0.283 4. 6. 0.5
1 1 4 0100000000. 0. 1
2 2 4 0100000000. 0. 1
3 3 7 0100000000. 0. 3
4 5 7 0100000000. 0. 3
5 6 10 0100000000. 0. 5
6 8 10 0100000000. 0. 5
7 9 13 0100000000. 0. 7
8 11 13 0100000000. 0. 7
9 12 16 0100000000. 0. 9
10 14 16 0100000000. 0. 9
11 15 19 0100000000. 0. 11
12 17 19 0100000000. 0. 11
13 18 22 0100000000. 0. 13
14 20 22 0100000000. 0. 13
15 21 25 0100000000. 0. 15
16 23 25 0100000000. 0. 15
17 24 28 0100000000. 0. 17
18 26 28 0100000000. 0. 17
19 27 31 0100000000. 0. 19
20 29 31 0100000000. 0. 19
21 30 36 0100000000. 0. 21
22 32 36 0100000000. 0. 21
23 33 37 0100000000. 0. 23
24 35 37 0100000000. 0. 23
25 36 40 0100000000. 0. 25
26 38 40 0100000000. 0. 25
27 39 43 0100000000. 0. 27
28 41 43 0100000000. 0. 27
29 42 46 0100000000. 0. 29
30 44 46 0100000000. 0. 29

```



31	3	4	2100000000.	0.	31
32	4	5	2100000000.	0.	31
33	5	6	2100000000.	0.	33
34	6	7	2100000000.	0.	33
35	7	8	2100000000.	0.	35
36	8	9	2100000000.	0.	35
37	9	10	2100000000.	0.	37
38	10	11	2100000000.	0.	37
39	11	12	2100000000.	0.	39
40	12	13	2100000000.	0.	39
41	13	14	2100000000.	0.	41
42	14	15	2100000000.	0.	41
43	15	16	2100000000.	0.	43
44	16	17	2100000000.	0.	43
45	17	18	2100000000.	0.	45
46	18	19	2100000000.	0.	45
47	19	20	2100000000.	0.	47
48	20	21	2100000000.	0.	47
49	21	22	2100000000.	0.	49
50	22	23	2100000000.	0.	49
51	23	24	2100000000.	0.	51
52	24	25	2100000000.	0.	51
53	25	26	2100000000.	0.	53
54	26	27	2100000000.	0.	53
55	27	28	2100000000.	0.	55
56	28	29	2100000000.	0.	55
57	29	30	2100000000.	0.	57
58	30	31	2100000000.	0.	57
59	31	32	2100000000.	0.	59
60	32	33	2100000000.	0.	59
61	33	34	2100000000.	0.	61
62	34	35	2100000000.	0.	61
63	35	36	2100000000.	0.	63
64	36	37	2100000000.	0.	63
65	37	38	2100000000.	0.	65
66	38	39	2100000000.	0.	65
67	39	40	2100000000.	0.	67
68	40	41	2100000000.	0.	67
69	41	42	2100000000.	0.	69
70	42	43	2100000000.	0.	69
71	43	44	2100000000.	0.	71
72	44	45	2100000000.	0.	71
73	45	46	2100000000.	0.	73
74	46	47	2100000000.	0.	73
75	47	48	2100000000.	0.	75
76	48	49	2100000000.	0.	75
77	49	50	2100000000.	0.	77
78	50	51	2100000000.	0.	77
79	51	52	2100000000.	0.	79
80	52	53	2100000000.	0.	79
81	53	54	2100000000.	0.	81
82	54	55	2100000000.	0.	81
83	55	56	2100000000.	0.	83
84	56	57	2100000000.	0.	83
85	57	58	2100000000.	0.	85
86	58	59	2100000000.	0.	85
87	59	60	2100000000.	0.	87
88	60	61	2100000000.	0.	87
89	61	62	2100000000.	0.	89
90	62	63	2100000000.	0.	89
1	0.	0.			
2	336.	0.			
3	0.	144.			
4	168.	144.			
5	336.	144.			
6	0.	288.			
7	168.	288.			
8	336.	288.			
9	0.	432.			
10	168.	432.			
11	336.	432.			
12	0.	576.			
13	168.	576.			
14	336.	576.			
15	0.	720.			
16	168.	720.			
17	336.	720.			
18	0.	864.			
19	168.	864.			
20	336.	864.			
21	0.	1008.			
22	168.	1008.			
23	336.	1008.			
24	0.	1152.			
25	168.	1152.			
26	336.	1152.			
27	0.	1296.			
28	168.	1296.			
29	336.	1296.			
30	0.	1440.			
31	168.	1440.			
32	336.	1440.			
33	0.	1584.			
34	168.	1584.			
35	336.	1584.			
36	0.	1728.			
37	168.	1728.			
38	336.	1728.			
39	0.	1872.			
40	168.	1872.			
41	336.	1872.			
42	0.	2016.			
43	168.	2016.			
44	336.	2016.			
45	0.	2160.			
46	168.	2160.			
47	336.	2160.			

```

1 2 3 4 5 6
0
1 3 144. 50000.
2 6 288. 50000.
3 9 432. 50000.
4 12 576. 50000.
5 15 720. 50000.
6 18 864. 50000.
7 21 1008. 50000.
8 24 1152. 50000.
9 27 1296. 50000.
10 30 1440. 50000.
11 33 1584. 50000.
12 36 1728. 50000.
13 39 1872. 50000.
14 42 2016. 50000.
15 45 2160. 50000.
7 7 2 1 0 3 1 1 0 1 1 1
6. 5. 336.
2 8 31 32 1 2 3 4 61 62 63 64
2 8 33 34 3 4 5 6 63 64 65 66
2 8 35 36 5 6 7 8 65 66 67 68
2 8 37 38 7 8 9 10 67 68 69 70
2 8 39 40 9 10 11 12 69 70 71 72
2 8 41 42 11 12 13 14 71 72 73 74
2 8 43 44 13 14 15 16 73 74 75 76
2 8 45 46 15 16 17 18 75 76 77 78
2 8 47 48 17 18 19 20 77 78 79 80
2 8 49 50 19 20 21 22 79 80 81 82
2 8 51 52 21 22 23 24 81 82 83 84
2 8 53 54 23 24 25 26 83 84 85 86
2 8 55 56 25 26 27 28 85 86 87 88
2 8 57 58 27 28 29 30 87 88 89 90
2 4 59 60 29 30 89 90
0. 0. 0. 1.
0. 0. 0. -1.
0 0 1 1 386.4
0 0 0 32.5 45. 32.5 32.5 65. 32.5
32.5 45. 32.5 32.5 45. 32.5 45. 32.5 45.
32.5 32.5 45. 32.5 32.5 45. 32.5 32.5
65. 32.5 32.5 45. 32.5 45. 32.5 45.
32.5 65. 32.5 32.5 45. 32.5 32.5 65.
32.5 32.5 65. 32.5 32.5 45. 32.5
15
0.432 1 30.864 1 61.296 1 9
1.728 1 122.16 1 152.592 1 18
3.024 1 213.456 1 243.888 1 27
4.32 1 304.752 1 335.182 1 36
5.616 1 396.048 1 426.48 1 45

```

f. Output Solution of Example 3.

```

STRUCTURE NO. 1: 15-ST, 1-BAY K-BRACED FRAME, EXAMPLE 3
TOTAL NUMBER OF STORIES----- 15
TOTAL NUMBER OF MEMBERS----- 90
TOTAL NUMBER OF NODES----- 47
TOTAL NUMBER OF RESTRAINED D.O.F.----- 6
TOTAL NUMBER OF LOADING CONDITIONS----- 2
DISPLACEMENT CONSTRAINT CODE----- 3
EQ. 0: NO DISPLACEMENT CONSTRAINTS
EQ. 1: DISPLACEMENT CONSTRAINT IS THE SAME FOR ALL NODES
EQ. 2: DISPLACEMENT CONSTRAINT CAN BE VARIED PER NODE
EQ. 3: DRIFT CONSTRAINT ON EACH STORY
TYPE OF DESIGN----- -1
EQ. 1: STATIC DESIGN
EQ. 0: STATIC AND DYNAMIC DESIGN
EQ. 1: DYNAMIC DESIGN
MAXIMUM NUMBER OF ITERATIONS----- 5
TYPE OF SECTIONS----- 1
EQ. 0: BUILT-UP SECTIONS
GT. 0: WF SECTIONS
LT. 0: TRUSS MEMBERS
GIRDER-COLUMN PROPERTIES:
MODULUS OF ELASTICITY----- 29000000.0
SPECIFIC WEIGHT----- 0.283
INITIAL MOMENT OF INERTIA----- 700.000
MAXIMUM MOMENT OF INERTIA----- 20300.000
MINIMUM MOMENT OF INERTIA----- 10.000
RATIO OF MINIMUM I TO MAXIMUM I----- 0.0

```


BRACING PROPERTIES:
 MODULUS OF ELASTICITY----- 29000000.0
 SPECIFIC WEIGHT----- 0.283
 MAXIMUM BRACING AREA----- 6.000
 MINIMUM BRACING AREA----- 0.500

GRADIENT PARAMETERS:
 GRADIENT ITERATION----- 10
 UPPER RANGE FOR DI----- 0.80000
 LOWER RANGE FOR DI----- 0.30000
 REDUCTION FACTOR----- 0.01000
 INCREMENTAL FACTOR----- 1.01000

MEMBER IDENTIFICATION DATA:

NO	J-END	K-END	TYPE	STRESS LIMIT	DEAD LOAD	LIVE LOAD	SNOW LOAD	DVM	NO	GIROER IDENTIFICATION NUMBERS:
1	1	4	0	100000000.	0.0	0.0	0.0	1	0	0
2	2	4	0	100000000.	0.0	0.0	0.0	1	0	0
3	3	7	0	100000000.	0.0	0.0	0.0	3	0	0
4	5	7	0	100000000.	0.0	0.0	0.0	3	0	0
5	6	10	0	100000000.	0.0	0.0	0.0	5	0	0
6	8	10	0	100000000.	0.0	0.0	0.0	5	0	0
7	9	13	0	100000000.	0.0	0.0	0.0	7	0	0
8	11	13	0	100000000.	0.0	0.0	0.0	7	0	0
9	12	16	0	100000000.	0.0	0.0	0.0	9	0	0
10	14	16	0	100000000.	0.0	0.0	0.0	9	0	0
11	15	19	0	100000000.	0.0	0.0	0.0	11	0	0
12	17	19	0	100000000.	0.0	0.0	0.0	11	0	0
13	18	22	0	100000000.	0.0	0.0	0.0	13	0	0
14	20	22	0	100000000.	0.0	0.0	0.0	13	0	0
15	21	25	0	100000000.	0.0	0.0	0.0	15	0	0
16	23	25	0	100000000.	0.0	0.0	0.0	15	0	0
17	24	28	0	100000000.	0.0	0.0	0.0	17	0	0
18	26	28	0	100000000.	0.0	0.0	0.0	17	0	0
19	27	31	0	100000000.	0.0	0.0	0.0	19	0	0
20	29	31	0	100000000.	0.0	0.0	0.0	19	0	0
21	30	34	0	100000000.	0.0	0.0	0.0	21	0	0
22	32	34	0	100000000.	0.0	0.0	0.0	21	0	0
23	33	37	0	100000000.	0.0	0.0	0.0	23	0	0
24	35	37	0	100000000.	0.0	0.0	0.0	23	0	0
25	36	40	0	100000000.	0.0	0.0	0.0	25	0	0
26	38	40	0	100000000.	0.0	0.0	0.0	25	0	0
27	39	43	0	100000000.	0.0	0.0	0.0	27	0	0
28	41	43	0	100000000.	0.0	0.0	0.0	27	0	0
29	42	46	0	100000000.	0.0	0.0	0.0	29	0	0
30	44	46	0	100000000.	0.0	0.0	0.0	29	0	0
31	3	4	2	100000000.	0.0	0.0	0.0	31	0	0
32	4	5	2	100000000.	0.0	0.0	0.0	31	0	0
33	6	7	2	100000000.	0.0	0.0	0.0	33	0	0
34	7	8	2	100000000.	0.0	0.0	0.0	33	0	0
35	9	10	2	100000000.	0.0	0.0	0.0	35	0	0
36	10	11	2	100000000.	0.0	0.0	0.0	35	0	0
37	12	13	2	100000000.	0.0	0.0	0.0	37	0	0
38	13	14	2	100000000.	0.0	0.0	0.0	37	0	0
39	15	16	2	100000000.	0.0	0.0	0.0	39	0	0
40	16	17	2	100000000.	0.0	0.0	0.0	39	0	0
41	18	19	2	100000000.	0.0	0.0	0.0	41	0	0
42	19	20	2	100000000.	0.0	0.0	0.0	41	0	0
43	21	22	2	100000000.	0.0	0.0	0.0	43	0	0
44	22	23	2	100000000.	0.0	0.0	0.0	43	0	0
45	24	25	2	100000000.	0.0	0.0	0.0	45	0	0
46	25	26	2	100000000.	0.0	0.0	0.0	45	0	0
47	27	28	2	100000000.	0.0	0.0	0.0	47	0	0
48	28	29	2	100000000.	0.0	0.0	0.0	47	0	0
49	30	31	2	100000000.	0.0	0.0	0.0	49	0	0
50	31	32	2	100000000.	0.0	0.0	0.0	49	0	0
51	33	34	2	100000000.	0.0	0.0	0.0	51	0	0
52	34	35	2	100000000.	0.0	0.0	0.0	51	0	0
53	36	37	2	100000000.	0.0	0.0	0.0	53	0	0
54	37	38	2	100000000.	0.0	0.0	0.0	53	0	0
55	39	40	2	100000000.	0.0	0.0	0.0	55	0	0
56	40	41	2	100000000.	0.0	0.0	0.0	55	0	0
57	42	43	2	100000000.	0.0	0.0	0.0	57	0	0
58	43	44	2	100000000.	0.0	0.0	0.0	57	0	0
59	45	46	2	100000000.	0.0	0.0	0.0	59	0	0
60	46	47	2	100000000.	0.0	0.0	0.0	59	0	0
61	1	3	1	100000000.	0.0	0.0	0.0	61	0	0
62	2	5	1	100000000.	0.0	0.0	0.0	61	0	0
63	3	4	1	100000000.	0.0	0.0	0.0	63	0	0
64	5	6	1	100000000.	0.0	0.0	0.0	63	0	0
65	6	9	1	100000000.	0.0	0.0	0.0	65	0	0
66	8	11	1	100000000.	0.0	0.0	0.0	65	0	0
67	9	12	1	100000000.	0.0	0.0	0.0	67	0	0
68	11	14	1	100000000.	0.0	0.0	0.0	67	0	0
69	12	15	1	100000000.	0.0	0.0	0.0	69	0	0
70	14	17	1	100000000.	0.0	0.0	0.0	69	0	0
71	15	18	1	100000000.	0.0	0.0	0.0	71	0	0
72	17	20	1	100000000.	0.0	0.0	0.0	71	0	0
73	18	21	1	100000000.	0.0	0.0	0.0	73	0	0
74	20	23	1	100000000.	0.0	0.0	0.0	73	0	0
75	21	24	1	100000000.	0.0	0.0	0.0	75	0	0
76	23	26	1	100000000.	0.0	0.0	0.0	75	0	0
77	24	27	1	100000000.	0.0	0.0	0.0	77	0	0
78	26	29	1	100000000.	0.0	0.0	0.0	77	0	0
79	27	30	1	100000000.	0.0	0.0	0.0	79	0	0
80	29	32	1	100000000.	0.0	0.0	0.0	79	0	0
81	30	33	1	100000000.	0.0	0.0	0.0	81	0	0
82	32	35	1	100000000.	0.0	0.0	0.0	81	0	0
83	33	36	1	100000000.	0.0	0.0	0.0	83	0	0

84	35	38	1	100000000.	0.0	0.0	0.0	83	0	0
85	36	39	1	100000000.	0.0	0.0	0.0	85	0	0
86	38	41	1	100000000.	0.0	0.0	0.0	85	0	0
87	39	42	1	100000000.	0.0	0.0	0.0	87	0	0
88	41	44	1	100000000.	0.0	0.0	0.0	87	0	0
89	42	45	1	100000000.	0.0	0.0	0.0	89	0	0
90	44	47	1	100000000.	0.0	0.0	0.0	89	0	0

MBAND WIDTH = 14

RESTRAINED DEGREES OF FREEDOM:

1	2	3	4	5	6
---	---	---	---	---	---

TOTAL ELEMENTS	NONZERO ELEMENTS	STORED ELEMENTS
10011	1092	1614

STARTING ROW NUMBERS FOR EACH COLUMN

1	1	1	4	4	4	1	1	1	1
1	1	4	4	4	7	7	7	7	7
7	13	13	13	16	16	16	16	16	16
22	22	22	25	25	25	25	25	25	31
31	31	34	34	34	34	34	34	40	40
40	43	43	43	43	43	43	49	49	49
52	52	52	52	52	52	58	58	58	61
61	61	61	61	61	67	67	67	70	70
70	70	70	70	76	76	76	79	79	79
79	79	79	85	85	85	88	88	88	88
88	88	94	94	94	97	97	97	97	97
97	103	103	103	106	106	106	106	106	106
112	112	112	115	115	115	115	115	115	121
121	121	124	124	124	124	124	124	130	130
130									

NUMBERS OF DIAGONAL ELEMENTS IN SINGLE ARRAY STIFFNESS MATRIX

1	3	4	7	9	12	19	27	36	46
57	69	79	90	102	112	123	135	148	162
177	187	198	210	220	231	243	256	270	285
295	306	318	328	339	351	364	378	393	403
414	426	436	447	459	472	486	501	511	522
534	544	555	567	580	594	609	619	630	642
652	663	675	688	702	717	727	738	750	760
771	783	796	810	825	835	846	858	868	879
891	904	918	933	943	954	966	976	987	999
1012	1026	1041	1051	1062	1074	1084	1095	1107	1120
1134	1149	1159	1170	1182	1192	1203	1215	1228	1242
1257	1267	1278	1290	1300	1311	1323	1336	1350	1365
1375	1386	1398	1408	1419	1431	1444	1458	1473	1483
1494	1506	1516	1527	1539	1552	1566	1581	1591	1602
1614									

TOTAL ELEMENTS MEAN THE TOTAL NUMBER OF ELEMENTS IN THE STIFFNESS MATRIX, NONZERO ELEMENTS ARE THE TOTAL NUMBER OF NON-ZERO ELEMENTS IN THE STIFFNESS MATRIX, AND STORED ELEMENTS ARE THE ELEMENTS IN THE STIFFNESS MATRIX NEEDED TO BE STORED BY USING THIS NUMBERING SCHEME. WHEN THE NUMBERING SCHEME IS AN EFFICIENT ONE, THE NUMBER OF NONZERO ELEMENTS IS CLOSE TO THE NUMBER OF STORED ELEMENTS.

LOADING INFORMATION :

STATIC LOADING INFORMATION:

STORY NO.	MODE	STORY HEIGHT	STORY WEIGHT
1	3	144.000	50000.000
2	4	288.000	50000.000
3	9	432.000	50000.000
4	12	576.000	50000.000
5	15	720.000	50000.000
6	18	864.000	50000.000
7	21	1008.000	50000.000
8	24	1152.000	50000.000
9	27	1296.000	50000.000
10	30	1440.000	50000.000
11	33	1584.000	50000.000
12	36	1728.000	50000.000
13	39	1872.000	50000.000
14	42	2016.000	50000.000
15	45	2160.000	50000.000

ATC-3 SEISMIC PROVISIONS

GENERAL INFORMATION :

AREA MAP NUMBER FOR AA -----	7
AREA MAP NUMBER FOR AV -----	7
SEISMIC COEFFICIENT AA ----- (TABLE 1-B) -----	0.400
SEISMIC COEFFICIENT AV ----- (TABLE 1-B) -----	0.400
SEISMIC INDEX ----- (TABLE 1-A) -----	4
SEISMIC HAZARD EXPOSURE GROUP (TABLE 1-A) -----	2
SEISMIC PERFORMANCE CATEGORY (TABLE 1-A) -----	C
RESPONSE MODIFICATION FACTOR (TABLE 3-B) -----	6.00
DEFLECTION AMPLIFICATION FACTOR (TABLE 3-B) -----	5.00
OVERALL LENGTH AT BASE (IN) -----	336.00
SOIL TYPE -----	3
STRUCTURAL TYPE -----	0
EQ. 1: MOMENT-RESISTING STRUCTURE	
EQ. 0: NON-MOMENT-RESISTING STRUCTURE	
MATERIAL OF THE STRUCTURE -----	1
EQ. 1: STEEL FRAME	
EQ. 0: CONCRETE FRAME	
STRUCTURAL CONFIGURATION :	
PLAN CONFIGURATION -----	1
VERTICAL CONFIGURATION -----	1
EQ. 1: REGULAR	
EQ. 0: IREGULAR	
SOIL PROFILE COEFFICIENT ---- (TABLE 3-A) -----	1.500

ALLOWABLE DRIFT ACCORDING TO TABLE 3-C

FLOOR NO.	ALLOWABLE VALUE (IN)
1	2.16000
2	2.16000
3	2.16000
4	2.16000
5	2.16000
6	2.16000
7	2.16000
8	2.16000
9	2.16000
10	2.16000
11	2.16000
12	2.16000
13	2.16000
14	2.16000
15	2.16000

MEMBERS CONTRIBUTING THEIR WEIGHT TO EACH FLOOR :

FLOOR LEVEL	NO. OF MEMBERS THAT CONTRIBUTING THEIR WEIGHT TO THE FLOOR									
1	31	32	1	2	3	4	61	62	63	64
2	33	34	3	4	5	6	63	64	65	66
3	35	36	5	6	7	8	65	66	67	68
4	37	38	7	8	9	10	67	68	69	70
5	39	40	9	10	11	12	69	70	71	72
6	41	42	11	12	13	14	71	72	73	74
7	43	44	13	14	15	16	73	74	75	76
8	45	46	15	16	17	18	75	76	77	78
9	47	48	17	18	19	20	77	78	79	80
10	49	50	19	20	21	22	79	80	81	82
11	51	52	21	22	23	24	81	82	83	84
12	53	54	23	24	25	26	83	84	85	86
13	55	56	25	26	27	28	85	86	87	88
14	57	58	27	28	29	30	87	88	89	90
15	59	60	29	30	89	90				

LOAD COMBINATION FACTORS FOR LOADING CONDITION 1

DEAD LOAD -----	0.0
LIVE LOAD -----	0.0
SNOW LOAD -----	0.0
EARTHQUAKE LOAD -----	1.0

LOAD COMBINATION FACTORS FOR LOADING CONDITION 2
 DEAD LOAD ----- 0.0
 LIVE LOAD ----- 0.0
 SNOW LOAD ----- 0.0
 EARTHQUAKE LOAD ----- -1.0

DYNAMIC PROPERTIES:
 FIRST MODE NUMBER----- 1
 LAST MODE NUMBER----- 1
 GRAVITY ACCELERATION----- 386.40
 TYPE OF MODE SUPERPOSITION----- 0

Eq. -1: DIRECT SUPERPOSITION OF MODES
 Eq. 0: ROOT-MEAN-SQUARE SUPERPOSITION OF MODES
 Eq. 1: ABSOLUTE SUPERPOSITION OF MODES

ADDITIONAL MODAL MASSES:
 0.0 0.0 32.500 65.000 32.500 32.500 65.000 32.500 32.500 65.000
 32.500 32.500 65.000 32.500 32.500 65.000 32.500 32.500 65.000 32.500
 32.500 65.000 32.500 32.500 65.000 32.500 32.500 65.000 32.500 65.000
 65.000 32.500 32.500 65.000 32.500 32.500 65.000 32.500 32.500 65.000
 32.500 32.500 65.000 32.500 32.500 65.000 32.500 65.000 32.500 65.000

DISPLACEMENT CONSTRAINTS:
 NODE DIRECTION VALUE
 3 1 0.432
 6 1 0.864
 9 1 1.296
 12 1 1.728
 15 1 2.160
 18 1 2.592
 21 1 3.024
 24 1 3.456
 27 1 3.888
 30 1 4.320
 33 1 4.752
 36 1 5.184
 39 1 5.616
 42 1 6.048
 45 1 6.480

RELATIVE SIZES OF MEMBERS:
 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861
 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861
 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861 0.0057142861
 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000
 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000
 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000
 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000
 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000
 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000 1.00000000

AFTER ATC-R-DLT SATISFIED, SCALING FACTOR= 0.174661E+04

STORY NO.	DRIFT(IN)	STABILITY COEFF	LATERAL FORCE(LB)	OVERTURNING MOMENT(IN-LB)	STORY WEIGHT(KIPS)
1	2.75692E-01	4.36217E-03	1.70028E+02	1.04511E+08	5.46643E+01
2	4.25047E-01	4.62564E-03	4.97596E+02	9.42172E+07	5.46643E+01
3	9.27550E-01	4.27844E-03	9.32563E+02	8.39891E+07	5.46643E+01
4	1.19683E+00	3.87199E-03	1.45624E+03	7.38873E+07	5.46643E+01
5	1.43104E+00	3.46632E-03	2.05763E+03	6.39867E+07	5.46643E+01
6	1.63085E+00	3.08420E-03	2.72919E+03	5.43765E+07	5.46643E+01
7	1.79692E+00	2.73251E-03	3.46536E+03	4.51594E+07	5.46643E+01
8	1.93031E+00	2.41286E-03	4.26177E+03	3.64412E+07	5.46643E+01
9	2.03239E+00	2.12455E-03	5.11488E+03	2.83367E+07	5.46643E+01
10	2.10491E+00	1.86595E-03	6.02176E+03	2.09688E+07	5.46643E+01
11	2.15010E+00	1.63514E-03	6.97990E+03	1.44680E+07	5.46643E+01
12	2.17047E+00	1.42999E-03	7.98714E+03	8.97228E+06	5.46643E+01
13	2.16920E+00	1.24857E-03	9.04160E+03	4.62673E+06	5.46643E+01
14	2.14984E+00	1.08898E-03	1.01416E+04	1.58317E+06	5.46643E+01
15	2.11682E+00	9.49379E-04	1.09942E+04	0.0	5.32530E+01

BASE MOMENT (IN-LB)----- 0.861965E+08
 TOTAL GRAVITY LOAD (LB)----- 0.818553E+06
 FUNDAMENTAL PERIOD----- 0.159841E+01
 ECCENTRICITY (IN)----- 0.105303E+03

OLD SCALING FACTOR-----	0.17382E+04
NEW SCALING FACTOR-----	0.17547E+04
ACTIVE STRESS IN LOADING CONDITION 1-----	61
ACTIVE STRESS IN LOADING CONDITION 2-----	61
ACTIVE DISPLACEMENT IN LOADING CONDITION 1-----	133
ACTIVE DISPLACEMENT IN LOADING CONDITION 2-----	133
STORY NUMBER WHICH HAS MAX. STORY DRIFT-----	12
MOST CRITICAL CONSTRAINT-----	4
NUMBER OF ANALYSIS-----	5

***** TOTAL WEIGHT OF STRUCTURE = 7.038569E+04 *****
 ***** BASE AE = 5.088725E+10; CYCLE NO. = 1 *****

NO CONSTRAINT ENERGY DENSITY RATIO

1	4	0.581659E-01
2	4	0.581729E-01
3	4	0.607840E-01
4	4	0.607239E-01
5	4	0.594666E-01
6	4	0.595075E-01
7	4	0.583727E-01
8	4	0.584413E-01
9	4	0.567945E-01
10	4	0.568957E-01
11	4	0.547558E-01
12	4	0.548942E-01
13	4	0.521945E-01
14	4	0.523733E-01
15	4	0.490906E-01
16	4	0.493035E-01
17	4	0.483657E-01
18	4	0.456176E-01
19	4	0.409649E-01
20	4	0.413803E-01
21	4	0.359500E-01
22	4	0.363104E-01
23	4	0.391772E-01
24	4	0.306000E-01
25	4	0.237010E-01
26	4	0.241064E-01
27	4	0.162719E-01
28	4	0.171174E-01
29	4	0.818794E-02
30	4	0.896758E-02
31	4	0.909178E-02
32	4	0.904932E-02
33	4	0.955225E-02
34	4	0.942518E-02
35	4	0.952803E-02
36	4	0.928989E-02
37	4	0.953311E-02
38	4	0.916128E-02
39	4	0.947861E-02
40	4	0.895230E-02
41	4	0.935757E-02
42	4	0.866296E-02
43	4	0.915364E-02
44	4	0.827862E-02
45	4	0.885597E-02
46	4	0.778327E-02
47	4	0.845345E-02
48	4	0.716584E-02
49	4	0.792433E-02
50	4	0.640834E-02
51	4	0.726083E-02
52	4	0.550301E-02
53	4	0.645531E-02
54	4	0.442342E-02
55	4	0.534905E-02
56	4	0.323373E-02
57	4	0.444660E-02
58	4	0.165389E-02
59	4	0.567803E-02
60	4	0.141105E-03
61	4	0.999999E+00
62	4	0.999946E+00
63	4	0.835242E+00
64	4	0.835175E+00
65	4	0.687946E+00
66	4	0.687858E+00
67	4	0.555163E+00
68	4	0.555044E+00
69	4	0.437541E+00
70	4	0.437397E+00
71	4	0.335129E+00
72	4	0.334955E+00
73	4	0.267863E+00
74	4	0.267671E+00
75	4	0.175441E+00

76 4 0.175291E+00
 77 4 0.117342E+00
 78 4 0.117133E+00
 79 4 0.727364E-01
 80 4 0.725273E-01
 81 4 0.404938E-01
 82 4 0.402904E-01
 83 4 0.191442E-01
 84 4 0.189628E-01
 85 4 0.685644E-02
 86 4 0.669644E-02
 87 4 0.135510E-02
 88 4 0.130303E-02
 89 4 0.227205E-03
 90 4 0.835006E-04

RELATIVE SIZES OF MEMBERS:
 0.0004031763 0.0004031763 0.0007743090 0.0007743090 0.0007413938 0.0007413938 0.0007405721 0.0007405721 0.0007327534 0.0007327534
 0.0007220474 0.0007220474 0.0007073549 0.0007073549 0.0006881570 0.0006881570 0.0006637240 0.0006637240 0.0006330854 0.0006330854
 0.0005949836 0.0005949836 0.0005475602 0.0005475602 0.0004874018 0.0004874018 0.0004105263 0.0004105263 0.0002964544 0.0002964544
 0.0050658621 0.0050658621 0.0127803087 0.0127803087 0.0120712072 0.0120712072 0.0121065006 0.0121065006 0.0119775459 0.0119775459
 0.0117728077 0.0117728077 0.0114667863 0.0114667863 0.0110501722 0.0110501722 0.0105096810 0.0105096810 0.0098331422 0.0098331422
 0.0090118833 0.0090118833 0.0080148056 0.0080148056 0.0068588182 0.0068588182 0.0071417727 0.0071417727 0.0075049363 0.0075049363
 1.00000000 1.00000000 0.783860087 0.783860087 0.654557347 0.654557347 0.534735322 0.534735322 0.427348932 0.427348932
 0.332400620 0.332400620 0.250131428 0.250131428 0.180557072 0.180557072 0.123813520 0.123813520 0.0785982609 0.0785982609
 0.0481299101 0.0481299101 0.0221235640 0.0221235640 0.0082353912 0.0082353912 0.0017615687 0.0017615687 0.0017615687 0.0017615687

AFTER ATC-P-DELTA SATISFIED, SCALING FACTOR= 0.643744E+04

STORY NO.	DRIFT(IN)	STABILITY COEFF	LATERAL FORCE(LB)	OVERTURNING MOMENT(IN-LB)	STORY WEIGHT(KIPS)
1	5.53349E-01	8.67530E-03	1.74566E+02	1.00192E+08	5.35624E+01
2	8.92768E-01	6.53495E-03	5.06956E+02	9.02804E+07	5.35561E+01
3	1.05508E+00	4.80500E-03	9.41337E+02	8.04361E+07	5.32991E+01
4	1.22088E+00	3.89380E-03	1.45883E+03	7.07191E+07	5.30619E+01
5	1.37698E+00	3.28388E-03	2.04685E+03	6.12037E+07	5.28191E+01
6	1.52476E+00	2.83611E-03	2.69692E+03	5.19772E+07	5.25741E+01
7	1.66276E+00	2.48485E-03	3.40253E+03	4.31392E+07	5.23269E+01
8	1.78969E+00	2.19713E-03	4.15854E+03	3.47910E+07	5.20789E+01
9	1.90410E+00	1.95406E-03	4.96071E+03	2.70416E+07	5.18293E+01
10	2.00411E+00	1.74368E-03	5.80552E+03	2.00066E+07	5.15804E+01
11	2.08743E+00	1.55793E-03	6.89000E+03	1.38876E+07	5.13331E+01
12	2.15120E+00	1.39100E-03	7.61156E+03	8.57201E+06	5.10877E+01
13	2.19057E+00	1.23778E-03	8.56859E+03	4.43243E+06	5.08486E+01
14	2.17934E+00	1.08422E-03	9.57613E+03	1.52674E+06	5.07050E+01
15	2.12563E+00	9.37202E-04	1.06023E+04	0.0	5.04860E+01

BASE MOMENT (IN-LB)----- 0.826709E+08
 TOTAL GRAVITY LOAD (LB)----- 0.781147E+06
 FUNDAMENTAL PERIOD----- 0.157651E+01
 ECCENTRICITY (IN)----- 0.105833E+03

OLD SCALING FACTOR----- 0.63476E+04
 NEW SCALING FACTOR----- 0.64078E+04
 ACTIVE STRESS IN LOADING CONDITION 1----- 61
 ACTIVE STRESS IN LOADING CONDITION 2----- 61
 ACTIVE DISPLACEMENT IN LOADING CONDITION 1----- 133
 ACTIVE DISPLACEMENT IN LOADING CONDITION 2----- 133
 STORY NUMBER WHICH HAS MAX. STORY DRIFT----- 13
 MOST CRITICAL CONSTRAINT----- 4
 NUMBER OF ANALYSIS----- 2

***** TOTAL WEIGHT OF STRUCTURE = 3.300637E+04 *****
 ***** BASE AE = 1.858254E+11; CYCLE NO. = 5 *****

MEMBER	LENGTH	AE	I	AREA	MA	MB	AXIAL FORCE	SHEAR	MOMENT	AXIAL FORCE	SHEAR	MOMENT
1	221.27	0.00040		2.5635	1	4	-1.97516E+04 -7.64539E+03 1.97516E+04 7.64539E+03					
2	221.27	0.00040		2.5635	2	4	1.97524E+04 7.64571E+03 -1.97524E+04 -7.64571E+03					
3	221.27	0.00077		4.9616	3	7	-4.65463E+04 -9.38130E+03 4.65463E+04 9.38130E+03					
4	221.27	0.00077		4.9616	5	7	4.65516E+04 9.38238E+03 -4.65516E+04 -9.38238E+03					
5	221.27	0.00074		4.7507	6	10	-4.40643E+04 -9.27577E+03 4.40643E+04 9.27577E+03					
6	221.27	0.00074		4.7507	8	10	4.40763E+04 9.27788E+03 -4.40763E+04 -9.27788E+03					
7	221.27	0.00074		4.7454	9	13	-4.38094E+04 -9.23194E+03 4.38094E+04 9.23194E+03					
8	221.27	0.00074		4.7454	11	13	4.38254E+04 9.23531E+03 -4.38254E+04 -9.23531E+03					
9	221.27	0.00073		4.6953	12	16	-4.28652E+04 -9.12935E+03 4.28652E+04 9.12935E+03					
10	221.27	0.00073		4.6953	14	16	4.28883E+04 9.13426E+03 -4.28883E+04 -9.13426E+03					

MEMBER	LENGTH	AE	I	AREA	MA	MB	AXIAL FORCE	SHEAR	MOMENT	AXIAL FORCE	SHEAR	MOMENT
11	221.27	0.00072		4.6267	15	19	-4.15819E+04 -8.98735E+03 4.15819E+04 8.98735E+03					
12	221.27	0.00072		4.6267	17	19	4.16123E+04 8.99392E+03 -4.16123E+04 -8.99392E+03					
13	221.27	0.00071		4.5326	18	22	-3.98667E+04 -8.79560E+03 3.98667E+04 8.79560E+03					
14	221.27	0.00071		4.5326	20	22	3.99063E+04 8.80434E+03 -3.99063E+04 -8.80434E+03					
15	221.27	0.00069		4.4096	21	25	-3.76904E+04 -8.54743E+03 3.76904E+04 8.54743E+03					
16	221.27	0.00069		4.4096	23	25	3.77387E+04 8.55840E+03 -3.77387E+04 -8.55840E+03					

17	221.27	0.00066	4.2530	24	28	-3.50176E+04 -8.23364E+03 3.50176E+04 8.23364E+03
18	221.27	0.00066	4.2530	26	28	3.50743E+04 8.24697E+03 -3.50743E+04 -8.24697E+03
19	221.27	0.00063	4.0567	27	31	-3.18172E+04 -7.84319E+03 3.18172E+04 7.84319E+03
20	221.27	0.00063	4.0567	29	31	3.18808E+04 7.85886E+03 -3.18808E+04 -7.85886E+03

MEMBER	LENGTH	AE	I	AREA	HA	HB	AXIAL FORCE	SHEAR	MOMENT	AXIAL FORCE	SHEAR	MOMENT
21	221.27	0.00059		3.8125	30	34	-2.80610E+04 -7.36023E+03 2.80610E+04 7.36023E+03					
22	221.27	0.00059		3.8125	32	34	2.81290E+04 7.37805E+03 -2.81290E+04 -7.37805E+03					
23	221.27	0.00059		3.5088	33	37	-2.37187E+04 -6.75982E+03 2.37187E+04 6.75982E+03					
24	221.27	0.00055		3.5088	35	37	2.37893E+04 6.77996E+03 -2.37893E+04 -6.77996E+03					
25	221.27	0.00049		3.1232	36	40	-1.87710E+04 -6.01025E+03 1.87710E+04 6.01025E+03					
26	221.27	0.00049		3.1232	38	40	1.88530E+04 6.03011E+03 -1.88530E+04 -6.03011E+03					
27	221.27	0.00041		2.6306	39	43	-1.32114E+04 -5.02226E+03 1.32114E+04 5.02226E+03					
28	221.27	0.00041		2.6306	41	43	1.32696E+04 5.04441E+03 -1.32696E+04 -5.04441E+03					
29	221.27	0.00030		1.8996	42	46	-6.90875E+03 -3.63693E+03 6.90875E+03 3.63693E+03					
30	221.27	0.00030		1.8996	44	46	6.95881E+03 3.66328E+03 -6.95881E+03 -3.66328E+03					

MEMBER	LENGTH	AE	I	AREA	MA	MB	AXIAL FORCE	Shear	MOMENT	AXIAL FORCE	Shear	MOMENT
31	168.00	0.00507	32.4609	2.6469	3	4	1.49859E+04 1.02648E+04 -1.49859E+04 -1.02648E+04	-9.24761E+01 0.0 9.24761E+01 0.0	-1.55210E+04 1.05886E+03 1.55210E+04 -1.05886E+03	-1.49859E+04 5.65723E+03 1.49859E+04 -5.65723E+03	9.24761E+01 0.0 -9.24761E+01 0.0	-1.50000E+01 5.66612E+03 1.50000E+01 -5.66612E+03
32	168.00	0.00507	32.4609	2.6469	4	5	-1.46668E+04 -5.62116E+03 1.46668E+04 5.62116E+03	-9.19336E+01 0.0 9.19336E+01 0.0	1.50078E+01 -5.61226E+03 -1.50078E+01 5.61226E+03	1.46668E+04 -1.02014E+04 -1.46668E+04 1.02014E+04	9.19336E+01 0.0 -9.19336E+01 0.0	-1.54599E+04 -1.03202E+03 1.54599E+04 1.03202E+03
33	168.00	0.01278	81.8933	4.2942	6	7	3.54321E+04 1.26669E+04 -3.54321E+04 -1.26669E+04	-2.14543E+02 0.0 2.14543E+02 0.0	-3.59206E+04 4.16874E+03 3.59206E+04 -4.16874E+03	-3.54321E+04 8.41329E+03 3.54321E+04 -8.41329E+03	2.14543E+02 0.0 -2.14543E+02 0.0	-1.22638E+02 8.44237E+03 1.22638E+02 -8.44237E+03
34	168.00	0.01278	81.8933	4.2942	7	8	-3.49208E+04 -8.32079E+03 3.49208E+04 8.32079E+03	-2.11029E+02 0.0 2.11029E+02 0.0	1.22781E+02 -8.29167E+03 -1.22781E+02 8.29167E+03	3.49208E+04 -1.25244E+04 -3.49208E+04 1.25244E+04	2.11029E+02 0.0 -2.11029E+02 0.0	-3.55756E+04 -4.08804E+03 3.55756E+04 4.08804E+03
35	168.00	0.01207	77.3495	4.0859	9	10	3.37652E+04 1.25889E+04 -3.37652E+04 -1.25889E+04	-2.06728E+02 0.0 2.06728E+02 0.0	-3.44797E+04 3.93889E+03 3.44797E+04 -3.93889E+03	-3.37652E+04 8.23246E+03 3.37652E+04 -8.23246E+03	2.06728E+02 0.0 -2.06728E+02 0.0	-2.50551E+02 8.29531E+03 2.50551E+02 -8.29531E+03
36	168.00	0.01207	77.3495	4.0859	10	11	-3.28438E+04 -8.06984E+03 3.28438E+04 8.06984E+03	-2.00030E+02 0.0 2.00030E+02 0.0	2.50758E+02 -8.00693E+03 -2.50758E+02 8.00693E+03	3.28438E+04 -1.22851E+04 -3.28438E+04 1.22851E+04	2.00030E+02 0.0 -2.00030E+02 0.0	-3.38558E+04 -3.79164E+03 3.38558E+04 3.79164E+03
37	168.00	0.01211	77.5757	4.0918	12	13	3.38312E+04 1.25669E+04 -3.38312E+04 -1.25669E+04	-2.07115E+02 0.0 2.07115E+02 0.0	-3.43708E+04 3.96899E+03 3.43708E+04 -3.96899E+03	-3.38312E+04 8.21486E+03 3.38312E+04 -8.21486E+03	2.07115E+02 0.0 -2.07115E+02 0.0	-4.24535E+02 8.32106E+03 4.24535E+02 -8.32106E+03
38	168.00	0.01211	77.5757	4.0918	13	14	-3.23956E+04 -7.97021E+03 3.23956E+04 7.97021E+03	-1.96167E+02 0.0 1.96167E+02 0.0	4.24457E+02 -7.86404E+03 -4.24457E+02 7.86404E+03	3.23956E+04 -1.20922E+04 -3.23956E+04 1.20922E+04	1.96167E+02 0.0 -1.96167E+02 0.0	-3.33805E+04 -3.74202E+03 3.33805E+04 3.74202E+03
39	168.00	0.01198	76.7494	4.0788	15	16	3.34094E+04 1.24759E+04 -3.34094E+04 -1.24759E+04	-2.04662E+02 0.0 2.04662E+02 0.0	-3.37858E+04 3.94151E+03 3.37858E+04 -3.94151E+03	-3.34094E+04 8.12977E+03 3.34094E+04 -8.12977E+03	2.04662E+02 0.0 -2.04662E+02 0.0	-6.24617E+02 8.28768E+03 -6.24617E+02 -8.28768E+03
40	168.00	0.01198	76.7494	4.0788	16	17	-3.13951E+04 -7.79273E+03 3.13951E+04 7.79273E+03	-1.88856E+02 0.0 1.88856E+02 0.0	6.24477E+02 -7.63486E+03 -6.24477E+02 7.63486E+03	3.13951E+04 -1.18033E+04 -3.13951E+04 1.18033E+04	1.88856E+02 0.0 -1.88856E+02 0.0	-3.23523E+04 -3.62433E+03 3.23523E+04 3.62433E+03
41	168.00	0.01177	75.4375	4.0351	18	19	3.27633E+04 1.23314E+04 -3.27633E+04 -1.23314E+04	-2.00003E+02 0.0 2.00003E+02 0.0	-3.27573E+04 3.90791E+03 3.27573E+04 -3.90791E+03	-3.27633E+04 8.01124E+03 3.27633E+04 -8.01124E+03	2.00003E+02 0.0 -2.00003E+02 0.0	-8.43285E+02 8.22809E+03 8.43285E+02 -8.22809E+03
42	168.00	0.01177	75.4375	4.0351	19	20	-3.01076E+04 -7.56990E+03 3.01076E+04 7.56990E+03	-1.78915E+02 0.0 1.78915E+02 0.0	8.43086E+02 -7.35310E+03 -8.43086E+02 7.35310E+03	3.01076E+04 -1.14346E+04 -3.01076E+04 1.14346E+04	1.78915E+02 0.0 -1.78915E+02 0.0	-3.09010E+04 -3.48842E+03 3.09010E+04 3.48842E+03
43	168.00	0.01147	73.4766	3.9823	21	22	3.18190E+04 1.21288E+04 -3.18190E+04 -1.21288E+04	-1.92697E+02 0.0 1.92697E+02 0.0	-3.13011E+04 3.85959E+03 3.13011E+04 -3.85959E+03	-3.18190E+04 7.84870E+03 3.18190E+04 -7.84870E+03	1.92697E+02 0.0 -1.92697E+02 0.0	-1.07205E+03 8.13164E+03 -1.07205E+03 -8.13164E+03
44	168.00	0.01147	73.4766	3.9823	22	23	-2.84670E+04 -7.28995E+03 2.84670E+04 7.28995E+03	-1.66094E+02 0.0 1.66094E+02 0.0	1.07234E+03 -7.00693E+03 -1.07234E+03 7.00693E+03	2.84670E+04 -1.09722E+04 -2.84670E+04 1.09722E+04	1.66094E+02 0.0 -1.66094E+02 0.0	-2.89763E+04 -3.32464E+03 2.89763E+04 3.32464E+03
45	168.00	0.01105	70.8070	3.9093	24	25	3.05510E+04 1.18329E+04 -3.05510E+04 -1.18329E+04	-1.82488E+02 0.0 1.82488E+02 0.0	-2.93543E+04 3.79715E+03 2.93543E+04 -3.79715E+03	-3.05510E+04 7.63657E+03 3.05510E+04 -7.63657E+03	1.82488E+02 0.0 -1.82488E+02 0.0	-1.30387E+03 7.99350E+03 1.30387E+03 -7.99350E+03

46	168.00	0.01105	70.8070	3.9093	25	26	-2.64530E+04	-1.50330E+02	1.30419E+03	2.64530E+04	1.50330E+02	-2.65597E+04	-6.94527E+03	0.0	-6.58825E+03	-1.04021E+04	0.0	-3.13138E+03	2.64530E+04	1.50330E+02	-2.65597E+04	-6.94527E+03	0.0	-6.58825E+03	-1.04021E+04	0.0	3.13138E+03			
47	168.00	0.01051	67.3436	3.8125	27	28	2.89320E+04	-1.69107E+02	-2.68827E+04	-2.89320E+04	1.69107E+02	-1.52750E+03	1.14552E+04	0.0	3.72241E+03	7.36912E+03	0.0	7.80851E+03	-2.89320E+04	1.69107E+02	-1.52750E+03	-2.89320E+04	1.69107E+02	-1.52750E+03	1.14552E+04	0.0	-3.72241E+03	-7.36912E+03	0.0	-7.80851E+03
48	168.00	0.01051	67.3436	3.8125	28	29	-2.40400E+04	-1.31626E+02	1.52793E+03	2.40400E+04	1.31626E+02	-2.36411E+04	-4.82541E+03	0.0	-6.08509E+03	-9.70583E+03	0.0	-2.90547E+03	2.40400E+04	1.31626E+02	-2.36411E+04	-4.82541E+03	0.0	-6.08509E+03	-9.70583E+03	0.0	2.90547E+03			
49	168.00	0.00963	63.0085	3.6877	30	31	2.69340E+04	-1.52281E+02	-2.38536E+04	-2.69340E+04	1.52281E+02	-1.72975E+03	1.09677E+04	0.0	3.63973E+03	7.03804E+03	0.0	2.64464E+03	2.69340E+04	-1.52281E+02	-1.72975E+03	-2.69340E+04	1.52281E+02	-1.72975E+03	1.09677E+04	0.0	-3.63973E+03	-7.03804E+03	0.0	-7.56943E+03
50	168.00	0.00963	63.0085	3.6877	31	32	-2.12070E+04	-1.10066E+02	1.73032E+03	2.12070E+04	1.10066E+02	-2.02215E+04	-6.01652E+03	0.0	-5.48495E+03	-8.85683E+03	0.0	-2.64464E+03	2.12070E+04	-1.10066E+02	-2.02215E+04	-6.01652E+03	0.0	-5.48495E+03	-8.85683E+03	0.0	2.64464E+03			
MEMBER LENGTH AE I AREA MA MEL AXIAL FORCE SHEAR MOMENT AXIAL FORCE SHEAR MOMENT																														
51	168.00	0.00901	57.7461	3.5303	33	34	2.45350E+04	-1.31629E+02	-2.02248E+04	-2.45350E+04	1.31629E+02	-1.88894E+03	1.03364E+04	0.0	3.56310E+03	6.63344E+03	0.0	7.26604E+03	2.45350E+04	-1.31629E+02	-1.88894E+03	-2.45350E+04	1.31629E+02	-1.88894E+03	1.03364E+04	0.0	-3.56310E+03	-6.63344E+03	0.0	-7.26604E+03
52	168.00	0.00901	57.7461	3.5303	34	35	-1.79300E+04	-8.58762E+01	1.68960E+03	1.79300E+04	8.58762E+01	-1.63168E+04	-5.39523E+03	0.0	-4.76241E+03	-7.81107E+03	0.0	-2.34657E+03	1.79300E+04	-8.58762E+01	-1.63168E+04	-5.39523E+03	0.0	-4.76241E+03	-7.81107E+03	0.0	2.34657E+03			
53	168.00	0.00801	51.3571	3.3293	36	37	2.17180E+04	-1.06326E+02	-1.58942E+04	-2.17180E+04	1.06326E+02	-1.96869E+03	9.51248E+03	0.0	3.53401E+03	6.15299E+03	0.0	6.89350E+03	2.17180E+04	-1.06326E+02	-1.96869E+03	-2.17180E+04	1.06326E+02	-1.96869E+03	9.51248E+03	0.0	-3.53401E+03	-6.15299E+03	0.0	-6.89350E+03
54	168.00	0.00801	51.3571	3.3293	37	38	-1.41880E+04	-5.93083E+01	1.96904E+03	1.41880E+04	5.93083E+01	-1.19329E+04	-4.43184E+03	0.0	-3.49121E+03	-6.50575E+03	0.0	-2.01729E+03	1.41880E+04	-5.93083E+01	-1.19329E+04	-4.43184E+03	0.0	-3.49121E+03	-6.50575E+03	0.0	2.01729E+03			
55	168.00	0.00686	43.9498	3.0799	39	40	1.84450E+04	-7.11689E+01	-1.01398E+04	-1.84450E+04	7.11689E+01	-1.81694E+03	8.21438E+03	0.0	3.76334E+03	5.59007E+03	0.0	6.18765E+03	1.84450E+04	-7.11689E+01	-1.81694E+03	-1.84450E+04	7.11689E+01	-1.81694E+03	8.21438E+03	0.0	-3.76334E+03	-5.59007E+03	0.0	-6.18765E+03
56	168.00	0.00686	43.9498	3.0799	40	41	-9.97500E+03	-3.04521E+01	1.81734E+03	9.97500E+03	3.04521E+01	-6.93350E+03	-3.63763E+03	0.0	-2.83988E+03	-4.76055E+03	0.0	-1.71697E+03	-9.97500E+03	-3.04521E+01	-6.93350E+03	-9.97500E+03	3.04521E+01	-6.93350E+03	-3.63763E+03	0.0	2.83988E+03	4.76055E+03	0.0	1.71697E+03
57	168.00	0.00714	45.7628	3.1428	42	43	1.47600E+04	-4.56147E+01	-5.75248E+03	-1.47600E+04	4.56147E+01	-1.91106E+03	5.98941E+03	0.0	3.44357E+03	4.29354E+03	0.0	5.0944E+03	1.47600E+04	-4.56147E+01	-1.91106E+03	-1.47600E+04	4.56147E+01	-1.91106E+03	5.98941E+03	0.0	-3.44357E+03	-4.29354E+03	0.0	-5.0944E+03
58	168.00	0.00714	45.7628	3.1428	43	44	-5.25900E+03	-7.93311E+00	1.91154E+03	5.25900E+03	7.93311E+00	-3.24456E+03	-2.07642E+03	0.0	-1.27031E+03	-2.35749E+03	0.0	-9.89244E+02	-5.25900E+03	-7.93311E+00	-3.24456E+03	-5.25900E+03	7.93311E+00	-3.24456E+03	-2.07642E+03	0.0	1.27031E+03	2.35749E+03	0.0	9.89244E+02
59	168.00	0.00750	48.0899	3.2217	45	46	1.05070E+04	-3.15923E+01	-3.40415E+03	-1.05070E+04	3.15923E+01	-1.90350E+03	3.94467E+03	0.0	2.57800E+03	2.87924E+03	0.0	3.64344E+03	1.05070E+04	-3.15923E+01	-1.90350E+03	-1.05070E+04	3.15923E+01	-1.90350E+03	3.94467E+03	0.0	-2.57800E+03	-2.87924E+03	0.0	-3.64344E+03
60	168.00	0.00750	48.0899	3.2217	46	47	-2.30000E+01	1.06006E+00	1.90372E+03	2.30000E+01	-1.06006E+00	-1.72569E+03	-3.75000E+02	0.0	3.89282E+02	-3.39267E+02	0.0	-3.53545E+02	-2.30000E+01	1.06006E+00	-1.72569E+03	-2.30000E+01	1.06006E+00	-1.72569E+03	-3.75000E+02	0.0	-3.89282E+02	3.39267E+02	0.0	3.53545E+02

MEMBER	LENGTH	AE	I	AREA	MA	MB	AXIAL FORCE	SHEAR	MOMENT	AXIAL FORCE	SHEAR	MOMENT
61	144.00	1.00000	6407.7734	37.1666	1	3	-2.96245E+05 -1.52779E+04 2.96245E+05 1.52779E+04	1.95892E+04 0.0 -1.95892E+04 0.0	2.90519E+06 -6.54090E+02 -2.90519E+06 6.54090E+02	2.96245E+05 -8.17832E+03 -2.96245E+05 8.17832E+03	-1.95892E+04 0.0 1.95892E+04 0.0	-8.43510E+04 -7.75372E+03 8.43510E+04 7.75372E+03
62	144.00	1.00000	6407.7734	37.1886	2	5	2.96232E+05 6.78941E+02 -2.96232E+05 -6.78941E+02	1.95440E+04 0.0 -1.95440E+04 0.0	2.89835E+06 1.52604E+04 -2.89835E+06 -1.52604E+04	-2.96232E+05 7.75418E+03 2.96232E+05 -7.75418E+03	-1.95440E+04 0.0 1.95440E+04 0.0	-8.40230E+04 8.17713E+03 -8.40230E+04 -8.17713E+03
63	144.00	0.78386	5022.7969	32.9252	3	6	-2.66004E+05 -8.37869E+03 2.66004E+05 8.37869E+03	-8.69055E+02 0.0 8.69055E+02 0.0	9.98720E+04 -7.77937E+03 -9.98720E+04 7.77937E+03	2.66004E+05 -8.75420E+03 -2.66004E+05 8.75420E+03	8.69055E+02 0.0 -8.69055E+02 0.0	-2.25024E+05 -7.40386E+03 2.25024E+05 7.40386E+03
64	144.00	0.78386	5022.7969	32.9252	5	8	2.65988E+05 7.77922E+03 -2.65988E+05 -7.77922E+03	-8.63500E+02 0.0 8.63500E+02 0.0	9.95240E+04 8.37715E+03 -9.95240E+04 -8.37715E+03	-2.65988E+05 7.40685E+03 2.65988E+05 -7.40685E+03	8.63500E+02 0.0 -8.63500E+02 0.0	-2.23862E+05 -6.75022E+03 2.23862E+05 -6.75022E+03
65	144.00	0.65456	4194.2539	30.0873	6	9	-2.37247E+05 -8.78014E+03 2.37247E+05 8.78014E+03	7.65904E+02 0.0 -7.65904E+02 0.0	2.60948E+05 -6.99044E+03 -2.60948E+05 6.99044E+03	2.37247E+05 -8.40195E+03 -2.37247E+05 8.40195E+03	-7.65904E+02 0.0 7.65904E+02 0.0	-1.50664E+05 -7.36863E+03 1.50664E+05 7.36863E+03
66	144.00	0.65456	4194.2539	30.0873	8	11	2.37228E+05 6.99498E+03 -2.37228E+05 -6.99498E+03	7.57402E+02 0.0 -7.57402E+02 0.0	2.59434E+05 8.77429E+03 -2.59434E+05 -8.77429E+03	-2.37228E+05 7.36897E+03 2.37228E+05 -7.36897E+03	-7.57402E+02 0.0 7.57402E+02 0.0	-1.50373E+05 6.40030E+03 1.50373E+05 -6.40030E+03
67	144.00	0.53474	3426.4626	27.1944	9	12	-2.08664E+05 -8.41323E+03 2.08664E+05 8.41323E+03	4.88219E+02 0.0 -4.88219E+02 0.0	1.85138E+05 -6.93291E+03 -1.85138E+05 6.93291E+03	2.08664E+05 -8.13220E+03 -2.08664E+05 8.13220E+03	-4.88219E+02 0.0 4.88219E+02 0.0	-1.14843E+05 -7.21395E+03 1.14843E+05 7.21395E+03
68	144.00	0.53474	3426.4626	27.1944	11	14	2.08640E+05 6.92573E+03 -2.08640E+05 -6.92573E+03	4.81887E+02 0.0 -4.81887E+02 0.0	1.84218E+05 8.40863E+03 -1.84218E+05 -8.40863E+03	-2.08640E+05 7.21308E+03 2.08640E+05 -7.21308E+03	-4.81887E+02 0.0 4.81887E+02 0.0	-1.14836E+05 -6.13128E+03 1.14836E+05 -6.13128E+03
69	144.00	0.42735	2738.3357	24.3108	12	15	-1.80692E+05 -8.14308E+03 1.80692E+05 8.14308E+03	4.76000E+02 0.0 -4.76000E+02 0.0	1.49234E+05 -7.2210E+03 -1.49234E+05 7.2210E+03	1.80692E+05 -7.81657E+03 -1.80692E+05 7.81657E+03	-4.76000E+02 0.0 4.76000E+02 0.0	-8.06520E+04 -7.04861E+03 8.06520E+04 7.04861E+03
70	144.00	0.42735	2738.3357	24.3108	14	17	1.80664E+05 6.72575E+03 -1.80664E+05 -6.72575E+03	4.67890E+02 0.0 -4.67890E+02 0.0	1.48224E+05 8.13711E+03 -1.48224E+05 -8.13711E+03	-1.80664E+05 7.04605E+03 1.80664E+05 -7.04605E+03	-4.67890E+02 0.0 4.67890E+02 0.0	-8.09450E+04 -7.81680E+03 8.09450E+04 7.81680E+03

MEMBER	LENGTH	AE	I	AREA	MA	MB	AXIAL FORCE	SHEAR	MOMENT	AXIAL FORCE	SHEAR	MOMENT
71	144.00	0.33240	2129.9478	21.4408	15	18	-1.53554E+05 -7.82825E+03 1.53554E+05 7.82825E+03	4.27562E+02 0.0 -4.27562E+02 0.0	1.14443E+05 -6.49526E+03 -1.14443E+05 6.49526E+03	1.53554E+05 -7.46949E+03 -1.53554E+05 7.46949E+03	-4.27562E+02 0.0 4.27562E+02 0.0	-5.28750E+04 -6.85382E+03 5.28750E+04 6.85382E+03
72	144.00	0.33240	2129.9478	21.4408	17	20	1.53521E+05 6.50025E+03 -1.53521E+05 -6.50025E+03	4.15473E+02 0.0 -4.15473E+02 0.0	1.13322E+05 7.82019E+03 -1.13322E+05 -7.82019E+03	-1.53521E+05 6.84874E+03 1.53521E+05 -6.84874E+03	-4.15473E+02 0.0 4.15473E+02 0.0	-5.34640E+04 -7.47170E+03 5.34640E+04 -7.47170E+03
73	144.00	0.25013	1602.7854	18.5992	18	21	-1.27492E+05 -7.48798E+03 1.27492E+05 7.48798E+03	3.81484E+02 0.0 -3.81484E+02 0.0	8.56650E+04 -6.22550E+03 -8.56650E+04 6.22550E+03	1.27492E+05 -7.08298E+03 -1.27492E+05 7.08298E+03	-3.81484E+02 0.0 3.81484E+02 0.0	-3.07030E+04 -6.30505E+03 3.07030E+04 6.30505E+03
74	144.00	0.25013	1602.7854	18.5992	20	23	1.27492E+05 6.23304E+03 -1.27492E+05 -6.23304E+03	3.66270E+02 0.0 -3.66270E+02 0.0	8.43680E+04 7.47640E+03 -8.43680E+04 -7.47640E+03	-1.27492E+05 6.62175E+03 1.27492E+05 -6.62175E+03	-3.66270E+02 0.0 3.66270E+02 0.0	-3.16160E+04 -7.08768E+03 3.16160E+04 7.08768E+03
75	144.00	0.18056	1156.9687	15.8022	21	24	-1.02921E+05 -7.11676E+03 1.02921E+05 7.11676E+03	3.33262E+02 0.0 -3.33262E+02 0.0	6.19960E+04 -5.90942E+03 -6.19960E+04 5.90942E+03	1.02921E+05 -6.64934E+03 -1.02921E+05 6.64934E+03	-3.33262E+02 0.0 3.33262E+02 0.0	-1.39930E+04 -6.37684E+03 1.39930E+04 6.37684E+03

76	144.00	0.18056	1156.9687	15.8022	23	26	1.02879E+05	3.15121E+02	6.05610E+04	-1.02879E+05	-3.15121E+02	-1.51980E+04	5.92074E+03	0.0	7.10013E+03	6.36245E+03	0.0	6.65842E+03	-1.02879E+05	-3.15121E+02	-1.51980E+04	5.92074E+03	0.0	7.10013E+03	6.36245E+03	0.0	6.65842E+03
77	144.00	0.12351	791.4465	13.0697	24	27	-8.00541E+04	2.85090E+02	4.33362E+04	8.00541E+04	-2.85090E+02	-2.28300E+03	-4.71553E+03	0.0	-5.53475E+03	-4.15625E+03	0.0	-4.09404E+03	8.00541E+04	-2.85090E+02	-2.28300E+03	4.71553E+03	0.0	5.53475E+03	4.15625E+03	0.0	4.09404E+03
78	144.00	0.12351	791.4465	13.0697	26	29	8.00061E+04	2.63496E+02	4.17187E+04	-8.00061E+04	-2.63496E+02	-3.77900E+03	5.55312E+03	0.0	6.68982E+03	6.06999E+03	0.0	6.17295E+03	8.00061E+04	-2.63496E+02	-3.77900E+03	5.55312E+03	0.0	6.68982E+03	6.06999E+03	0.0	6.17295E+03
79	144.00	0.07660	503.6396	10.4260	27	30	-5.92730E+04	2.37129E+02	2.91361E+04	5.92730E+04	-2.37129E+02	5.01100E+03	-6.28517E+03	0.0	-5.08509E+03	-5.58193E+03	0.0	-5.78833E+03	-5.92730E+04	-2.37129E+02	5.01100E+03	6.28517E+03	0.0	5.08509E+03	5.58193E+03	0.0	5.78833E+03
80	144.00	0.07660	503.6396	10.4260	29	32	5.92203E+04	2.12707E+02	2.73859E+04	-5.92203E+04	-2.12707E+02	3.24100E+03	-5.11608E+03	0.0	-2.73859E+04	5.74683E+03	0.0	5.61333E+03	5.92203E+04	-2.12707E+02	3.24100E+03	5.11608E+03	0.0	2.73859E+04	5.74683E+03	0.0	5.61333E+03

MEMBER	LENGTH	AE	I	AREA	MA	ML	AXIAL FORCE	SHEAR	MOMENT	AXIAL FORCE	SHEAR	MOMENT
81	144.00	0.04513	289.1821	7.9003	30	33	-4.09438E+04	1.90559E+02	1.88427E+04	4.09438E+04	-1.90559E+02	8.59744E+03
82	144.00	0.04513	289.1821	7.9003	32	35	4.08892E+04	1.63516E+02	1.69746E+04	-4.08892E+04	-1.63516E+02	6.56425E+03
83	144.00	0.02212	141.7628	5.5314	33	36	-2.54472E+04	1.43438E+02	1.16241E+04	2.54472E+04	-1.43438E+02	9.03012E+03
84	144.00	0.02212	141.7628	5.5314	35	38	2.53950E+04	1.15369E+02	9.75162E+03	-2.53950E+04	-1.15369E+02	6.86319E+03
85	144.00	0.00824	52.7705	3.3748	36	39	-1.31802E+04	1.02315E+02	6.86073E+03	1.31802E+04	-1.02315E+02	7.87250E+03
86	144.00	0.00824	52.7705	3.3748	38	41	1.31385E+04	7.30259E+01	5.06648E+03	-1.31385E+04	-7.30259E+01	5.45000E+03
87	144.00	0.00176	11.2877	1.5608	39	42	-4.55131E+03	3.54927E+01	2.26664E+03	4.55131E+03	-3.54927E+01	2.84371E+03
88	144.00	0.00176	11.2877	1.5608	41	44	4.51381E+03	2.28582E+01	1.48225E+03	-4.51381E+03	-2.28582E+01	1.80923E+03
89	144.00	0.00176	11.2877	1.5608	42	45	-3.08125E+01	4.38454E+01	2.90864E+03	3.08125E+01	-4.38454E+01	3.40446E+03
90	144.00	0.00176	11.2877	1.5608	44	47	-1.18750E+00	2.19478E+01	1.43449E+03	1.18750E+00	-2.19478E+01	1.72566E+03

GIRDER-COLUMN SECTION PROPERTIES:

MEMBER	TYPE	SECTION MODULUS	TF/D	DEPTH	TM	TF
31	2	3.372	0.0	0.0	0.0	0.0
32	2	3.372	0.0	0.0	0.0	0.0
33	2	6.434	0.0	0.0	0.0	0.0
34	2	6.434	0.0	0.0	0.0	0.0
35	2	7.972	0.0	0.0	0.0	0.0
36	2	7.972	0.0	0.0	0.0	0.0
37	2	7.995	0.0	0.0	0.0	0.0
38	2	7.995	0.0	0.0	0.0	0.0
39	2	7.911	0.0	0.0	0.0	0.0
40	2	7.911	0.0	0.0	0.0	0.0
41	2	7.778	0.0	0.0	0.0	0.0
42	2	7.778	0.0	0.0	0.0	0.0
43	2	7.578	0.0	0.0	0.0	0.0
44	2	7.578	0.0	0.0	0.0	0.0
45	2	7.386	0.0	0.0	0.0	0.0
46	2	7.386	0.0	0.0	0.0	0.0
47	2	6.953	0.0	0.0	0.0	0.0
48	2	6.953	0.0	0.0	0.0	0.0
49	2	6.510	0.0	0.0	0.0	0.0
50	2	6.510	0.0	0.0	0.0	0.0
51	2	5.972	0.0	0.0	0.0	0.0
52	2	5.972	0.0	0.0	0.0	0.0
53	2	5.317	0.0	0.0	0.0	0.0
54	2	5.317	0.0	0.0	0.0	0.0
55	2	4.556	0.0	0.0	0.0	0.0
56	2	4.556	0.0	0.0	0.0	0.0
57	2	4.743	0.0	0.0	0.0	0.0
58	2	4.743	0.0	0.0	0.0	0.0
59	2	4.982	0.0	0.0	0.0	0.0
60	2	4.982	0.0	0.0	0.0	0.0
61	1	397.322	0.0	0.0	0.0	0.0
62	1	397.322	0.0	0.0	0.0	0.0
63	1	333.283	0.0	0.0	0.0	0.0
64	1	333.283	0.0	0.0	0.0	0.0
65	1	291.611	0.0	0.0	0.0	0.0
66	1	291.611	0.0	0.0	0.0	0.0
67	1	250.133	0.0	0.0	0.0	0.0
68	1	250.133	0.0	0.0	0.0	0.0
69	1	210.043	0.0	0.0	0.0	0.0
70	1	210.043	0.0	0.0	0.0	0.0

Reproduced from
best available copy.



71	1	171.708	0.0	0.0	0.0	0.0
72	1	171.708	0.0	0.0	0.0	0.0
73	1	135.710	0.0	0.0	0.0	0.0
74	1	135.710	0.0	0.0	0.0	0.0
75	1	102.699	0.0	0.0	0.0	0.0
76	1	102.699	0.0	0.0	0.0	0.0
77	1	73.403	0.0	0.0	0.0	0.0
78	1	73.403	0.0	0.0	0.0	0.0
79	1	46.557	0.0	0.0	0.0	0.0
80	1	46.557	0.0	0.0	0.0	0.0
81	1	28.786	0.0	0.0	0.0	0.0
82	1	28.786	0.0	0.0	0.0	0.0
83	1	14.452	0.0	0.0	0.0	0.0
84	1	14.452	0.0	0.0	0.0	0.0

85	1	5.462	0.0	0.0	0.0	0.0
86	1	5.462	0.0	0.0	0.0	0.0
87	1	1.177	0.0	0.0	0.0	0.0
88	1	1.177	0.0	0.0	0.0	0.0
89	1	1.177	0.0	0.0	0.0	0.0
90	1	1.177	0.0	0.0	0.0	0.0

JOINT	-X	-Y	DISPL-X	DISPL-Y	ROTATION
1	0.0	0.0	0.0 0.0	0.0 0.0	0.0 0.0
2	336.000	0.0	0.0 0.0	0.0 0.0	0.0 0.0
3	0.0	144.000	1.09630823E-01 -1.09630823E-01	3.95554192E-02 -3.95554192E-02	-1.15832896E-03 1.15832896E-03
4	168.000	144.000	7.68321753E-02 -7.68321753E-02	-1.88956528E-06 1.88956528E-06	2.25305310E-04 -2.25305310E-04
5	336.000	144.000	1.09370351E-01 -1.09370351E-01	-3.95536348E-02 3.95536348E-02	-1.15555199E-03 1.15555199E-03
6	0.0	288.000	2.86508381E-01 -2.86508381E-01	7.96719790E-02 -7.96719790E-02	-1.31892436E-03 1.31892436E-03
7	168.000	288.000	2.37685025E-01 -2.37685025E-01	1.46518927E-04 -1.46518927E-04	-5.27555094E-05 5.27555094E-05
8	336.000	288.000	2.85803914E-01 -2.85803914E-01	-7.96677470E-02 7.96677470E-02	-1.31540117E-03 1.31540117E-03
9	0.0	432.000	4.95543420E-01 -4.95543420E-01	1.18826568E-01 -1.18826568E-01	-1.56257511E-03 1.56257511E-03
10	168.000	432.000	4.47669864E-01 -4.47669864E-01	4.00811899E-04 -4.00811899E-04	-2.80776527E-04 2.80776527E-04
11	336.000	432.000	4.94237065E-01 -4.94237065E-01	-1.18819058E-01 1.18819058E-01	-1.55798346E-03 1.55798346E-03
12	0.0	576.000	7.37426639E-01 -7.37426639E-01	1.56927347E-01 -1.56927347E-01	-1.77993602E-03 1.77993602E-03
13	168.000	576.000	6.89529479E-01 -6.89529479E-01	7.45907892E-04 -7.45907892E-04	-5.12434868E-04 5.12434868E-04
14	336.000	576.000	7.35394180E-01 -7.35394180E-01	-1.56915426E-01 1.56915426E-01	-1.77466101E-03 1.77466101E-03

15	0.0	720.000	1.01023579E+00 -1.01023579E+00	1.93034066E-01 -1.93034066E-01	-1.98836974E-03 1.98836974E-03
16	168.000	720.000	9.62662366E-01 -9.62662366E-01	1.16273738E-03 -1.16273738E-03	-7.37881055E-04 7.37881055E-04

JOINT	-X	-Y	DISPL-X	DISPL-Y	ROTATION
17	336.000	720.000	1.00736904E+00 -1.00736904E+00	-1.93816364E-01 1.93816364E-01	-1.98243884E-03 1.98243884E-03
18	0.0	864.000	1.31232548E+00 -1.31232548E+00	2.29395926E-01 -2.29395926E-01	-2.18339614E-03 2.18339614E-03
19	168.000	864.000	1.26528740E+00 -1.26528740E+00	1.64296012E-03 -1.64296012E-03	-9.57999611E-04 9.57999611E-04
20	336.000	864.000	1.30851269E+00 -1.30851269E+00	-2.29370594E-01 2.29370594E-01	-2.17686896E-03 2.17686896E-03
21	0.0	1008.000	1.64175415E+00 -1.64175415E+00	2.63443232E-01 -2.63443232E-01	-2.36364221E-03 2.36364221E-03
22	168.000	1008.000	1.59546661E+00 -1.59546661E+00	2.18554540E-03 -2.18554540E-03	-1.17196748E-03 1.17196748E-03
23	336.000	1008.000	1.63687801E+00 -1.63687801E+00	-2.63407826E-01 2.63407826E-01	-2.35652947E-03 2.35652947E-03
24	0.0	1152.000	1.99633312E+00 -1.99633312E+00	2.95786116E-01 -2.95786116E-01	-2.52673239E-03 2.52673239E-03
25	168.000	1152.000	1.95106030E+00 -1.95106030E+00	2.79774796E-03 -2.79774796E-03	-1.37925078E-03 1.37925078E-03
26	336.000	1152.000	1.99026012E+00 -1.99026012E+00	-2.95735538E-01 2.95735538E-01	-2.51909881E-03 2.51909881E-03
27	0.0	1296.000	2.37357712E+00 -2.37357712E+00	3.26198637E-01 -3.26198637E-01	-2.46983989E-03 2.46983989E-03
28	168.000	1296.000	2.32961464E+00 -2.32961464E+00	3.48858209E-03 -3.48858209E-03	-1.57926790E-03 1.57926790E-03
29	336.000	1296.000	2.36614323E+00 -2.36614323E+00	-3.26131880E-01 3.26131880E-01	-2.66182609E-03 2.66182609E-03
30	0.0	1440.000	2.77063656E+00 -2.77063656E+00	3.54428351E-01 -3.54428351E-01	-2.78877141E-03 2.78877141E-03
31	168.000	1440.000	2.72832584E+00 -2.72832584E+00	4.27796692E-03 -4.27796692E-03	-1.77171407E-03 1.77171407E-03
32	336.000	1440.000	2.76163960E+00 -2.76163960E+00	-3.54336381E-01 3.54336381E-01	-2.78085540E-03 2.78085540E-03

JOINT	-X	-Y	DISPL-X	DISPL-Y	ROTATION
33	0.0	1584.000	3.18420696E+00 -3.18420696E+00	3.80162537E-01 -3.80162537E-01	-2.87672784E-03 2.87672784E-03
34	168.000	1584.000	3.14394665E+00 -3.14394665E+00	5.18789515E-03 -5.18789515E-03	-1.95699674E-03 1.95699674E-03
35	336.000	1584.000	3.17336750E+00 -3.17336750E+00	-3.80036294E-01 3.80036294E-01	-2.87022954E-03 2.87022954E-03

36	0.0	1728.000	3.61040783E+00 -3.61040783E+00	4.03006375E-01 -4.03006375E-01	-2.92215603E-03 2.92215603E-03
37	168.000	1728.000	3.57261944E+00 -3.57261944E+00	6.26730546E-03 -6.26730546E-03	-2.13674945E-03 2.13674945E-03
38	336.000	1728.000	3.59730625E+00 -3.59730625E+00	-4.02833223E-01 4.02833223E-01	-2.92081642E-03 2.92081642E-03
39	0.0	1872.000	4.04440880E+00 -4.04440880E+00	4.22398925E-01 -4.22398925E-01	-2.87456508E-03 2.87456508E-03
40	168.000	1872.000	4.00971317E+00 -4.00971317E+00	7.61288777E-03 -7.61288777E-03	-2.32603261E-03 2.32603261E-03
41	336.000	1872.000	4.02847672E+00 -4.02847672E+00	-4.22164381E-01 4.22164381E-01	-2.90275831E-03 2.90275831E-03
42	0.0	2016.000	4.47618294E+00 -4.47618294E+00	4.36876026E-01 -4.36876026E-01	-2.74763792E-03 2.74763792E-03
43	168.000	2016.000	4.44897747E+00 -4.44897747E+00	9.28089023E-03 -9.28089023E-03	-2.50448752E-03 2.50448752E-03
44	336.000	2016.000	4.45866966E+00 -4.45866966E+00	-4.36524272E-01 4.36524272E-01	-2.83083878E-03 2.83083878E-03
45	0.0	2160.000	4.89731693E+00 -4.89731693E+00	4.36976075E-01 -4.36976075E-01	-2.63858261E-03 2.63858261E-03
46	168.000	2160.000	4.87842274E+00 -4.87842274E+00	1.02383569E-02 -1.02383569E-02	-2.54819030E-03 2.54819030E-03
47	336.000	2160.000	4.87838173E+00 -4.87838173E+00	-4.36520398E-01 4.36520398E-01	-2.76679243E-03 2.76679243E-03

REFERENCES

1. Cheng, F. Y. and Juang, D. S., "Optimum Design of Braced and Unbraced Frames for Static, Earthquake, and Wind Forces with UBC ATC-3 and TJ-11," Final Report for NSF, UMR Structural Series 85-11, 1985.
2. Cheng, F. Y. and Juang, D. S., "Assesment of ATC-3-06 for Steel Structures Based on Optimization Algorithm," Proceedings of the 8th World Conference on Earthquake Engineering, San Francisco, July, 1984, Vol. 5, PP. 435-442.
3. Cheng, F. Y., "Optimum Design of 2-D and 3-D Structures Subjected to Multicomponent Seismic Excitations," Proceedings of the AIT-CCNNA (U.S.-Taiwan) Joint Seminar on Research for Multiple Hazards Mitigation, Taiwan, January, 1984, pp. 182-202.
4. Cheng, F. Y. and Juang, D. S., "Computer Aided Structural Optimization for Static and Wind Forces," Proceedings of the 5th U.S. National Conference on Wind Engineering, Texas Tech University, Nov. 1985.
5. Cheng, F. Y., "Evaluation of Frame Systems Based on Optimality Criteria with Multicomponent Seismic Inputs, Performance Constraints, and P-Delta Effect," published in Vol. I of the NSF-NATO Advanced Study Institute on Optimization of Distributed Parameter Structural Systems, Sijthoff & Noordhoff International, pp. 650-684.
6. Cheng, F. Y., "Optimum Design of Seismic Structures with Risk Consideration," Proceedings of the U.S.-Japan Joint Seminar on Seismic Risk and Its Use in Code Formulation, Hawaii, March, 1983, pp #10, 1-28.

