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SARCF USER'S GUIDE
SEISMIC ANALYSIS OF REINFORCED
CONCRETE FRAMES

by

Y.S. Chung and M. Shinozuka

Department of Civil Engineering and Operations Research
Princeton University
Princeton, New Jersey 08544

and

C. Meyer

Department of Civil Engineering and Engineering Mechanics
Columbia University
New York, New York 10027-6699

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Y.S. Chung¹, C. Meyer² and M. Shinozuka³

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- 1 Research Associate, Dept. of Civil Engineering and Operations Research, Princeton University
- 2 Associate Professor, Dept. of Civil Engineering, Columbia University
- 3 Professor, Dept. of Civil Engineering and Operations Research, Princeton University

NATIONAL CENTER FOR EARTHQUAKE ENGINEERING RESEARCH
State University of New York at Buffalo
Red Jacket Quadrangle, Buffalo, NY 14261

PREFACE

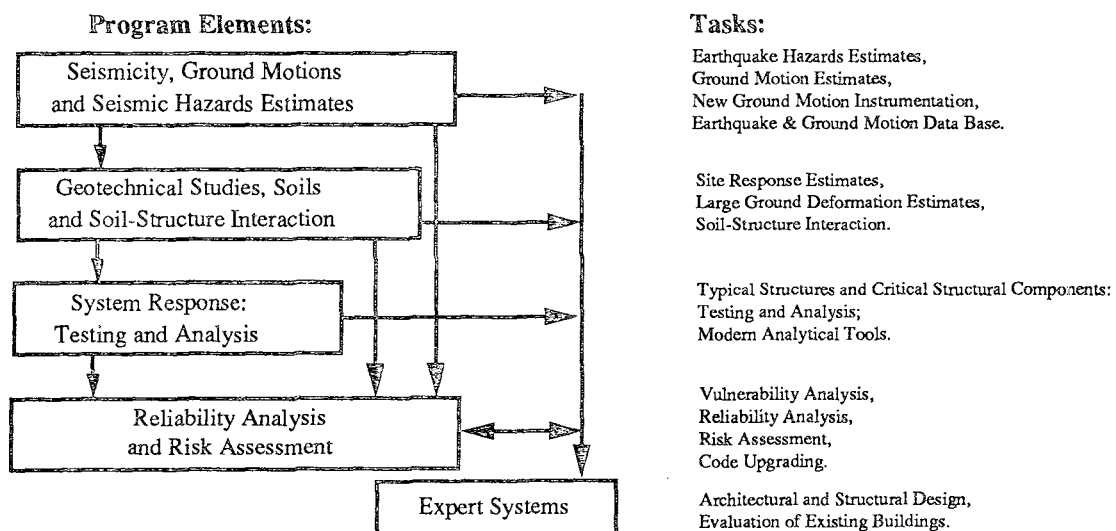
The National Center for Earthquake Engineering Research (NCEER) is devoted to the expansion and dissemination of knowledge about earthquakes, the improvement of earthquake-resistant design, and the implementation of seismic hazard mitigation procedures to minimize loss of lives and property. The emphasis is on structures and lifelines that are found in zones of moderate to high seismicity throughout the United States.

NCEER's research is being carried out in an integrated and coordinated manner following a structured program. The current research program comprises four main areas:

- Existing and New Structures
- Secondary and Protective Systems
- Lifeline Systems
- Disaster Research and Planning

This technical report pertains to Program 1, Existing and New Structures, and more specifically to system response investigations.

The long term goal of research in Existing and New Structures is to develop seismic hazard mitigation procedures through rational probabilistic risk assessment for damage or collapse of structures, mainly existing buildings, in regions of moderate to high seismicity. The work relies on improved definitions of seismicity and site response, experimental and analytical evaluations of systems response, and more accurate assessment of risk factors. This technology will be incorporated in expert systems tools and improved code formats for existing and new structures. Methods of retrofit will also be developed. When this work is completed, it should be possible to characterize and quantify societal impact of seismic risk in various geographical regions and large municipalities. Toward this goal, the program has been divided into five components, as shown in the figure below:



System response investigations constitute one of the important areas of research in Existing and New Structures. Current research activities include the following:

1. Testing and analysis of lightly reinforced concrete structures, and other structural components common in the eastern United States such as semi-rigid connections and flexible diaphragms.
2. Development of modern, dynamic analysis tools.
3. Investigation of innovative computing techniques that include the use of interactive computer graphics, advanced engineering workstations and supercomputing.

The ultimate goal of projects in this area is to provide an estimate of the seismic hazard of existing buildings which were not designed for earthquakes and to provide information on typical weak structural systems, such as lightly reinforced concrete elements and steel frames with semi-rigid connections. An additional goal of these projects is the development of modern analytical tools for the nonlinear dynamic analysis of complex structures.

The systems response area is, in part, concerned with the design and analysis of structures using sophisticated computer programs. This report describes improved models for the analysis and design of reinforced concrete frames and for damage estimation. These can be used to assess the expected distribution of damage throughout the frame. The program can help to estimate the response of reinforced concrete frames and in damage assessment. The program and the new models are described in detail and a listing of the program is provided.

ABSTRACT

A computer program for the automatic damage controlled design of reinforced concrete frames is described. The program is an extension of DRAIN-2D with various enhancements, such as an improved model for RC frame elements, automatic generation of random earthquakes, a new damage model, and an automated damage-controlled design. The current version covers only beam and beam-column elements. It is intended to implement other structural element types in future program versions.

This report reviews a few important aspects of the program, some of them retained from DRAIN-2D, such as basic structural analysis assumptions and the step-by-step solution of the nonlinear equations of motion. The frame element and damage model are described in some detail, as are the basis for the automated damage-controlled design procedure. Several examples illustrate the use and capabilities of the program.

Appendices contain the user's manual and the complete source listing of the program.

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1. Introduction

Modern seismic design of concrete buildings relies on the energy dissipation of structural components through large inelastic deformations. A consistent design approach thus requires a rational assessment of the reliability of a given design subjected to a hypothetical seismic event of specified intensity. As long as the structural response is limited to linear elastic behavior, standard methods of structural analysis suffice for this task, and very approximate techniques have proven adequate for most practical purposes.

If inelastic frame action is taken into account, accurate response analyses are considerably more difficult. Even though grossly simplified analysis methods have been proposed for design purposes (9), their application to actual structures is not straightforward, and their accuracy varies widely. Thus, more refined mathematical models and analysis techniques are needed for unusual situations. They also can be used for calibration purposes in cases where data from physical experiments are not available.

It is the purpose of this report to document SARCF, a computer program for seismic analysis of reinforced concrete frames, which has been partially developed at Columbia University. It contains a refined mathematical model for reinforced concrete frame elements, which can simulate the strength and stiffness degradation observed in laboratory experiments under strong load reversals. The large computational effort required for full-scale nonlinear seismic analyses will preclude the use of this program in the day-to-day operations of a design firm, but its claim to high accuracy commends it for special and comparative purposes.

A significant feature of the program is its automated damage-controlled design procedure. It incorporates a set of design rules, with which it can perform iterations on a preliminary design entered by the user, until a uniform damage distribution of

specified value is achieved. Thus the least amount of damage and high reliability of the frame for a seismic event of given intensity is assured.

The program is based on the well-known structural analysis program DRAIN-2D (8). Over the years, the following major modifications and enhancements have been added to the program.

- 1) The Roufaiel-Meyer frame element model (11,12), with enhancements due to Chung (4,5), was incorporated to replace the element models supplied with the standard DRAIN-2D version.
- 2) At any time during a dynamic time history analysis it is possible to interrupt the analysis to compute mode shapes and natural frequencies using the current tangent stiffness of the frame.
- 3) At each time step during the dynamic time history analysis, a damage index is computed for both ends of each element in the frame.
- 4) Whereas DRAIN-2D expects the user to supply a ground motion time history and performs only one time history analysis per run, SARCF may also generate an arbitrary number of artificial ground motion histories upon demand, using Monte Carlo simulation.
- 5) The program can perform a static analysis for gravity loads before executing the dynamic time history analysis. Thus the problem of nonlinear superposition is solved correctly.
- 6) Following the analysis of a frame structure for a specified number of ground motions, the program computes mean value and standard deviation of all element damage indices.
- 7) The program contains a number of design rules which it utilizes to modify the given frame such that the damage distribution becomes as uniform and as low as specified by the user.

The next chapter summarizes some of the important theoretical aspects of the original DRAIN-2D program, especially the numerical method used to solve the nonlinear equations of dynamic motion. Also the eigenvalue solver and artificial ground motion generator will be reviewed briefly.

Chapter 3 contains summary descriptions of the hysteresis model for RC frame elements, as well as of our damage index. The automated damage-controlled design procedure is described in chapter 4. The examples in chapter 5 illustrate the various capabilities and the use of the program. The user's guide in Appendix A contains detailed input specifications as well as instructions for preparing the data file and executing the program. The complete Fortran source listing of the program is given in Appendix B.

2. Nonlinear Dynamic Frame Analysis

This chapter summarizes some of the theoretical background for nonlinear dynamic analysis of frames. Much of this material is based on program DRAIN-2D (8). The eigenvalue solver utilizes the determinant search algorithm of the well-known SAP4 program (3).

2.1 Basic Assumptions

For analysis, a structure can be idealized as a finite number of nodes, or joints, interconnected by a finite number of deformable elements, or members. The nodes may have finite dimensions, but are commonly idealized as points. The elements may in general be one-, two- or three-dimensional, but in the present version of the program only one-dimensional or line elements are incorporated. Loads may be applied to the nodes or to the elements.

The analysis is based on the following assumptions:

- 1) The input structure is idealized as a plane frame. Out-of-plane motion is ignored.
- 2) Each member is treated as a massless prismatic line member represented by its centroidal axis, and all the mass in the structure is assumed to be lumped at the corresponding nodes.
- 3) The $P-\Delta$ effect is taken into account by adding geometric stiffness to the column stiffness, using the axial forces produced by the static loads.
- 4) Axial and shear deformations of the frame are ignored.
- 5) The idealized frame is assumed to be fixed at the base of the first story columns on a rigid foundation.

Based on the above assumptions, the equilibrium equations to be solved at any stage of the analysis can be obtained as described in the next section. They

are solved by an efficient algorithm based on the Gaussian elimination method. The structure stiffness is stored in a compacted form to optimize the use of core storage, and during the elimination operation virtually all unnecessary arithmetic operations are avoided. The initial elastic stiffness is stored separately in the case that the automated design option is exercised.

2.2 Solution of Equations of Motion

Basic analysis procedures are the same as those for DRAIN-2D (8). That is, member forces are computed for each load increment, and the tangent stiffness matrix is updated to account for changes in any of the element stiffnesses. The equations of motion to be solved at any stage of the analysis are written as

$$[M]\{\Delta\ddot{u}\} + [C_t]\{\Delta\dot{u}\} + [K_t]\{\Delta u\} = \{\Delta p\} \quad (2.1)$$

where

$[K_t]$: current tangent stiffness matrix

$[M]$: mass matrix

$[C_t]$: current damping matrix

$\{\Delta p\} = -[M]\{I\}\{\Delta\ddot{u}_g\}$
: load increment due to earthquake excitation

$\{I\}$: modal influence vector

$\Delta\ddot{u}_g$: earthquake ground acceleration increment

The above system of equations is solved using the constant acceleration method (2). That is, using velocity and acceleration at the previous time step, the finite increments are

$$\begin{aligned} \{\Delta\ddot{u}\} &= \frac{4}{\Delta t^2}\{\Delta u\} - \frac{4}{\Delta t}\{\dot{u}_o\} - 2\{\ddot{u}_o\} \\ \{\Delta\dot{u}\} &= \frac{2}{\Delta t}\{\Delta u\} - 2\{\dot{u}_o\} \end{aligned} \quad (2.2)$$

where the subscript “o” refers to the previous time step. The damping matrix is assumed to be a linear combination of mass and stiffness proportional components, known as Rayleigh damping,

$$[C_t] = \alpha[M] + \beta[K_t] \quad (2.3)$$

Substitution of Eqs (2.2) and (2.3) into Eq (2.1) gives

$$\begin{aligned} \left(\frac{4}{\Delta t^2} + \frac{2\alpha}{\Delta t} \right) [M]\{\Delta u\} + [K_t] \left(1 + \frac{2\beta}{\Delta t} \right) \{\Delta u\} - 2\beta[K_t]\{\dot{u}_o\} \\ = \{\Delta p\} + [M]\{2\ddot{u}_o + \frac{4}{\Delta t}\dot{u}_o + 2\alpha\dot{u}_o\} \end{aligned} \quad (2.4)$$

To avoid the need to compute the product $\beta[K_t]\{2\dot{u}_o\}$, Wilson (16) suggested the following transformation.

$$\{\Delta x\} = \{\Delta u\} + \beta\{\Delta \dot{u}\} = \left(1 + \frac{2\beta}{\Delta t} \right) \{\Delta u\} - 2\beta\{\dot{u}_o\} \quad (2.5)$$

Using Eq (2.5), Eq (2.4) becomes

$$[\gamma M + K_t]\{\Delta x\} = \{\Delta p\} + [M]\{2\ddot{u}_o + \frac{4}{\Delta t}\dot{u}_o + 2\alpha\dot{u}_o\} \quad (2.6)$$

where

$$\gamma = \frac{\left(\frac{4}{\Delta t^2} + \frac{2\alpha}{\Delta t} \right)}{\left(1 + \frac{2\beta}{\Delta t} \right)}$$

After the solution of Eq (2.6) for $\{\Delta x\}$, the nodal displacement increments, $\{\Delta u\}$, are obtained from Eq (2.5).

The proportionality factors α and β of Eq (2.3) can be determined by specifying damping ratios, λ_1 and λ_2 , for any two modes of vibration, say the first and second modes. The set of two simultaneous equations used to obtain α and β is

$$\lambda_n = \frac{\alpha}{2\omega_n} + \frac{\beta\omega_n}{2} \quad ; \quad n = 1, 2 \quad (2.7)$$

where λ_n indicates the proportion of critical damping in the n -th mode and ω_n denotes the circular frequency of the n -th mode.

2.3 Generation of Artificial Earthquakes

For nonlinear dynamic analysis of structures, the representation of earthquake ground motions as a stationary random process is of limited use because of the time dependency of the mean peak acceleration envelope and the duration of strong ground motion. Artificial ground acceleration histories, $\ddot{x}(t)$, can be generated by multiplying an envelope function, $s(t)$, and a stationary Gaussian process, $g(t)$. The envelope is here assumed to be either of a trapezoidal form or an exponential function, Fig 2.1 (14),

$$s(t) = \frac{e^{-\alpha t} - e^{-\beta t}}{e^{-\alpha t_n} - e^{-\beta t_n}} \quad t > 0 \quad (2.8)$$

where α and β are positive constants and $t_n = \frac{1}{\alpha - \beta} \ln \left(\frac{\alpha}{\beta} \right)$ with $\alpha > \beta$. A Gaussian process, $g(t)$, can be obtained by using the well-known Kanai-Tajimi spectrum as the power spectral density function,

$$S(\omega) = S_o \times \frac{1 + 4\zeta_g^2 \left(\frac{\omega}{\omega_g} \right)^2}{\left[1 - \left(\frac{\omega}{\omega_g} \right)^2 \right]^2 + 4\zeta_g^2 \left(\frac{\omega}{\omega_g} \right)^2} \quad (2.9)$$

where ω_g is the characteristic ground frequency, ζ_g is the predominant damping coefficient, S_o is the intensity of Gaussian white noise over the range $-\infty < \omega < \infty$. The Gaussian process, $g(t)$, can be generated by using Monte Carlo technique (15),

$$g(t) = \sqrt{2} \sum_{k=1}^N \sqrt{G(\omega_k) \Delta\omega} \cdot \cos(\omega_k t - \phi_k) \quad (2.10)$$

where ϕ_k is the random phase angle, uniformly distributed between 0 and 2π . $\omega_k = k\Delta\omega$, and $\omega_u = N\Delta\omega$ is the upper cut-off frequency. $G(\omega_k) = 2S(\omega_k)$ is the one-sided power spectrum. To generate an artificial earthquake, Shinozuka (13)

suggested the following relationship between the intensity, S_o , and the peak ground acceleration, PGA . With

$$\sigma_g^2 = E[\ddot{x}_g^2] = \int S(\omega) d\omega = \frac{S_o \pi \omega_g (1 + 4\zeta_g^2)}{2\zeta_g} \quad (2.11)$$

the peak ground acceleration can be written as

$$PGA = \alpha_g S_o^{\frac{1}{2}} \quad (2.12)$$

where $\alpha_g = p_g \left[\pi \omega_g \left(\frac{1}{2\zeta_g} + 2\zeta_g \right) \right]^{\frac{1}{2}}$, and p_g denotes the peak factor, empirically assumed to be 3.0 in this study.

Because of the random nature of earthquake acceleration histories, structure response quantities (such as damage indices) are more meaningful if formed as averages for an ensemble of responses, rather than responses to individual input functions. In order to determine the minimum number of sample functions necessary to give useful mean responses, the running mean values of damage indices for certain structural elements of a frame can be computed as functions of the number of sample earthquake input histories (6). The number of sample functions to be generated is input by the user.

2.4 Eigenvalue Solver

The eigenvalue solver is based on the determinant search algorithm and has been adopted with minor modifications from the SAP4 program (3). The theoretical background has been described in detail by Bathe (2).

The user has the option of computing the natural frequencies and mode shapes, by interrupting a time history analysis at any point to solve the eigenvalue problem

$$[K_t]\{\phi_i\} = \omega_i^2 [M]\{\phi_i\} \quad (2.13)$$

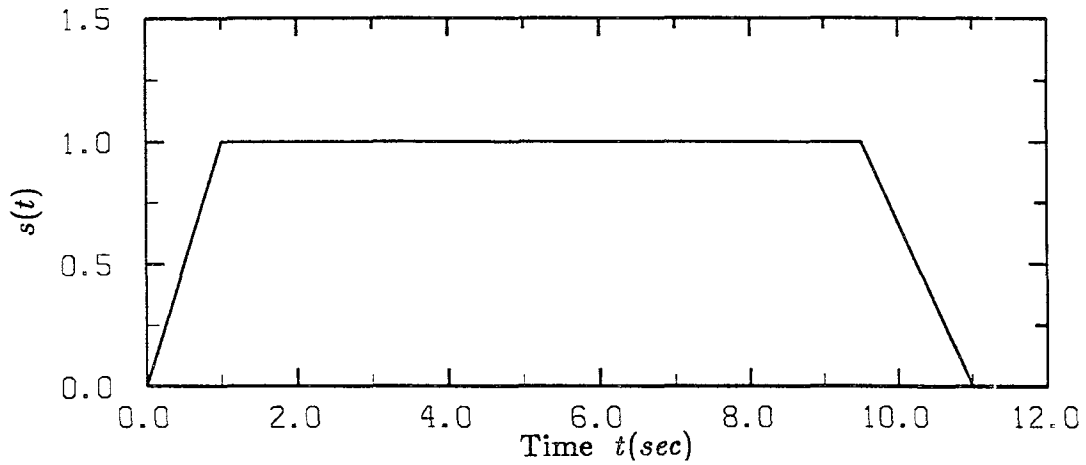
where

$[K_t]$: current tangent stiffness matrix

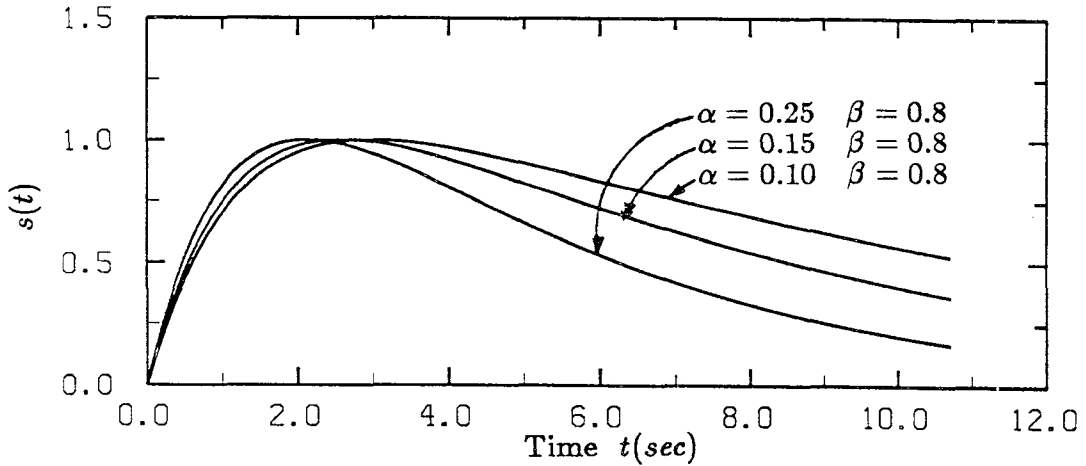
$[M]$: mass matrix

$\{\phi_i\}$: i-th modal vector

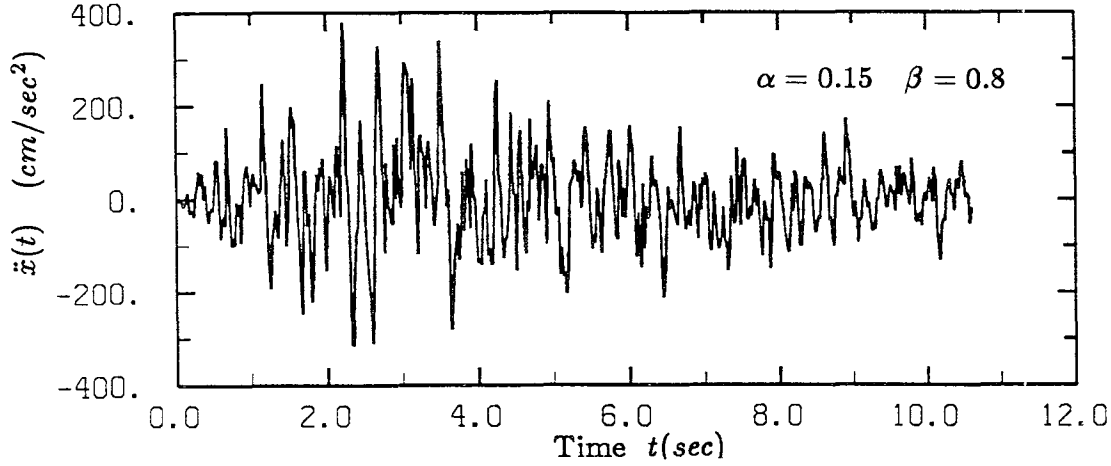
ω_i : the natural frequency corresponding to i-th mode



a) Trapezoidal Envelope Function



b) Exponential Envelope Function



c) Nonstationary Ground Motion Based on Exponential Envelope Function

Fig. 2.1 - Generation of Artificial Earthquakes

3. Frame Element and Damage Index

This chapter describes the reinforced concrete frame element model of SARCF. Originally proposed by Roufaiel and Meyer (11,12), several aspects of the model have been refined by Chung (4,5), particularly for the representation of stiffness and strength degradation. This model takes into account the finite size of the plastic regions and realistically simulates the experimental hysteretic behavior under cyclic load. In addition, the computer program contains a new damage model as an objective measure of strength deterioration.

3.1 Primary Moment-Curvature Curve

The primary moment-curvature curve relates moment to curvature under monotonically increasing loading. If the stress-strain laws for steel and concrete are specified and the cross-sectional dimensions are known, it is relatively straightforward to compute the moment associated with any specified curvature (4).

First, the neutral axis has to be positioned for a given moment or curvature, based on the corresponding concrete or tensile steel strain. The complete moment-curvature is obtained by increasing the concrete strain or steel strain in small increments from zero until any one of the possible failure modes is reached, and at each step computing the neutral axis position, the curvature and the bending moment. This analysis is based on the following assumptions:

- 1) The stress-strain curves of reinforcing steel and concrete are idealized as shown in Figs A.2 and A.3;
- 2) The tensile strength of concrete is ignored;
- 3) Plane sections remain plane after deformation;
- 4) The axial force, if any, is acting at the plastic centroid of the section.

3.2 Hysteresis Model

Under load reversals, the stiffness of a RC member experiences a progressive reduction due to cracking of the concrete and bond deterioration of the steel-concrete interface. In a Takeda-type model, a set of rules is specified, with which it is possible to characterize the hysteretic behavior more realistically than either with a simple bilinear or degrading bilinear formulation. The model of Roufaiel and Meyer utilizes such a set of rules and therefore has been adopted herein, together with certain improvements to better represent stiffness and strength degradation, Fig 3.1.

The hysteresis response can be characterized by five different types of branches:

- 1) Elastic loading and unloading: If the maximum moment does not exceed the yield moment M_y , the moment-curvature relationship is given by

$$\Delta M = (EI)_1 \Delta \phi \quad (3.1)$$

where $(EI)_1 = (EI)_e$ is the initial elastic sectional stiffness.

- 2) Inelastic loading: If the moment exceeds the yield moment and is still increasing, the moment-curvature relationship is given by

$$\Delta M = (EI)_2 \Delta \phi \quad (3.2)$$

where $(EI)_2 = p(EI)_e = \frac{M_u^+ - M_y^+}{\phi_u^+ - \phi_y^+}$.

- 3) Inelastic unloading: If the moment decreases after the yield moment has been exceeded, the moment-curvature relationship becomes

$$\Delta M = (EI)_3 \Delta \phi \quad (3.3)$$

where $(EI)_3 = \frac{M_x^+}{\phi_x^+ - \phi_r^+}$. The “+” superscript denotes loading in the positive sense. Likewise, a “-” superscript stands for loading in the negative sense.

- 4) Inelastic reloading during closing of cracks: In a reversed load cycle, previously opened cracks tend to close, leading to an increase in stiffness and

a characteristic “pinched” shape of the moment-curvature curve. This effect is a function of the shear span. If the absolute value of the moment increases but is still less than a certain “crack-closing moment”, M_p^+ , the moment-curvature relationship is in this case

$$\Delta M = (EI)_4 \Delta \phi \quad (3.4)$$

where $(EI)_4 = \frac{M_p^+}{\phi_p^+ - \phi_r^-}$.

- 5) Inelastic reloading after closing of cracks: Once the absolute value of the moment exceeds the “crack-closing moment”, M_p^+ , and is still increasing, then the moment-curvature relationship is

$$\Delta M = (EI)_5 \Delta \phi \quad (3.5)$$

where $(EI)_5 = \frac{\bar{M}_x^+ - M_p^+}{\phi_x^+ - \phi_p^+}$.

For further details see Ref (4). It should be emphasized that this model, unlike most other models, does not depend on the program user’s input of either stiffness or strength degradation parameters. These parameters are determined internally from the basic material and section properties that are input.

3.3 Tangent Stiffness Matrix

To compute the tangent stiffness matrix of a general frame member, the element is subdivided into three regions, Fig 3.2:

- 1) an inelastic region of length x_i at node i, having the average stiffness $(\bar{EI})_i$,
 - 2) an inelastic region of length x_j at node j, having the average stiffness $(\bar{EI})_j$,
- and
- 3) an elastic region of length $L - x_i - x_j$ with the initial stiffness $(EI)_e$.

For the six planar degrees of freedom identified in Fig 3.2, the tangent stiffness

of this frame element can be written as

$$[K_e] = \begin{bmatrix} k_{11} & 0 & 0 & k_{14} & 0 & 0 \\ & k_{22} & k_{23} & 0 & k_{25} & k_{26} \\ & & k_{33} & 0 & k_{35} & k_{36} \\ & & & k_{44} & 0 & 0 \\ & & sym. & & k_{55} & k_{56} \\ & & & & & k_{66} \end{bmatrix}$$

The coefficients, $k_{11} = k_{44} = -k_{14} = \frac{EA}{L}$, are assumed to remain constant. k_{33} , k_{36} , and k_{66} are obtained from their flexibility counterparts, which in turn can be computed by integrating the moment-curvature expressions over the entire length of the member.

Denoting by

$$\begin{aligned} Q_i &= \frac{(EI)_e}{(EI)_i} \\ Q_j &= \frac{(EI)_e}{(EI)_j} \end{aligned} \quad (3.6)$$

the stiffness ratios for the end regions i and j , the flexibility coefficients are given by Roufaiel (12).

$$\begin{aligned} f_{ii} &= \frac{1}{3(EI)_e L^2} [(Q_j - 1)x_j^3 - (Q_i - 1)(L - x_i)^3 + Q_i L^3] \\ f_{jj} &= \frac{1}{3(EI)_e L^2} [(Q_i - 1)x_i^3 - (Q_j - 1)(L - x_j)^3 + Q_j L^3] \\ f_{ij} &= \frac{1}{3(EI)_e L^2} \left[(Q_j - 1)x_j^2(1.5L - x_j) + (Q_i - 1)x_i^2(1.5L - x_i) + \frac{L^3}{2} \right] \end{aligned} \quad (3.7)$$

The corresponding stiffness coefficients follow as

$$\begin{aligned} k_{33} &= \frac{f_{jj}}{f_{ii}f_{jj} - f_{ij}^2} \\ k_{66} &= \frac{f_{ii}}{f_{ii}f_{jj} - f_{ij}^2} \\ k_{36} &= -\frac{f_{ij}}{f_{ii}f_{jj} - f_{ij}^2} \end{aligned} \quad (3.8)$$

The remaining coefficients follow from statics

$$\begin{aligned}
 k_{23} = -k_{35} &= \frac{(k_{33} + k_{36})}{L} \\
 k_{26} - k_{56} &= \frac{(k_{36} + k_{66})}{L} \\
 k_{22} = k_{55} = -k_{25} &= \frac{(k_{33} + 2 \times k_{36} + k_{66})}{L^2}
 \end{aligned} \tag{3.9}$$

The length x_i and stiffness ratio Q_i of the plastic region at node i depend on the current branch of the moment-curvature diagram. For elastic loading or unloading, we have

$$\begin{aligned}
 x_i &= 0.0 \\
 Q_i^1 &= 1.0
 \end{aligned} \tag{3.10}$$

For inelastic loading (see branch 2 in Fig 3.1), the length of the plastic region is determined by

$$x_i = \frac{M_i - M_y}{M_i + M_j} L \tag{3.11}$$

because bending moments are assumed to vary linearly along the beam length. The stiffness ratio within the plastic region is assumed to be constant over the length x_i and equal to the value at node i , i.e.

$$Q_i^2 = \frac{(EI)_e}{(EI)_2} \tag{3.12}$$

Upon inelastic unloading, x_i remains the maximum plastic region length reached in any previous inelastic loading cycle. But now the stiffness varies over the length of the plastic region, and for an accurate analysis, it would be necessary to compute the stiffnesses at all sections. This would be a time-consuming task and require considerable computer storage. In order to simplify this task, an empirical averaging process is used.

Directly at node i the stiffness has to be equal to $(EI)_3$, while at the border line between plastic and elastic regions it is $(EI)_e$. We approximate the variable

stiffness by an average value, assumed to be constant over the length of the plastic region and given by

$$(\overline{EI})_3 = (EI)_3 \frac{(EI)_e}{c(EI)_e + (1-c)(EI)_3} \quad (3.13)$$

where c is an empirical constant, for which values between 0.5 and 0.75 have been found to give most accurate results. In the present analysis a value of 0.5 is used.

The stiffness ratio for the plastic region at node i during inelastic unloading follows as

$$Q_i^3 = \frac{(EI)_e}{(\overline{EI})_3} = c \left(\frac{(EI)_e}{(EI)_3} - 1 \right) + 1 \quad (3.14)$$

Similarly, the stiffness ratios during inelastic reloading(branches 4 and 5 in Fig 3.1) are

$$\begin{aligned} Q_i^4 &= \frac{(EI)_e}{(\overline{EI})_4} = c \left(\frac{(EI)_e}{(EI)_4} - 1 \right) + 1 \\ Q_i^5 &= \frac{(EI)_e}{(\overline{EI})_5} = c \left(\frac{(EI)_e}{(EI)_5} - 1 \right) + 1 \end{aligned} \quad (3.15)$$

3.4 Nodal Damage Index

This damage index, D_e , quantifies the damage of a member section in a plastic hinge. It takes into consideration the nonlinear relationship between maximum displacement and dissipated energy, the strength deterioration rate and the number of load cycles to failure. The damage index is expressed in the form of a modified Miner's Rule. It contains damage modifiers, which reflect the effect of the loading history, and it considers the fact that RC members typically respond differently to positive and negative moments:

$$D_e = \sum_i \left(\alpha_i^+ \frac{n_i^+}{N_i^+} + \alpha_i^- \frac{n_i^-}{N_i^-} \right) \quad (3.16)$$

where

i : indicator of different displacement or curvature levels

$N_i = \frac{M_i - M_{fi}}{\Delta M_i}$: number of cycles up to curvature level i to cause failure

ΔM_i : strength drop in one load cycle up to curvature level i , Fig 3.3

n_i : number of cycles up to curvature level i actually applied

α_i : damage modifier

$+, -$: indicator of loading sense

$(M_i - M_{fi})$ denotes the total strength drop at curvature level i . The strength drop in a single load cycle up to curvature level i , ΔM_i (Fig 3.3), is given by

$$\Delta M_i = \left(\frac{\phi_i - \phi_f}{\phi_f - \phi_y} \right)^\omega \Delta M_f \quad (3.17)$$

The loading history effect is captured by including the damage modifier α_i , which, for positive moment loading, is defined as

$$\alpha_i^+ = \frac{\frac{1}{n_i^+} \sum_{j=1}^{n_i^+} k_{ij}^+}{\bar{k}_i^+} \cdot \frac{\phi_i^+ + \phi_{i-1}^+}{2\phi_i^+} \quad (3.18)$$

where

$$k_{ij}^+ = \frac{M_{ij}^+}{\phi_i^+} \quad (3.19)$$

is the stiffness during the j -th cycle up to load level i , Fig 3.4, and

$$\bar{k}_i^+ = \frac{1}{N_i^+} \sum_{j=1}^{N_i^+} k_{ij}^+ \quad (3.20)$$

is the average stiffness during N_i^+ cycles up to load level i . Denoting with

$$M_{ij}^+ = M_{i1}^+ - (j-1)\Delta M_i^+ \quad (3.21)$$

the moment reached after j cycles up to load level i , Fig 3.4, the damage modifier α_i^+ can be expressed as

$$\alpha_i^+ = \frac{M_{i1}^+ - \frac{1}{2}(n_i^+ - 1)\Delta M_i^+}{M_{i1}^+ - \frac{1}{2}(N_i^+ - 1)\Delta M_i^+} \cdot \frac{\phi_i^+ + \phi_{i-1}^+}{2\phi_i^+} \quad (3.22)$$

As Fig 3.4 illustrates, the energy that can be dissipated during a single cycle up to a given load level i decreases in successive cycles. That means the damage increments also decrease. In a constant-amplitude loading sequence, the first load cycle will cause more damage than the last one. As a result, the α_i -factor decreases as load cycling proceeds. This has been considered by incorporating the stiffness ratio into the damage modifier. The factor $\frac{\phi_i^+ + \phi_{i-1}^+}{2\phi_i^+}$ has been introduced to normalize the damage increments in the case of changing load amplitudes. For negative loading, “+” superscripts are replaced by “-” superscripts. For further details refer to Refs (4,6).

3.5 Structural Damage Index

A structural damage index can be composed of individual story damage indices (10), each of which is a weighted average of the damage indices of all potential plastic hinges in the story under consideration,

$$D_{S_k} = \frac{\sum_{i=1}^{n_k} D_i^k \cdot E_i^k}{\sum_{i=1}^{n_k} E_i^k} \quad (3.23)$$

where D_{S_k} and D_i^k denote the damage index for k -th story and for joint i in story k , respectively. n_k is the number of potential plastic hinges in the k -th story ($2 \times$ number of elements in story k). E_i^k is the energy dissipated in joint i of story k . Then, the structural damage index, D_g , can be defined as,

$$D_g = \sum_{k=1}^N D_{S_k} I_k \quad (3.24)$$

where N is the total number of stories. $I_k = \frac{N+1-k}{N}$ is the weighting factor for story k , which expresses the greater importance of the lower stories of a building ($I_k = 1$ for $k = 1$).

By combining the detailed damage information of an entire frame into a single number (D_g), too much information is lost to make this single structural damage index a useful estimator of the structure's residual strength and capacity to withstand further loading. However, for other purposes such as insurance risk evaluations this kind of single number may be of use. Any rational evaluation of a structure's reliability appears to be meaningful only if the mechanical deterioration process of all significant structural members are accurately accounted for.

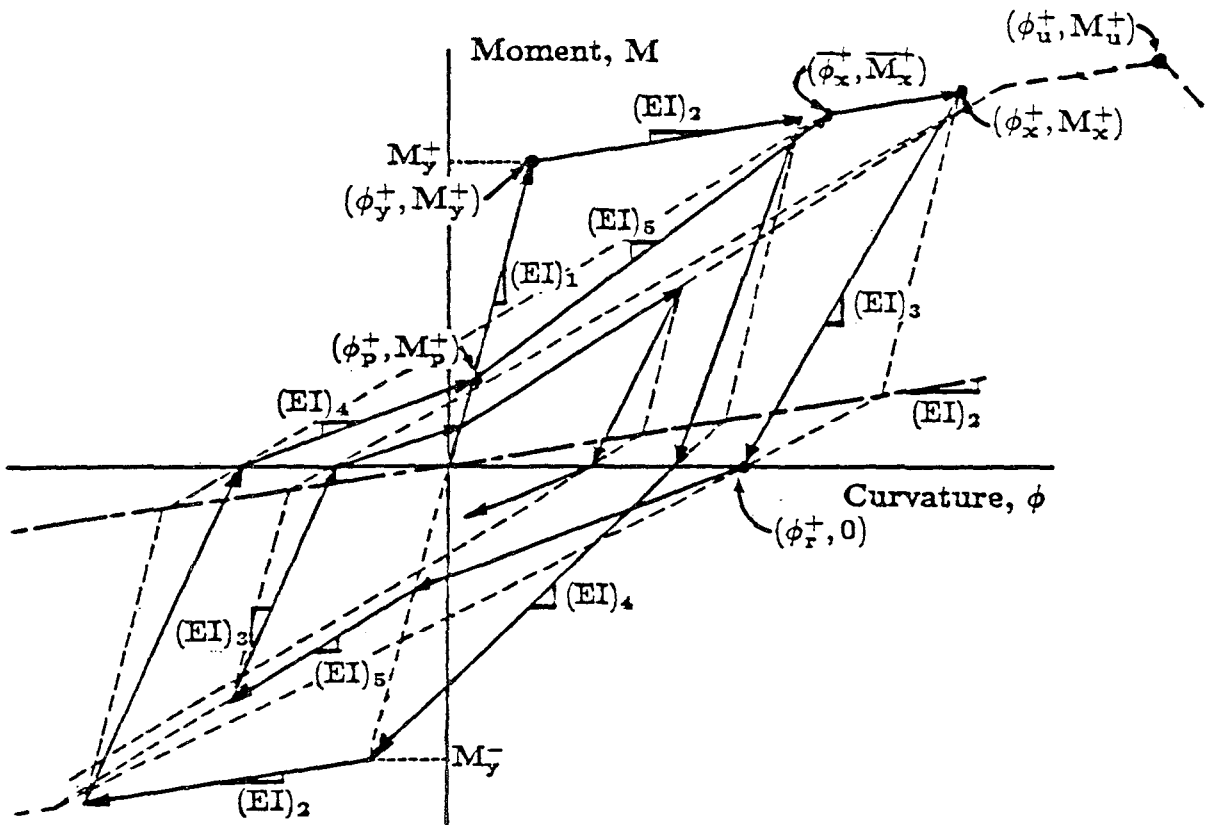


Fig. 3.1 - Typical Hysteretic Moment-Curvature Relationship

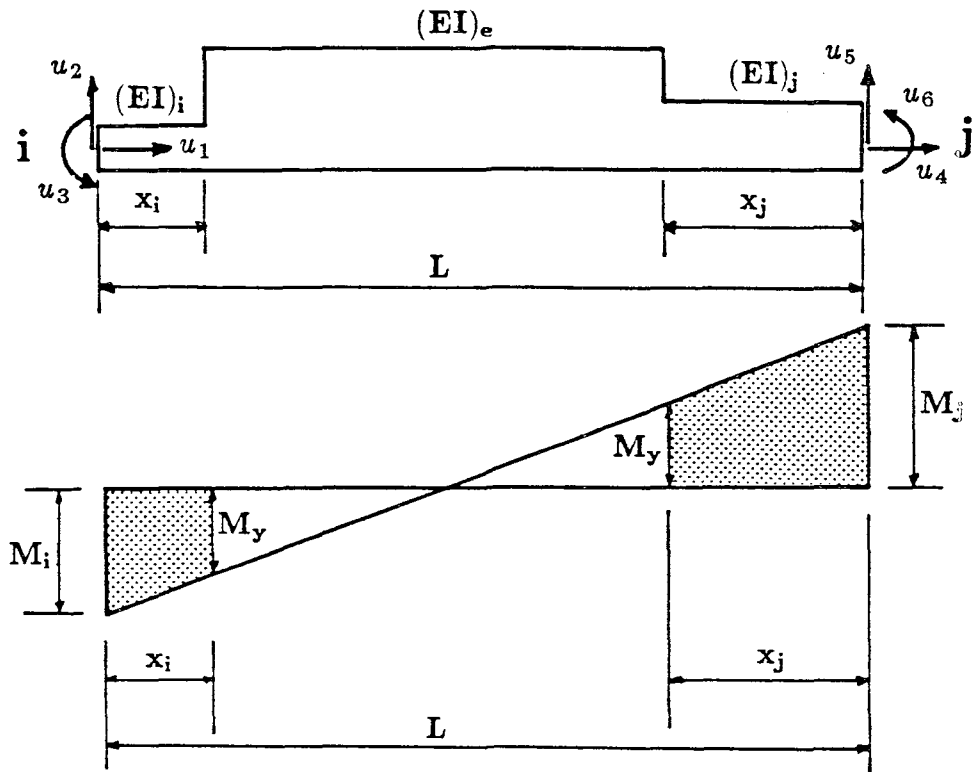


Fig. 3.2 - Member Size Model

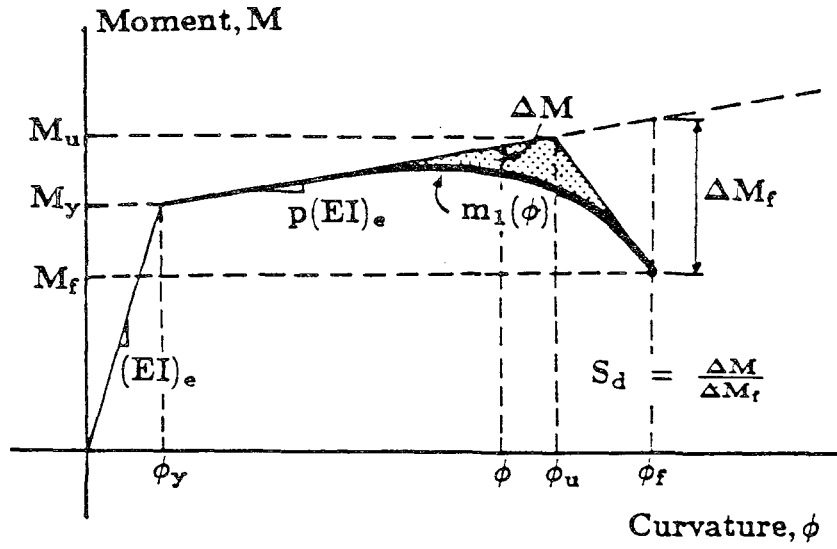


Fig. 3.3 - Strength Degradation Curve

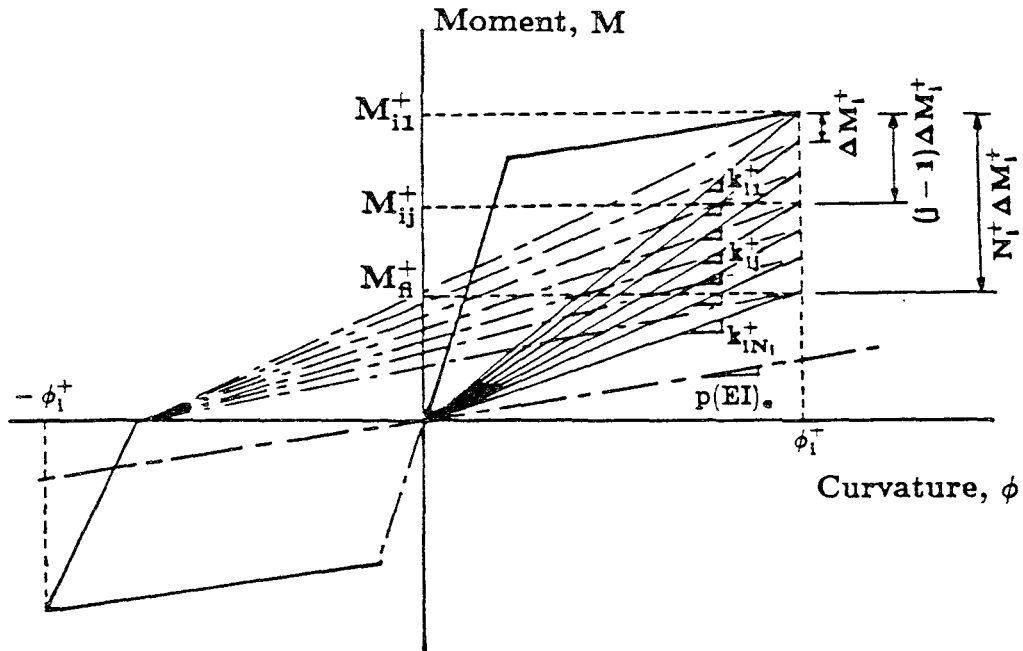


Fig. 3.4 - Strength Drop Due to Cyclic Loading

4. Automatic Design Procedure

The automatic design method of Fig 4.1 is based on a thorough study of the effects of three important design variables, the longitudinal reinforcement, the confinement steel, and member depth. Design rules derived from a large number of numerical studies allow an iterative improvement of a preliminary design until the distribution of damage indices has reached a user-specified degree of uniformity, Ref (6). The rationale for this design philosophy is that a structure, which is shown to dissipate energy uniformly in its main components, can be expected to survive an earthquake of given intensity with the least amount of damage possible.

The key components of the design procedure are, 1) an algorithm to evaluate the computed damage distribution by comparing it with user-specified acceptance criteria; 2) a set of design rules which permit the automatic modification of the structure such that improved performance is guaranteed.

The damage acceptance algorithm contains the following components:

- 1) Damage (and plastic hinges) in columns is unacceptable, as required by the strong-column weak-beam concept. Specifically, a column damage index greater than the user-specified allowable index shall be flagged as unacceptable in any column except at the base of the first story.
- 2) The mean value of all beam damage indices shall not exceed a user-specified acceptance level, allowing for a small prescribed tolerance.
- 3) The damage index of any beam element shall not deviate from the mean value computed for all beams by more than a user-specified allowance. Thus, individual beam elements may be flagged as having too much or too little damage.

If the damage index of at least one frame member is unacceptable, corrective action has to be taken, i.e. the design will have to be modified such that an improved

performance in a reanalysis is guaranteed and convergence towards an acceptable design is assured.

Structural designers normally rely on their experience when designing a structure to withstand seismic loads. They can fall back on both knowledge of rational principles of structural theory and intuition. The design task is complicated by the fact that a typical reinforced concrete frame is a highly redundant structure with intricate load-resistant mechanisms. In addition, the random nature of the earthquake loading makes the design task more difficult.

By performing numerous numerical parameter studies, we accumulated a store of experience with a certain regular building frame that structural engineers would possibly gain in years of practice. The rules summarized below, which are contained in program SARCF, are considered to form a useful starting point for an automatic design procedure.

- 1) For any beam element which showed an unacceptable level of damage in the preliminary analysis, the longitudinal steel will be increased (or decreased) by 5%,

$$\Delta A_s^1 = 0.05 \times A_s \times \text{SIGN}[D - D_{all}] \quad (4.1)$$

where A_s is the original amount of steel, D is the amount of damage determined in the preliminary analysis, and D_{all} is the allowable damage index of the beam element. The steel increments (reductions) of Eq (4.1) are only trial amounts introduced to determine in a first design iteration the influence of these changes.

- 2) In a subsequent design iteration "i", the amount of steel in any beam with unacceptable damage is changed according to,

$$\Delta A_s^i = \Delta A_s^{i-1} \times \frac{D^i - D_{all}}{D^{i-1} - D^i} \quad (4.2)$$

where ΔA_s^i denotes the additional(or deductible) amount of longitudinal steel

for the element in question, ΔA_s^{i-1} denotes the steel increased (or decreased) for the previous iteration, D^i and D^{i-1} represent damage values in the (i)th and (i-1)th iteration, respectively.

- 3) To adhere to the strong-column weak-beam concept, any column with unacceptable damage has to satisfy the requirement, $M_y^{col} \geq 1.25 \times M_y^{beam}$, where M_y^{col} is the yield moment of the column considered, and M_y^{beam} is the yield moment of the beam framing into the same joint. There are four categories of joints; 1) one beam and one column meeting at a joint, 2) one beam and two columns, 3) two beams and one column, in which case M_y^{beam} is the sum of the absolute value of the two beam yield moments, and 4) two beams and two columns. In the last case, M_y^{beam} is the average of the two beam yield moments. Then, the reinforcing steel of each column will be linearly increased (or decreased) by the amount,

$$\Delta A_s^i = \Delta A_s^{col} \times \frac{M_y^i - M_y^{i-1}}{\Delta M_y^{col}} \quad (4.3)$$

where the superscript indicates the iteration number. ΔM_y^{col} denotes the increment of the yield moment of the column when the longitudinal steel of the corresponding column is increased by ΔA_s^{col} . Since only one reinforcing steel area is used for the entire column, the more critical joint (top or bottom) controls.

- 4) At any section of an element, the longitudinal steel ratio ρ shall not be less than the minimum required by the ACI 318-83 Code (1), and shall not be greater than the maximum permitted by the ACI 318-83 Code (1), $\rho \leq \rho_{max} = \frac{3}{4}\rho_b + \rho'$ where ρ_b is the balanced steel and ρ' is the compression steel. For any element which does not satisfy the above steel requirements, the member depth will be

reduced(or increased) by:

$$\Delta d = \frac{A_s}{b \cdot (\rho_{min} - \rho)} \quad \left(\text{or } \Delta d = \frac{A_s}{b \cdot (\rho - \rho_{max})} \right) \quad (4.4)$$

where b is the width of the element section in question.

The steel ratios of all beams with unacceptable damage indices are either increased or decreased simultaneously. Thus, full use of the superposition principle is made. Design rule 4 is subjected to practical constraints, which at this time are not yet fully implemented. This design procedure assures that concentrations of heavy damage in some vulnerable structural members are avoided. Such local damage concentrations have led to many collapses in recent earthquakes. It is also felt that by keeping the damage in a frame uniform, an optimum response to an earthquake of given intensity is achieved.

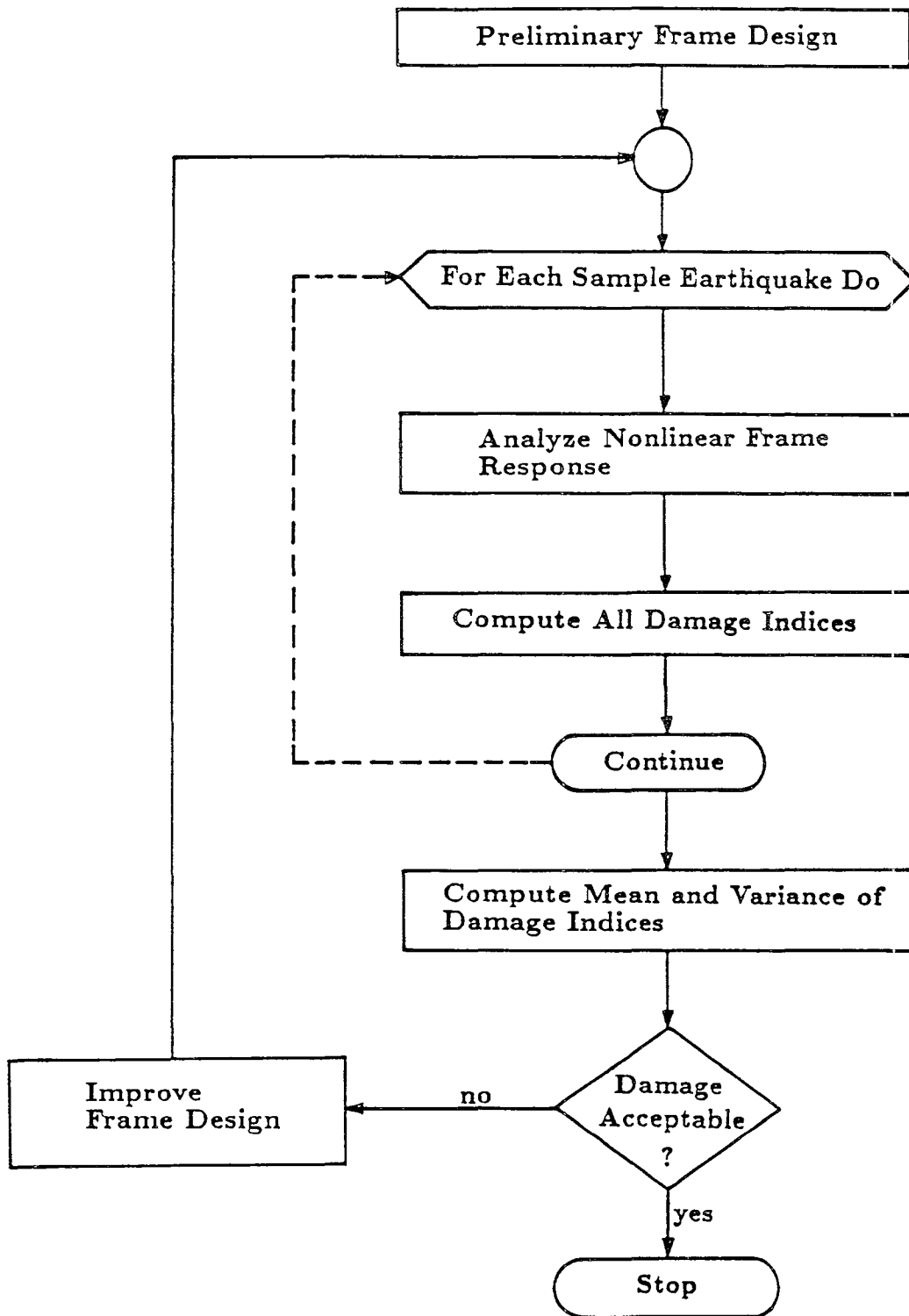


Fig. 4.1 - Automatic Design Method

5. Examples

This chapter presents four examples to demonstrate the usage of the different analysis and design options of program SARCF. Two examples utilize deterministically specified earthquake ground motions, whereas in the third example the ground accelerations are generated artificially, using an envelope function of the exponential type. In addition, one example of an automatic design is presented herein. Detailed input data and some basic output are included with further explanations.

5.1 Deterministic Analysis of Four-Story Three-Bay Frame

This example illustrates the input for one-half of a four-story and three-bay building frame, by making use of symmetry, Fig 5.1. The nonlinear response to the El Centro North-South earthquake is to be computed. In addition, the fundamental natural frequency and the damage indices are requested. The input data are listed in Table 5.1. The printout of the fundamental natural frequency and corresponding mode shape is listed in Table 5.3, and the maximum story displacements are plotted in Fig 5.3.

5.2 Deterministic Analysis of Irregular Frame

The input data for the building frame shown in Fig 5.2 are listed in Table 5.2. As in example 1, the nonlinear response, fundamental natural frequency and the damage indices are to be computed for the El Centro North-South earthquake. The output damage indices are listed in Table 5.4.

5.3 Random Vibration Analysis of Four-Story Three-Bay Frame

Input data for this example are almost the same as those for the first example, except that ten artificial earthquake ground motions are now to be generated. After the ten analyses, mean and standard deviation of beam damage indices will be

computed. The output damage information is given in Table 5.5 and plotted in Fig 5.4. The input data for this case are identical to those for example 2 of Table 5.1, except that lines 1 and 2 read now:

```

1      START      RANDOM VIBRATION ANALYSIS FOR FOUR STORY - THREE BAY FRAME
2          1      1      0      -1

```

and line 78 through line 147 are replaced by

```

78          0      0      12000.010      1.00      1.0      1.0      0.0      300.0
79          10      1      1.5 11.5 13.0 386.4000 28.2743 0.6 300.0 3.0 1.0
80          512      0      0      0 INPUT ARTIFICIAL GROUND MOTIONS
81          1      0
82          0      400 400 400
83      STOP

```

This input specifies the necessary information for the generation of random artificial earthquakes.

5.4 Automatic Design Example

The automatic design procedure is to be applied to the frame of Fig 5.1. Design iterations are to continue until the average damage values of all beams have reached user-specified values. The input data for this case are identical to those for example 3, except that lines 1 and 2 read now:

```

1      START      AUTOMATIC DESIGN ANALYSIS FOR FOUR STORY - THREE BAY FRAME
2          1      1      1      -1

```

and line 83 is replaced by

```

83          0      20      0.15      0.05      0.10      0.01
84      STOP

```

This input specifies a target mean value of 0.15 for the beam damage indices, with tolerance 0.05, and maximum deviation of 0.1. For columns, the maximum allowable damage index is specified to be 0.01. The damage indices obtained after 13 design iterations are plotted in Fig 5.5. A maximum of 20 iterations was allowed.

Table 5.1 – Input Data for Deterministic Analysis of Four-Story Three-Bay Frame

Line No.	Input Data														
1	START	HALF OF FOUR STORY - THREE BAY FRAME													
2	0	1	0	-1											
3	4	3	14	6	3	2	4	8	2	0	1	30000			
4	10.0		480.0												
5	2240.0		480.0												
6	3360.0		480.0												
7	12360.0		120.0												
8	130.0		0.0												
9	14240.0		0.0												
10	1	13	3	3											
11	2	14	3	3											
12	3	12	2	3											
13	13	1	1	1	14	1									
14	3	0	1	0	12	3									
15	1	3	1	2	3										
16	1	3	4	5	6										
17	1	3	7	8	9										
18	1	3	10	11	12										
19	10	0.067638	0.067638		0.0	0	0	1.							
20	11	0.135276	0.135276		0.0	0	0	1.							
21	7	0.070280	0.070280		0.0	0	0	1.							
22	8	0.140560	0.140560		0.0	0	0	1.							
23	4	0.068366	0.068366		0.0	0	0	1.							
24	5	0.136732	0.136732		0.0	0	0	1.							
25	1	0.074081	0.074081		0.0	0	0	1.							
26	2	0.148163	0.148163		0.0	0	0	1.							
27	0.075	0.004		0.0	0.0										
28	1	8	1	2	4	5	0	0							
29	1	29000.000		0.01	60.00			0.100							
30	1	4.0	0.0030	0.024											
31	2	4.0	0.0030	0.030											
32	-1	20.0	12.00	2.00	2.280	1.50									
33	-2	20.0	12.00	2.00	2.622	1.50									
34	-3	22.0	12.00	2.00	2.622	1.50									
35	-4	22.0	12.00	2.00	2.736	1.50									
36	1	7.5	-7.5	0.0	0.0										
37	2	7.5	-9.0	0.0	0.0										
38	3	9.0	-9.0	0.0	0.0										
39	4	7.5	0.0	0.0	0.0										
40	5	9.0	0.0	0.0	0.0										
41	1	1	2	0	1	1	1	1	0	0	0	0	0	0	0
42	-2	2	3	0	1	1	1	4	0	0	0	0	0	0	0
43	3	4	5	0	1	1	2	1	0	0	0	0	0	0	0
44	-4	5	6	0	1	1	2	4	0	0	0	0	0	0	0
45	5	7	8	0	2	1	3	2	0	0	0	0	0	0	0
46	-6	8	9	0	2	1	3	5	0	0	0	0	0	0	0
47	7	10	11	0	2	1	4	3	0	0	0	0	0	0	0
48	-8	11	12	0	2	1	4	5	0	0	0	0	0	0	0
49	2	8	1	3	4	4	8	0							
50	1	29000.000		0.01	60.00			0.100							
51	1	4.0	0.0030	0.012											
52	2	4.0	0.0030	0.010											
53	3	4.0	0.0030	0.008											
54	-1	15.0	12.00	1.50	1.800	1.50									
55	-2	15.0	12.00	1.50	2.850	1.50									
56	-3	18.0	15.00	1.875	2.850	1.50									
57	-4	18.0	15.00	1.875	2.964	1.50									
58	1	0.0	0.0	-11.0	0.0										
59	2	0.0	0.0	-11.0	11.0										
60	3	0.0	0.0	-10.0	11.0										
61	4	0.0	0.0	-10.0	10.0										
62	1	0	26.833	0.0	0.0	-26.833	0.0	0.0							
63	2	0	53.667	0.0	0.0	-53.667	0.0	0.0							
64	3	0	53.250	0.0	0.0	-53.250	0.0	0.0							
65	4	0	106.500	0.0	0.0	-106.500	0.0	0.0							

66	5	0	80.250		0.0	0.0	-80.250		0.0	0.0							
67	6	0	160.500		0.0	0.0	-160.500		0.0	0.0							
68	7	0	107.563		0.0	0.0	-107.563		0.0	0.0							
69	8	0	215.125		0.0	0.0	-215.125		0.0	0.0							
70	1	1	4	0	1	1	4	0	0	0	1	0	1.0	0.0	0	0	0.0
71	2	2	5	0	1	1	4	0	0	0	2	0	1.0	0.0	0	0	0.0
72	3	4	7	0	1	1	3	0	0	0	3	0	1.0	0.0	0	0	0.0
73	4	5	8	0	2	1	2	3	0	0	0	4	0	1.0	0.0	0	0.0
74	5	7	10	0	2	1	2	2	0	0	0	5	0	1.0	0.0	0	0.0
75	6	8	11	0	3	1	3	2	0	0	0	6	0	1.0	0.0	0	0.0
76	7	10	13	0	3	1	3	1	0	0	0	7	0	1.0	0.0	0	0.0
77	8	11	14	0	3	1	4	1	0	0	0	8	0	1.0	0.0	0	0.0
78	0	0	7680.010			38.64	1.0		1.0		0.0	300.0					
79	384	0	0	0	EL CENTRO NORTH-SOUTH EARTHQUAKES												
80	0.000	0.000	0.020	-0.014	0.040	-0.110	0.060	-0.103	0.080	-0.090	0.100	-0.097					
81	0.120	-0.122	0.140	-0.145	0.160	-0.130	0.180	-0.112	0.200	-0.087	0.220	-0.087					
82	0.240	-0.133	0.260	-0.179	0.280	-0.198	0.300	-0.165	0.320	-0.147	0.340	-0.110					
83	0.360	-0.084	0.380	-0.043	0.400	-0.067	0.420	-0.133	0.440	-0.194	0.460	-0.200					
84	0.480	-0.067	0.500	0.031	0.520	0.144	0.540	-0.050	0.560	-0.130	0.580	-0.147					
85	0.600	-0.207	0.620	-0.265	0.640	-0.331	0.660	-0.312	0.680	-0.175	0.700	-0.201					
86	0.720	-0.166	0.740	-0.167	0.760	-0.068	0.780	0.026	0.800	0.153	0.820	0.240					
87	0.840	0.257	0.860	0.342	0.880	0.472	0.900	0.501	0.920	0.427	0.940	0.366					
88	0.960	0.276	0.980	0.239	1.000	0.345	1.020	0.420	1.040	0.540	1.060	0.651					
89	1.080	0.746	1.100	0.664	1.120	0.610	1.140	0.408	1.160	0.408	1.180	0.064					
90	1.200	-0.525	1.220	-0.802	1.240	-0.614	1.260	-0.493	1.280	-0.255	1.300	-0.060					
91	1.320	0.137	1.340	0.314	1.360	0.508	1.380	0.723	1.400	1.014	1.420	1.242					
92	1.440	1.558	1.460	1.476	1.480	1.177	1.500	0.953	1.520	0.909	1.540	0.943					
93	1.560	0.855	1.580	0.918	1.600	1.012	1.620	1.232	1.640	0.334	1.660	-1.503					
94	1.680	-2.105	1.700	-2.027	1.720	-2.072	1.740	-1.850	1.760	-1.758	1.780	-1.785					
95	1.800	-1.786	1.820	-1.839	1.840	-1.661	1.860	-1.372	1.880	-1.108	1.900	-0.797					
96	1.920	-0.437	1.940	-0.017	1.960	0.367	1.980	0.800	2.000	1.186	2.020	1.628					
97	2.040	1.997	2.060	2.458	2.080	2.781	2.100	3.093	2.120	3.260	2.140	3.482					
98	2.160	2.874	2.180	2.368	2.200	-1.221	2.220	-2.418	2.240	-1.671	2.260	-1.900					
99	2.280	-1.116	2.300	-0.767	2.320	-0.176	2.340	0.115	2.360	0.543	2.380	0.912					
100	2.400	1.208	2.420	1.790	2.440	0.587	2.460	-2.681	2.480	-1.576	2.500	-1.762					
101	2.520	-1.031	2.540	-0.590	2.560	0.241	2.580	-0.683	2.600	-2.017	2.620	-1.672					
102	2.640	-1.717	2.660	-1.509	2.680	-1.254	2.700	-1.020	2.720	-0.765	2.740	-0.533					
103	2.760	-0.276	2.780	-0.045	2.800	0.192	2.820	-0.097	2.840	-0.441	2.860	-0.854					
104	2.880	-0.969	2.900	-0.730	2.920	-0.610	2.940	-0.340	2.960	-0.110	2.980	0.188					
	*	*	*	*	*	*	*	*	*	*	*	*					
	*	*	*	*	*	*	*	*	*	*	*	*					
	*	*	*	*	*	*	*	*	*	*	*	*					
130	6.000	0.590	6.020	0.260	6.040	-0.042	6.060	-0.436	6.080	-0.136	6.100	0.097					
131	6.120	0.234	6.140	-0.131	6.160	-0.051	6.180	0.082	6.200	0.214	6.220	0.387					
132	6.240	0.520	6.260	0.160	6.280	-0.033	6.300	-0.113	6.320	0.005	6.340	0.077					
133	6.360	0.036	6.380	-0.097	6.400	-0.037	6.420	-0.016	6.440	0.039	6.460	0.087					
134	6.480	-0.057	6.500	-0.310	6.520	-0.429	6.540	-0.249	6.560	-0.240	6.580	-0.180					
135	6.600	-0.131	6.620	-0.018	6.640	0.207	6.660	-0.110	6.680	-0.093	6.700	-0.035					
136	6.720	-0.108	6.740	-0.113	6.760	-0.101	6.780	-0.002	6.800	0.074	6.820	0.239					
137	6.840	0.362	6.860	0.718	6.880	0.794	6.900	0.187	6.920	-0.268	6.940	-0.126					
138	6.960	-0.043	6.980	0.162	7.000	0.049	7.020	-0.223	7.040	-0.476	7.060	-0.436					
139	7.080	-0.220	7.100	-0.044	7.120	0.162	7.140	0.326	7.160	0.427	7.180	0.125					
140	7.200	-0.163	7.220	-0.208	7.240	-0.084	7.260	-0.210	7.280	-0.140	7.300	-0.056					
141	7.320	0.054	7.340	0.137	7.360	0.271	7.380	0.236	7.400	0.081	7.420	-0.008					
142	7.440	0.204	7.460	0.443	7.480	0.501	7.500	0.195	7.520	0.094	7.540	-0.022					
143	7.560	-0.021	7.580	0.053	7.600	0.095	7.620	0.260	7.640	0.375	7.660	0.535					
144	1	0															
145	0	400	400	400													
146	0	0	0	0	0	0	0	0	0	0	0	0					
147	STOP																

Table 5.2 – Input Data for Deterministic Analysis of Irregular Frame

Line No.	Input Data														
1	START	NONSYMMERIC FOUR-STORY FRAME													
2	0	1	0	-1											
3	4	0	19	10	4	1	4	7	2	0	0	30000			
4	4	3	2	1											
5	10.		480.												
6	2240.		480.												
7	30.		360.												
8	5480.		360.												
9	60.		240.												
10	9720.		240.												
11	100.		120.												
12	14960.		120.												
13	150.		0.												
14	19960.		0.												
15	3	5	1	1											
16	6	9	2	1											
17	10	14	3	1											
18	15	19	3	1											
19	15	1	1	1	19	1									
20	1	2	1	2											
21	1	3	3	4	5										
22	1	4	6	7	8	9									
23	1	5	10	11	12	13	14								
24	1	0.072733	0.072733				0.0	2	0	1.					
25	3	0.069094	0.069094				0.0	5	2	1.					
26	4	0.138188	0.138188				0.0	0	0	1.					
27	6	0.071251	0.071251				0.0	9	3	1.					
28	7	0.142501	0.142501				0.0	8	0	1.					
29	10	0.068474	0.068474				0.0	14	4	1.					
30	11	0.136948	0.136948				0.0	13	1	1.					
31	0.075	0.004			0.0		0.0								
32	1	10	1	2	4	2	0	0							
33	1	29000.000			0.01	60.00		0.100							
34	1	4.0	0.0030	0.024											
35	2	4.0	0.0030	0.030											
36	-1	18.0	12.00	2.00			1.596	1.50							
37	-2	20.0	12.00	2.00			2.400	1.50							
38	-3	22.0	12.00	2.00			2.622	1.50							
39	-4	22.0	12.00	2.00			2.736	1.50							
40	1	7.5	-7.5	0.0			0.0								
41	2	9.0	-9.0	0.0			0.0								
42	1	1	2	0	1	1	1	0	0	0	0	0	0	0	0
43	2	3	4	0	1	1	2	0	0	0	0	0	0	0	0
44	3	4	5	0	1	1	2	0	0	0	0	0	0	0	0
45	4	6	7	0	2	1	3	0	0	0	0	0	0	0	0
46	5	7	8	0	2	1	3	0	0	0	0	0	0	0	0
47	6	8	9	0	2	1	3	0	0	0	0	0	0	0	0
48	7	10	11	0	2	1	4	0	0	0	0	0	0	0	0
49	8	11	12	0	2	1	4	0	0	0	0	0	0	0	0
50	9	12	13	0	2	1	4	0	0	0	0	0	0	0	0
51	10	13	14	0	2	1	4	0	0	0	0	0	0	0	0
52	2	14	1	3	4	4	7	0							
53	1	29000.000			0.01	60.00		0.100							
54	1	4.0	0.0030	0.012											
55	2	4.0	0.0030	0.010											
56	3	4.0	0.0030	0.008											
57	-1	15.0	12.00	1.50			2.160	1.50							
58	-2	18.0	12.00	1.875			2.993	1.50							
59	-3	18.0	15.00	1.875			3.135	1.50							
60	-4	18.0	15.00	1.875			3.260	1.50							
61	1	0.0	0.0	-11.0			0.0								
62	2	0.0	0.0	-11.0			11.0								
63	3	0.0	0.0	-10.0			11.0								
64	4	0.0	0.0	-9.00			10.0								
65	1	0	26.250		0.0	0.0	-26.250		0.0	0.0					

66	2	0	52.792		0.0		0.0	-52.792		0.0	0.0	
67	3	0	105.583		0.0		0.0	-105.583		0.0	0.0	
68	4	0	80.229		0.0		0.0	-80.229		0.0	0.0	
69	5	0	160.458		0.0		0.0	-160.458		0.0	0.0	
70	6	0	107.854		0.0		0.0	-107.854		0.0	0.0	
71	7	0	215.708		0.0		0.0	-215.708		0.0	0.0	
72	1	1	3	0	1	1	1	4	0	0	0	
73	2	2	4	0	1	1	1	4	0	0	0	
74	3	3	6	0	1	1	2	3	0	0	0	
75	4	4	7	0	2	1	3	3	0	0	0	
76	5	5	8	0	1	1	2	3	0	0	0	
77	6	6	10	0	2	1	3	2	0	0	0	
78	7	7	11	0	2	1	4	2	0	0	0	
79	8	8	12	0	2	1	4	2	0	0	0	
80	9	9	13	0	2	1	3	2	0	0	0	
81	10	10	15	0	2	1	3	1	0	0	0	
82	11	11	16	0	3	1	4	1	0	0	0	
83	12	12	17	0	3	1	4	1	0	0	0	
84	13	13	18	0	3	1	4	1	0	0	0	
85	14	14	19	0	2	1	3	1	0	0	0	
86	0	0										
87	384	0	0	0	7680.010		38.64	1.0	1.0	0.0	300.0	
EL CENTRO NORTH-SOUTH EARTHQUAKES												
88	0.000	0.000	0.020	-0.014	0.040	-0.110	0.060	-0.103	0.080	-0.090	0.100	-0.097
89	0.120	-0.122	0.140	-0.145	0.160	-0.130	0.180	-0.112	0.200	-0.087	0.220	-0.087
90	0.240	-0.133	0.260	-0.179	0.280	-0.198	0.300	-0.165	0.320	-0.147	0.340	-0.110
91	0.360	-0.084	0.380	-0.043	0.400	-0.067	0.420	-0.133	0.440	-0.194	0.460	-0.200
92	0.480	-0.067	0.500	0.031	0.520	0.144	0.540	-0.050	0.560	-0.130	0.580	-0.147
93	0.600	-0.207	0.620	-0.265	0.640	-0.331	0.660	-0.312	0.680	-0.175	0.700	-0.201
94	0.720	-0.166	0.740	-0.167	0.760	-0.068	0.780	0.026	0.800	0.153	0.820	0.240
95	0.840	0.257	0.860	0.342	0.880	0.472	0.900	0.501	0.920	0.427	0.940	0.366
96	0.960	0.276	0.980	0.239	1.000	0.345	1.020	0.420	1.040	0.540	1.060	0.651
97	1.080	0.746	1.100	0.664	1.120	0.610	1.140	0.408	1.160	0.408	1.180	0.064
98	1.200	-0.525	1.220	-0.802	1.240	-0.614	1.260	-0.493	1.280	-0.255	1.300	-0.060
99	1.320	0.137	1.340	0.314	1.360	0.508	1.380	0.723	1.400	1.014	1.420	1.242
100	1.440	1.558	1.460	1.476	1.480	1.177	1.500	0.953	1.520	0.909	1.540	0.943
101	1.560	0.855	1.580	0.918	1.600	1.012	1.620	1.232	1.640	0.334	1.660	-1.503
102	1.680	-2.105	1.700	-2.027	1.720	-2.072	1.740	-1.850	1.760	-1.758	1.780	-1.785
103	1.800	-1.786	1.820	-1.839	1.840	-1.661	1.860	-1.372	1.880	-1.108	1.900	-0.797
104	1.920	-0.437	1.940	-0.017	1.960	0.367	1.980	0.800	2.000	1.186	2.020	1.628
105	2.040	1.997	2.060	2.458	2.080	2.781	2.100	3.093	2.120	3.260	2.140	3.482
106	2.160	2.874	2.180	2.368	2.200	-1.221	2.220	-2.418	2.240	-1.671	2.260	-1.900
	*	*	*	*	*	*	*	*	*	*	*	*
	*	*	*	*	*	*	*	*	*	*	*	*
	*	*	*	*	*	*	*	*	*	*	*	*
138	6.000	0.590	6.020	0.260	6.040	-0.042	6.060	-0.436	6.080	-0.136	6.100	0.097
139	6.120	0.234	6.140	-0.131	6.160	-0.051	6.180	0.082	6.200	0.214	6.220	0.387
140	6.240	0.520	6.260	0.160	6.280	-0.033	6.300	-0.113	6.320	0.005	6.340	0.077
141	6.360	0.036	6.380	-0.097	6.400	-0.037	6.420	-0.016	6.440	0.039	6.460	0.087
142	6.480	-0.057	6.500	-0.310	6.520	-0.429	6.540	-0.249	6.560	-0.240	6.580	-0.180
143	6.600	-0.131	6.620	-0.018	6.640	0.207	6.660	-0.110	6.680	-0.093	6.700	-0.035
144	6.720	-0.108	6.740	-0.113	6.760	-0.101	6.780	-0.002	6.800	0.074	6.820	0.239
145	6.840	0.362	6.860	0.718	6.880	0.794	6.900	0.187	6.920	-0.268	6.940	-0.126
146	6.960	-0.043	6.980	0.162	7.000	0.049	7.020	-0.223	7.040	-0.476	7.060	-0.436
147	7.080	-0.220	7.100	-0.044	7.120	0.162	7.140	0.326	7.160	0.427	7.180	0.125
148	7.200	-0.163	7.220	-0.208	7.240	-0.084	7.260	-0.210	7.280	-0.140	7.300	-0.056
149	7.320	0.054	7.340	0.137	7.360	0.271	7.380	0.236	7.400	0.081	7.420	-0.008
150	7.440	0.204	7.460	0.443	7.480	0.501	7.500	0.195	7.520	0.094	7.540	-0.022
151	7.560	-0.021	7.580	0.053	7.600	0.095	7.620	0.260	7.640	0.375	7.660	0.535
152	1	0										
153	0	400	400	400								
154	0	0	0	0	0	0	0	0	0	0	0	0
155	STOP											

Table 5.3 — Example 1, Output for Fundamental Natural Frequency and Mode Shape

FIRST NATUARAL FREQUENCY = 0.11419E+01 (Hz)

FIRST MODE SHAPE:

NODE	X	Y	R
1	0.1000E+01	0.7361E-02	-0.9242E-03
2	0.1000E+01	-0.5171E-03	-0.5822E-03
3	0.1000E+01	0.0000E+00	0.3562E-03
4	0.7871E+00	0.6929E-02	-0.1621E-02
5	0.7871E+00	-0.4371E-03	-0.1326E-02
6	0.7871E+00	0.0000E+00	0.8012E-03
7	0.4948E+00	0.5548E-02	-0.1700E-02
8	0.4948E+00	-0.2984E-03	-0.1579E-02
9	0.4948E+00	0.0000E+00	0.9858E-03
10	0.2125E+00	0.3095E-02	-0.1829E-02
11	0.2125E+00	-0.1970E-03	-0.1536E-02
12	0.2125E+00	0.0000E+00	0.9574E-03
13	0.0000E+00	0.0000E+00	0.0000E+00
14	0.0000E+00	0.0000E+00	0.0000E+00

Table 5.4 – Example 2, Output for Damage Indices

NODAL DAMAGE INDICES TIME = 7.690

DAMAGE INDICES FOR BEAM ELEMENT : GROUP 1

ELEMENT NO.	NODE NO	NODE NO	DAMAGE INDICES	
			AT I	AT J
1	1	0.0160	2	0.0150
2	3	0.0069	4	0.0046
3	4	0.0109	5	0.0118
4	6	0.0046	7	0.0036
5	7	0.0033	8	0.0032
6	8	0.0110	9	0.0069
7	10	0.0067	11	0.0008
8	11	0.0007	12	0.0007
9	12	0.0007	13	0.0007
10	13	0.0004	14	0.0003

DAMAGE INDICES FOR BEAM-COLUMN ELEMENT : GROUP 2

ELEMENT NO.	NODE NO	NODE NO	DAMAGE INDICES	
			AT I	AT J
1	1	0.0000	3	0.0154
2	2	0.0000	4	0.0371
3	3	0.0000	6	0.0000
4	4	0.0000	7	0.0076
5	5	0.0000	8	0.0136
6	6	0.0000	10	0.0000
7	7	0.0000	11	0.0000
8	8	0.0000	12	0.0000
9	9	0.0000	13	0.0155
10	10	0.0000	15	0.0095
11	11	0.0000	16	0.0109
12	12	0.0000	17	0.0143
13	13	0.0000	18	0.0234
14	14	0.0000	19	0.0408

*** STORY DAMAGE INDEX ***

FLOOR	BEAM	COLUMN	STORY
4	0.01553	0.02908	0.02400
3	0.00883	0.01061	0.00956
2	0.00587	0.01509	0.00831
1	0.00290	0.02156	0.01866

GLOBAL DAMAGE = 0.03567

Table 5.5 — Example 3, Output of Statistics of All Element Damage Indices

STATISTICS FOR DAMAGE INDICES

BEAM ELEMENTS GROUP 1

ELEM NO.	NODE NO	NODE NO	NO. OF EARTHQUAKE	MEAN VALUE		STAND DEV		VARIANCE		MAXIMUM		MINIMUM	
				AT I	AT J	AT I	AT J	AT I	AT J	AT I	AT J	AT I	AT J
1	1	2	10	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	2	3	10	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
3	4	5	10	0.0231	0.0012	0.0171	0.0015	0.0003	0.0000	0.0608	0.0048	0.0004	0.0000
4	5	6	10	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
5	7	8	10	0.0987	0.0727	0.0422	0.0380	0.0018	0.0014	0.1682	0.1268	0.0277	0.0238
6	8	9	10	0.1895	0.0000	0.1144	0.0000	0.0131	0.0000	0.4071	0.0000	0.0305	0.0000
7	10	11	10	0.1880	0.1557	0.0924	0.0837	0.0085	0.0070	0.4012	0.3637	0.0688	0.0614
8	11	12	10	0.2812	0.0000	0.1516	0.0000	0.0230	0.0000	0.5057	0.0000	0.0934	0.0000

BEAM-COLUMN ELEMENTS GROUP 2

ELEM NO.	NODE NO	NODE NO	NO. OF EARTHQUAKE	MEAN VALUE		STAND DEV		VARIANCE		MAXIMUM		MINIMUM	
				AT I	AT J	AT I	AT J	AT I	AT J	AT I	AT J	AT I	AT J
1	1	4	10	0.0546	0.0215	0.0516	0.0271	0.0027	0.0007	0.1376	0.0826	0.0000	0.0000
2	2	5	10	0.1443	0.1042	0.0989	0.1103	0.0098	0.0122	0.3083	0.3637	0.0298	0.0004
3	4	7	10	0.0803	0.0169	0.0561	0.0186	0.0032	0.0003	0.1956	0.0550	0.0152	0.0000
4	5	8	10	0.1652	0.0390	0.0796	0.0417	0.0063	0.0017	0.2933	0.1465	0.0545	0.0049
5	7	10	10	0.0066	0.0000	0.0141	0.0000	0.0002	0.0000	0.0480	0.0000	0.0000	0.0000
6	8	11	10	0.0562	0.0136	0.0374	0.0156	0.0014	0.0002	0.1140	0.0532	0.0035	0.0000
7	10	13	10	0.0000	0.1793	0.0000	0.0895	0.0000	0.0080	0.0000	0.3489	0.0000	0.0652
8	11	14	10	0.0004	0.1592	0.0011	0.0742	0.0000	0.0055	0.0036	0.3212	0.0000	0.0674

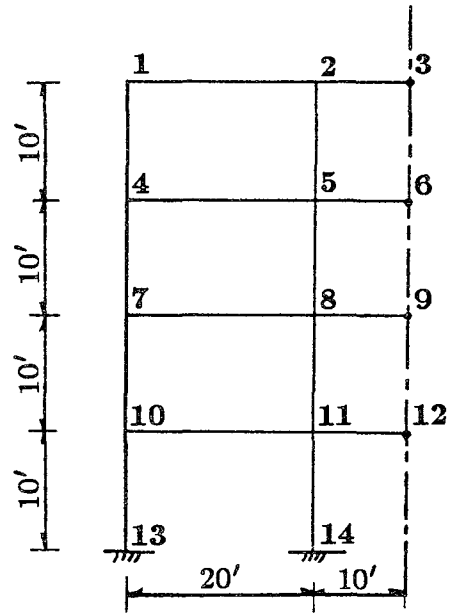


Fig. 5.1 – Half of Four-Story Three-Bay Building Frame

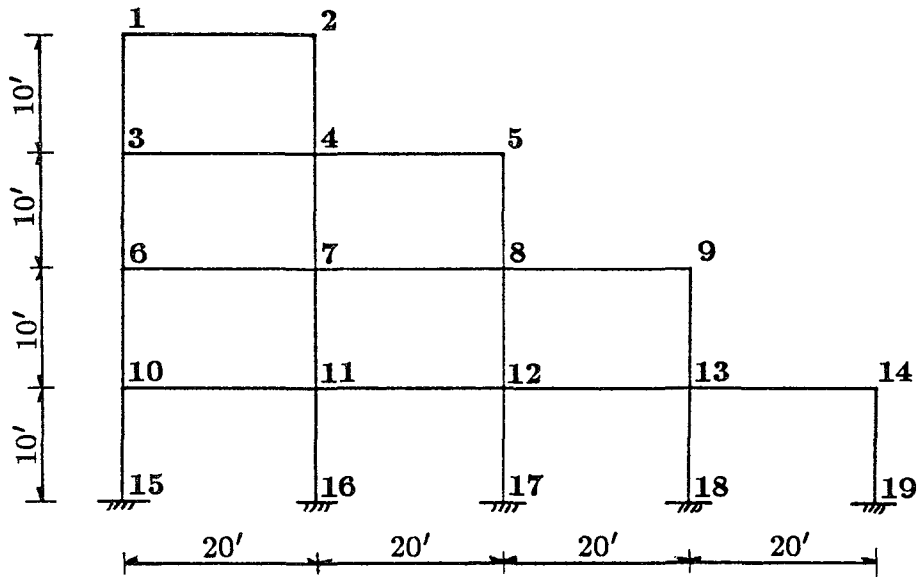


Fig. 5.2 – Nonsymmetric Building Frame

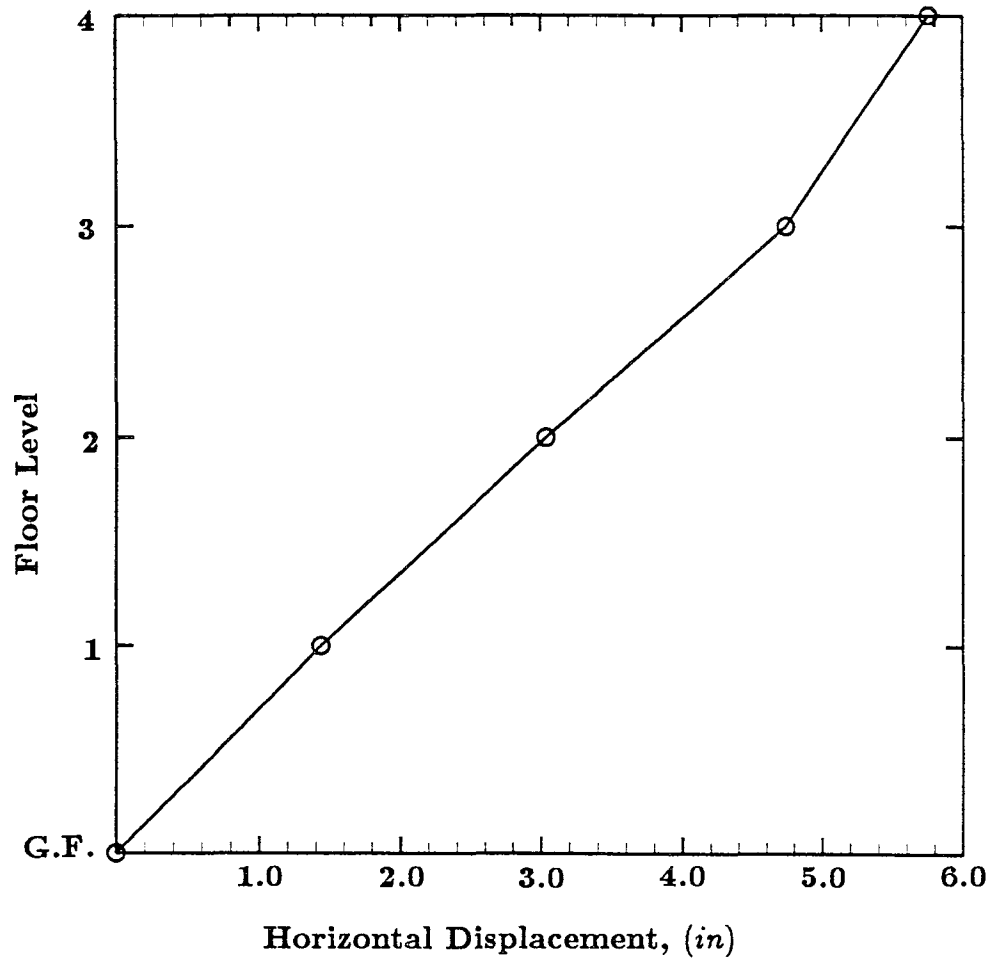


Fig. 5.3 – Maximum Horizontal Floor Displacements

	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
0.0215	0.0546		0.1042	0.1443		0.0546
	0.0231	0.0012	0.0000	0.0000	0.0012	0.0231
0.0169	0.0803		0.0390	0.1652		0.0803
	0.0987	0.0727	0.1895	0.1895	0.0727	0.0987
0.0000	0.0066		0.0136	0.0562		0.0066
	0.1880	0.1557	0.2812	0.2812	0.1557	0.1880
0.1793	0.0000		0.1592	0.0004		0.0000

Mean Damage of All Beams : $\bar{D} = 0.084$

Standard Deviation of All Beams : $\sigma_D = 0.094$

Fig. 5.4 - Mean Damage Indices for Example Office Building

0.0796	0.0684	0.1114	0.1114	0.0684	0.0796
0.0000	0.0000	0.0003	0.0000	0.0003	0.0000
0.0667	0.0532	0.1616	0.1616	0.0532	0.0667
0.0000	0.0000	0.0000	0.0008	0.0000	0.0000
0.1089	0.0917	0.2055	0.2056	0.0917	0.1089
0.0000	0.0000	0.0039	0.0054	0.0039	0.0000
0.1613	0.1206	0.2119	0.2119	0.1206	0.1613
0.0000	0.0000	0.0025	0.0025	0.0025	0.0000
0.1557		0.1517		0.1517	0.1557

Mean Damage of All Beams : $\bar{D} = 0.120$

Standard Deviation of All Beams : $\sigma_D = 0.051$

Fig. 5.5 – Mean Damage Indices for Example Office Building
After 13 Design Iterations

6. References

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Appendix – A
SARCF User's Guide

Appendix A – SARCF User's Guide

The purpose of program SARCF (“Seismic Analysis of Reinforced Concrete Frames”) is to compute nonlinear responses of reinforced concrete frames subject to deterministic and/or randomly generated earthquake ground motions, including expected damage values, with an option to perform automatic design iterations until a user-specified damage distribution has been achieved.

This program has been written in Fortran-77 for VAX/VMS computer systems and for the Sun3 micro-computer system. It is a derivative of DRAIN-2D, a general purpose computer program for dynamic analysis of inelastic plane structures, with various enhancements. At the present, the program can handle only reinforced concrete frames made up of beam and beam-column elements. Input data are entered in a batch mode consisting of 7 items arranged in the following sequence.

- 1) “START” card and analysis control data.
- 2) Structure information.
- 3) Element information.
- 4) Load information.
 - * Static load information.
 - * Earthquake data, either deterministic or randomly generated.
- 5) Analysis information.
 - * Eigenvalue information (optional).
 - * Damage index information (optional).
 - * Automatic design procedure (optional).
- 6) Output specifications.
- 7) “STOP” card.

Static loads may be applied to the structure prior to the application of the dynamic loading, but the response to such static loads must remain elastic.

The present program version makes limited use of fixed dimension statements, so that some important input variables are subject to upper limits. These restrictions are clearly indicated in the input specifications below. However, because of the use of PARAMETER statements, it is relatively easy to relax any one of these capacity restrictions if necessary.

1. Description of Problem

1.1 START Card (A5,3X,18A4)

Provide a single card with the following information:

Columns 1 - 5 : Enter the word "START".

6 - 80 : Designate the title of this problem.

1.2 Analysis Control Data (4I5)

Provide a single card with the following control data:

Columns 1 - 5 : Code for type of earthquake data ("KEARTH").

1 : for randomly generated earthquake data.

0 : for deterministic earthquake data.

6 - 10 : Code for damage index ("KDAMAGE").

1 : compute damage indices.

0 : do not compute damage indices.

11 - 15 : Code for an automatic design analysis. ("KAUTO")

1 : perform automatic design.

0 : do not perform automatic design.

16 - 20 : Data checking code ("KDATA"). This code specifies two items of information: 1) whether to perform a complete analysis or only a data check run; 2) whether to store all element data in core or on a scratch file with the result of increased peripheral processing cost.

1 : data check run only.

0 : complete analysis execution, with element data stored on a

scratch file.

-1 : complete analysis execution, with element data stored in core.

2. Structure Information

All data necessary to describe the structure are to be supplied in the order and format as described below. Some data have to be input specifically, while others will default to previously defined values. Consistent units have to be used throughout. If the automatic design option is exercised, then only U.S. customary units (foot, pound and kips) are permitted.

2.1 Structural Geometry Control Card (11I5,I10)

Columns 1 - 5 : Number of stories (“NSTORY”).

6 - 10 : Number of bays (“NBAY”).

If the number of bays is the same for each story, enter this number here. If it is variable, enter zero here and specify the numbers of bays in Section 2.2 below.

11 - 15 : Number of nodes (“NJTS”).

(e.g. $(NBAY+1) \times NSTORY$)

16 - 20 : Number of control nodes, of which x and y coordinates are to be specified (“NCONJT”). See Section 2.3.A.

21 - 25 : Number of node generation commands (“NCDJT”).

See Section 2.3.B.

26 - 30 : Number of zero displacements commands (“NCDDOF”).

See Section 2.4.

31 - 35 : Number of identical displacements commands (“NCDDIS”).

See Section 2.5.

36 - 40 : Number of lumped mass commands (“NCDMS”).

See Section 2.6.

- 41 - 45 : Number of different element groups in structure (“NELGR”).
- 46 - 50 : Structure stiffness storage code (“KODST”). A duplicate structure stiffness matrix is always retained and periodically updated and stored in core, if sufficient memory is available, else it is stored on a scratch file. Whether the stiffness duplicate can fit into core or not can be determined in a data check run (KDATA = 1, see Section 1.2) by setting KODST = 0.
- 0 : store stiffness duplicate in core.
- 1 : store stiffness duplicate on scratch file.
- 51 - 55 : Symmetry option code (“KSYM”).
- 1 : only left half of structure is modeled.
- 0 : no use of symmetry is made.
- 1 : only right half of structure is modeled.
- 56 - 65 : Blank COMMON length to be allocated.
- Enter the number of double-precision words. The length of blank COMMON to be allocated depends on the size of the problem and is difficult to compute by hand. This important information is provided in a data check run (KDATA = 1, Section 1.2). The current program defaults to COMMON A(50000). If this memory allocation turns out to be insufficient, the main program of SARCF has to be recompiled with an appropriately increased COMMON allocation.

2.2 Number of Bays (16I5)

Omit if the structure has the same number of bays in each story, i.e. if a non-zero value for NBAY was entered in Section 2.1. If the number of bays varies, enter for each story the actual number of bays, starting with the ground story and preceding to the top.

If the number of stories exceeds 16, use two or more cards, as needed.

2.3 Node Generation Cards

The node generation cards allow the omission of input data for frames which exhibit some regularity. For example, if all story heights are equal, it suffices to specify the coordinates of only the top and bottom nodes (defined as “control nodes”) and to prompt the automatic generation of the coordinates for all nodes inbetween. Note that all control nodes are to be defined first, one node per card, followed by all node generation commands, with one command per card.

Node numbers can be assigned in any arbitrary sequence. However, if use of the automatic design option is made, nodes have to be numbered sequentially, starting from the top story as shown in Fig A.1. If no use of the node generation option is made, enter all nodes as “control nodes”.

2.3.A Control Node Cards (I5,2F10.0)

Columns 1 - 5 : Node number.

6 - 15 : X coordinate of node.

16 - 25 : Y coordinate of node.

2.3.B Node Generation Commands(4I5)

Omit if NCDJT = 0. (See Section 2.1)

Columns 1 - 5 : First node number in the line of nodes.

6 - 10 : Last node number in the line of nodes.

11 - 15 : Number of nodes to be generated along the line.

16 - 20 : Node number increment between any two successive nodes.

Default value = 1.

2.4 Zero Displacements Commands (6I5)

These commands allow the specification of a series of nodes having identical boundary

conditions, identified by the code

1 : For fixed boundary condition.

0 : For free boundary condition.

Enter NCDDOF cards, with one command per card. See Section 2.1.

Columns 1 - 5 : First node number in series.

6 - 10 : Code for X displacements.

11 - 15 : Code for Y displacements.

16 - 20 : Code for rotations.

21 - 25 : Last node number in series. Leave blank for a single node.

26 - 30 : Node number increment between any two successive nodes in series.

Default = 1.

2.5 Identical Displacements Commands (16I5)

One command for each card. Omit if NCDDIS = 0. See Section 2.1.

Columns 1 - 5 : Displacement code:

1 : For X displacement.

2 : For Y displacement.

3 : For rotation.

6 - 10 : Number of nodes having identical displacement (Maximum =14).

11 - 15 : First node.

16 - 20 : Second node etc.

List up to 14 nodes in this card. If there are more than 14 nodes with identical displacement, two or more commands will be used, with the nodes in increasing order in each command. The smallest node number has to appear on each command card.

2.6 Lumped Mass Commands (I5,3F10.0,2I5,F10.0)

One command for each card. Omit if NCDMS = 0. See Section 2.1.

Columns 1 - 5 : First node number in series.

6 - 15 : Mass associated with X displacement.

16 - 25 : Mass associated with Y displacement.

26 - 35 : Rotary inertia.

36 - 40 : Last node number in series. Leave blank for a single node.

41 - 45 : Node number difference between any two successive nodes in series.

Default = 1.

46 - 55 : Scale factor by which input masses are to be divided ("SCALE").

Default value is the one specified in the preceding command, so that the same factor applies to all subsequent commands until it changes again. Thus, it needs to be specified at least for the first command. If masses are input as weights, enter the gravity constant for SCALE. For example, a 100 *kip* weight (or $\frac{100}{386.4} = 0.2588$ *k - sec²/in* mass) may be input as a mass "100.", with scale factor "386.4".

2.7 Damping Information (4F10.0)

Four different types of damping may be specified singly or in combination.

Columns 1 - 10 : Mass proportional damping factor, α .

11 - 20 : Tangent stiffness proportional damping factor, β .

21 - 30 : Original stiffness proportional damping factor, β_o .

31 - 40 : Structural damping factor, δ .

Note : Use of structural damping may be problematic, especially for inelastic structures. A possible cause is that the damping forces tend to accentuate small oscillations in numerical computations. From past experience, the following

values give realistic results: $\alpha = 0.07$, $\beta = 0.004$, $\beta_o = 0$ and $\delta = 0$.

3. Element Information

Only frame elements with or without axial force are incorporated in the current program version. That is, there are two different element types, beam-columns and beams. All elements of a frame must be divided into groups. All elements in any one group must be of the same type, and typically all elements of the same type will be included in a single group. However, elements of the same type may be subdivided into more than one group if desired. The number of groups, NELGR, was specified in Section 2.1.

If the automated design option is not exercised, element groups may be input in any convenient sequence. Otherwise, the beam element group has to be input before the beam-column element group. In any case, the elements within a group must be numbered in sequence. In addition, in the automatic design option, elements are to be sequentially numbered from the top story as shown in Fig A.1.

Each group needs all the following data.

3.1 Group Control Information (8I5)

Columns 1 - 5 : Group type number.

1 : for beam element.

2 : for beam-column element.

6 - 10 : Number of elements in this group ("NMEM").

11 - 15 : Number of different reinforcing steel types ("NSTL").

See Section 3.2.

16 - 20 : Number of different concrete types ("NCON").

See Section 3.3.

21 - 25 : Number of different cross section types ("NSEC").

See Section 3.4.

26 - 30 : Number of different end eccentricity types (“NECC”).

See Section 3.5.

31 - 35 : Number of different fixed-end forces patterns (“NFEF”).

See Section 3.6.

36 - 40 : Number of different initial element force patterns (“NINT”).

See Section 3.7.

3.2 Reinforcing Steel Types (I5,F15.4,F10.4,F10.2,F10.5)

Supply NSTL cards (see Section 3.1), one for each different reinforcing steel. See Fig A.2 for definitions. Assign each type a number, starting with 1, up to maximum 6.

Columns 1 - 5 : Type number.

6 - 20 : Young’s modulus, E_s .

21 - 30 : Strain hardening ratio, as a fraction of Young’s modulus, P_s .

31 - 40 : Yield stress, f_{sy} .

41 - 50 : Ultimate strain, ϵ_{su} .

3.3 Concrete Types (I5,3F10.4)

Supply NCON cards (see Section 3.1), one for each different concrete type. See Fig A.3 for definitions. Assign each type a number, starting with 1, up to maximum 9.

Columns 1 - 5 : Type number.

6 - 15 : Uniaxial concrete strength, f'_c .

16 - 25 : Strain at maximum stress, ϵ_o .

26 - 35 : Confinement steel ratio, ρ'' .

3.4 Cross Section Types (I5,4F10.4,F5.2,3F10.4)

Supply NSEC cards (see Section 3.1), one for each different cross section. See Fig A.4 for definitions. Assign each type a number, starting with 1, up to maximum 9. Input negative type number for the section which is symmetrical about horizontal axis.

Columns 1 - 5 : Type number (Negative for symmetrical section).

6 - 15 : Height of cross section (“HT”).

16 - 25 : Bottom width of cross section (“BB”).

26 - 35 : Distance from the bottom face to the centroid of bottom reinforcing steel (“DCB”).

36 - 45 : Area of bottom reinforcing steel (“ASB”).

46 - 50 : Strength degradation parameter, ω .

This parameter depends on various factors, such as the longitudinal steel ratio, the confinement ratio, the axial force. Values between 1.5 and 2.0 have been found to lead to realistic results.

51 - 60 : Top width of cross section (“BT”).

Leave blank or zero for symmetrical section.

61 - 70 : Distance from the top face to the centroid of top reinforcing steel (“DCT”).

Leave blank or zero for symmetrical section.

71 - 80 : Area of top reinforcing steel (“AST”).

Leave blank or zero for symmetrical section.

3.5 End Eccentricities (I5,4F10.4)

Plastic hinges may form near the faces of a connection rather than inside a beam-column joint. This behavior can be modeled with rigid links connecting nodes with the respective element ends, as shown in Fig A.5.

Supply NECC cards (see Section 3.1), one for each different kind of eccentricity with which members are attached to a node. Omit if NECC = 0. All eccentricities are measured from the node to the element end. Assign each different eccentricity type a number, starting with 1, up to maximum 15.

Columns 1 - 5 : Type number.

- 6 - 15 : $X_i = X$ eccentricity at end i.
- 16 - 25 : $X_j = X$ eccentricity at end j.
- 26 - 35 : $Y_i = Y$ eccentricity at end i.
- 36 - 45 : $Y_j = Y$ eccentricity at end j.

3.6 Fixed-End Force Patterns (2I5,7F10.0)

Static loads applied along the lengths of beams and beam-column elements may be taken into account by specifying fixed-end forces as shown in Fig A.6. These forces are those which must act on the element ends to prevent end displacements. The sign convention for these forces is as shown in Fig A.6.

Supply NFEF cards (see Section 3.1), one for each different fixed-end force pattern. Omit if NFEF = 0. Assign each different fixed-end force pattern a number, starting with 1, up to maximum 35.

Columns 1 - 5 : Pattern number.

6 - 10 : Coordinate system code.

0 : Forces refer to element coordinate system, (Fig A.6.a).

1 : Forces refer to global coordinate system, (Fig A.6.b).

11 - 20 : Fixed end force, F_{x_i} .

21 - 30 : Fixed end force, F_{y_i} .

31 - 40 : Fixed end moment, M_i .

41 - 50 : Fixed end force, F_{x_j} .

51 - 60 : Fixed end force, F_{y_j} .

61 - 70 : Fixed end moment, M_j .

71 - 80 : Live load reduction factor. The fixed-end forces specified for each element may account for the live load reduction as permitted, e.g. by the Uniform Building Code for members with large tributary areas. For dead loads, however, this reduction factor is ignored.

3.7 Initial Element Force Patterns (I5,6F10.0)

For structures for which static analyses are carried out separately, initial member forces such as those due to prestress may be specified by use of initial element force patterns. These forces are converted internally to nodal loads, using the same sign convention as indicated for fixed-end force patterns. The geometric stiffness, if used, is based on the initial axial force plus any axial force due to static loading, and may be included for the dynamic loading, if required.

Supply NINT cards (see Section 3.1), one for each different initial element force pattern. Omit if $NINT = 0$. Assign each different initial element force pattern a number, starting with 1, up to maximum 30.

- Columns 1 - 5 : Pattern number.
- 6 - 15 : Initial axial force, F_{x_i} .
- 16 - 25 : Initial shear force, F_{y_i} .
- 26 - 35 : Initial moment, M_i .
- 36 - 45 : Initial axial force, F_{x_j} .
- 46 - 55 : Initial shear force, F_{y_j} .
- 56 - 65 : Initial moment, M_j .

3.8 Element Generation Commands (8I5,5I4,2F5.0,I5,F5.0)

For structures with similar elements, the program can automatically generate data for repetitive elements. Provided all data for a sequence of elements are identical (except node numbers), only two cards, one for the first and one for the last element in the sequence (the "key elements") need to be provided. In the printout of the element data, generated elements are identified by an asterisk at the beginning of the printed line.

Assign a sequential number for all the elements in the same group, starting with 1, up to NMEM (See Section 3.1). Supply one card for each key element in increasing numerical order of the assigned element number.

- Columns 1 - 5 : Element number. If KSYM is not equal to zero, input a negative element number for the beam element, which is located at the symmetrical axis. For example, input -2 , -4 and -6 for element No. ②, ④ and ⑥ in Figs. A.1.b) or A.1.c), respectively.
- 6 - 10 : Node number at element end i.
- 11 - 15 : Node number at element end j.
- 16 - 20 : Node number increment for element generation.
Default = 1.
- 21 - 25 : Concrete type number.
- 26 - 30 : Steel type number.
- 31 - 35 : Cross section type number.
- 36 - 40 : End eccentricity type number. Leave blank or input zero if there is no end eccentricity.
- 41 - 44 : Geometric stiffness code.
1 : include geometric stiffness.
0 : ignore geometric stiffness.
- 45 - 48 : Time history output code. If a time history of element results is not required for the element covered by this command, input zero or leave blank. If a time history printout, at the intervals specified in Section 6.1, is required, input 1.
- 49 - 52 : Code for the output of hysteretic curve. If hysteretic response information for this element is not required, input zero or leave blank. If such information is required, input node number at element end “i” or “j”, of this element.
- 53 - 56 : Fixed-end force pattern number for static dead loads on element.
Leave blank or input zero if there are no dead loads.
- 57 - 60 : Fixed-end force pattern number for static live loads on element.

Leave blank or input zero if there are no live loads.

61 - 65 : Scale factor to be applied to fixed-end forces due to static dead loads.

66 - 70 : Scale factor to be applied to fixed-end forces due to static live loads.

71 - 75 : Initial force pattern number. Leave blank or input zero if there are no initial forces.

76 - 80 : Scale factor to be applied to initial element forces.

4. Load Information

Static loads may be applied to the structure prior to the application of the dynamic loading, but the response to static load must remain elastic. For a deterministic analysis, ground acceleration data are to be input in the format described in Section 4.4.B. If random earthquake data are to be generated, only the data described in Section 4.1 are to be entered.

4.1 Load Control Data (2I5,1I10,6F10.0)

Columns 1 - 5 : Static load code ("KSTAT").

1 : Static loads are to be applied prior to dynamic loads.

0 : No static loads are to be included in the analysis.

6 - 10 : Number of commands specifying static loads applied directly at the nodes ("NCDLD"). See Section 4.2.

Leave blank or input zero if there are no static loads.

11 - 20 : Number of integration time steps to be considered in the dynamic analysis ("NSTEPS").

21 - 30 : Integration time step, Δt ("DT").

31 - 40 : Scale factor to be applied to the ground X-accelerations ("FACAXH").

- 41 - 50 : Scale factor to be applied to the time coordinates of the X-acceleration record (“FACAMH”).
- 51 - 60 : Scale factor to be applied to the ground Y-accelerations (“FACAXV”).
- 61 - 70 : Scale factor to be applied to the time coordinates of the Y-acceleration record (“FACAMV”).
- 71 - 80 : Absolute value of the maximum displacement response permitted (“DISMAX”). The specification of such a displacement limit presumes that exceedance of this limit corresponds to failure, at which point the execution is terminated. Default = 10^5 .

4.2 Commands for Static Nodal Loads (I5,3F10.0,2I5)

These commands allow the specification of a series of loads having the same static nodal loads with the sign convention of Fig A.6. Omit if there are no static loads applied directly at nodes. One command for each card.

- Columns 1 - 5 : First node number in series.
- 6 - 15 : Load in X direction.
- 16 - 25 : Load in Y direction.
- 26 - 35 : Moment.
- 36 - 40 : Last node number in series. Leave blank or zero for a single node.
- 41 - 45 : Node number difference between any two successive nodes in series. Default = 1.

Note : A loaded node may appear in two or more commands if desired, for example, if it is a part of two series. In such a case, the total load applied at the node will be the sum of the load from the separate commands.

4.3 Data for Randomly Generated Earthquakes (2I5,3F5.2,2F10.4,F5.4,2F5.0)

Omit this card if KEARTH = 0 (See Section 1.2), i.e. for a deterministic analysis, and

proceed to Section 4.4.

Columns 1 - 5 : Number of artificial earthquakes to be generated (“NEAR”).

6 - 10 : Code for envelope function type (See Fig 2.1).

1 : for trapezoidal envelope.

2 : for exponential envelope.

11 - 15 : Initial peak time, t_1 , for the trapezoidal envelope function;
coefficient α , for the exponential envelope function.

16 - 20 : Last peak time, t_2 , for the trapezoidal envelope function;
coefficient β , for the exponential envelope function.

21 - 25 : Strong motion duration, t_3 , for the trapezoidal envelope function,
but leave blank or input zero for the exponential envelope function.

26 - 35 : Intensity factor for the input spectrum, S_o . For this program, the
one-sided Kanai-Tajimi spectrum is used, Fig. A.7.

36 - 45 : Characteristic dominant frequency, ω_g .

46 - 50 : Characteristic dominant damping ratio, τ_g .

51 - 55 : Upper cut-off frequency, ω_u .

56 - 60 : Peak factor for the earthquake simulation, p_g .

Note : For firm soil conditions, the following parameter values are recommended: for
 $1g$ peak acceleration data, $S_o = 0.6378(ft^2/sec^3)$; $\omega_g = 9\pi(rad/sec)$; $\tau_g = 0.6$;
 $\omega_u = 300(rad/sec)$ for one-sided Kanai-Tajimi spectrum, and $p_g = 3.0$. For $0.1g$
peak acceleration earthquake, only the S_o value changes to $0.006378(ft^2/sec^3)$.
For further information see Ref. 13.

4.4 Deterministic Acceleration Records

Omit this set of data if KEARTH = 1, i.e. for randomly generated earthquakes.

4.4.A Control Information (4I5,10A4)

Columns 1 - 5 : Number of time-acceleration pairs defining ground motion in X di-

rection (NPTH). Input zero or leave blank for no ground motion in this direction.

6 - 10 : Number of time-acceleration pairs defining ground motion in Y direction (NPTV). Input zero or leave blank for no ground motion in this direction.

11 - 15 : Code for echo printing accelerations as input. Leave blank or zero for no output.

1 : print.

0 : do not print.

16 - 20 : Code for echo printing accelerations as interpolated at intervals of Δt .

1 : print.

0 : do not print.

21 - 60 : Title to identify acceleration record.

4.4.B Ground Acceleration Time History in X-Direction (6(F6.3,F7.3))

Omit if NPTH = 0. Otherwise, enter 6 pairs of time and acceleration records per card. The first time-acceleration pair has to be (0.0,0.0). Note that both the accelerations and time coordinates may be scaled if desired. See Section 4.1.

4.4.C Ground Acceleration Time History in Y-Direction (6(F6.3,F7.3))

Omit if NPTV = 0. Otherwise, enter 6 pairs of time and acceleration records per card. The first time-acceleration pair has to be (0.0,0.0). Note that both the accelerations and time coordinates may be scaled if desired. See Section 4.1.

5. Analysis Information

5.1 Control Information for Eigenvalue Analysis(2I5)

Columns 1 - 5 : Code for natural frequencies.

1 : compute natural frequencies at specified time intervals.

0 : do not compute natural frequencies.

6 - 10 : Time intervals, at which natural frequencies are to be computed, expressed as a multiple of the time step, Δt .

5.2 Control Information for Damage Indices (4I5)

Omit this card if KDAMAGE = 0 in Section 1.2. Otherwise, all nodal damage indices as well as global and story damage indices may be obtained at selected time intervals. If KDAMAGE = 1, all damage indices will be automatically computed at the end of the time history analysis.

Columns 1 - 5 : Code for time history of damage index.

1 : compute and print time history of damage indices.

0 : do not compute.

6 - 10 : Time interval for nodal damage indices to be computed, expressed as a multiple of the time step, Δt .

11 - 15 : Time interval for story damage indices to be computed, expressed as a multiple of the time step, Δt .

16 - 20 : Time interval for structural damage indices to be computed, expressed as a multiple of the time step, Δt .

5.3 Data for Automatic Design Procedure (5X,1I5,4F10.5)

Omit this card if KAUTO = 0 in Section 1.2.

Columns 6 - 10 : Maximum number of automatic design iterations.

11 - 20 : Target mean value of beam damage indices.

21 - 30 : Tolerance by which the actual mean may deviate from the target mean value.

31 - 40 : Maximum tolerable deviation of individual beam damage indices from the actual mean value.

41 - 50 : Allowable damage index for beam-columns.

6. Time History Output Specifications

Omit all the cards for this section if KEARTH = 1, i.e. for randomly generated earthquake data. However, envelope values of all nodal displacements and element results are automatically printed at the end of the computation for each randomly generated earthquake, except if the specified maximum displacement has been exceeded.

For the deterministic earthquake data, i.e. KEARTH = 0, printed time histories of selected nodal displacements and element results at selected time intervals may be obtained if desired. Similarly, envelope values of all nodal displacements and element results are printed at the end of the computation if the specified maximum displacement was not exceeded. Intermediate result envelopes are also printed at selected time intervals.

6.1 Control Information (6I5)

Columns 1 - 5 : Time interval for printout of nodal displacement time histories, expressed as a multiple of a time step Δt . Leave blank for no printout. The nodes for which time histories are required are specified in Sections 6.2, 6.3 and 6.4.

6 - 10 : Time interval for printout of time histories of element results, expressed as a multiple of the time step Δt . Leave blank for no printout. The elements for which time histories are required are specified in Section 3.8.

11 - 15 : Time interval for intermediate printout of envelope values, expressed as a multiple of the time step Δt . Leave blank for no intermediate printout. Envelope values are automatically printed at the end of the response period.

16 - 20 : Number of nodes (NHOUT) for which X displacement time histories are required.

21 - 25 : Number of nodes (NVOUT) for which Y displacement time histories

are required.

26 - 30 : Number of nodes (NROUT) for which rotation time histories are required.

6.2 List of Nodes for X-Displacement Time Histories (10I5)

As many cards as needed to specify NHOUT node numbers, with up to 10 nodes per card. Omit if NHOUT = 0.

6.3 List of Nodes for Y-Displacement Time Histories (10I5)

As many cards as needed to specify NVOUT node numbers, with up to 10 nodes per card. Omit if NVOUT = 0.

6.4 List of Nodes for Rotation Time Histories (10I5)

As many cards as needed to specify NROUT node numbers, with up to 10 nodes per card. Omit if NROUT = 0.

7. Termination

One card (A4) to terminate the complete data.

Columns 1 - 4 : Print the word "STOP".

8. Installation and Execution

SARCF is written in Fortran-77 language for VAX/VMS computer systems. All the calculations need to be performed in double precision. Otherwise, truncation errors would cause excessive errors in the solution and numerical instabilities. This program has also been installed with relatively little additional effort on SUN3 micro-computer systems. It can be installed on most small or large computers as well.

The SARCF source consists of about 6000 statements, listed in Appendix B, and is organized in a number of "base" subroutines. These subroutines read and print the structure and loading data, assemble the structure stiffness and loading, compute the displacement histories of the structure, eigenvalues, the statistics of damage indices and perform automatic design modifications of a preliminary frame design. It is in particular noted that SARCF calls some IMSL subroutines to generate random earthquakes (Ref. 7). Because the IMSL library is proprietary, these subroutines are not listed in the Appendix B. These subroutines are

FFTCC, GGUD and GGUBFS

and are called in lines 1022, 1052 and 1082 of the listing in Appendix B.

A typical procedure to execute SARCF on the VAX 780 under a VMS system or to execute the program on the SUN3 micro-computer, is listed below:

<u>For VAX 780</u>	<u>For SUN3</u>
\$ assign <u>Datafile</u> for\$read	% f77 -ffpa -O -o sarcf.exe sarcf.f
\$ assign <u>Outputfile</u> for\$print	% sarcf.exe < <u>Datafile</u> > <u>Outputfile</u>
\$ for sarcf	
\$ lin sarcf	
\$ run sarcf	

After executing SARCF, the following output or scratch files will be generated on the

for002.dat : scratch file for element information.

for007.dat : scratch file for element information only if the automatic design is required.

for008.dat : scratch file for time history of horizontal or vertical displacement if required (see Section 6.2 and 6.3).

for009.dat : scratch file for time history of rotational displacement if required (see Section 6.4).

for012.dat : scratch file for element stiffness information only if natural frequencies are required.

for013.dat : scratch file for element lumped mass information only if natural frequencies are required.

for016.dat : output file of the hysteretic curve information only for the element required (see Section 3.8).

for020.dat : output file of time history of damage indices if damage analysis is required.

for033.dat : output file of mean value of all the element damage indices only if random earthquake analysis is required.

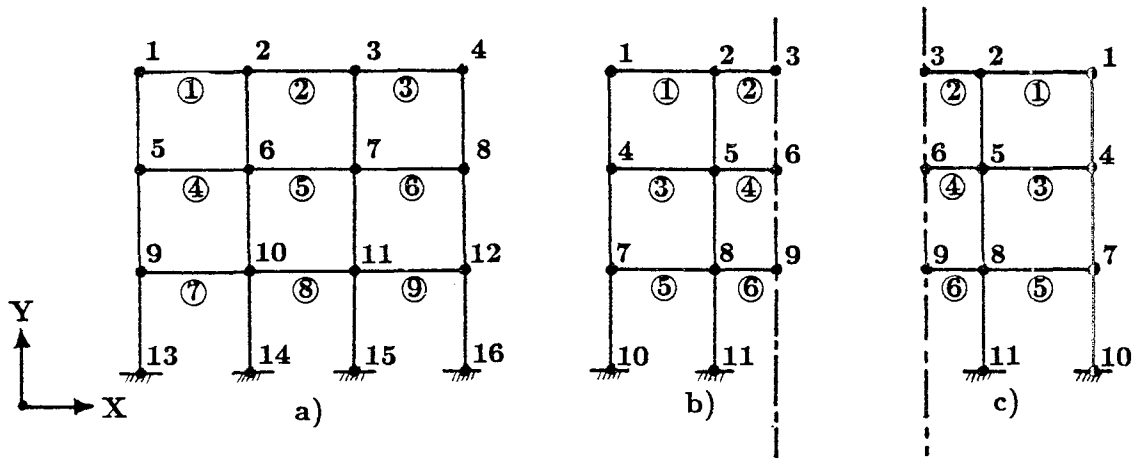


Fig. A.1 – Node and Beam Element Numbering Sequence
(Mandatory if the Automatic Design Option is Required.)

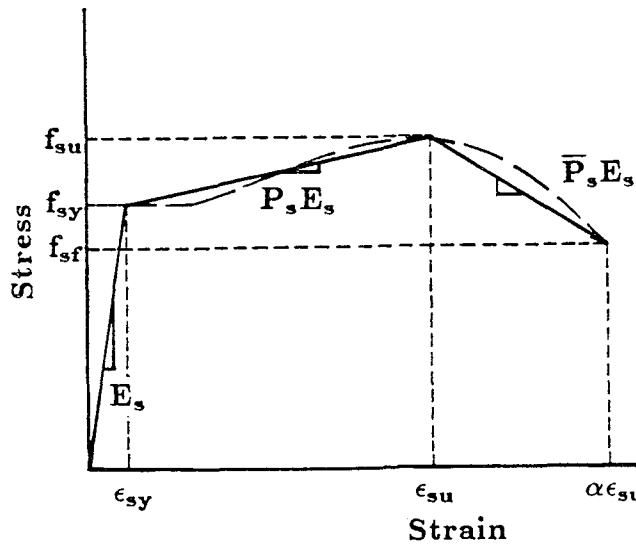


Fig. A.2 – Stress-Strain Curve for Reinforcing Steel

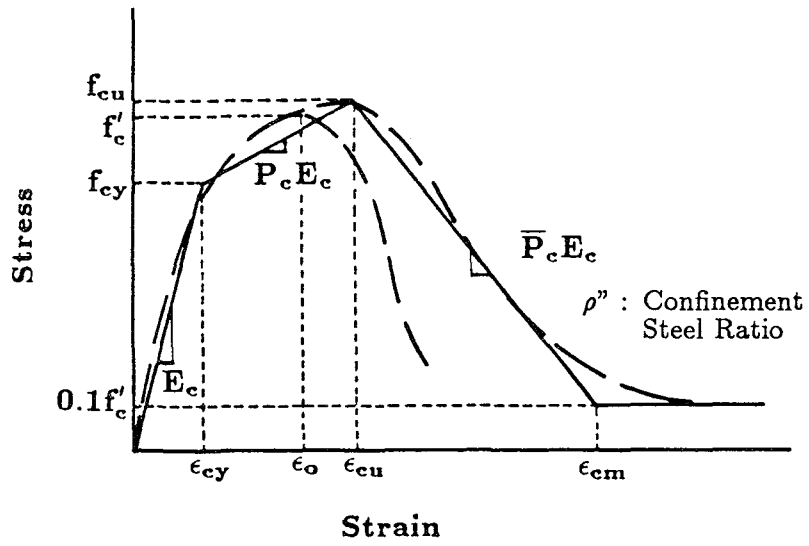


Fig. A.3 – Stress-Strain Curve for Concrete

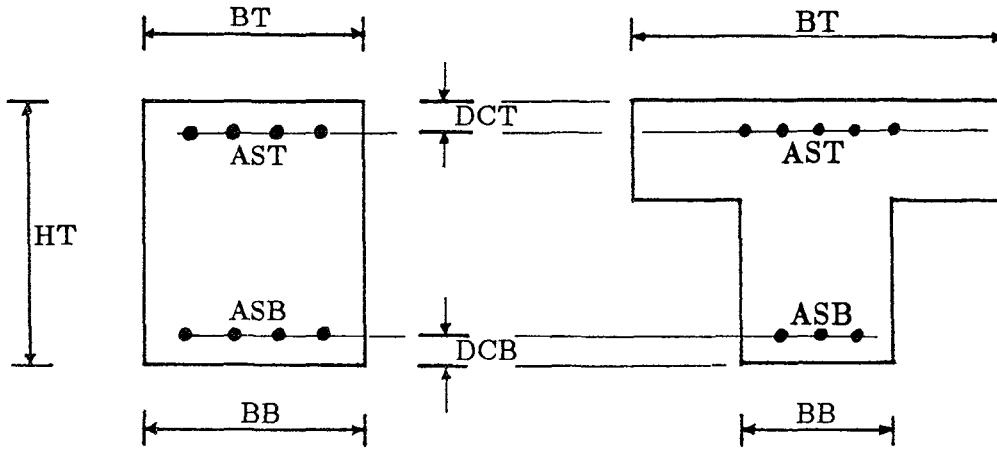


Fig. A.4 – Idealized Concrete Cross Sections

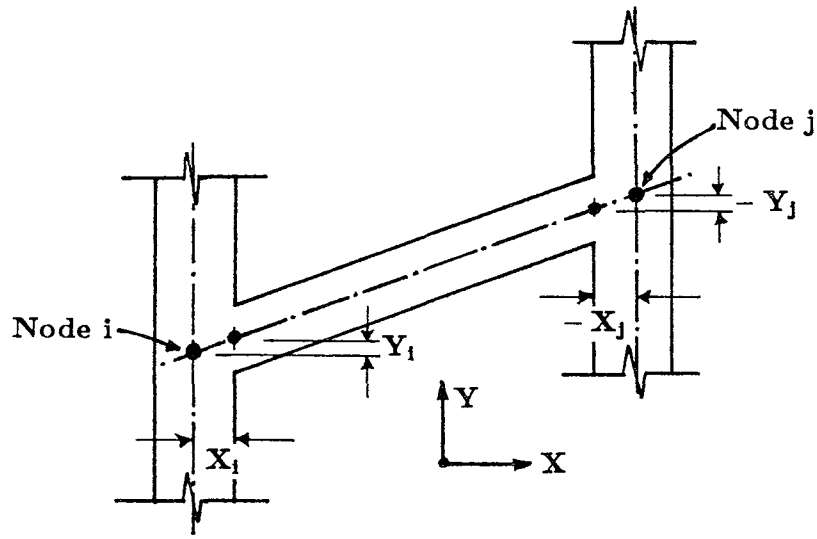


Fig. A.5 – End Eccentricities of Frame Element

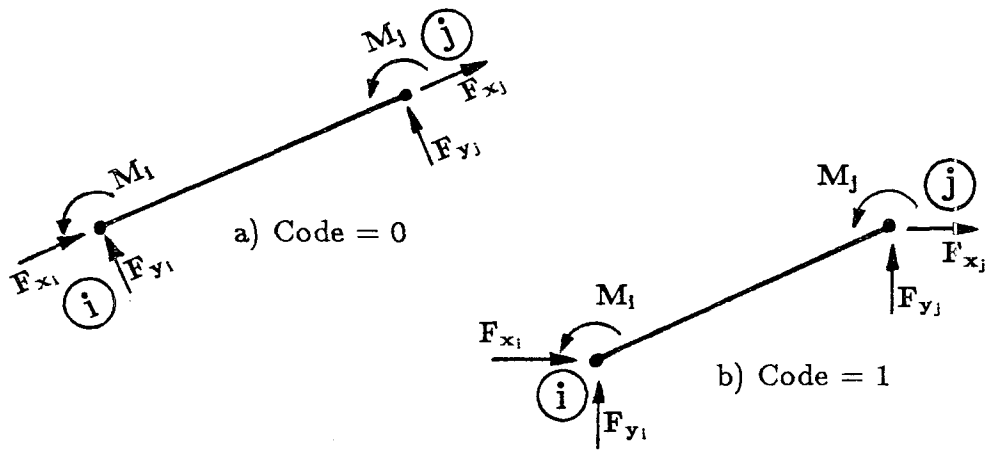


Fig. A.6 – Fixed End Forces and Initial Forces Pattern

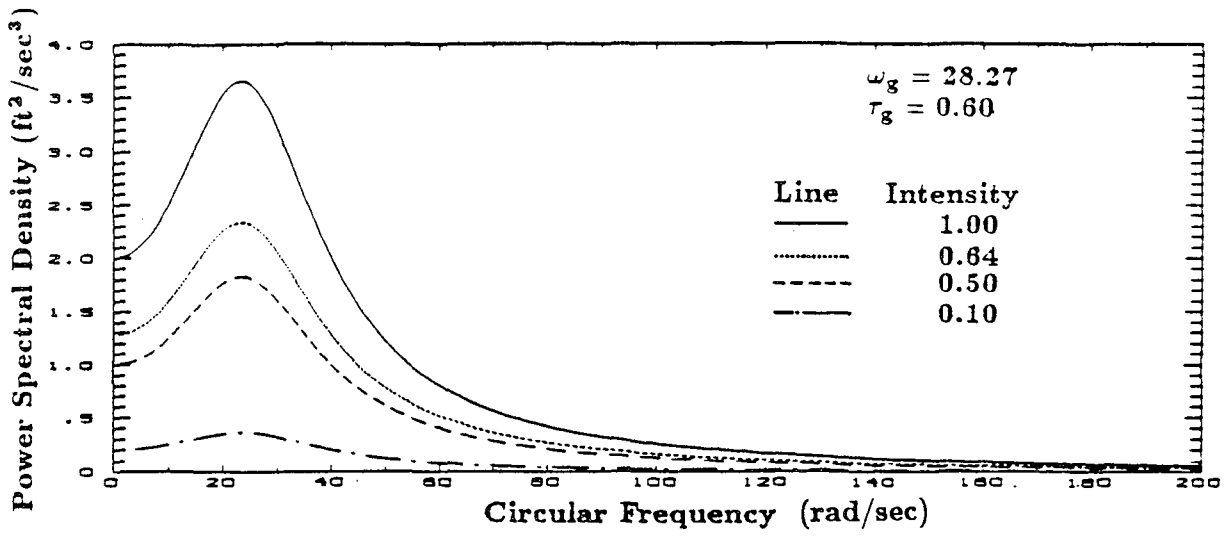


Fig. A.7 - One-Sided Kanai Tajimi Spectrum

Appendix – B

(Fortran Source Listing of Program SARCF)

Please contact NCEER for the magnetic tape of program source file.


```

1 C ***** MAIN 1
2 C ***** MAIN 2
3 C * SEISMIC ANALYSIS MAIN 3
4 C * OF MAIN 4
5 C * REINFORCED CONCRETE FRAMES MAIN 5
6 C * ( SARCF ) MAIN 6
7 C * MAIN 7
8 C * YOUNG SOD CHUNG, PH.D. MAIN 8
9 C * RESEARCH ASSOCIATE MAIN 9
10 C * MAIN 9
11 C * MAIN 9
12 C * DEPARTMENT OF CIVIL ENGINEERING AND OPERATIONS RESEARCH MAIN 10
13 C * PRINCETON UNIVERSITY MAIN 11
14 C * MAIN 12
15 C * DATED ON NOVEMBER 1, 1988 MAIN 13
16 C * MAIN 14
17 C * MAIN 15
18 C ***** MAIN 16
19 C ***** MAIN 17
20 C IMPLICIT REAL*8(A-H,O-Z) MAIN 18
21 C MAIN 19
22 C SET STORAGE CAPACITY AND CALL DRAIN MAIN 20
23 C MAIN 21
24 C COMMON A(50000) MAIN 22
25 C NNTST=5000 MAIN 23
26 C MAIN 24
27 C CALL SARCF(A,NNTST) MAIN 25
28 C MAIN 26
29 C STOP MAIN 27
30 C END MAIN 28
31 C *****MAIN 29
32 C SUBROUTINE SARCF (A,NNTST) SARCF 1
33 C IMPLICIT REAL*8(A-H,O-Z) SARCF 2
34 C SARCF 3
35 C COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY SARCF 4
36 C COMMON/GENINF/KCONT(10),KELEM(10),NELEM(10),NINF(10),NDOF(10) SARCF 5
37 C 1 ,FCONT(3),NUMEM(10) SARCF 6
38 C COMMON/DAMP/ ALPHA,BETA,DT,GAXCTE,CON1,CON2,CON3,CON4,CON5, SARCF 7
39 C 1 C1,C2,C3,C4,C5,C6,C7,C8,C9,C10,C11,C12,BETAO,DELTA SARCF 8
40 C COMMON/PASS/ IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM,IEAR,NEAR, SARCF 9
41 C 1 KSYM,KSMD SARCF 10
42 C COMMON/STOR/ NAVST,NF1,NF2,NF3,NF4,NTST,KODST,KDATA,NF17 SARCF 11
43 C COMMON/OUTN/ IPJ,IPE,KNTJ,KNTE,NHOUT,NVOUT,NROUT SARCF 12
44 C COMMON/WORK/HED1(18),KFORM1(2),TITLE1(10),W1(1571) SARCF 13
45 C COMMON/WORK1/ HED(18),KFORM(2),TITLE(10) SARCF 14
46 C COMMON/THIST/ITHOUT(10),THOUT(20),ITHP,ISAVE,NELTH,NSTH,NF7,ISE SARCF 15
47 C COMMON/INFEL/ IMEM,IMEMD,KST,KSTD,LM(1),LMD(1),DUM(212) SARCF 16
48 C COMMON/THISTJ/ITHPJ,NF5,NSTHJ,ISJ SARCF 17
49 C COMMON/THISTR/ITHPR,NF6,NSTHR,NHR,NVR,LRH1(50),LRH2(50),LRV1(50), SARCF 18
50 C 1 LRV2(50) SARCF 19
51 C COMMON/EQUAKE/DSEED,PGA,WG,TAU,UWG,PG,IEVL,KIEVL,ENA,ENB,ENC SARCF 20
52 C COMMON/DAMAGE/KDAMAGE,ITDAM,KIDAMT,NNSKIP,NSSKIP,NGSKIP,GLHYS, SARCF 21
53 C 1GLDAM SARCF 22
54 C COMMON/AUTO/KAUTO,KAUTOD,KECO,KECOD,NDSGN,NDSGND,KFREQ,KFREQD, SARCF 23
55 C 1 DBALL,DCALL,DBSTD,CONC,STEEL,IECO,BMAVG,BMDEV,ICONV SARCF 24
56 C SARCF 25
57 C PARAMETER (NQKE=20,NELN=40,NELG=2) SARCF 26
58 C DIMENSION NIBAY(NELN/3),STIN(6,NELN),CONIN(9,NELN),SECIN(9,NELN), SARCF 27
59 C 1ITY(3,NELN),YBM(2,NELG,NELN),RHOM(2,NELG,NELN),DDIN(2,NELN), SARCF 28
60 C 2DMY(2,NELN),DCAVG(NELN),DBAVG(NELN),DEDIF(NELN),PDEDIF(NELN), SARCF 29
61 C 3IECHK(NELN),ICOR(NELN),IICHK(NELN),DD1(NQKE),DD2(NQKE),DA(NELN), SARCF 30
62 C 4NOD(2*NELN),DDAM(NQKE,2*NELN),STHYS(NELG,NELN/3), SARCF 31
63 C 5STDAM(NELG,NELN/3),STRDAM(NELN/3),STRHYS(NELN/3),ELDAM(NELG,NELN), SARCF 32
64 C 6ELHYS(NELG,NELN),IR(NQKE),S(2*NELN),IP(5*NELN),KIP(5*NELN), SARCF 33
65 C 7PR(5*NELN),PPR(2,5*NELN) SARCF 34
66 C SARCF 35
67 C DIMENSION A(1) SARCF 36
68 C DIMENSION CHEK(2),HDAT(3,3),HSTF(2,2),SLOD(2,2) SARCF 37
69 C DATA CHEK/5HSTART,5HSTOP / SARCF 38
70 C DATA HDAT/8HEXECUTE ,8H ,8H , SARCF 39
71 C 1 8HDATA CHE,8HCKING ON,8HLY , SARCF 40
72 C 2 8HEXECUTE ,8HIF SINGL,8HE BLOCK / SARCF 41
73 C DATA HSTF/8HSTORED I,8HN CORE / SARCF 42
74 C 1 8HSTORED O,8HN TAPE / SARCF 43
75 C DATA SLOD/8HLOADS AP,8HPLIED , SARCF 44
76 C 1 8HLOADS IG,8HNORED / SARCF 45
77 C SARCF 46
78 C START AND TITLE CARD SARCF 47
79 C SARCF 48
80 C 20 FORMAT (A5,3X,18A4) SARCF 49

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81      10 READ 20, OPER,HED                      SARCF 50
82      IF (OPER.EQ.CHEK(2)) CALL EXIT          SARCF 51
83      IF (OPER.NE.CHEK(1)) GO TO 10          SARCF 52
84      PRINT 30, HED                          SARCF 53
85      30 FORMAT (9H1ERDARCS ,59X,4H1986//72(1H*)//1X,18A4//72(1H*)) SARCF 54
86      READ 25, KEARTH,KDAMAGE,KAUTO,KDATA    SARCF 55
87      25 FORMAT (415)                        SARCF 56
88      C                                       SARCF 57
89      NF1=1                                  SARCF 58
90      NF2=2                                  SARCF 59
91      NF3=3                                  SARCF 60
92      NF4=4                                  SARCF 61
93      NF5=8                                  SARCF 62
94      NF6=9                                  SARCF 63
95      NF7=10                                 SARCF 64
96      NF17=7                                 SARCF 65
97      REWIND NF17                            SARCF 66
98      REWIND NF1                             SARCF 67
99      REWIND NF2                             SARCF 68
100     REWIND NF3                             SARCF 69
101     C                                       SARCF 70
102     C   CONTROL CARD                       SARCF 71
103     C                                       SARCF 72
104     READ 40, NSTORY,NBAY,NJTS,NCONJT,NCDJT,NCDDOF,NCDDIS,NCDDMS,NELGR, SARCF 73
105     1KDOST,KSVM,NTST                       SARCF 74
106     40 FORMAT (1115,I10)                   SARCF 75
107     IF(NELGR .LE. NELG) GO TO 42           SARCF 76
108     PRINT 43, NELGR                         SARCF 77
109     43 FORMAT('INCREASE THE PARAMETER, NELG, FOR NUMBER OF ELEMENT GROUP SARCF 78
110     1, I.E. INPUT NELG >= ',I3)          SARCF 79
111     GO TO 999                               SARCF 80
112     42 IF(NBAY .EQ. 0) READ 41, (NBAY(I),I=1,NSTORY) SARCF 81
113     41 FORMAT(1615)                         SARCF 82
114     C                                       SARCF 83
115     IF (NTST.EQ.0) NTST=NNTST              SARCF 84
116     I=1                                      SARCF 85
117     IF (KDATA.GT.0) I=2                    SARCF 86
118     IF (KDATA.LT.0) I=3                    SARCF 87
119     J=1                                      SARCF 88
120     IF (KODST.NE.0) J=2                    SARCF 89
121     PRINT 50, NJTS,NCONJT,NCDJT,NCDDOF,NCDDIS,NCDDMS,NELGR,KDATA,(HDAT SARCF 90
122     1K,I),K=1,3),KODST,(HSTF(K,J),K=1,2),NTST SARCF 91
123     50 FORMAT (////                        SARCF 92
124     1 41H TOTAL NUMBER OF NODES           =15// SARCF 93
125     2 41H NO. OF CONTROL NODES           =15/ SARCF 94
126     3 41H NO. OF NODE GENERATION COMMANDS =15// SARCF 95
127     4 41H NO. OF ZERO DISPLACEMENT COMMANDS =15/ SARCF 96
128     5 41H NO. OF IDENTICAL DISPLACEMENT COMMANDS =15// SARCF 97
129     6 41H NO. OF MASS GENERATION COMMANDS =15// SARCF 98
130     7 41H NO. OF ELEMENT GROUPS         =15/// SARCF 99
131     8 41H DATA CHECKING CODE           =15,6X,3A8/ SARCF100
132     9 41H STRUCTURE STIFFNESS STORAGE CODE =15,6X,2A8/// SARCF101
133     9 41H BLANK COMMON TO BE ASSUMED    =110) SARCF102
134     C                                       SARCF103
135     KID=1                                   SARCF104
136     KX=KID+3*NJTS                          SARCF105
137     KY=KX+NJTS                             SARCF106
138     C                                       SARCF107
139     C   NODE COORDINATES, ETC             SARCF108
140     C                                       SARCF109
141     CALL INJTS (A(KX),A(KY),A(KID),NJTS,NCONJT,NCDJT,NCDDOF,NCDDIS) SARCF110
142     C                                       SARCF111
143     C   MASS DATA                       SARCF112
144     C                                       SARCF113
145     KFM=KY+NJTS                            SARCF114
146     KEQM=KFM+NEQ+1                         SARCF115
147     KD=KEQM+NEQ+1                         SARCF116
148     KDDS=KD+NEQ+1                         SARCF117
149     KM=KDDS+NEQ+1                         SARCF118
150     NEQK=KD+NEQ                           SARCF119
151     DO 72 I=KDDS,KM-1                      SARCF120
152     72 A(I)=0.0                            SARCF121
153     C                                       SARCF122
154     CALL INMASS (A(KFM),A(KEQM),A(KID),NCDDMS,NJTS) SARCF123
155     C                                       SARCF124
156     C   DATA FOR DAMPING COEFFICIENTS   SARCF125
157     C                                       SARCF126
158     READ 11, ALPHA,BETA,BETA0,DELTA        SARCF127
159     11 FORMAT(4E10.0)                      SARCF128
160     PRINT 21, ALPHA,BETA,BETA0,DELTA       SARCF129

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161      21 FORMAT( 21H1DAMPING COEFFICIENTS  ////          SARCF130
162      1          5X,39HMASS PROPORTION, ALPHA          =F12.6//  SARCF131
163      2          5X,39HTANGENT STIFFNESS PROPORTION, BETA  =F12.6//  SARCF132
164      3          5X,39HORIGINAL STIFFNESS PROPORTION, BETA-O =F12.6//  SARCF133
165      4          5X,39HSTRUCTURAL DAMPING, DELTA          =F12.6)   SARCF134
166      C          SARCF135
167      C          ELEMENT DATA                          SARCF136
168      C          SARCF137
169      CALL INELEM (A(KID),A(KX),A(KY),A(KM),NJTS,NELTOT,ITY,STIN,CONIN, SARCF138
170      1SECIN,DDIN,RHOM,YBM,NELG,NELN,IP,KIP,PR,PPR,DMY)   SARCF139
171      C          SARCF140
172      C          LOAD CONTROL DATA                      SARCF141
173      C          SARCF142
174      READ 61, KSTAT,NCDLD,NSTEPS,DT,FACAXH,FACTMH,FACAXV,FACTMV,DISMAX SARCF143
175      61 FORMAT (215,1110,6F10.0)                         SARCF144
176      KSTAT=0                                              SARCF145
177      IF (FACTMV.EQ.0.) FACTMV=1.                         SARCF146
178      IF (DISMAX.EQ.0.) DISMAX=100000.                   SARCF147
179      C          SARCF148
180      C          COEFFICIENTS FOR EQUATION OF MOTION     SARCF149
181      C          SARCF150
182      CALL CONSTANT                                       SARCF151
183      C          SARCF152
184      C          STATIC NODAL LOADS                      SARCF153
185      C          SARCF154
186      CALL INEXLD (A(KDDS),A(KID),NCDLD,NJTS,A(KEQM))    SARCF155
187      C          SARCF156
188      C          INPUT INFORMATION FOR EARTHQUAKE        SARCF157
189      C          SARCF158
190      IF(KEARTH.EQ.1) GO TO 64                            SARCF159
191      C          SARCF160
192      C          INPUT INFORMATION FOR DETERMINISTIC EARTHQUAKE SARCF161
193      C          SARCF162
194      J=1                                                  SARCF163
195      IF(KSTAT.EQ.0)J=2                                    SARCF164
196      PRINT 71, KSTAT,(SLOD(K,J),K=1,2),NCDLD,NSTEPS,DT,FACAXH,FACTMH,FASARCF165
197      1CAXV,FACTMV,DISMAX                                  SARCF166
198      71 FORMAT (32H STATIC LOAD CONTROL INFORMATION  ///  SARCF167
199      1          29H STATIC LOAD CODE          =15,1X,2A8//  SARCF168
200      2          29H NO. OF NODAL LOAD COMMANDS =15////////  SARCF169
201      1          31H1EARTHQUAKE CONTROL INFORMATION  ///  SARCF170
202      1          32H DETERMINISTIC EARTHQUAKE        /      SARCF171
203      2          32H NO. OF INTEGRATION TIME STEPS =15/    SARCF172
204      3          32H INTEGRATION STEP SIZE          =F9.4///  SARCF173
205      4          40H MAGNIFICATION FACTORS FOR X EARTHQUAKE /  SARCF174
206      5          18X, 14HACCELERATION =, F9.2, /          SARCF175
207      6          18X, 14HTIME          =, F9.2, //        SARCF176
208      7          40H MAGNIFICATION FACTORS FOR Y EARTHQUAKE /  SARCF177
209      8          18X, 14HACCELERATION =, F9.2, /          SARCF178
210      9          18X, 14HTIME          =, F9.2, ///       SARCF179
211      9          32H MAX. PERMISSIBLE DISPLACEMENT =F10.2) SARCF180
212      GO TO 125                                           SARCF181
213      C          SARCF182
214      C          INPUT INFORMATION FOR ARTIFICIAL EARTHQUAKE SARCF183
215      C          SARCF184
216      64 READ 126, NEAR,IEVL,ENA,ENB,ENC,PGA,WG,TAU,UWG,PG  SARCF185
217      126 FORMAT(215,3F5.2,2F10.4,F5.4,2F5.0)            SARCF186
218      C          SARCF187
219      J=1                                                  SARCF188
220      IF(KSTAT.EQ.0)J=2                                    SARCF189
221      PRINT 73, KSTAT,(SLOD(K,J),K=1,2),NCDLD,NEAR,NSTEPS,DT,FACAXH,FACTSARCF190
222      1MH,FACAXV,FACTMV,DISMAX                            SARCF191
223      73 FORMAT (32H STATIC LOAD CONTROL INFORMATION  ///  SARCF192
224      1          29H STATIC LOAD CODE          =15,1X,2A8//  SARCF193
225      2          29H NO. OF NODAL LOAD COMMANDS =15////////  SARCF194
226      1          31H1EARTHQUAKE CONTROL INFORMATION///     SARCF195
227      1          32H NO. OF INPUT EARTHQUAKE          =15/    SARCF196
228      2          32H NO. OF INTEGRATION TIME STEPS =15/    SARCF197
229      3          32H INTEGRATION STEP SIZE          =F9.4///  SARCF198
230      4          40H MAGNIFICATION FACTORS FOR X EARTHQUAKE /  SARCF199
231      5          18X, 14HACCELERATION =, F9.2, /          SARCF200
232      6          18X, 14HTIME          =, F9.2, //        SARCF201
233      7          40H MAGNIFICATION FACTORS FOR Y EARTHQUAKE /  SARCF202
234      8          18X, 14HACCELERATION =, F9.2, /          SARCF203
235      9          18X, 14HTIME          =, F9.2, ///       SARCF204
236      9          32H MAX. PERMISSIBLE DISPLACEMENT =F10.2) SARCF205
237      C          SARCF206
238      IF(NEAR .LE. NQKE) GO TO 122                        SARCF207
239      PRINT 123, NEAR                                      SARCF208
240      123 FORMAT('INCREASE THE PARAMETER, NQKE, FOR NUMBER OF RANDOM EARTHQUASARCF209

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241      1AKE, I.E. INPUT NQKE >=' ,15)          SARCF210
242      GO TO 999                                SARCF211
243      122 IF(IEVL .EQ. 1) PRINT 127, PGA,WG,TAU,UWG,PG,ENA,ENB,ENC SARCF212
244      127 FORMAT(35HARTIFICIAL EARTHQUAKE INFORMATIONS /// SARCF213
245      1      43HKANAI-TAJIMI SPECTRAL DENSITY FUNCTION WITH// SARCF214
246      2      5X,25HINTENSITY FOR SPECTRUM =F8.4,' (FT**2/SC**3)'// SARCF215
247      3      5X,25HDOMINANT FREQUENCY =F8.4,' (RAD/SECOND)'// SARCF216
248      4      5X,25HDOMINANT DAMAPING RATIO =F8.4,// SARCF217
249      5      5X,25HUPPER CUT-OFF FREQUENCY =F10.2,' (RAD/SECOND)'// SARCF218
250      6      5X,25HPEAK FACTOR =F10.2,// SARCF219
251      7      35HTRAPEZOIDAL ENVELOPE FUNCTION WITH /// SARCF220
252      8      5X,25HINITIAL PEAK TIME =F8.4,' (FT**2/SC**3)'// SARCF221
253      9      5X,25HLAST PEAK TIME =F8.4,// SARCF222
254      9      5X,25HSTRONG MOTION DURATION =F8.4,///) SARCF223
255      C                                          SARCF224
256      IF(IEVL .EQ. 2) PRINT 128, PGA,WG,TAU,UWG,PG,ENA,ENB SARCF225
257      128 FORMAT(35HARTIFICIAL EARTHQUAKE INFORMATIONS /// SARCF226
258      1      43HKANAI-TAJIMI SPECTRAL DENSITY FUNCTION WITH// SARCF227
259      2      5X,25HINTENSITY FOR SPECTRUM =F8.4,' (FT**2/SC**3)'// SARCF228
260      3      5X,25HDOMINANT FREQUENCY =F8.4,' (RAD/SECOND)'// SARCF229
261      4      5X,25HDOMINANT DAMAPING RATIO =F8.4,// SARCF230
262      5      5X,25HUPPER CUT-OFF FREQUENCY =F10.2,' (RAD/SECOND)'// SARCF231
263      6      5X,25HPEAK FACTOR =F10.2,// SARCF232
264      7      35HEXPOENTIAL ENVELOPE FUNCTION WITH /// SARCF233
265      8      5X,25HCOEFFICIENT FOR ALPHA =F8.4,' (FT**2/SC**3)'// SARCF234
266      9      5X,25HCOEFFICIENT FOR BETA =F8.4,///) SARCF235
267      C                                          SARCF236
268      IDSGN=0 SARCF237
269      ICONV=0 SARCF238
270      DO 92 I=1,20 SARCF239
271      ICOR(I)=0 SARCF240
272      92 IECHK(I)=0 SARCF241
273      C                                          SARCF242
274      IF(KEARTH .EQ. 0) GO TO 125 SARCF243
275      CALL RANINT(NEAR,IR) SARCF244
276      91 IEAR=IEAR+1 SARCF245
277      IF(NEAR .GE. 1) DSEED=DFLOAT(IR(IEAR)) SARCF246
278      C                                          SARCF247
279      C      EARTHQUAKE RECORDS SARCF248
280      C                                          SARCF249
281      IF(IEAR .GE. 2 .OR. IDSGN .GE. 1) GO TO 81 SARCF250
282      125 READ 90, NPTH,NPTV,KFORM,TITLE SARCF251
283      90 FORMAT (4I5,10A4) SARCF252
284      IF(KEARTH .EQ. 1) GO TO 63 SARCF253
285      PRINT 100, TITLE,NPTH,NPTV,KFORM SARCF254
286      100 FORMAT (//////32H1EARTHQUAKE ACCELERATION RECORDS,3H - ,10A4,/// SARCF255
287      1      30H NO. OF X INPUT PAIRS =, 15, / SARCF256
288      2      30H NO. OF Y INPUT PAIRS =, 15, /// SARCF257
289      3      12H PRINT CODES // SARCF258
290      4      30H ACCELERATIONS AS INPUT =, 15, / SARCF259
291      5      30H INTERPOLATED ACCELERATIONS =, 15) SARCF260
292      GO TO 81 SARCF261
293      C                                          SARCF262
294      63 PRINT 101, TITLE,IEAR,NPTH,NPTV,KFORM SARCF263
295      101 FORMAT (//////32H1EARTHQUAKE ACCELERATION RECORDS,3H - ,10A4,3HNO., SARCF264
296      115/// SARCF265
297      1      30H NO. OF X INPUT PAIRS =, 15, / SARCF266
298      2      30H NO. OF Y INPUT PAIRS =, 15, /// SARCF267
299      3      12H PRINT CODES // SARCF268
300      4      30H ACCELERATIONS AS INPUT =, 15, / SARCF269
301      5      30H INTERPOLATED ACCELERATIONS =, 15) SARCF270
302      C                                          SARCF271
303      81 DO 80 I=KD,NEQK SARCF272
304      80 A(I)=0.0 SARCF273
305      C                                          SARCF274
306      IF(IEAR .GE. 2) PRINT 103,TITLE,IEAR SARCF275
307      103 FORMAT (//////32H1EARTHQUAKE ACCELERATION RECORDS,3H - ,10A4,3HNO., SARCF276
308      115) SARCF277
309      C                                          SARCF278
310      KTH=KM+NEQ+1 SARCF279
311      KGH=KTH+NPTH SARCF280
312      KTV=KGH+NPTH SARCF281
313      KGV=KTV+NPTV SARCF282
314      KAXH=KGV+NPTV SARCF283
315      KAXV=KAXH+NSTEPS SARCF284
316      C                                          SARCF285
317      DO 104 I=KTH,KAXV+NSTEPS-1 SARCF286
318      104 A(I)=0.0 SARCF287
319      C                                          SARCF288
320      CALL INAXL (KFORM,A(KTH),A(KGH),A(KTV),A(KGV),A(KAXH),A(KAXV),NSTESARCF289

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321      1PS,DT,FACAXH,FACTMH,FACAXV,FACTMV,KEARTH,IEAR)
322      C
323      C ANALYSIS INFORMATION DATA
324      C
325      IF(IEAR.GE.2 .OR. IDSGN.GE.1) GO TO 121
326      C
327      C ANALYSIS FOR FUNDAMENTAL NATURAL FREQUENCY
328      C
329      READ 105, KFREQ,IFREQ
330      105 FORMAT(2I5)
331      IF(KFREQ.NE. 1) PRINT 106, KFREQ
332      106 FORMAT(42HANALYSIS FOR FUNDAMENTAL NATURAL FREQUENCY///
333      1 5X,35HCODE FOR ANALYSIS =,15,/
334      2 20X,'*** DO NOT COMPUTE NATURAL FREQUENCY ',///)
335      IF(KFREQ.EQ. 1) PRINT 107, KFREQ,IFREQ
336      107 FORMAT(42HANALYSIS FOR FUNDAMENTAL NATURAL FREQUENCY///
337      1 5X,35HCODE FOR ANALYSIS =,15,/
338      2 20X,'*** COMPUTE NATURAL FREQUENCY ',///
339      3 10X,35HINTERVAL FOR NATURAL FREQUENCY =,15,/)
340      C
341      C ANALYSIS FOR DAMAGE INDEX
342      C
343      IF(KDAMAGE.EQ. 1) READ 129, ITDAM,NSSKIP,NNSKIP,NGSKIP
344      129 FORMAT(4I5)
345      IF(KDAMAGE.NE. 1) PRINT 136, KDAMAGE
346      136 FORMAT(22H1DAMAGE INDEX ANALYSIS///
347      1 5X,42HCODE FOR THE DAMAGE INDEX =,15,///
348      2 20X,' *** NOT PERFORM THE DAMAGE ANALYSIS ***')
349      IF(KDAMAGE.GE. 1) PRINT 137, KDAMAGE,ITDAM,NSSKIP,NNSKIP,NGSKIP
350      137 FORMAT(22H1DAMAGE INDEX ANALYSIS///
351      1 5X,42HCODE FOR THE DAMAGE INDEX =,15,///
352      2 5X,42HCODE FOR TIME HISTORIES OF DAMAGE INDEX =,15,/
353      3 10X,37HINTERVAL FOR NODE DAMAGE INDEX =,15,/
354      4 10X,37HINTERVAL FOR STORY DAMAGE INDEX =,15,/
355      5 10X,37HINTERVAL FOR GLOBAL DAMAGE INDEX =,15,///)
356      C
357      C ANALYSIS FOR AUTOMATIC DESIGN PROCEDURE
358      C
359      IF(KAUTO.EQ. 1) READ 131, KECO,NDSGN,BMAVG,BMDEV,DBALL,DCALL
360      131 FORMAT(2I5,4F10.5)
361      IF(KAUTO.NE. 1) PRINT 132, KAUTO
362      132 FORMAT(35HAUTOMATIC DESIGN ANALYSIS ///
363      1 5X,37HCODE FOR AUTOMATIC DESIGN ANALYSIS =,15,/
364      2 20X,'*** NOT PERFORM THE AUTOMTIC DESIGN ANALYSIS',///)
365      IF(KAUTO.EQ. 1) PRINT 133, KAUTO,NDSGN,DBALL,DCALL
366      133 FORMAT(35HAUTOMATIC DESIGN ANALYSIS ///
367      1 5X,37HCODE FOR AUTOMATIC DESIGN ANALYSIS =,15,/
368      2 20X,'*** DO THE AUTOMTIC DESIGN ANALYSIS',///
369      35X,49HMAXIMUM ITERATION NUMBER OF AUTOMATIC DESIGN =,15,//
370      45X,49HALLOWABLE MEAN VALUE OF BEAM DAMAGE INDEX =,F10.5,//
371      45X,49HALLOWABLE DEV FROM BEAM MEAN VALUE =,F10.5,//
372      45X,49HTOLERABLE DEV OF INDIVIDUAL BEAM DAMAGE INDEX =,F10.5,//
373      45X,49HALLOWABLE DAMAGE INDICES FOR COLUMN =,F10.5,///)SARCF342
374      C
375      C OUTPUT CONTROL DATA
376      C
377      IF(KEARTH.EQ.0) READ 110, IPJ,IPE,IENV,NHOUT,NVOUT,NROUT,NHR,NVR,
378      1ITHPJ,ITHPR,ITHP,ISJ,ISE
379      110 FORMAT (13I5)
380      121 IF(IPJ.LE. 0) IPJ=0
381      IF(IPE.LE. 0) IPE=0
382      IF(IDSGN.EQ.0 .AND. IEAR.EQ.1)
383      1PRINT 120, IPJ,IPE,IENV,NHOUT,NVOUT,NROUT,NHR,NVR,ITHPJ,ITHPR,ITHPSARCF352
384      2,ISJ,ISE
385      120 FORMAT (30H1TIME HISTORY OUTPUT INTERVALS //
386      1 5X,21H NODE DISPLACEMENTS =, 15, /
387      2 5X,21H ELEMENT RESULTS =, 15, ///
388      3 40H OUTPUT INTERVAL FOR RESULTS ENVELOPES =,15,////
389      4 35H NO. OF NODES FOR X DISPL HISTORY =, 15,/
390      5 35H NO. OF NODES FOR Y DISPL HISTORY =, 15,/
391      6 35H NO. OF NODES FOR ROTATION HISTORY=, 15,///
392      7 46H NO. OF PRS OF NODES FOR REL X DISPL HISTORY =,15,/
393      8 46H NO. OF PRS OF NODES FOR REL Y DISPL HISTORY =,15,///
394      9 40H CODE FOR JOINT TIME HISTORY PRINT =, 15,/
395      9 40H CODE FOR REL DISPL TIME HISTORY PRINT =,15,/
396      9 40H CODE FOR ELEMENT TIME HISTORY PRINT =, 15,///
397      9 48H CODE FOR SAVING DISPL TIME HISTORIES ON TAPE =,15,/
398      9 48H CODE FOR SAVING ELEMENT TIME HISTORIES ON TAPE=,15)
399      C
400      NSTH=0

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401	NSTHR=0	SARCF370
402	NELTH=0	SARCF371
403	NSTHJ=0	SARCF372
404	IF (ITHPJ.GT.0) REWIND NF5	SARCF373
405	IF (ITHPR.GT.0) REWIND NF6	SARCF374
406	IF (ITHP.GT.0) REWIND NF4	SARCF375
407	C	SARCF376
408	KJH=KAXV+NSTEPS	SARCF377
409	KJV=KJH+NHOUT	SARCF378
410	KJR=KJV+NVOUT	SARCF379
411	IF (IPJ.EQ.0) IPJ=-1	SARCF380
412	IF (IPE.EQ.0) IPE=-1	SARCF381
413	C	SARCF382
414	IF(KEARTH.EQ.0) CALL OUTJT (A(KJH),A(KJV),A(KJR),A(KID),NJTS)	SARCF383
415	C	SARCF384
416	IF (NHR.EQ.0) GO TO 150	SARCF385
417	READ 130,(LRH1(I),LRH2(I),I=1,NHR)	SARCF386
418	130 FORMAT (10I5)	SARCF387
419	PRINT 140, (LRH1(I),LRH2(I),I=1,NHR)	SARCF388
420	140 FORMAT (////39H PAIRS OF NODES FOR REL X DISPL HISTORY//10(3X,I4,ISARCF389	
421	14,1H,))	SARCF390
422	150 IF (NVR.EQ.0) GO TO 170	SARCF391
423	READ 130,(LRV1(I),LRV2(I),I=1,NVR)	SARCF392
424	PRINT 160, (LRV1(I),LRV2(I),I=1,NVR)	SARCF393
425	160 FORMAT (////39H PAIRS OF NODES FOR REL Y DISPL HISTORY//10(3X,I4,ISARCF394	
426	14,1H,))	SARCF395
427	170 CONTINUE	SARCF396
428	IF(IEAR.GE.2 .OR. IDSGN.GE.1) CALL REINT(IEAR,IDSGN,NELG,NELN,ICORSARCF397	
429	1,DEDIF,PDEDIF,I1CHK,DA,SECIN,STIN,CONIN,YBM,RHOM,DDIN,ITY,DMY)	SARCF398
430	IF(IEAR.GE.2 .OR. IDSGN.GE.1) GO TO 171	SARCF399
431	C	SARCF400
432	IF(NELTOT .LE. NELN) GO TO 171	SARCF401
433	PRINT 173, NELTOT	SARCF402
434	173 FORMAT('INCREASE THE PARAMETER, NELN, FOR TOTAL NUMBER OF ELEMENTSSARCF403	
435	1, I.E. INPUT NELN >=',15)	SARCF404
436	GO TO 999	SARCF405
437	C	SARCF406
438	C COMPACT STORAGE	SARCF407
439	C	SARCF408
440	171 J=KJR+NR0UT-KAXH	SARCF409
441	KMM=KM+NEQ	SARCF410
442	KAXHH=KAXH-1	SARCF411
443	DO 190 I=1,J	SARCF412
444	190 A(KMM+I)=A(KAXHH+I)	SARCF413
445	J=KAXHH-KMM	SARCF414
446	KAXH=KAXH-J	SARCF415
447	KAXV=KAXV-J	SARCF416
448	KJH=KJH-J	SARCF417
449	KJV=KJV-J	SARCF418
450	KJR=KJR-J	SARCF419
451	C	SARCF420
452	C ARRAY ADDRESSES FOR REMAINING COMPUTATION	SARCF421
453	C	SARCF422
454	KA=KJR+NR0UT	SARCF423
455	KAA=KA+NSTO	SARCF424
456	IF (KODST.NE.0) KAA=KA	SARCF425
457	KDIS=KAA+NSTO	SARCF426
458	KVEL=KDIS+NEQ+1	SARCF427
459	KACC=KVEL+NEQ+1	SARCF428
460	KENP=KACC+NEQ+1	SARCF429
461	KENN=KENP+NEQ+1	SARCF430
462	KTP=KENN+NEQ+1	SARCF431
463	KTN=KTP+NEQ+1	SARCF432
464	KIAD=KTN+NEQ+1	SARCF433
465	KBL=KIAD+NELTOT+1	SARCF434
466	C	SARCF435
467	NAVST=NTST-KBL+1	SARCF436
468	LSOFAR=KBL-1	SARCF437
469	C	SARCF438
470	C INITIALIZE	SARCF439
471	C	SARCF440
472	DO 200 I=KA,KIAD	SARCF441
473	200 A(I)=0.	SARCF442
474	KNTJ=0	SARCF443
475	KNTE=0	SARCF444
476	C	SARCF445
477	C CONSOLIDATE ELEMENT DATA	SARCF446
478	C	SARCF447
479	CALL CONSOL (A(KIAD),A(KBL),LSOFAR)	SARCF448
480	C	SARCF449

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481          IF (KDATA.GT.0.AND.IEAR.LT.NEAR) GO TO 91          SARCF450
482          IF (KDATA.GT.0.AND.IEAR.EQ.NEAR) GO TO 10         SARCF451
483          IF (KDATA.GE.0.OR.NBLOK.EQ.1) GO TO 220          SARCF452
484          PRINT 210                                          SARCF453
485          210 FORMAT (///42H MORE THAN ONE BLOCK, EXECUTION SUPPRESSED) SARCF454
486          GO TO 10                                          SARCF455
487          C                                                SARCF456
488          C EXECUTE                                          SARCF457
489          C                                                SARCF458
490          220 PRINT 230                                       SARCF459
491          230 FORMAT (1H1,20X,20(1H*),5X,7HRESULTS,5X,20(1H*)////////) SARCF460
492          C                                                SARCF461
493          C COMPUTE STATIC STIFFNESS                          SARCF462
494          C                                                SARCF463
495          ISTEP=0                                           SARCF464
496          KVARY=100000000                                     SARCF465
497          CALL STIFF (A(KBL),A(KAA),A(KD),A(KIAD),A(KFM),A(KVEL),A(KM),A(KBL)SARCF466
498          1))                                               SARCF467
499          C                                                SARCF468
500          C COMPUTE NATURAL FREQUENCIES                      SARCF469
501          C                                                SARCF470
502          IF(KFREQ.LT.1) GO TO 255                          SARCF471
503          IF(IEAR.GE.2) GO TO 255                          SARCF472
504          S(NEQ+1)=0.                                       SARCF473
505          CALL FNFQ(NEQ,A(KAA),A(KFM),A(KM),WE,S)          SARCF474
506          CALL PRTFQ(WE,S,NJTS,NEQ,A(KID))                  SARCF475
507          C                                                SARCF476
508          C STATIC LOAD EFFECTS                              SARCF477
509          C                                                SARCF478
510          255 IF (KSTAT.EQ.0) GO TO 260                     SARCF479
511          CALL RESERV (A(KA),A(KAA),A(KM),NSTO,NF3,KODST,NEQ,1,2) SARCF480
512          CALL OPTSOL (A(KA),A(KDDS),A(KM),NEQ,1,3)         SARCF481
513          CALL RESPON (A(KBL),A(KD),A(KDIS),A(KVEL),A(KACC),A(KDDS),A(KENP),SARCF482
514          1A(KEN),A(KIAD),A(KJH),A(KJV),A(KJR),A(KTP),A(KTN),DISMAX,A(KBL),ASARCF483
515          2(KID),NJTS,NELG,NELN,ELDAM,ELHYS,STDAM,STHYS,STRDAM,STRHYS,NIBAY) SARCF484
516          IF (KSTAT.NE.-1.AND.KVARY.EQ.0) GO TO 250        SARCF485
517          IF (KVARY.NE.0) PRINT 240                          SARCF486
518          240 FORMAT (///32H ERROR - YIELD UNDER STATIC LOAD) SARCF487
519          GO TO 10                                          SARCF488
520          C                                                SARCF489
521          C MODIFY STIFFNESS FOR GEOMETRIC AND INERTIA EFFECTS SARCF490
522          C                                                SARCF491
523          250 IF (KODST.NE.0) CALL RESERV (A(KA),A(KAA),A(KM),NSTO,NF3,KODST,NEQ)SARCF492
524          1,JCOL,1)                                         SARCF493
525          260 IF (NSTEPS.EQ.0) GO TO 10                      SARCF494
526          ISTEP=1                                           SARCF495
527          IENVY=IENV                                         SARCF496
528          KVARY=100000000                                     SARCF497
529          CALL STIFF (A(KBL),A(KAA),A(KD),A(KIAD),A(KFM),A(KVEL),A(KM),A(KBL)SARCF498
530          1))                                               SARCF499
531          CALL RESERV (A(KA),A(KAA),A(KM),NSTO,NF3,KODST,NEQ,1,2) SARCF500
532          CALL OPTSOL (A(KA),A(KDDS),A(KM),NEQ,1,1)         SARCF501
533          C                                                SARCF502
534          C SOLVE, STEP BY STEP                              SARCF503
535          C                                                SARCF504
536          DO 280 ISTEP=1,NSTEPS                              SARCF505
537          C                                                SARCF506
538          C COMPUTE TIME HISTORY FOR FUNDAMENTAL NATURAL FREQUENCIES SARCF507
539          C                                                SARCF508
540          IF(KFREQ.LT.1) GO TO 271                          SARCF509
541          IF(IFREQ.EQ.0) IFREQ=NSTEPS+1                     SARCF510
542          REM=DFLOAT(ISTEP)/DFLOAT(IFREQ)-DFLOAT(ISTEP/IFREQ) SARCF511
543          IF(REM.NE.0.0) GO TO 271                          SARCF512
544          S(NEQ+1)=0.                                       SARCF513
545          CALL FNFQ(NEQ,A(KAA),A(KFM),A(KM),WE,S)          SARCF514
546          CALL PRTFQ(WE,S,NJTS,NEQ,A(KID))                  SARCF515
547          C                                                SARCF516
548          C MODIFY STIFFNESS IF NECESSARY                    SARCF517
549          C                                                SARCF518
550          271 IF (KVARY.EQ.0) GO TO 270                     SARCF519
551          IF (KODST.NE.0) CALL RESERV (A(KA),A(KAA),A(KM),NSTO,NF3,KODST,NEQ)SARCF520
552          1,JCOL,1)                                         SARCF521
553          CALL STIFF (A(KBL),A(KAA),A(KD),A(KIAD),A(KFM),A(KVEL),A(KM),A(KBL)SARCF522
554          1))                                               SARCF523
555          CALL RESERV (A(KA),A(KAA),A(KM),NSTO,NF3,KODST,NEQ,JCOL,2) SARCF524
556          CALL OPTSOL (A(KA),A(KDDS),A(KM),NEQ,JCOL,1)      SARCF525
557          C                                                SARCF526
558          C SET UP EFFECTIVE LOAD                            SARCF527
559          C                                                SARCF528
560          270 CALL FORCE (A(KD),A(KDDS),A(KAXH),A(KAXV),A(KFM),A(KEQM),A(KVEL),ASARCF529

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561      1(KACC))                                SARCF530
562      C                                        SARCF531
563      C   RESPONSE                            SARCF532
564      C                                        SARCF533
565      CALL OPTSOL (A(KA),A(KDDS),A(KM),NEQ,JCOL,2) SARCF534
566      CALL RESPON (A(KBL),A(KD),A(KDIS),A(KVEL),A(KACC),A(KDDS),A(KENP),SARCF535
567      1A(KENN),A(KIAD),A(KJH),A(KJV),A(KJR),A(KTP),A(KTN),DISMAX,A(KBL),ASARCF536
568      2(KID),NJTS,NELG,NELN,ELDAM,ELHYS,STDAM,STHYS,STRDAM,STRHYS,NIBAY) SARCF537
569      IF (KSTAT.EQ.-1) GO TO 290                SARCF538
570      IF (ISTEP.NE.IENV.OR.IENV.GE.NSTEPS) GO TO 280 SARCF539
571      CALL OUTEND (A(KBL),A(KIAD),A(KENP),A(KENN),A(KTP),A(KTN),A(KID),NSARCF540
572      1JTS)                                      SARCF541
573      IF(KDAMAGE .GE. 1) CALL OUTDAM (A(KBL),A(KIAD),A(KID),NJTS,NQKE,NESARCF542
574      1LG,NELN,STDAM,STRDAM,NOD,DDAM)          SARCF543
575      C   IENV=IENV+IENVY                      SARCF544
576      C                                        SARCF545
577      280 CONTINUE                             SARCF546
578      C                                        SARCF547
579      C   PRINT FINAL ENVELOPES                SARCF548
580      C                                        SARCF549
581      290 CALL OUTEND (A(KBL),A(KIAD),A(KENP),A(KENN),A(KTP),A(KTN),A(KID),NSARCF550
582      1JTS)                                      SARCF551
583      IF(KDAMAGE .GE. 1) CALL OUTDAM (A(KBL),A(KIAD),A(KID),NJTS,NQKE,NESARCF552
584      1LG,NELN,STDAM,STRDAM,NOD,DDAM)          SARCF553
585      C                                        SARCF554
586      C   PRINT REORGANISED DISPLACEMENT TIME HISTORIES SARCF555
587      C                                        SARCF556
588      IF (ITHPJ.EQ.0) GO TO 300                 SARCF557
589      CALL THPRJ (A(KJH),A(KJV),A(KJR),NF7)    SARCF558
590      C                                        SARCF559
591      C   PRINT REORGANISED RELATIVE NODE DISPLACEMENT TIME HISTORIES SARCF560
592      C                                        SARCF561
593      300 IF (ITHPR.EQ.0) GO TO 310             SARCF562
594      CALL THPRR (NF7,ISJ)                     SARCF563
595      C                                        SARCF564
596      C   PRINT REORGANISED ELEMENT TIME HISTORIES SARCF565
597      C                                        SARCF566
598      310 IF (ITHP.EQ.0) GO TO 85              SARCF567
599      CALL THPREL (NF4)                         SARCF568
600      85 IF(IEAR. LT. NEAR) GO TO 91           SARCF569
601      IF(KEARTH .EQ. 0 .OR. NEAR .EQ. 1) GO TO 10 SARCF570
602      C                                        SARCF571
603      C   COMPUTE STATISTICS FOR DAMAGE INDICES SARCF572
604      C                                        SARCF573
605      IF(NEAR .GE. 2) CALL DSTATIS(A(KX),A(KY),NJTS,NELTOT,ICLK,IDSGN,NQSARCF574
606      1KE,NELN,DDAM,NIBAY,DBAVG,DCAVG,DEDIF,PDEDIF,ICOR,IICLK,IECHK,NOD,DSARCF575
607      1D1,DD2)                                  SARCF576
608      IF(KAUTO .LE. 0 .OR. ICHK .EQ. 0) GO TO 10 SARCF577
609      IEAR=0                                    SARCF578
610      IDSGN=IDSGN+1                             SARCF579
611      PRINT 350, IDSGN                          SARCF580
612      GO TO 91                                  SARCF581
613      350 FORMAT(1H1////,15(8H***** ),/15(8H***** ),////, SARCF582
614      110X,'NUMBERS OF CORRECTIVE DESIGN FOR COLUMNS AND BEAMS ',112///) SARCF583
615      C                                        SARCF584
616      999 STOP                                  SARCF585
617      END                                       SARCF586
618      SUBROUTINE INJTS (X,Y,ID,NJTS,NCONJT,NCDJT,NCDDOF,NCDDIS) INJTS 1
619      IMPLICIT REAL*8(A-H,O-Z)                 INJTS 2
620      C                                        INJTS 3
621      C   SET UP JOINT COORDINATES AND ID ARRAY INJTS 4
622      C                                        INJTS 5
623      COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY INJTS 6
624      COMMON/WORK/ KDOF(3),IJOINT(14),IDUM,W(1591) INJTS 7
625      C                                        INJTS 8
626      DIMENSION X(1),Y(1),ID(NJTS,1)         INJTS 9
627      C                                        INJTS 10
628      C   INITIALIZE CCORDINATES              INJTS 11
629      C                                        INJTS 12
630      DO 10 IJ=1,NJTS                           INJTS 13
631      Y(IJ)=999999.                             INJTS 14
632      10 X(IJ)=999999.                          INJTS 15
633      C                                        INJTS 16
634      C   CONTROL JOINT COORDINATES          INJTS 17
635      C                                        INJTS 18
636      PRINT 20                                  INJTS 19
637      20 FORMAT (25H1CONTROL NODE COORDINATES/// INJTS 20
638      1      5H NODE, 6X, 7HX-COORD, 6X, 7HY-COORD /) INJTS 21
639      DO 50 IJ=1,NCONJT                         INJTS 22
640      READ 30, IJT,X(IJT),Y(IJT)              INJTS 23

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641      30 FORMAT(I5,2F10.0)                                INJTS 24
642      PRINT 40, IJT,X(IJT),Y(IJT)                        INJTS 25
643      40 FORMAT (I5,2F13.3)                              INJTS 26
644      50 CONTINUE                                        INJTS 27
645      C                                                  INJTS 28
646      C      GENERATION COMMANDS                          INJTS 29
647      C                                                  INJTS 30
648      PRINT 60                                           INJTS 31
649      60 FORMAT (////25H NODE GENERATION COMMANDS  /)   INJTS 32
650      IF (NCDJT.NE.0) GO TO 80                           INJTS 33
651      PRINT 70                                           INJTS 34
652      70 FORMAT (//, 5H NONE)                            INJTS 35
653      GO TO 130                                          INJTS 36
654      80 PRINT 90                                         INJTS 37
655      90 FORMAT (/, 6H FIRST, 4X, 4HLAST, 3X, 5HNO.OF, 4X, 4HNODE, 5X,
656      1      8HDISTANCE, /, 6H NODE, 4X, 4HNODE, 3X, 5HNODES, 4X,
657      2      4HDIFF, /)                                  INJTS 40
658      DO 120 IJ=1,NCDJT                                  INJTS 41
659      READ 100,IJT,JJT,NJT,KDIF,PROP                     INJTS 42
660      100 FORMAT(4I5,F10.0)                              INJTS 43
661      IF (KDIF.EQ.0) KDIF=1                              INJTS 44
662      PRINT 110, IJT,JJT,NJT,KDIF,PROP                  INJTS 45
663      110 FORMAT (I6, 3I8, F13.3)                       INJTS 46
664      CALL LINGEN (X,Y,IJT,JJT,NJT,KDIF,PROP)           INJTS 47
665      120 CONTINUE                                       INJTS 48
666      C                                                  INJTS 49
667      C      GENERATE UNSPECIFIED JOINT COORDINATES      INJTS 50
668      C                                                  INJTS 51
669      130 IJ=1                                           INJTS 52
670      140 IJ=IJ+1                                       INJTS 53
671      IF (IJ.GT.NJTS) GO TO 160                          INJTS 54
672      IF (X(IJ).NE.999999.) GO TO 140                   INJTS 55
673      IJT=IJ-1                                           INJTS 56
674      JJT=IJT                                           INJTS 57
675      150 JJT=JJT+1                                       INJTS 58
676      IF (JJT.GT.NJTS) GO TO 160                        INJTS 59
677      IF (X(JJT).EQ.999999.) GO TO 150                 INJTS 60
678      NJT=JJT-IJT-1                                      INJTS 61
679      PROP=0.                                            INJTS 62
680      CALL LINGEN (X,Y,IJT,JJT,NJT,1,PROP)              INJTS 63
681      IJ=JJT                                             INJTS 64
682      GO TO 140                                          INJTS 65
683      160 CONTINUE                                       INJTS 66
684      C                                                  INJTS 67
685      PRINT 170, (IJ,X(IJ),Y(IJ),IJ=1,NJTS)             INJTS 68
686      170 FORMAT (26H1COMPLETE NODE COORDINATES///
687      1      5H NODE, 6X, 7HX-COORD, 6X, 7HY-COORD, //, (I5, 2F13.3))
688      C                                                  INJTS 71
689      C      INITIALIZE ID MATRIX                          INJTS 72
690      C                                                  INJTS 73
691      DO 180 I=1,NJTS                                     INJTS 74
692      DO 180 J=1,3                                        INJTS 75
693      180 ID(I,J)=0                                       INJTS 76
694      C                                                  INJTS 77
695      C      ZERO DISPLACEMENTS                           INJTS 78
696      C                                                  INJTS 79
697      PRINT 190                                           INJTS 80
698      190 FORMAT (27H1ZERO DISPLACEMENT COMMANDS  /)   INJTS 81
699      IF (NCDDOF.NE.0) GO TO 200                         INJTS 82
700      PRINT 70                                           INJTS 83
701      GO TO 280                                          INJTS 84
702      200 PRINT 210                                       INJTS 85
703      210 FORMAT (/6H FIRST, 4X, 4H X , 4X, 4H Y , 4X, 4HROTN,
704      1      4X, 4HLAST, 4X, 4HNODE, /, 6H NODE, 4X, 4HCODE, 4X,
705      2      4HCODE, 4X, 4HCODE, 4X, 4HNODE, 4X, 4HDIFF, /)
706      DO 270 IJ=1,NCDDOF                                  INJTS 89
707      READ 220,IJT,(KDOF(J),J=1,3),JJT,KDIF              INJTS 90
708      220 FORMAT(6I5)                                     INJTS 91
709      PRINT 230, IJT,(KDOF(J),J=1,3),JJT,KDIF           INJTS 92
710      230 FORMAT (I6, 5I8)                              INJTS 93
711      DO 240 J=1,3                                       INJTS 94
712      240 ID(IJT,J)=KDOF(J)                              INJTS 95
713      IF (JJT.EQ.0) GO TO 270                            INJTS 96
714      IF (KDIF.EQ.0) KDIF=1                              INJTS 97
715      NJT=(JJT-IJT)/KDIF                                 INJTS 98
716      DO 260 II=1,NJT                                     INJTS 99
717      IJT=IJT+KDIF                                       INJTS100
718      DO 250 J=1,3                                       INJTS101
719      250 ID(IJT,J)=KDOF(J)                              INJTS102
720      260 CONTINUE                                       INJTS103

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721      270 CONTINUE
722      C
723      IDENTICAL DISPLACEMENTS
724      C
725      280 CONTINUE
726      PRINT 290
727      290 FORMAT (//// 28H EQUAL DISPLACEMENT COMMANDS  /)
728      IF (NCDDIS.NE.0) GO TO 300
729      PRINT 70
730      GO TO 380
731      300 PRINT 310
732      310 FORMAT (/, 5H DISP, 3X, 5HNO.OF, /,
733      1      5H CODE, 3X, 5HNODES, 6X, 14H LIST OF NODES, /)
734      DO 370 IJ=1,NCDDIS
735      READ 320,KODOF,NJT,(IJOINT(I),I=1,NJT)
736      320 FORMAT (16I5)
737      PRINT 330, KODOF,NJT,(IJOINT(I),I=1,NJT)
738      330 FORMAT (I5, I8, 6X, 14I5)
739      II=IJOINT(1)
740      IF (ID(II,KODOF).LT.0) GO TO 350
741      DO 340 I=2,NJT
742      IK=IJOINT(I)
743      340 ID(IK,KODOF)=-II
744      GO TO 370
745      350 DO 360 I=2,NJT
746      IK=IJOINT(I)
747      360 ID(IK,KODOF)=ID(II,KODOF)
748      370 CONTINUE
749      C
750      SET UP ID ARRAY
751      C
752      380 KOUNT=0
753      DO 410 I=1,NJTS
754      DO 410 J=1,3
755      IF (ID(I,J).NE.0) GO TO 390
756      KOUNT=KOUNT+1
757      ID(I,J)=KOUNT
758      GO TO 410
759      390 IF (ID(I,J).NE.1) GO TO 400
760      ID(I,J)=0
761      GO TO 410
762      400 II=-ID(I,J)
763      ID(I,J)=ID(II,J)
764      410 CONTINUE
765      C
766      PRINT 420, (I,(ID(I,J),J=1,3),I=1,NJTS)
767      420 FORMAT(24H1ID ARRAY (FOR INTEREST)///
768      1      5H NODE,7X,1HX,7X,1HY,7X,1HR/(15,318))
769      C
770      NEQ=KOUNT
771      KOUNT=KOUNT+1
772      DO 430 I=1,NJTS
773      DO 430 J=1,3
774      IF (ID(I,J).EQ.0) ID(I,J)=KOUNT
775      430 CONTINUE
776      C
777      RETURN
778      END
779      SUBROUTINE LINGEN (X,Y,IJT,JJT,NJT,KDIF,PROP)
780      IMPLICIT REAL*8(A-H,O-Z)
781      C
782      SUBROUTINE TO GENERATE JOINTS ALONG STRAIGHT LINE
783      C
784      DIMENSION X(1),Y(1)
785      C
786      XI=X(IJT)
787      YI=Y(IJT)
788      DX=X(JJT)-XI
789      DY=Y(JJT)-YI
790      IF (PROP.LT.1.) GO TO 10
791      PROP=PROP/DSQRT(DX**2+DY**2)
792      10 IF (PROP.EQ.0.) PROP=1./DFLOAT(NJT+1)
793      DX=DX*PROP
794      DY=DY*PROP
795      C
796      DO 20 IJ=1,NJT
797      IJT=IJT+KDIF
798      XI=XI+DX
799      YI=YI+DY
800      X(IJT)=XI

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INJTS104
INJTS105
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LINGE 22

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801		20 Y(IJT)=YI	LINGE 23
802	C		LINGE 24
803		RETURN	LINGE 25
804		END	LINGE 26
805		SUBROUTINE INMASS (FM,IEQFM,ID,NCDMS,NJTS)	INMAS 1
806		IMPLICIT REAL*8(A-H,O-Z)	INMAS 2
807	C		INMAS 3
808	C	SET UP MASS MATRIX	INMAS 4
809	C		INMAS 5
810		COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY	INMAS 6
811		COMMON/WORK/ FMAS(3),W(1597)	INMAS 7
812	C		INMAS 8
813		DIMENSION FM(1),IEQFM(1),ID(NJTS,1)	INMAS 9
814	C		INMAS 10
815		NEQ1=NEQ+1	INMAS 11
816		DO 10 J=1,NEQ1	INMAS 12
817		IEQFM(J)=1	INMAS 13
818		10 FM(J)=0.	INMAS 14
819	C		INMAS 15
820		PRINT 20	INMAS 16
821		20 FORMAT (25H1MASS GENERATION COMMANDS///	INMAS 17
822		1 6H FIRST,8X,6H X ,8X,6H Y ,10X,4HROTN,4X,4X,4HLAST,	INMAS 18
823		2 4X, 4HNODE, 4X, 9HMODIFYING/	INMAS 19
824		3 6H NODE, 8X, 6H MASS, 8X, 6H MASS, 10X, 4HMASS, 4X,	INMAS 20
825		4 4X, 4HNODE, 4X, 4HDIFF, 4X, 9H FACTOR /)	INMAS 21
826		DO 90 IJ=1,NCDMS	INMAS 22
827	C		INMAS 23
828		READ 30, IJT, (FMAS(I), I=1,3), JJT, KDIF, SSCALE	INMAS 24
829		30 FORMAT(15,3F10.0,2I5,F10.0)	INMAS 25
830		IF (SSCALE.EQ.0.) GO TO 40	INMAS 26
831		SCALE=SSCALE	INMAS 27
832		40 PRINT 50, IJT, (FMAS(I), I=1,3), JJT, KDIF, SCALE	INMAS 28
833		50 FORMAT(16,3E14.4,4X,2I8,F13.2)	INMAS 29
834	C		INMAS 30
835		DO 60 J=1,3	INMAS 31
836		IF (FMAS(J).EQ.0.) GO TO 60	INMAS 32
837		IEQ=ID(IJT,J)	INMAS 33
838		FM(IEQ)=FM(IEQ)+FMAS(J)/SCALE	INMAS 34
839		IEQFM(IEQ)=J+1	INMAS 35
840		60 CONTINUE	INMAS 36
841	C		INMAS 37
842		IF (JJT.EQ.0) GO TO 90	INMAS 38
843		IF (KDIF.EQ.0) KDIF=1	INMAS 39
844		NJT=(JJT-IJT)/KDIF	INMAS 40
845		DO 80 IK=1,NJT	INMAS 41
846		IJT=IJT+KDIF	INMAS 42
847		DO 70 J=1,3	INMAS 43
848		IF (FMAS(J).EQ.0.) GO TO 70	INMAS 44
849		IEQ=ID(IJT,J)	INMAS 45
850		FM(IEQ)=FM(IEQ)+FMAS(J)/SCALE	INMAS 46
851		IEQFM(IEQ)=J+1	INMAS 47
852		70 CONTINUE	INMAS 48
853		80 CONTINUE	INMAS 49
854	C		INMAS 50
855		90 CONTINUE	INMAS 51
856		FM(NEQ+1)=0.	INMAS 52
857	C		INMAS 53
858		PRINT 100	INMAS 54
859		100 FORMAT (////22H COMPLETE NODAL MASSES///	INMAS 55
860		1 5H NODE,11X, 6HX-MASS,11X, 6HY-MASS,11X, 6HR-MASS /)	INMAS 56
861	C		INMAS 57
862		DO 140 IJ=1,NJTS	INMAS 58
863		DO 120 J=1,3	INMAS 59
864		IEQ=ID(IJ,J)	INMAS 60
865		IJT=IEQFM(IEQ)	INMAS 61
866		IF (IJT.LE.1) GO TO 110	INMAS 62
867		FMAS(J)=FM(IEQ)	INMAS 63
868		IEQFM(IEQ)=-IJT	INMAS 64
869		GO TO 120	INMAS 65
870		110 FMAS(J)=0.	INMAS 66
871		120 CONTINUE	INMAS 67
872	C		INMAS 68
873		PRINT 130, IJ, (FMAS(J), J=1,3)	INMAS 69
874		130 FORMAT (15, 3F17.6)	INMAS 70
875	C		INMAS 71
876		140 CONTINUE	INMAS 72
877	C		INMAS 73
878		DO 150 J=1,NEQ	INMAS 74
879		150 IEQFM(J)=IABS(IEQFM(J))	INMAS 75
880	C		INMAS 76

881	RETURN	INMAS 77
882	END	INMAS 78
883	SUBROUTINE INEXLD (D,ID,NCCLD,NJTS,IEQFM)	INEXL 1
884	IMPLICIT REAL*8(A-H,O-Z)	INEXL 2
885	C	INEXL 3
886	C SET UP STATIC LOADS DIRECTLY ON JOINTS	INEXL 4
887	C	INEXL 5
888	COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY	INEXL 6
889	COMMON/WORK/ FLD(3),W(1597)	INEXL 7
890	C	INEXL 8
891	DIMENSION D(1),ID(NJTS,1),IEQFM(1)	INEXL 9
892	C	INEXL 10
893	DO 10 I=1,NEQ	INEXL 11
894	10 D(I)=0.	INEXL 12
895	D(NEQ+1)=0.	INEXL 13
896	IF (NCCLD.EQ.0) RETURN	INEXL 14
897	PRINT 20	INEXL 15
898	20 FORMAT(29H1STATIC NODAL LOAD GENERATION///	INEXL 16
899	1 6H FIRST,8X,6H X ,8X,6H Y ,8X,6HMOMENT,8X,4HLAST,	INEXL 17
900	2 4X,4HNODE/	INEXL 18
901	3 6H NODE,8X,6H LOAD,8X,6H LOAD,8X,6H LOAD ,8X,4HNODE,	INEXL 19
902	4 4X,4HDIFF/)	INEXL 20
903	C	INEXL 21
904	DO 80 IJ=1,NCCLD	INEXL 22
905	C	INEXL 23
906	READ 30,IJT,(FLD(I),I=1,3),JJT,KDIF	INEXL 24
907	30 FORMAT(15,3F10.0,2I5)	INEXL 25
908	PRINT 40, IJT,(FLD(I),I=1,3),JJT,KDIF	INEXL 26
909	40 FORMAT (16, 3F14.3, 4X, 2I8)	INEXL 27
910	DO 50 J=1,3	INEXL 28
911	IEQ=ID(IJT,J)	INEXL 29
912	50 D(IEQ)=D(IEQ)+FLD(J)	INEXL 30
913	C	INEXL 31
914	IF (JJT.EQ.0) GO TO 80	INEXL 32
915	IF (KDIF.EQ.0) KDIF=1	INEXL 33
916	NJT=(JJT-IJT)/KDIF	INEXL 34
917	DO 70 IK=1,NJT	INEXL 35
918	IJT=IJT+KDIF	INEXL 36
919	DO 60 J=1,3	INEXL 37
920	IEQ=ID(IJT,J)	INEXL 38
921	60 D(IEQ)=D(IEQ)+FLD(J)	INEXL 39
922	70 CONTINUE	INEXL 40
923	80 CONTINUE	INEXL 41
924	D(NEQ+1)=0.	INEXL 42
925	C	INEXL 43
926	PRINT 90	INEXL 44
927	90 FORMAT(///28H COMPLETE STATIC NODAL LOADS///	INEXL 45
928	1 5H NODE, 6X, 6HX-LOAD, 6X, 6HY-LOAD, 6X, 6HMOMENT/)	INEXL 46
929	C	INEXL 47
930	DO 130 IJ=1,NJTS	INEXL 48
931	DO 110 J=1,3	INEXL 49
932	IEQ=ID(IJ,J)	INEXL 50
933	IJT=IEQFM(IEQ)	INEXL 51
934	IF (IJT.LT.0) GO TO 100	INEXL 52
935	FLD(J)=D(IEQ)	INEXL 53
936	IEQFM(IEQ)=-IJT	INEXL 54
937	GO TO 110	INEXL 55
938	100 FLD(J)=0.	INEXL 56
939	110 CONTINUE	INEXL 57
940	C	INEXL 58
941	PRINT 120, IJ,(FLD(J),J=1,3)	INEXL 59
942	120 FORMAT (15, 3F12.3)	INEXL 60
943	130 CONTINUE	INEXL 61
944	C	INEXL 62
945	DO 140 J=1,NEQ	INEXL 63
946	140 IEQFM(J)=IABS(IEQFM(J))	INEXL 64
947	C	INEXL 65
948	RETURN	INEXL 66
949	END	INEXL 67
950	SUBROUTINE INAXL (KFORM,TH,GH,TV,GV,GAXH,GAXV,NSTEPS,DT,FACAXH,FACINAXL	INAXL 1
951	1TMH,FACAXV,FACTMV,KEARTH,IEAR)	INAXL 2
952	IMPLICIT REAL*8(A-H,O-Z)	INAXL 3
953	C	INAXL 4
954	C SET UP EARTHQUAKE RECORDS	INAXL 5
955	C	INAXL 6
956	COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY	INAXL 7
957	COMMON/EQUAKE/DSEED,PGA,WG,TAU,UWG,PG,IEVL,KIEVL,ENA,ENB,ENC	INAXL 8
958	C	INAXL 9
959	DIMENSION KFORM(1),TH(1),GH(1),TV(1),GV(1),GAXH(1),GAXV(1),	INAXL 10
960	1 IEQFM(1)	INAXL 11

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961      C                                     INAXL 12
962      DATA XPR, YPR /3H X , 3H Y /          INAXL 13
963      C                                     INAXL 14
964      IF (NPTH.EQ.0) GO TO 80                INAXL 15
965      IF (KEARTH.EQ.0) GO TO 20             INAXL 16
966      CALL QUAKE(TH,GH,NPTH,FACAXH,FACTMH,IEAR) INAXL 17
967      GO TO 35                               INAXL 18
968      20 READ 30, (TH(I),GH(I),I=1,NPTH)     INAXL 19
969      30 FORMAT(6(F6.3,F7.3))                INAXL 20
970      35 IF (KFORM(1).NE.0) PRINT 40, XPR,(TH(I),GH(I),I=1,NPTH) INAXL 21
971      40 FORMAT (24H1GROUND ACCELERATIONS IN, A3, 19HDIRECTION, AS INPUT//INAXL 22
972      1      5(4X, 4HTIME, 7X, 5HACCEL, 3X) // INAXL 23
973      2      (5(F 8.3, F12.4, 3X)))          INAXL 24
974      C                                     INAXL 25
975      DO 50 I=1,NPTH                          INAXL 26
976      GH(I)=GH(I)*FACAXH                     INAXL 27
977      50 TH(I)=TH(I)*FACTMH                 INAXL 28
978      C                                     INAXL 29
979      55 IF (NSTEPS.LE.0) GO TO 80           INAXL 30
980      CALL INTPOL (TH,GH,GAXH,DT,NSTEPS)     INAXL 31
981      C                                     INAXL 32
982      IF (KFORM(2).NE.0) PRINT 60, XPR,(N,GAXH(N),N=1,NSTEPS) INAXL 33
983      60 FORMAT (24H1GROUND ACCELERATIONS IN, A3, INAXL 34
984      1      37HDIRECTION, AS SCALED AND INTERPOLATED /// INAXL 35
985      2      5(5H STEP, 7X, 5HACCEL, 5X) // INAXL 36
986      3      (5(I5, F12.3, 5X)))          INAXL 37
987      C                                     INAXL 38
988      GA=0.                                  INAXL 39
989      DO 70 I=1,NSTEPS                       INAXL 40
990      GAXH(I)=GAXH(I)-GA                    INAXL 41
991      70 GA=GAXH(I)+GA                      INAXL 42
992      C                                     INAXL 43
993      80 IF (NPTV.EQ.0) GO TO 130           INAXL 44
994      IF (KEARTH.EQ.0) GO TO 100           INAXL 45
995      CALL QUAKE(TV,GV,NPTV,FACAXV,FACTMV,IEAR) INAXL 46
996      GO TO 105                              INAXL 47
997      100 READ 30, (TV(I),GV(I),I=1,NPTV)   INAXL 48
998      105 IF (KFORM(1).NE.0) PRINT 40, YPR,(TV(N),GV(N),N=1,NPTV) INAXL 49
999      C                                     INAXL 50
1000     DO 110 I=1,NPTV                       INAXL 51
1001     GV(I)=GV(I)*FACAXV                    INAXL 52
1002     110 TV(I)=TV(I)*FACTMV                INAXL 53
1003     C                                     INAXL 54
1004     IF (NSTEPS.LE.0) GO TO 130           INAXL 55
1005     CALL INTPOL (TV,GV,GAXV,DT,NSTEPS)     INAXL 56
1006     C                                     INAXL 57
1007     IF (KFORM(2).NE.0) PRINT 60, YPR,(N,GAXV(N),N=1,NSTEPS) INAXL 58
1008     C                                     INAXL 59
1009     GA=0.                                  INAXL 60
1010     DO 120 I=1,NSTEPS                     INAXL 61
1011     GAXV(I)=GAXV(I)-GA                    INAXL 62
1012     120 GA=GAXV(I)+GA                      INAXL 63
1013     C                                     INAXL 64
1014     130 RETURN                             INAXL 65
1015     END                                     INAXL 66
1016     SUBROUTINE RANINT(N,IR)                 RANIN 1
1017     IMPLICIT REAL*8(A-H,O-Z)               RANIN 2
1018     DIMENSION IR(1)                        RANIN 3
1019     C                                     RANIN 4
1020     DSEED=123457.0D0                       RANIN 5
1021     K=2147483647                            RANIN 6
1022     CALL GGUD(DSEED,K,N,IR)                 RANIN 7
1023     RETURN                                   RANIN 8
1024     END                                     RANIN 9
1025     SUBROUTINE QUAKE(T,G,N,FACA,FACB,IEAR)  QUAKE 1
1026     IMPLICIT REAL*8(A-H,O-Z)               QUAKE 2
1027     COMMON/EQUAKE/DSEED,PGA,WG,TAU,UWG,PG,IEVL,KIEVL,ENA,ENB,ENC QUAKE 3
1028     COMPLEX*16 AGL,A(1024)                 QUAKE 4
1029     DIMENSION G(N),T(N),RR(1024),WK(6294),IWK(6294) QUAKE 5
1030     PARAMETER(PI=3.14159D0)                QUAKE 6
1031     EQUIVALENCE(IWK(1),WK(1))              QUAKE 7
1032     C                                     QUAKE 8
1033     EN=DFLOAT(N)                            QUAKE 9
1034     DF=UWG/(2.D0*PI*EN)                     QUAKE 10
1035     DT=1.D0/(DF*EN)                          QUAKE 11
1036     FG=WG/(2.D0*PI)                          QUAKE 12
1037     C                                     QUAKE 13
1038     SO=(PGA**2)/((PG**2)*PI*WG*(0.5/TAU+2.D0*TAU)) QUAKE 14
1039     C                                     QUAKE 15
1040     DO 10 I=1,N                              QUAKE 16

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1041      T(I)=DT*DFLOAT(I-1)/FACB          QUAKE 17
1042      G(I)=DF*DFLOAT((I))              QUAKE 18
1043      ALPHA=4.DO*((G(I)*FG*TAU)**2)     QUAKE 19
1044      10 G(I)=2.DO*SO*(FG**4+ALPHA)/((G(I)**2-FG**2)**2+ALPHA) QUAKE 20
1045      C                                  QUAKE 21
1046      CALL RANAGL(N,RR,PI,DSEED)        QUAKE 22
1047      C                                  QUAKE 23
1048      DO 40 I=1,N                        QUAKE 24
1049      AGL=DCMLX(DCOS(RR(I)),DSIN(-RR(I))) QUAKE 25
1050      40 A(I)=DSQRT(G(I)*2.DO*PI*DF)*AGL QUAKE 26
1051      C                                  QUAKE 27
1052      CALL FFTCC(A,N,IWK,WK)            QUAKE 28
1053      C                                  QUAKE 29
1054      DO 50 I=1,N                        QUAKE 30
1055      50 G(I)=DREAL(DSQRT(2.DO)*A(I))    QUAKE 31
1056      C                                  QUAKE 32
1057      IF(IEVL .GE. 2) GO TO 70          QUAKE 33
1058      DO 60 I=1,N                        QUAKE 34
1059      EI=DFLOAT(I-1)*DT                QUAKE 35
1060      EG=1.DO                           QUAKE 36
1061      IF(T(I) .LE. ENA) EG=(1.DO/ENA)*EI QUAKE 37
1062      IF(T(I) .GE. ENB) EG=(1.DO/(ENC-ENB))*(ENB-EI)+1.DO QUAKE 38
1063      IF(T(I) .GE. ENC) EG=0.DO         QUAKE 39
1064      60 G(I)=G(I)*EG/FACA              QUAKE 40
1065      GO TO 90                           QUAKE 41
1066      C                                  QUAKE 42
1067      70 TO=1.DO/(ENA-ENB)*DLOG(ENA/ENB) QUAKE 43
1068      GAMMA=1.DO/(EXP(-ENA*TO)-EXP(-ENB*TO)) QUAKE 44
1069      DO 80 I=1,N                        QUAKE 45
1070      EI=DFLOAT(I-1)*DT                QUAKE 46
1071      EG=GAMMA*(EXP(-ENA*T(I))-EXP(-ENB*T(I))) QUAKE 47
1072      G(I)=G(I)*EG/FACA                 QUAKE 48
1073      80 CONTINUE                       QUAKE 49
1074      C                                  QUAKE 50
1075      90 RETURN                          QUAKE 51
1076      END                                QUAKE 52
1077      SUBROUTINE RANAGL(N,RR,PI,DSEED)   RANAG 1
1078      DOUBLE PRECISION DSEED,RR(N),PI   RANAG 2
1079      REAL RAN(1024)                     RANAG 3
1080      C                                  RANAG 4
1081      DO 10 I=1,N                        RANAG 5
1082      10 RAN(I)=GGUBFS(DSEED)           RANAG 6
1083      C                                  RANAG 7
1084      DO 20 I=1,N                        RANAG 8
1085      RR(I)=RAN(I)*2.DO*PI              RANAG 9
1086      20 CONTINUE                       RANAG 10
1087      RETURN                             RANAG 11
1088      END                                  RANAG 12
1089      SUBROUTINE INTPOL (T,G,GAX,DT,NPMAX) INTPO 1
1090      IMPLICIT REAL*8(A-H,O-Z)          INTPO 2
1091      C                                  INTPO 3
1092      C      INTERPOLATE EARTHQUAKE DATA INTPO 4
1093      C                                  INTPO 5
1094      DIMENSION T(1),G(1),GAX(1)       INTPO 6
1095      C                                  INTPO 7
1096      N=1                                 INTPO 8
1097      TIM=0.                             INTPO 9
1098      DO 30 MSTEP=1,NPMAX               INTPO 10
1099      TIM=TIM+DT                         INTPO 11
1100      10 IF (TIM.LE.T(N+1)) GO TO 20   INTPO 12
1101      N=N+1                              INTPO 13
1102      GO TO 10                           INTPO 14
1103      20 PP=(TIM-T(N))/(T(N+1)-T(N))   INTPO 15
1104      30 GAX(MSTEP)=PP*G(N+1)+(1.-PP)*G(N) INTPO 16
1105      C                                  INTPO 17
1106      RETURN                             INTPO 18
1107      END                                  INTPO 19
1108      SUBROUTINE CONSTANT                CONST 1
1109      IMPLICIT REAL*8(A-H,O-Z)          CONST 2
1110      C                                  CONST 3
1111      C      DAMPING DATA AND INTEGRATION COEFFICIENTS CONST 4
1112      C                                  CONST 5
1113      COMMON/DAMP/ ALPHA,BETA,DT,GAXCTE,CON1,CON2,CON3,CON4,CON5, CONST 6
1114      1      C1,C2,C3,C4,C5,C6,C7,C8,C9,C10,C11,C12,BETA0,DELTA CONST 7
1115      C                                  CONST 8
1116      C      COEFFICIENTS FOR CONSTANT ACCN METHOD CONST 9
1117      C                                  CONST 10
1118      CON1=4./DT**2                     CONST 11
1119      CON2=2./DT                         CONST 12
1120      CON3=4./DT                         CONST 13

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1121      CON4=2.                                CONST 14
1122      CON5=0.                                CONST 15
1123      C                                         CONST 16
1124      C6=1.0/(1.0+BETA*CON2)                 CONST 17
1125      C1=(CON1+ALPHA*CON2)*C6                 CONST 18
1126      C2=CON4+ALPHA*CON5                       CONST 19
1127      C3=CON3+ALPHA*CON4                       CONST 20
1128      C4=CON5*BETA*C1                           CONST 21
1129      C5=CON4*BETA*C1                           CONST 22
1130      C7=CON4*BETA*C6                           CONST 23
1131      C8=CON5*BETA*C6                           CONST 24
1132      C9=C2-C4                                   CONST 25
1133      C10=C3-C5                                  CONST 26
1134      C11=CON4*BETA*C6                           CONST 27
1135      C12=CON2*BETA*C6                           CONST 28
1136      C                                         CONST 29
1137      RETURN                                    CONST 30
1138      END                                        CONST 31
1139      SUBROUTINE OUTJT (LJTH,LJTV,LJTR,ID,NJTS)  OUTJT  1
1140      IMPLICIT REAL*8(A-H,O-Z)                 OUTJT  2
1141      C                                         OUTJT  3
1142      C LIST OUTPUT JOINTS FOR TIME HISTORY     OUTJT  4
1143      C                                         OUTJT  5
1144      COMMON/OUTN/ IPJ,IPE,KNTJ,KNTE,NHOUT,NVOUT,NROUT OUTJT  6
1145      C                                         OUTJT  7
1146      DIMENSION LJTH(1),LJTV(1),LJTR(1),ID(NJTS,1) OUTJT  8
1147      C                                         OUTJT  9
1148      IF (NHOUT.EQ.0) GO TO 30                   OUTJT 10
1149      READ 10,(LJTH(I),I=1,NHOUT)                OUTJT 11
1150      10 FORMAT(10I5)                            OUTJT 12
1151      PRINT 20, (LJTH(I),I=1,NHOUT)              OUTJT 13
1152      20 FORMAT (//// 26H NODES FOR X DISPL HISTORY// OUTJT 14
1153      1      (5X, 20I5))                          OUTJT 15
1154      30 IF (NVOUT.EQ.0) GO TO 60                 OUTJT 16
1155      READ 40,(LJTV(I),I=1,NVOUT)                OUTJT 17
1156      40 FORMAT(10I5)                            OUTJT 18
1157      PRINT 50, (LJTV(I),I=1,NVOUT)              OUTJT 19
1158      50 FORMAT (//// 26H NODES FOR Y DISPL HISTORY// OUTJT 20
1159      1      (5X, 20I5))                          OUTJT 21
1160      60 IF (NROUT.EQ.0) GO TO 90                 OUTJT 22
1161      READ 70,(LJTR(I),I=1,NROUT)                OUTJT 23
1162      70 FORMAT(10I5)                            OUTJT 24
1163      PRINT 80, (LJTR(I),I=1,NROUT)              OUTJT 25
1164      80 FORMAT (//// 27H NODES FOR ROTATION HISTORY// OUTJT 26
1165      1      (5X, 20I5))                          OUTJT 27
1166      90 RETURN                                    OUTJT 28
1167      END                                        OUTJT 29
1168      SUBROUTINE INELEM (ID,X,Y,M,NJTS,NELTOT,ITY,STIN,CONIN,SECIN,DDIN,INELE  1
1169      1RHOM,YBM,NELG,NELN,IP,KIP,PR,PPR,DMY)     INELE  2
1170      IMPLICIT REAL*8(A-H,O-Z)                 INELE  3
1171      C                                         INELE  4
1172      C INPUT ELEMENT DATA                     INELE  5
1173      C                                         INELE  6
1174      COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY INELE  7
1175      COMMON/GENINF/KCONT(10),KELEM(10),NELEM(10),NINF(10),NDOF(10) INELE  8
1176      1      ,FCONT(3),NUMEM(10)                 INELE  9
1177      COMMON/PASS/ IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM,IDUM(4) INELE 10
1178      COMMON/STOR/ NAVST,NF1,NF2,NF3,NF4,NTST,KODST,KDATA,NF17 INELE 11
1179      COMMON/DAMAGE/KDAMAGE,ITDAM,KIDAMT,NNSKIP,NSSKIP,NGSKIP,GLHYS, INELE 12
1180      1GLDAM                                     INELE 13
1181      COMMON/AUTO/KAUTO,KAUTOD,KECO,KECOD,NDSGN,NDSGND,KFREQ,KFREQD, INELE 14
1182      1 DBALL,DCALL,DBSTD,CONC,STEEL,IECO,BMAVG,BMDEV,ICONV INELE 15
1183      C                                         INELE 16
1184      DIMENSION STIN(6,1),CONIN(9,1),SECIN(9,1),DDIN(2,1),YBM(2,NELG,1), INELE 17
1185      1ITY(3,1),RHOM(2,NELG,1),DMY(NELG,1) INELE 18
1186      DIMENSION M(1),IP(1),KIP(1),PR(1),PPR(2,1) INELE 19
1187      DIMENSION X(1),Y(1),ID(NJTS,1)           INELE 20
1188      C                                         INELE 21
1189      NEQ1=NEQ+1                                  INELE 22
1190      DO 10 I=1,NEQ1                              INELE 23
1191      10 M(I)=I                                    INELE 24
1192      C                                         INELE 25
1193      C STORE NODE COORDINATES FOR THE PLOT OF DAMAGE INDEX INELE 26
1194      C                                         INELE 27
1195      IF(KDAPT.EQ.0) GO TO 15                     INELE 28
1196      C                                         INELE 29
1197      C PRINT NUMBER OF STORY AND BAY FOR A OUTPUT OF DAMAGE INDEX INELE 30
1198      C                                         INELE 31
1199      15 IC=0                                       INELE 32
1200      NELTOT=0                                       INELE 33

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1201	C		INELE 34
1202		CONC=0.0	INELE 35
1203		STEEL=0.0	INELE 36
1204		DO 140 IGR=1,NELGR	INELE 37
1205		READ 20,KCONT,FCONT	INELE 38
1206	20	FORMAT(1015,3E10.0)	INELE 39
1207		PRINT 30, IGR	INELE 40
1208	30	FORMAT(29H1ELEMENT SPECIFICATION, GROUP,13//)	INELE 41
1209		KEL=KCONT(1)	INELE 42
1210		KELEM(IGR)=KEL	INELE 43
1211		NELEM(IGR)=KCONT(2)	INELE 44
1212		NELTOT=NELTOT+KCONT(2)	INELE 45
1213	C		INELE 46
1214		GO TO (40,50), KEL	INELE 47
1215	40	CALL INELL(KCONT,FCONT,NDOF(IGR),NINF(IGR),ID,X,Y,NJTS,CONC,STEEL,	INELE 48
1216		1ITY,STIN,CONIN,SECIN,DDIN,RHOM,YBM,NELG,NELN,IP,KIP,PR,PPR,DMY)	INELE 49
1217		GO TO 140	INELE 50
1218	50	CALL INELL(KCONT,FCONT,NDOF(IGR),NINF(IGR),ID,X,Y,NJTS,CONC,STEEL,	INELE 51
1219		1ITY,STIN,CONIN,SECIN,DDIN,RHOM,YBM,NELG,NELN,IP,KIP,PR,PPR,DMY)	INELE 52
1220	140	CONTINUE	INELE 53
1221	C		INELE 54
1222		REWIND NF17	INELE 55
1223		REWIND NF2	INELE 56
1224		CALL MODIFY (M,NEQ,NSTO)	INELE 57
1225	C		INELE 58
1226		RETURN	INELE 59
1227		END	INELE 60
1228		SUBROUTINE CONSOL (IAD,BL,LSOFAR)	CONSO 1
1229		IMPLICIT REAL*8(A-H,O-Z)	CONSO 2
1230	C		CONSO 3
1231	C	COMPACT ELEMENT DATA IN AVAILABLE STORAGE, INITIALIZE ARRAYS	CONSO 4
1232	C		CONSO 5
1233		COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY	CONSO 6
1234		COMMON/GENINF/KCONT(10),KELEM(10),NELEM(10),NINF(10),NDOF(10)	CONSO 7
1235	1	,FCONT(3),NUMEM(10)	CONSO 8
1236		COMMON/STOR/ NAVST,NF1,NF2,NF3,NF4,NTST,KODST,KDATA	CONSO 9
1237		COMMON/PASS/ IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM, IDUM(4)	CONSO 10
1238	C		CONSO 11
1239		DIMENSION IAD(1),BL(1)	CONSO 12
1240	C		CONSO 13
1241		NBLOK=1	CONSO 14
1242		IC=0	CONSO 15
1243		NSELM=LSOFAR	CONSO 16
1244	C		CONSO 17
1245		DO 70 IGR=1,NELGR	CONSO 18
1246		NEL=NELEM(IGR)	CONSO 19
1247		NIN=NINF(IGR)	CONSO 20
1248		IF (NIN.LE.NAVST) GO TO 20	CONSO 21
1249		PRINT 10	CONSO 22
1250	10	FORMAT(40H1INSUFFICIENT STORAGE FOR SINGLE ELEMENT)	CONSO 23
1251		NAVST=NIN	CONSO 24
1252	20	DO 60 IEL=1,NEL	CONSO 25
1253		NSELM=NSELM+NIN	CONSO 26
1254		IC=IC+1	CONSO 27
1255		IF (IC.GT.1) GO TO 30	CONSO 28
1256		IAD(IC)=1	CONSO 29
1257		KBB=1	CONSO 30
1258		GO TO 40	CONSO 31
1259	30	IAD(IC)=IAD(IC-1)+NINP	CONSO 32
1260		KBB=IAD(IC)	CONSO 33
1261	40	IF (KBB+NIN.LE.NAVST) GO TO 50	CONSO 34
1262		CALL STORE (BL(1),NAVST,NF1,2)	CONSO 35
1263		NBLOK=NBLOK+1	CONSO 36
1264		IAD(IC)=1	CONSO 37
1265		KBB=1	CONSO 38
1266	50	CALL STORE (BL(KBB),NIN,NF2,1)	CONSO 39
1267	C		CONSO 40
1268	60	NINP=NIN	CONSO 41
1269	70	CONTINUE	CONSO 42
1270	C		CONSO 43
1271		PRINT 80, LSOFAR,NBLOK,NSELM	CONSO 44
1272	80	FORMAT(21H1STORAGE REQUIREMENTS///	CONSO 45
1273	1	5X,34HSTORAGE EXCLUDING ELEMENT DATA =I6//	CONSO 46
1274	2	5X,34HNUMBER OF BLOCKS OF ELEMENT DATA =I6//	CONSO 47
1275	3	5X,34HTOTAL STORAGE IF SINGLE BLOCK =I6)	CONSO 48
1276	C		CONSO 49
1277		IF (NBLOK.GT.1) CALL STORE (BL(1),NAVST,NF1,2)	CONSO 50
1278		REWIND NF1	CONSO 51
1279		REWIND NF2	CONSO 52
1280	C		CONSO 53

1281	RETURN	CONSO 54
1282	END	CONSO 55
1283	SUBROUTINE FINISH	FINIS 1
1284	IMPLICIT REAL*8(A-H,O-Z)	FINIS 2
1285	C	FINIS 3
1286	COMMON/GENINF/KCONT(10),KELEM(10),NELEM(10),NINF(10),NDOF(10)	FINIS 4
1287	1 ,FCONT(3),NUMEM(10)	FINIS 5
1288	COMMON/PASS/ IGR, ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM, IDUM(4)	FINIS 6
1289	COMMON/STOR/ NAVST,NF1,NF2,NF3,NF4,NTST,KODST,KDATA,NF17	FINIS 7
1290	COMMON/INFEL/ COM(215)	FINIS 8
1291	C	FINIS 9
1292	CALL STORE (COM,NINF(IGR),NF2,2)	FINIS 10
1293	CALL STORE (COM,NINF(IGR),NF17,2)	FINIS 11
1294	C	FINIS 12
1295	RETURN	FINIS 13
1296	END	FINIS 14
1297	SUBROUTINE STORE (A,N,NF,K)	STORE 1
1298	IMPLICIT REAL*8(A-H,O-Z)	STORE 2
1299	C	STORE 3
1300	DIMENSION A(N)	STORE 4
1301	C	STORE 5
1302	GO TO (10,20), K	STORE 6
1303	10 READ (NF) A	STORE 7
1304	RETURN	STORE 8
1305	20 WRITE (NF) A	STORE 9
1306	C	STORE 10
1307	RETURN	STORE 11
1308	END	STORE 12
1309	SUBROUTINE BAND	BAND 1
1310	IMPLICIT REAL*8(A-H,O-Z)	BAND 2
1311	C	BAND 3
1312	COMMON/GENINF/KCONT(10),KELEM(10),NELEM(10),NINF(10),NDOF(10)	BAND 4
1313	1 ,FCONT(3),NUUMEM(10)	BAND 5
1314	COMMON/PASS/ IGR, ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM, IDUM(4)	BAND 6
1315	COMMON/INFEL/ IMEM,IMEMD,KST,KSTD,LM(1),LMD(1),DUM(212)	BAND 7
1316	COMMON A(1)	BAND 8
1317	C	BAND 9
1318	CALL SBAND (A(KM),LM,NDOF(IGR))	BAND 10
1319	C	BAND 11
1320	RETURN	BAND 12
1321	END	BAND 13
1322	SUBROUTINE SBAND (M,LM,NDF)	SBAND 1
1323	IMPLICIT REAL*8(A-H,O-Z)	SBAND 2
1324	C	SBAND 3
1325	DIMENSION M(1),LM(1)	SBAND 4
1326	C	SBAND 5
1327	DO 10 J=1,NDF	SBAND 6
1328	JJ=LM(J)	SBAND 7
1329	NN=M(JJ)	SBAND 8
1330	C	SBAND 9
1331	DO 10 I=1,NDF	SBAND 10
1332	II=LM(I)	SBAND 11
1333	IF (JJ.LT.II.OR.II.GE.NN) GO TO 10	SBAND 12
1334	M(JJ)=II	SBAND 13
1335	NN=II	SBAND 14
1336	10 CONTINUE	SBAND 15
1337	C	SBAND 16
1338	RETURN	SBAND 17
1339	END	SBAND 18
1340	SUBROUTINE MODIFY (M,NEQ,NSTO)	MODIF 1
1341	IMPLICIT REAL*8(A-H,O-Z)	MODIF 2
1342	C	MODIF 3
1343	DIMENSION M(1)	MODIF 4
1344	C	MODIF 5
1345	NSTO=0	MODIF 6
1346	DO 10 J=1,NEQ	MODIF 7
1347	NSTO=NSTO+1+J-M(J)	MODIF 8
1348	10 M(J)=NSTO	MODIF 9
1349	C	MODIF 10
1350	RETURN	MODIF 11
1351	END	MODIF 12
1352	SUBROUTINE MULTST (A,ST,ATK,FK,NN,MM)	MULTS 1
1353	IMPLICIT REAL*8(A-H,O-Z)	MULTS 2
1354	C	MULTS 3
1355	DIMENSION FK(NN,1),A(MM,1),ST(MM,1),ATK(NN,1)	MULTS 4
1356	C	MULTS 5
1357	C	MULTS 6
1358	FORM MATRIX PRODUCTS A(T)*ST=ATK AND ATK*A=FK	MULTS 7
1359	DO 10 I=1,NN	MULTS 8
1360	DO 10 J=1,MM	MULTS 9

1361		10 ATK(I,J)=0.	MULTS 10
1362	C		MULTS 11
1363		DO 30 K=1,MM	MULTS 12
1364		DO 30 I=1,NN	MULTS 13
1365		AA=A(K,I)	MULTS 14
1366		IF (AA.EQ.0.) GO TO 30	MULTS 15
1367		DO 20 J=1,MM	MULTS 16
1368		20 ATK(I,J)=ATK(I,J)+AA*ST(K,J)	MULTS 17
1369		30 CONTINUE	MULTS 18
1370	C		MULTS 19
1371		DO 40 I=1,NN	MULTS 20
1372		DO 40 J=1,NN	MULTS 21
1373		40 FK(I,J)=0.	MULTS 22
1374	C		MULTS 23
1375		DO 60 K=1,MM	MULTS 24
1376		DO 60 J=1,NN	MULTS 25
1377		AA=A(K,J)	MULTS 26
1378		IF (AA.EQ.0.) GO TO 60	MULTS 27
1379		DO 50 I=J,NN	MULTS 28
1380		50 FK(I,J)=FK(I,J)+ATK(I,K)*AA	MULTS 29
1381		60 CONTINUE	MULTS 30
1382	C		MULTS 31
1383		DO 70 I=1,NN	MULTS 32
1384		DO 70 J=I,NN	MULTS 33
1385		70 FK(I,J)=FK(J,I)	MULTS 34
1386	C		MULTS 35
1387		RETURN	MULTS 36
1388		END	MULTS 37
1389		SUBROUTINE MULT (A,B,C,II,KK,JJ)	MULT 1
1390		IMPLICIT REAL*8(A-H,O-Z)	MULT 2
1391	C		MULT 3
1392	C	FORM MATRIX PRODUCT A*B=C	MULT 4
1393	C		MULT 5
1394		DIMENSION A(II,1),B(KK,1),C(II,1)	MULT 6
1395	C		MULT 7
1396		DO 10 I=1,II	MULT 8
1397		DO 10 J=1,JJ	MULT 9
1398		10 C(I,J)=0.	MULT 10
1399	C		MULT 11
1400		DO 30 I=1,II	MULT 12
1401		DO 30 K=1,KK	MULT 13
1402		AA=A(I,K)	MULT 14
1403		IF (AA.EQ.0.) GO TO 30	MULT 15
1404		DO 20 J=1,JJ	MULT 16
1405		20 C(I,J)=C(I,J)+AA*B(K,J)	MULT 17
1406		30 CONTINUE	MULT 18
1407	C		MULT 19
1408		RETURN	MULT 20
1409		END	MULT 21
1410		SUBROUTINE MULTT (A,B,C,II,KK,JJ)	MULTT 1
1411		IMPLICIT REAL*8(A-H,O-Z)	MULTT 2
1412	C		MULTT 3
1413	C	FORM MATRIX PRODUCT A(T)*B=C	MULTT 4
1414	C		MULTT 5
1415		DIMENSION A(II,1),B(II,1),C(KK,1)	MULTT 6
1416	C		MULTT 7
1417		DO 10 K=1,KK	MULTT 8
1418		DO 10 J=1,JJ	MULTT 9
1419		10 C(K,J)=0.	MULTT 10
1420	C		MULTT 11
1421		DO 30 K=1,KK	MULTT 12
1422		DO 30 I=1,II	MULTT 13
1423		AA=A(I,K)	MULTT 14
1424		IF (AA.EQ.0.) GO TO 30	MULTT 15
1425		DO 20 J=1,JJ	MULTT 16
1426		20 C(K,J)=C(K,J)+AA*B(I,J)	MULTT 17
1427		30 CONTINUE	MULTT 18
1428	C		MULTT 19
1429		RETURN	MULTT 20
1430		END	MULTT 21
1431		SUBROUTINE ELDIS (DDIS,VEL,LLM)	ELDIS 1
1432		IMPLICIT REAL*8(A-H,O-Z)	ELDIS 2
1433	C		ELDIS 3
1434		COMMON/GENINF/ KCONT(10),KELEM(10),NELEM(10),NINF(10),NDOF(10)	ELDIS 4
1435		1 ,FCONT(3),NUMEM(10)	ELDIS 5
1436		COMMON/PASS/ IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM,IDUM(4)	ELDIS 6
1437		COMMON/DISVEL/ DDISE(10),VELE(10),DD(10)	ELDIS 7
1438	C		ELDIS 8
1439		DIMENSION DDIS(1),VEL(1),LLM(1)	ELDIS 9
1440	C		ELDIS 10


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1441      NDF=NDOF(IGR)
1442      DO 10 K=1,NDF
1443      LL=LLM(K)
1444      DDISE(K)=DDIS(LL)
1445      10 VELE(K)=VEL(LL)
1446      C
1447      RETURN
1448      END
1449      SUBROUTINE SFORCE (DD)
1450      IMPLICIT REAL*8(A-H,O-Z)
1451      C
1452      C ADD CLAMPING FORCES TO STATIC LOAD VECTOR
1453      C
1454      COMMON/GENINF/KCONT(10),KELEM(10),NELEM(10),NINF(10),NDOF(10)
1455      1 ,FCONT(3),NUMEM(10)
1456      COMMON/PASS/ IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM,IDUM(4)
1457      COMMON/INFEL/ IMEM,IMEMD,KST,KSTD,LM(1),LMD(1),DUM(212)
1458      COMMON A(1)
1459      C
1460      DIMENSION DD(1)
1461      C
1462      KDD=KDDS-1
1463      NDF=NDOF(IGR)
1464      DO 10 I=1,NDF
1465      LL=LM(I)
1466      10 A(KDD+LL)=A(KDD+LL)-DD(I)
1467      RETURN
1468      END
1469      SUBROUTINE FORCE (D,DDIS,GAXH,GAXV,FM,IEQFM,VEL,ACC)
1470      IMPLICIT REAL*8(A-H,O-Z)
1471      C
1472      C SET UP DYNAMIC LOAD VECTOR FOR CURRENT STEP
1473      C
1474      COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY
1475      COMMON/PASS/ IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM,IDUM(4)
1476      COMMON/DAMP/ ALPHA,BETA,DT,GAXCTE,CON1,CON2,CON3,CON4,CON5,
1477      1 C1,C2,C3,C4,C5,C6,C7,C8,C9,C10,C11,C12,BETA0,DELTA
1478      C
1479      DIMENSION D(1),DDIS(1),GAXH(1),GAXV(1),FM(1),IEQFM(1),VEL(1),
1480      1 ACC(1)
1481      C
1482      DO 50 I=1,NEQ
1483      IEQ=IEQFM(I)
1484      C IF(NPTH.EQ.0 .AND. IEQ.EQ.2) IEQ=4
1485      C IF(NPTV.EQ.0 .AND. IEQ.EQ.3) IEQ=4
1486      GO TO (40,10,20,30), IEQ
1487      10 D(I)=D(I)-FM(I)*GAXH(ISTEP)
1488      GO TO 30
1489      20 D(I)=D(I)-FM(I)*GAXV(ISTEP)
1490      30 DDIS(I)=D(I)+FM(I)*(C9*ACC(I)+C10*VEL(I))
1491      GO TO 50
1492      40 DDIS(I)=D(I)
1493      50 CONTINUE
1494      C
1495      RETURN
1496      END
1497      SUBROUTINE RESERV (A,AA,M,L,NF,KODST,NEQ,JCOL,K)
1498      IMPLICIT REAL*8(A-H,O-Z)
1499      C
1500      DIMENSION A(L),AA(L),M(NEQ)
1501      C
1502      IF (KODST.EQ.0) GO TO 10
1503      C
1504      CALL STORE (A,L,NF,K)
1505      REWIND NF
1506      GO TO 60
1507      C
1508      10 IF (JCOL.GT.1) GO TO 30
1509      DO 20 I=1,L
1510      20 A(I)=AA(I)
1511      GO TO 60
1512      30 JDP=M(JCOL-1)
1513      JJ=1
1514      DO 50 J=JCOL,NEQ
1515      JD=M(J)
1516      JH=JD-JDP
1517      JDP=JD
1518      NE=MINO(JH,JJ)
1519      DO 40 I=1,NE
1520      A(JD)=AA(JD)

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ELDIS 11
ELDIS 12
ELDIS 13
ELDIS 14
ELDIS 15
ELDIS 16
ELDIS 17
ELDIS 18
SFORC 1
SFORC 2
SFORC 3
SFORC 4
SFORC 5
SFORC 6
SFORC 7
SFORC 8
SFORC 9
SFORC 10
SFORC 11
SFORC 12
SFORC 13
SFORC 14
SFORC 15
SFORC 16
SFORC 17
SFORC 18
SFORC 19
SFORC 20
FORCE 1
FORCE 2
FORCE 3
FORCE 4
FORCE 5
FORCE 6
FORCE 7
FORCE 8
FORCE 9
FORCE 10
FORCE 11
FORCE 12
FORCE 13
FORCE 14
FORCE 15
FORCE 16
FORCE 17
FORCE 18
FORCE 19
FORCE 20
FORCE 21
FORCE 22
FORCE 23
FORCE 24
FORCE 25
FORCE 26
FORCE 27
FORCE 28
RESER 1
RESER 2
RESER 3
RESER 4
RESER 5
RESER 6
RESER 7
RESER 8
RESER 9
RESER 10
RESER 11
RESER 12
RESER 13
RESER 14
RESER 15
RESER 16
RESER 17
RESER 18
RESER 19
RESER 20
RESER 21
RESER 22
RESER 23
RESER 24

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1521      40 JD=JD-1                      RESER 25
1522      50 JJ=JJ+1                      RESER 26
1523      C                               RESER 27
1524      60 RETURN                        RESER 28
1525      END                              RESER 29
1526      SUBROUTINE OPTSOL (A,B,NA,NEQ,JCOL,KEX) OPTSO 1
1527      IMPLICIT REAL*8(A-H,O-Z)         OPTSO 2
1528      C                               OPTSO 3
1529      DIMENSION A(1),B(NEQ),NA(NEQ)    OPTSO 4
1530      C                               OPTSO 5
1531      NEQQ=NEQ-1                        OPTSO 6
1532      GO TO (10,150,10), KEX           OPTSO 7
1533      C *****OPTSO 8
1534      C REDUCE COEFFICIENT MATRIX A     OPTSO 9
1535      C *****OPTSO 10
1536      10 JF=MAX0(JCOL,2)                OPTSO 11
1537      J1=JF+1                          OPTSO 12
1538      IL=JF-1                          OPTSO 13
1539      JCL=JCOL-1                        OPTSO 14
1540      NAJP=NA(IL)                       OPTSO 15
1541      DO 140 J=JF,NEQ                  OPTSO 16
1542      NAJ=NA(J)                        OPTSO 17
1543      IF=J1-NAJ+NAJP                   OPTSO 18
1544      IF (IF.GE.J) GO TO 130           OPTSO 19
1545      IF1=MAX0(IF+1,JCOL)              OPTSO 20
1546      JK=NAJ-J                         OPTSO 21
1547      IF (IF1.GT.IL) GO TO 80         OPTSO 22
1548      JIA=JK+IF1                       OPTSO 23
1549      I1=IF1+1                         OPTSO 24
1550      KL=IF1-1                         OPTSO 25
1551      NAIP=NA(KL)                      OPTSO 26
1552      DO 70 I=IF1,IL                   OPTSO 27
1553      NAI=NA(I)                        OPTSO 28
1554      IK=NAI-I                         OPTSO 29
1555      II=I1-NAI+NAIP                   OPTSO 30
1556      IF (II.GE.I) GO TO 60           OPTSO 31
1557      KF=MAX0(II,IF)                  OPTSO 32
1558      JKA=JK+KF                        OPTSO 33
1559      IKA=IK+KF                        OPTSO 34
1560      AA=A(JIA)                        OPTSO 35
1561      IF (KF.GE.JCOL) GO TO 30        OPTSO 36
1562      DO 20 K=KF,JCL                   OPTSO 37
1563      NAK=NA(K)                        OPTSO 38
1564      AA=AA-A(JKA)*A(IKA)*A(NAK)      OPTSO 39
1565      JKA=JKA+1                        OPTSO 40
1566      20 IKA=IKA+1                     OPTSO 41
1567      IF (JCOL.GT.KL) GO TO 50        OPTSO 42
1568      KF=JCOL                          OPTSO 43
1569      C                               OPTSO 44
1570      30 DO 40 K=KF,KL                 OPTSO 45
1571      AA=AA-A(JKA)*A(IKA)             OPTSO 46
1572      JKA=JKA+1                       OPTSO 47
1573      40 IKA=IKA+1                     OPTSO 48
1574      50 A(JIA)=AA                     OPTSO 49
1575      60 JIA=JIA+1                     OPTSO 50
1576      I1=I1+1                         OPTSO 51
1577      KL=KL+1                         OPTSO 52
1578      70 NAIP=NAI                     OPTSO 53
1579      C                               OPTSO 54
1580      80 KF=IF                         OPTSO 55
1581      JKA=JK+IF                       OPTSO 56
1582      AA=A(NAJ)                       OPTSO 57
1583      IF (IF.GE.JCOL) GO TO 100       OPTSO 58
1584      DO 90 K=IF,JCL                   OPTSO 59
1585      NAI=NA(K)                        OPTSO 60
1586      AA=AA-A(NAI)*A(JKA)**2         OPTSO 61
1587      90 JKA=JKA+1                     OPTSO 62
1588      IF (JCOL.GT.IL) GO TO 120      OPTSO 63
1589      KF=JCOL                          OPTSO 64
1590      C                               OPTSO 65
1591      100 DO 110 K=KF,IL               OPTSO 66
1592      NAI=NA(K)                        OPTSO 67
1593      CC=A(JKA)/A(NAI)                 OPTSO 68
1594      AA=AA-A(JKA)*CC                  OPTSO 69
1595      A(JKA)=CC                        OPTSO 70
1596      110 JKA=JKA+1                    OPTSO 71
1597      120 A(NAJ)=AA                    OPTSO 72
1598      130 IL=IL+1                      OPTSO 73
1599      J1=J1+1                          OPTSO 74
1600      140 NAJP=NAJ                    OPTSO 75

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1601 C OPTSO 76
1602 GO TO (250,250,150), KEX OPTSO 77
1603 C *****OPTSO 78
1604 C REDUCE VECTOR B AND BACK SUBSTITUTE OPTSO 79
1605 C *****OPTSO 80
1606 150 DO 160 N=1,NEQQ OPTSO 81
1607 IF (B(N).NE.0.) GO TO 170 OPTSO 82
1608 160 CONTINUE OPTSO 83
1609 N=NEQQ OPTSO 84
1610 170 N1=N+1 OPTSO 85
1611 I1=N1+1 OPTSO 86
1612 KL=N OPTSO 87
1613 NAIP=NA(N) OPTSO 88
1614 DO 200 I=N1,NEQ OPTSO 89
1615 NAI=NA(I) OPTSO 90
1616 II=I1-NAI+NAIP OPTSO 91
1617 IF (II.GE.I) GO TO 190 OPTSO 92
1618 KF=MAX0(II,N) OPTSO 93
1619 IK=NAI-I OPTSO 94
1620 IKA=IK+KF OPTSO 95
1621 BB=B(I) OPTSO 96
1622 DO 180 K=KF,KL OPTSO 97
1623 BB=BB-A(IKA)*B(K) OPTSO 98
1624 180 IKA=IKA+1 OPTSO 99
1625 B(I)=BB OPTSO100
1626 190 I1=I1+1 OPTSO101
1627 KL=KL+1 OPTSO102
1628 200 NAIP=NAI OPTSO103
1629 DO 210 I=N,NEQ OPTSO104
1630 NAI=NA(I) OPTSO105
1631 210 B(I)=B(I)/A(NAI) OPTSO106
1632 C OPTSO107
1633 J=NEQ OPTSO108
1634 J1=J+1 OPTSO109
1635 KL=NEQQ OPTSO110
1636 NAJ=NA(NEQ) OPTSO111
1637 DO 240 I=1,NEQQ OPTSO112
1638 NAJP=NA(J-1) OPTSO113
1639 II=J1-NAJ+NAJP OPTSO114
1640 IF (II.GE.J) GO TO 230 OPTSO115
1641 JK=NAJ-J OPTSO116
1642 KF=II OPTSO117
1643 JKA=JK+KF OPTSO118
1644 BB=B(J) OPTSO119
1645 DO 220 K=KF,KL OPTSO120
1646 B(K)=B(K)-A(JKA)*BB OPTSO121
1647 220 JKA=JKA+1 OPTSO122
1648 230 J1=J1-1 OPTSO123
1649 KL=KL-1 OPTSO124
1650 J=J-1 OPTSO125
1651 240 NAJ=NAJP OPTSO126
1652 C OPTSO127
1653 250 RETURN OPTSO128
1654 END OPTSO129
1655 SUBROUTINE STIFF (BL,SA,D,IAD,FM,VEL,M,IBL) STIFF 1
1656 IMPLICIT REAL*8(A-H,O-Z) STIFF 2
1657 C STIFF 3
1658 C ASSEMBLE STIFFNESS MATRIX STIFF 4
1659 C STIFF 5
1660 COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY STIFF 6
1661 COMMON/GENINF/KCONT(10),KELEM(10),NELEM(10),NINF(10),NDOF(10) STIFF 7
1662 1 ,FCONT(3),NUMEM(10) STIFF 8
1663 COMMON/PASS/ IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM,IDUM(4) STIFF 9
1664 COMMON/DAMP/ ALPHA,BETA,DT,GAXCTE,CON1,CON2,CON3,CON4,CON5, STIFF 10
1665 1 C1,C2,C3,C4,C5,C6,C7,C8,C9,C10,C11,C12,BETA0,DELTA STIFF 11
1666 COMMON/STOR/ NAVST,NF1,NF2,NF3,NF4,NTST,KODST,KDATA STIFF 12
1667 COMMON/INFEL/ IMEM,IMEMD,KST,KSTD,LM(1),LMD(1),DUM(212) STIFF 13
1668 C STIFF 14
1669 DIMENSION BL(1),SA(1),IAD(1),D(1),FM(1),VEL(1),M(1),IBL(1) STIFF 15
1670 DIMENSION FK(100) STIFF 16
1671 C STIFF 17
1672 C SELECT ELEMENTS IN TURN STIFF 18
1673 C STIFF 19
1674 JCOL=NEQ STIFF 20
1675 IC=0 STIFF 21
1676 DO 180 IGR=1,NELGR STIFF 22
1677 NEL=NELEM(IGR) STIFF 23
1678 KEL=KELEM(IGR) STIFF 24
1679 NIN=NINF(IGR) STIFF 25
1680 NDF=NDOF(IGR) STIFF 26

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1681	C		STIFF 27
1682		DO 170 IEL=1,NEL	STIFF 28
1683	C		STIFF 29
1684		IC=IC+1	STIFF 30
1685		IF (IC.GT.KVARY) GO TO 220	STIFF 31
1686		IDD=IAD(IC)	STIFF 32
1687		IF (NBLOK.EQ.1) GO TO 10	STIFF 33
1688		IF (IDD.EQ.1) CALL STORE (BL,NAVST,NF1,1)	STIFF 34
1689	C		STIFF 35
1690		10 IF (ISTEP.LT.2) GO TO 20	STIFF 36
1691		IF (KST.EQ.0) GO TO 170	STIFF 37
1692		20 GO TO (30,40), KEL	STIFF 38
1693		30 CALL STIF (ISTEP,NDF,NIN,BL(IDD),FK,C12)	STIFF 39
1694		GO TO 130	STIFF 40
1695		40 CALL STIF (ISTEP,NDF,NIN,BL(IDD),FK,C12)	STIFF 41
1696		130 CONTINUE	STIFF 42
1697	C		STIFF 43
1698	C	ADD ELEMENT STIFFNESS FK TO TOTAL STIFFNESS	STIFF 44
1699	C		STIFF 45
1700		DO 150 L=1,NDF	STIFF 46
1701		LML=LM(L)	STIFF 47
1702		IF (LML.GT.NEQ) GO TO 150	STIFF 48
1703		IF (LML.LT.JCOL) JCOL=LML	STIFF 49
1704		DO 140 K=1,NDF	STIFF 50
1705		LMK=LM(K)	STIFF 51
1706		IF (LMK.GT.NEQ) GO TO 140	STIFF 52
1707		IF (LMK.LT.LML) GO TO 140	STIFF 53
1708		LK=(K-1)*NDF+L	STIFF 54
1709		JJ=M(LMK)-(LMK-LML)	STIFF 55
1710		SA(JJ)=SA(JJ)+FK(LK)	STIFF 56
1711		140 CONTINUE	STIFF 57
1712		150 CONTINUE	STIFF 58
1713	C		STIFF 59
1714	C	CORRECT FORCE VECTOR FOR STIFFNESS CHANGES	STIFF 60
1715	C		STIFF 61
1716		IF (BETA.EQ.0.0.OR.ISTEP.LT.2) GO TO 170	STIFF 62
1717		DO 160 L=1,NDF	STIFF 63
1718		LML=LM(L)	STIFF 64
1719		LK=L	STIFF 65
1720		DO 160 K=1,NDF	STIFF 66
1721		LMK=LM(K)	STIFF 67
1722		D(LML)=D(LML)-BETA*FK(LK)*VEL(LMK)	STIFF 68
1723		160 LK=LK+NDF	STIFF 69
1724	C		STIFF 70
1725		170 CONTINUE	STIFF 71
1726		180 CONTINUE	STIFF 72
1727	C		STIFF 73
1728	C	ADD MASS DEPENDING FACTOR TO TOTAL STIFFNESS	STIFF 74
1729	C		STIFF 75
1730		IF (ISTEP.GT.1) GO TO 220	STIFF 76
1731		IF (ISTEP.EQ.0) GO TO 200	STIFF 77
1732		DO 190 I=1,NEQ	STIFF 78
1733		II=M(I)	STIFF 79
1734		190 SA(II)=SA(II)+C1*FM(I)	STIFF 80
1735		GO TO 220	STIFF 81
1736	C		STIFF 82
1737	C	CHECK FOR ZEROS ON DIAGONAL	STIFF 83
1738	C		STIFF 84
1739		200 DO 210 I=1,NEQ	STIFF 85
1740		II=M(I)	STIFF 86
1741		IF (SA(II).EQ.0.) SA(II)=0.000001	STIFF 87
1742		210 CONTINUE	STIFF 88
1743	C		STIFF 89
1744		220 KVARY=0	STIFF 90
1745		REWIND NF1	STIFF 91
1746	C		STIFF 92
1747		RETURN	STIFF 93
1748		END	STIFF 94
1749		SUBROUTINE RESPON (BL,D,DIS,VEL,ACC,DDIS,DISENP,DISENN,IAD,LJTH,LJRESPO	1
1750		1TV,LJTR,TIMENP,TIMENN,DISMAX,IBL,ID,NJTS,NELG,NELN,ELDAM,ELHYS,STDRESPO	2
1751		2AM,STHYS,STRDAM,STRHYS,NIBAY)	RESPO 3
1752		IMPLICIT REAL*8(A-H,O-Z)	RESPO 4
1753	C		RESPO 5
1754	C	SOLVE STATE DETERMINATION PROBLEM FOR ELEMENTS	RESPO 6
1755	C	PRINT TIME HISTORY RESULTS	RESPO 7
1756	C		RESPO 8
1757		COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY	RESPO 9
1758		COMMON/GENINF/KCONT(10),KELEM(10),NELEM(10),NINF(10),NDOF(10)	RESPO 10
1759		1 ,FCONT(3),NUMEM(10)	RESPO 11
1760		COMMON/PASS/ IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM, IDUM(2),	RESPO 12

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1761      1ISYM,ISYMD                                RESPO 13
1762      COMMON/DAMP/ ALPHA,BETA,DT,GAXCTE,CON1,CON2,CON3,CON4,CON5, RESPO 14
1763      1      C1,C2,C3,C4,C5,C6,C7,C8,C9,C10,C11,C12,BETA0,DELTA RESPO 15
1764      COMMON/STOR/ NAVST,NF1,NF2,NF3,NF4,NTST,KODST,KDATA RESPO 16
1765      COMMON/OUTN/ IPJ,IPE,KNTJ,KNTE,NHOUT,NVOUT,NROUT RESPO 17
1766      COMMON/DISVEL/ DDISE(10),VELE(10),DD(10) RESPO 18
1767      COMMON/INFEL/ IMEM,IMEMD,KST,KSTD,LM(1),LMD(1),DUM(212) RESPO 19
1768      COMMON/WORK/POUT(1600) RESPO 20
1769      COMMON/THIST/ITHOUT(10),THOUT(20),ITHP,ISAVE,NELTH,NSTH,NF7,ISE RESPO 21
1770      COMMON/THISTJ/ITHPJ,NF5,NSTHJ,ISJ RESPO 22
1771      COMMON/THISTR/ITHPR,NF6,NSTHR,NHR,NVR,LRH1(50),LRH2(50),LRV1(50), RESPO 23
1772      1      LRV2(50) RESPO 24
1773      COMMON/DAMAGE/KDAMAGE,ITDAM,KIDAMT,NNSKIP,NSSKIP,NGSKIP,GLHYS, RESPO 25
1774      1GLDAM RESPO 26
1775      COMMON/AUTO/KAUTO,KAUTOD,KECO,KECOD,NDSGN,NDSGND,KFREQ,KFREQD, RESPO 27
1776      1 DBALL,DCALL,DBSTD,CONC,STEEL,IECO,BMAVG,BMDEV,ICONV RESPO 28
1777      C RESPO 29
1778      DIMENSION ELDAM(NELG,1),ELHYS(NELG,1),STHYS(NELG,1), RESPO 30
1779      1STDAM(NELG,1),STRDAM(1),STRHYS(1),NIBAY(1) RESPO 31
1780      DIMENSION BL(1),D(1),DIS(1),VEL(1),ACC(1),DDIS(1),DISENP(1), RESPO 32
1781      1      DISENN(1),IAD(1),TIMENP(1),TIMENN(1),LJTH(1),LJTV(1), RESPO 33
1782      2      LJTR(1),IBL(1),ID(NJTS,1) RESPO 34
1783      C RESPO 35
1784      TIME=DT*DFLOAT(ISTEP) RESPO 36
1785      C RESPO 37
1786      C      STATIC DISPLACEMENTS RESPO 38
1787      C RESPO 39
1788      IF (ISTEP.NE.0) GO TO 60 RESPO 40
1789      DO 10 I=1,NEQ RESPO 41
1790      10 DIS(I)=DIS(I)+DDIS(I) RESPO 42
1791      PRINT 20 RESPO 43
1792      20 FORMAT(///27H STATIC NODAL DISPLACEMENTS///5X, RESPO 44
1793      1      5H NODE,6X,7HX-DISPL,6X,7HY-DISPL,5X,8HROTATION/) RESPO 45
1794      DO 40 I=1,NJTS RESPO 46
1795      DO 30 J=1,3 RESPO 47
1796      IJ=ID(I,J) RESPO 48
1797      30 POUT(IJ)=DIS(IJ) RESPO 49
1798      40 PRINT 50, I,(POUT(IJ),J=1,3) RESPO 50
1799      50 FORMAT(I10,2F13.3,F13.5) RESPO 51
1800      GO TO 80 RESPO 52
1801      C RESPO 53
1802      C      DYNAMIC DISPLACEMENTS, VELOCITIES AND ACCELERATIONS RESPO 54
1803      C RESPO 55
1804      60 KNTJ=KNTJ+1 RESPO 56
1805      KNTE=KNTE+1 RESPO 57
1806      DO 70 I=1,NEQ RESPO 58
1807      VVEL=VEL(I) RESPO 59
1808      AACC=ACC(I) RESPO 60
1809      C      DDD=C6*DDIS(I)+C7*VEL(I)+C8*ACC(I) RESPO 61
1810      DDD=C6*DDIS(I)+C7*VEL(I) RESPO 62
1811      DDIS(I)=DDD RESPO 63
1812      DIS(I)=DIS(I)+DDD RESPO 64
1813      C      VEL(I)=VEL(I)+CON2*DDD-CON4*VVEL-CON5*AACC RESPO 65
1814      VEL(I)=VEL(I)+CON2*DDD-CON4*VVEL RESPO 66
1815      70 ACC(I)=ACC(I)+CON1*DDD-CON3*VVEL-CON4*AACC RESPO 67
1816      C RESPO 68
1817      C      CHECK FOR DISPLACEMENT ENVELOPES RESPO 69
1818      C RESPO 70
1819      80 DO 100 I=1,NEQ RESPO 71
1820      IF (DIS(I).LT.0.) GO TO 90 RESPO 72
1821      IF (DIS(I).LE.DISENP(I)) GO TO 100 RESPO 73
1822      DISENP(I)=DIS(I) RESPO 74
1823      TIMENP(I)=TIME RESPO 75
1824      GO TO 100 RESPO 76
1825      90 IF (DIS(I).GE.DISENN(I)) GO TO 100 RESPO 77
1826      DISENN(I)=DIS(I) RESPO 78
1827      TIMENN(I)=TIME RESPO 79
1828      100 CONTINUE RESPO 80
1829      DDIS(NEQ+1)=0. RESPO 81
1830      C RESPO 82
1831      C      PRINT DISPLACEMENT TIME HISTORIES AND SAVE ON TAPE RESPO 83
1832      C RESPO 84
1833      IF (ISTEP.EQ.0.AND.ITHPJ.GT.0) GO TO 110 RESPO 85
1834      IF (KNTJ.NE.IPJ) GO TO 290 RESPO 86
1835      110 IF (NHOUT.EQ.0) GO TO 180 RESPO 87
1836      DO 120 I=1,NHOUT RESPO 88
1837      IJ=LJTH(I) RESPO 89
1838      IJ=ID(IJ,1) RESPO 90
1839      120 POUT(I)=DIS(IJ) RESPO 91
1840      IF (ITHPJ.GT.1.OR.ISTEP.EQ.0) GO TO 170 RESPO 92

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1841      PRINT 130, TIME                      RESPO 93
1842      130 FORMAT (/// 34H X-DISPLACEMENTS AT NODES, TIME = F7.3 / ) RESPO 94
1843      N2=0                                  RESPO 95
1844      140 N1=N2+1                           RESPO 96
1845      N2=N1+9                               RESPO 97
1846      IF (N2.GT.NHOUT) N2=NHOUT            RESPO 98
1847      PRINT 150, (LJTH(I),I=N1,N2)         RESPO 99
1848      PRINT 160, (POUT(I),I=N1,N2)         RESPO100
1849      150 FORMAT(13H NODE NO. ,10I10)     RESPO101
1850      160 FORMAT(13H DISPLACEMENT,2X,10F10.3) RESPO102
1851      IF (N2.LT.NHOUT) GO TO 140           RESPO103
1852      170 IF (ITHPJ.LT.1) GO TO 180        RESPO104
1853      NON=NHOUT+1                          RESPO105
1854      POUT(NON)=TIME                       RESPO106
1855      CALL STORE (POUT,NON,NF5,2)          RESPO107
1856      NSTHJ=NSTHJ+1                        RESPO108
1857      180 IF (NVOUT.EQ.0) GO TO 230       RESPO109
1858      DO 190 I=1,NVOUT                     RESPO110
1859      IJ=LJTV(I)                           RESPO111
1860      IJ=ID(IJ,2)                          RESPO112
1861      190 POUT(I)=DIS(IJ)                  RESPO113
1862      IF (ITHPJ.GT.1.OR.ISTEP.EQ.0) GO TO 220 RESPO114
1863      PRINT 200, TIME                       RESPO115
1864      200 FORMAT (/// 34H Y-DISPLACEMENTS AT NODES, TIME = F7.3 /) RESPO116
1865      N2=0                                  RESPO117
1866      210 N1=N2+1                           RESPO118
1867      N2=N1+9                               RESPO119
1868      IF (N2.GT.NVOUT) N2=NVOUT           RESPO120
1869      PRINT 150, (LJTV(I),I=N1,N2)         RESPO121
1870      PRINT 160, (POUT(I),I=N1,N2)         RESPO122
1871      IF (N2.LT.NVOUT) GO TO 210          RESPO123
1872      220 IF (ITHPJ.LT.1) GO TO 230       RESPO124
1873      NON=NVOUT+1                          RESPO125
1874      POUT(NON)=TIME                       RESPO126
1875      CALL STORE (POUT,NON,NF5,2)          RESPO127
1876      NSTHJ=NSTHJ+1                        RESPO128
1877      230 IF (NROUT.EQ.0) GO TO 290       RESPO129
1878      DO 240 I=1,NROUT                     RESPO130
1879      IJ=LJTR(I)                           RESPO131
1880      IJ=ID(IJ,3)                          RESPO132
1881      240 POUT(I)=DIS(IJ)                  RESPO133
1882      IF (ITHPJ.GT.1.OR.ISTEP.EQ.0) GO TO 280 RESPO134
1883      PRINT 250, TIME                       RESPO135
1884      250 FORMAT (/// 28H ROTATIONS AT NODES, TIME = F7.3 /) RESPO136
1885      N2=0                                  RESPO137
1886      260 N1=N2+1                           RESPO138
1887      N2=N1+9                               RESPO139
1888      IF (N2.GT.NROUT) N2=NROUT           RESPO140
1889      PRINT 150, (LJTR(I),I=N1,N2)         RESPO141
1890      PRINT 270, (POUT(I),I=N1,N2)         RESPO142
1891      270 FORMAT(13H ROTATION ,2X,10F10.5) RESPO143
1892      IF (N2.LT.NROUT) GO TO 260          RESPO144
1893      280 IF (ITHPJ.LT.1) GO TO 290       RESPO145
1894      NON=NROUT+1                          RESPO146
1895      POUT(NON)=TIME                       RESPO147
1896      CALL STORE (POUT,NON,NF5,2)          RESPO148
1897      NSTHJ=NSTHJ+1                        RESPO149
1898      290 CONTINUE                          RESPO150
1899      C                                      RESPO151
1900      C PRINT RELATIVE DISPLACEMENTS AND SAVE ON TAPE RESPO152
1901      C                                      RESPO153
1902      IF (ISTEP.EQ.0.AND.ITHPR.GT.0) GO TO 300 RESPO154
1903      IF (KNTJ.NE.IPJ) GO TO 420          RESPO155
1904      300 IF (NHR.EQ.0) GO TO 370          RESPO156
1905      DO 310 I=1,NHR                        RESPO157
1906      IJ1=LRH1(I)                          RESPO158
1907      IJ2=LRH2(I)                          RESPO159
1908      IJ1=ID(IJ1,1)                        RESPO160
1909      IJ2=ID(IJ2,1)                        RESPO161
1910      310 POUT(I)=DIS(IJ1)-DIS(IJ2)        RESPO162
1911      IF (ISTEP.EQ.0.OR.ITHPR.GT.1) GO TO 360 RESPO163
1912      PRINT 320, TIME                       RESPO164
1913      320 FORMAT(///46H RELATIVE X-DISPLACEMENT BETWEEN NODES, TIME =,F7.3/) RESPO165
1914      N2=0                                  RESPO166
1915      330 N1=N2+1                           RESPO167
1916      N2=N1+9                               RESPO168
1917      IF (N2.GT.NHR) N2=NHR                RESPO169
1918      PRINT 340, (LRH1(I),LRH2(I),I=N1,N2) RESPO170
1919      PRINT 350, (POUT(I),I=N1,N2)         RESPO171
1920      340 FORMAT(11H NODE PAIRS,10(15,2H -,13)) RESPO172

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1921	350	FORMAT(11H REL DISPL.,10F10.4)	RESP0173
1922		IF (N2.LT.NHR) GO TO 330	RESP0174
1923	360	IF (ITHPR.LT.1) GO TO 370	RESP0175
1924		NON=NHR+1	RESP0176
1925		POUT(NON)=TIME	RESP0177
1926		CALL STORE (POUT,NON,NF6,2)	RESP0178
1927		NSTHR=NSTHR+1	RESP0179
1928	370	IF (NVR.EQ.0) GO TO 420	RESP0180
1929		DO 380 I=1,NVR	RESP0181
1930		IJ1=LRV1(I)	RESP0182
1931		IJ2=LRV2(I)	RESP0183
1932		IJ1=ID(IJ1,2)	RESP0184
1933		IJ2=ID(IJ2,2)	RESP0185
1934	380	POUT(I)=DIS(IJ1)-DIS(IJ2)	RESP0186
1935		IF (ISTEP.EQ.0.OR.ITHPR.GT.1) GO TO 410	RESP0187
1936		PRINT 390, TIME	RESP0188
1937	390	FORMAT(///46H RELATIVE Y-DISPLACEMENT BETWEEN NODES, TIME =,F7.3/)	RESP0189
1938		N2=0	RESP0190
1939	400	N1=N2+1	RESP0191
1940		N2=N1+9	RESP0192
1941		IF (N2.GT.NVR) N2=NVR	RESP0193
1942		PRINT 340, (LRV1(I),LRV2(I),I=N1,N2)	RESP0194
1943		PRINT 350, (POUT(I),I=N1,N2)	RESP0195
1944		IF (N2.LT.NVR) GO TO 400	RESP0196
1945	410	IF (ITHPR.LT.1) GO TO 420	RESP0197
1946		NON=NVR+1	RESP0198
1947		POUT(NON)=TIME	RESP0199
1948		CALL STORE (POUT,NON,NF6,2)	RESP0200
1949		NSTHR=NSTHR+1	RESP0201
1950	420	CONTINUE	RESP0202
1951	C		RESP0203
1952	C	CHECK FOR COLLAPSE	RESP0204
1953	C		RESP0205
1954		DO 460 I=1,NEQ,3	RESP0206
1955		IF (DABS(DIS(I)).LT.DISMAX) GO TO 450	RESP0207
1956	430	PRINT 440	RESP0208
1957	440	FORMAT(30H1MAXIMUM DISPLACEMENT EXCEEDED)	RESP0209
1958		KSTAT=-1	RESP0210
1959		GO TO 470	RESP0211
1960	450	IF (DABS(DIS(I+1)).GE.DISMAX) GO TO 430	RESP0212
1961	460	CONTINUE	RESP0213
1962	C		RESP0214
1963	C	INITIALIZE FORCE VECTOR	RESP0215
1964	C		RESP0216
1965	470	DO 480 I=1,NEQ	RESP0217
1966	480	D(I)=0.	RESP0218
1967		D(NEQ+1)=0.	RESP0219
1968	C		RESP0220
1969	C	STATE DETERMINATION FOR ELEMENTS	RESP0221
1970	C		RESP0222
1971		IC=0	RESP0223
1972		KPR=0	RESP0224
1973		IF (NBLOK.GT.1) CALL STORE (BL,NAVST,NF1,1)	RESP0225
1974	C		RESP0226
1975		DO 640 IGR=1,NELGR	RESP0227
1976		NEL=NELEM(IGR)	RESP0228
1977		KEL=KELEM(IGR)	RESP0229
1978		NDF=NDOF(IGR)	RESP0230
1979		NIN=NINF(IGR)	RESP0231
1980		IF (KNTE.EQ.IPE) KPR=IGR	RESP0232
1981		IF (ISTEP.EQ.0) KPR=-IGR	RESP0233
1982	C		RESP0234
1983		DO 640 IEL=1,NEL	RESP0235
1984		IMEM=IEL	RESP0236
1985		KBAL=0	RESP0237
1986		IC=IC+1	RESP0238
1987	C	IISLP=0	RESP0239
1988		IDD=IAD(IC)	RESP0240
1989		IF (NBLOK.EQ.1.OR.IDD.NE.1.OR.IC.EQ.1) GO TO 490	RESP0241
1990		CALL STORE (BL,NAVST,NF2,2)	RESP0242
1991		CALL STORE (BL,NAVST,NF1,1)	RESP0243
1992	C		RESP0244
1993	C	ELEMENT NODAL DISPLACEMENTS AND VELOCITIES	RESP0245
1994	C		RESP0246
1995	490	CALL ELDIS (DDIS,VEL,BL(IDD+2))	RESP0247
1996	C		RESP0248
1997	C	GET RESPONSE OF EVERY ELEMENT	RESP0249
1998	C		RESP0250
1999		GO TO (500,510), KEL	RESP0251
2000	500	CALL RESP (NDF,NIN,KBAL,KPR,BL(IDD),DDISE,DD,TIME,VELE,C11,DELTA,	RESP0252

2001	1ELDAM(IGR,IEL),ELHYS(IGR,IEL))	RESPO253
2002	GO TO 600	RESPO254
2003	510 CALL RESP (NDF,NIN,KBAL,KPR,BL(DD),DDISE,DD,TIME,VELE,C11,DELTA,	RESPO255
2004	1ELDAM(IGR,IEL),ELHYS(IGR,IEL))	RESPO256
2005	600 CONTINUE	RESPO257
2006	C	RESPO258
2007	C SAVE ELEMENT TIME HISTORIES ON TAPE	RESPO259
2008	C	RESPO260
2009	IF (ISAVE.EQ.0) GO TO 610	RESPO261
2010	C CALL STORE (ITHOUT,30,NF4,2)	RESPO262
2011	CALL STORE (ITHOUT,25,NF4,2)	RESPO263
2012	NSTH=NSTH+1	RESPO264
2013	610 CONTINUE	RESPO265
2014	C	RESPO266
2015	C CORRECT FOR OUT OF BALANCE FORCES, ADD DAMPING LOADS	RESPO267
2016	C	RESPO268
2017	IF (KBAL.EQ.0) GO TO 630	RESPO269
2018	DO 620 K=1,NDF	RESPO270
2019	LL=LM(K)	RESPO271
2020	620 D(LL)=D(LL)+DD(K)	RESPO272
2021	C	RESPO273
2022	630 IF (KST.NE.0) KVARY=IC	RESPO274
2023	640 CONTINUE	RESPO275
2024	C	RESPO276
2025	C COMPUTE STORY DAMAGE INDEX	RESPO277
2026	C	RESPO278
2027	IF(KDAMAGE.LT.1) GO TO 990	RESPO279
2028	DO 485 II=1,NSTORY	RESPO280
2029	STRHYS(II)=0.0	RESPO281
2030	STRDAM(II)=0.0	RESPO282
2031	DO 485 JJ=1,NELGR	RESPO283
2032	STHYS(JJ,II)=0.0	RESPO284
2033	STDAM(JJ,II)=0.0	RESPO285
2034	485 CONTINUE	RESPO286
2035	C	RESPO287
2036	DO 660 IGR=1,NELGR	RESPO288
2037	NJ=0	RESPO289
2038	DO 660 II=1,NSTORY	RESPO290
2039	ISTORY=NSTORY+1-II	RESPO291
2040	IF(ISYM .EQ. 0) GO TO 661	RESPO292
2041	NSYBAY=INT(NBAY/2)	RESPO293
2042	KBAY=NBAY-2*NSYBAY	RESPO294
2043	IF(NBAY.EQ.0) NSYBAY=INT(NIBAY(ISTORY)/2)	RESPO295
2044	IF(NBAY.EQ.0) KBAY=NIBAY(ISTORY)-2*NSYBAY	RESPO296
2045	IF(KBAY .NE. 0) NSYBAY=NSYBAY+1	RESPO297
2046	NSBAY=NSYBAY	RESPO298
2047	IF(KBAY .EQ. 0 .AND. IGR .EQ. 2) NSBAY=NSYBAY+1	RESPO299
2048	GO TO 662	RESPO300
2049	661 NSBAY=NBAY	RESPO301
2050	IF(IGR .EQ. 2) NSBAY=NBAY+1	RESPO302
2051	IF(NBAY .EQ. 0) NSBAY=NIBAY(ISTORY)	RESPO303
2052	IF(NBAY .EQ. 0 .AND. IGR .EQ. 2) NSBAY=NSBAY+1	RESPO304
2053	662 DO 670 J=1,NSBAY	RESPO305
2054	IJ=J+NJ	RESPO306
2055	STHYS(IGR,ISTORY)=STHYS(IGR,ISTORY)+ELHYS(IGR,IJ)	RESPO307
2056	STDAM(IGR,ISTORY)=STDAM(IGR,ISTORY)+ELDAM(IGR,IJ)	RESPO308
2057	STRHYS(ISTORY)=STRHYS(ISTORY)+ELHYS(IGR,IJ)	RESPO309
2058	STRDAM(ISTORY)=STRDAM(ISTORY)+ELDAM(IGR,IJ)	RESPO310
2059	IF(ELDAM(IGR,IJ).EQ.0.) ELDAM(IGR,IJ)=0.0	RESPO311
2060	IF(ELDAM(IGR,IJ).NE.0.) ELDAM(IGR,IJ)=ELDAM(IGR,IJ)/ELHYS(IGR,IJ)	RESPO312
2061	670 CONTINUE	RESPO313
2062	NJ=IJ	RESPO314
2063	IF(STDAM(IGR,ISTORY).EQ.0.) STDAM(IGR,ISTORY)=0.0	RESPO315
2064	IF(STDAM(IGR,ISTORY).NE.0.) STDAM(IGR,ISTORY)=STDAM(IGR,ISTORY)/	RESPO316
2065	1STHYS(IGR,ISTORY)	RESPO317
2066	660 CONTINUE	RESPO318
2067	C	RESPO319
2068	C COMPUTE STORY AND GLOBAL DAMAGE INDEX	RESPO320
2069	C	RESPO321
2070	GLHYS=0.0	RESPO322
2071	GLDAM=0.0	RESPO323
2072	DO 675 Istory=1,NSTORY	RESPO324
2073	STRDAM(ISTORY)=STRDAM(ISTORY)/STRHYS(ISTORY)	RESPO325
2074	WEIGHT=DFLOAT(NSTORY+1-ISTORY)/DFLOAT(NSTORY)	RESPO326
2075	GLHYS=GLHYS+STRHYS(ISTORY)	RESPO327
2076	GLDAM=GLDAM+STRDAM(ISTORY)*WEIGHT	RESPO328
2077	675 CONTINUE	RESPO329
2078	C	RESPO330
2079	C PRINT DAMAGE INDICES	RESPO331
2080	C	RESPO332

2081	IF(KDAMAGE.LT.1) GO TO 990	RESP0333
2082	EI=DFLOAT(ISTEP-1)	RESP0334
2083	REM1=DMOD(EI,DFLOAT(NNSKIP))	RESP0335
2084	REM2=DMOD(EI,DFLOAT(NSSKIP))	RESP0336
2085	REM3=DMOD(EI,DFLOAT(NGSKIP))	RESP0337
2086	C	RESP0338
2087	C PRINT STORY DAMAGE INDEX IN FILE (FORO20.DAT)	RESP0339
2088	C	RESP0340
2089	IF(ISTEP.EQ.1) WRITE(20,700)	RESP0341
2090	700 FORMAT(///15X,'***** SUMMARY OF DAMAGE INDEX *****')	RESP0342
2091	C	RESP0343
2092	IF(ISTEP .EQ. NSTEPS) GO TO 715	RESP0344
2093	IF(ITDAM.LT.1 .OR. ISTEP .EQ. 1) GO TO 990	RESP0345
2094	IF(REM1.NE.0.) GO TO 810	RESP0346
2095	715 WRITE(20,710) TIME	RESP0347
2096	710 FORMAT(//2X,' AT TIME =',F7.3)	RESP0348
2097	WRITE(20,720)	RESP0349
2098	720 FORMAT(//10X,'*** ELEMENT DAMAGE INDEX ***')	RESP0350
2099	C	RESP0351
2100	IEL=0	RESP0352
2101	NEEL=0	RESP0353
2102	NEL=0	RESP0354
2103	DO 740 II=1,NSTORY	RESP0355
2104	ISTORY=NSTORY+1-II	RESP0356
2105	WRITE(20,750) IISTORY	RESP0357
2106	750 FORMAT(/2X,'FOR STORY =',15/)	RESP0358
2107	C	RESP0359
2108	IF(ISYM .EQ. 0) GO TO 761	RESP0360
2109	IF(NBAY.EQ.0) NSYBAY=INT(NIBAY(ISTORY)/2)	RESP0361
2110	IF(NBAY.NE.0) NSYBAY=INT(NBAY/2)	RESP0362
2111	KBAY=NBAY-2*NSYBAY	RESP0363
2112	IF(NBAY.EQ.0) KBAY=NIBAY(ISTORY)-2*NSYBAY	RESP0364
2113	IF(KBAY .NE. 0) NSYBAY=NSYBAY+1	RESP0365
2114	NSBAY=NSYBAY	RESP0366
2115	GO TO 762	RESP0367
2116	761 NSBAY=NBAY	RESP0368
2117	IF(NBAY .EQ. 0) NSBAY=NIBAY(ISTORY)	RESP0369
2118	C	RESP0370
2119	762 IEL=NEEL+1	RESP0371
2120	NEEL=IEL+NSBAY-1	RESP0372
2121	DO 740 IGR=1,NELGR	RESP0373
2122	C	RESP0374
2123	IF(IGR .EQ. 1) GO TO 767	RESP0375
2124	IF(ISYM.NE.0 .AND. KBAY.NE.0) GO TO 767	RESP0376
2125	763 IF(ISYM .EQ. 0) GO TO 764	RESP0377
2126	IF(KBAY .EQ. 0) NSBAY=NSYBAY+1	RESP0378
2127	GO TO 765	RESP0379
2128	764 NSBAY=NBAY+1	RESP0380
2129	IF(NBAY .EQ. 0) NSBAY=NIBAY(ISTORY)+1	RESP0381
2130	765 IEL=NEL+1	RESP0382
2131	NEL=IEL+NSBAY-1	RESP0383
2132	WRITE(20,772) (I,I=IEL,NEL)	RESP0384
2133	WRITE(20,775) (ELDAM(2,I),I=IEL,NEL)	RESP0385
2134	GO TO 740	RESP0386
2135	767 IF(IGR .EQ. 1) WRITE(20,771) (I,I=IEL,NEEL)	RESP0387
2136	IF(IGR .EQ. 2) WRITE(20,772) (I,I=IEL,NEEL)	RESP0388
2137	WRITE(20,785) (ELDAM(IGR,I),I=IEL,NEEL)	RESP0389
2138	740 CONTINUE	RESP0390
2139	771 FORMAT(5X,' BEAM NO ',8(5X,13,2X))	RESP0391
2140	772 FORMAT(5X,' COLUMN NO ',8(5X,13,2X))	RESP0392
2141	775 FORMAT(5X,' DAMAGE ',8(F10.4)/)	RESP0393
2142	785 FORMAT(5X,' DAMAGE ',8(F10.4))	RESP0394
2143	IF(ISTEP .EQ. NSTEPS) GO TO 815	RESP0395
2144	C	RESP0396
2145	810 IF(REM1 .NE. 0. .AND. REM2 .NE. 0.) GO TO 910	RESP0397
2146	815 WRITE(20,820)	RESP0398
2147	820 FORMAT(//2X,'*** STORY DAMAGE INDEX ***',	RESP0399
2148	1//10X,'FLOOR',9X,' BEAM ',9X,' COLUMN ',9X,' STORY '/')	RESP0400
2149	DO 830 II=1,NSTORY	RESP0401
2150	ISTORY=NSTORY+1-II	RESP0402
2151	830 WRITE(20,840) IISTORY,(STDAM(J,IISTORY),J=1,2),STRDAM(ISTORY)	RESP0403
2152	840 FORMAT(8X,15,11X,F9.5,9X,F9.5,9X,F9.5)	RESP0404
2153	IF(ISTEP .EQ. NSTEPS) GO TO 915	RESP0405
2154	C	RESP0406
2155	910 IF(REM1 .NE. 0. .AND. REM2 .NE. 0. .AND. REM3 .NE. 0.) GO TO 990	RESP0407
2156	915 WRITE(20,920) GLDAM	RESP0408
2157	920 FORMAT(///8X,'GLOBAL DAMAGE = ',F10.5)	RESP0409
2158	C	RESP0410
2159	990 IF (KNTJ.EQ.IPJ) KNTJ=0	RESP0411
2160	IF (KNTE.EQ.IPE) KNTE=0	RESP0412

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2161      IF (NBLOK.EQ.1) GO TO 995                      RESPO413
2162      CALL STORE (BL,NAVST,NF2,2)                    RESPO414
2163      REWIND NF1                                      RESPO415
2164      REWIND NF2                                      RESPO416
2165      NF=NF1                                          RESPO417
2166      NF1=NF2                                        RESPO418
2167      NF2=NF                                          RESPO419
2168      C                                              RESPO420
2169      995 RETURN                                      RESPO421
2170      END                                             RESPO422
2171      SUBROUTINE OUTEND (BL,IAD,DISENP,DISENN,TIMENP,TIMENN,ID,NJTS) OUTEN 1
2172      IMPLICIT REAL*8(A-H,O-Z)                        OUTEN 2
2173      C                                              OUTEN 3
2174      C      OUTPUT DISPLACEMENT AND FORCE ENVELOPES  OUTEN 4
2175      C                                              OUTEN 5
2176      C                                              OUTEN 6
2177      COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY OUTEN 7
2178      COMMON/GENINF/KCONT(10),KELEM(10),NELEM(10),NINF(10),NDOF(10) OUTEN 8
2179      1      ,FCONT(3),NUMEM(10)                      OUTEN 9
2180      COMMON/PASS/ IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM,IEAR,NEAR, OUTEN 10
2181      1      KSYM,KSYPD                                OUTEN 11
2182      COMMON/STOR/ NAVST,NF1,NF2,NF3,NF4,NTST,KODST,KDATA OUTEN 12
2183      COMMON/DAMP/ ALPHA,BETA,DT,GAXCTE,CON1,CON2,CON3,CON4,CON5, OUTEN 13
2184      1      C1,C2,C3,C4,C5,C6,C7,C8,C9,C10,C11,C12,BETA0,DELTA OUTEN 14
2185      COMMON/AUTO/KAUTO,KAUTOD,KECO,KECOD,NDSGN,NDSGND,KFREQ,KFREQD, OUTEN 15
2186      1 DBALL,DCALL,DBSTD,CONC,STEEL,IECO,BMAVG,BMDEV,ICONV OUTEN 16
2187      C                                              OUTEN 17
2188      DIMENSION BL(1),IAD(1),DISENP(1),DISENN(1),TIMENP(1),TIMENN(1), OUTEN 18
2189      1      ID(NJTS,1)                                OUTEN 19
2190      C                                              OUTEN 20
2191      C      DISPLACEMENTS                            OUTEN 21
2192      C                                              OUTEN 22
2193      TIME=DT*FLOAT(ISTEP)                            OUTEN 23
2194      PRINT 10, TIME                                  OUTEN 24
2195      10 FORMAT (37H1NODAL DISPLACEMENT ENVELOPES, TIME =F8.3///) OUTEN 25
2196      1      18X, 15HX-DISPLACEMENT , 22X, 15HY-DISPLACEMENT , 28X, OUTEN 26
2197      2      9HROTATION /                               OUTEN 27
2198      3      8H NODE , 3(8HPOSITIVE, 3X, 4HTIME, 3X, 8HNEGATIVE, 3X, OUTEN 28
2199      4      4HTIME, 6X) / )                          OUTEN 29
2200      DO 20 I=1,NJTS                                  OUTEN 30
2201      II=ID(I,1)                                       OUTEN 31
2202      IJ=ID(I,2)                                       OUTEN 32
2203      IK=ID(I,3)                                       OUTEN 33
2204      20 PRINT 30, I,DISENP(IJ),TIMENP(IJ),DISENN(IJ),TIMENN(IJ),DISENP(IJ) OUTEN 34
2205      1,TIMENP(IJ),DISENN(IJ),TIMENN(IJ),DISENP(IK),TIMENP(IK),DISENN(IK) OUTEN 35
2206      2,TIMENN(IK)                                     OUTEN 36
2207      30 FORMAT(I5,2(F11.3,F7.2,F11.3,F7.2,3X),2(F11.5,F7.2)) OUTEN 37
2208      IF(KAUTO.EQ.1) GO TO 175                        OUTEN 38
2209      C                                              OUTEN 39
2210      C      ELEMENT FORCES, ETC.                    OUTEN 40
2211      C                                              OUTEN 41
2212      IC=0                                             OUTEN 42
2213      DO 160 IGR=1,NELGR                              OUTEN 43
2214      C                                              OUTEN 44
2215      PRINT 40, IGR,TIME                              OUTEN 45
2216      40 FORMAT(33H1RESULTS ENVELOPES, ELEMENT GROUP,I3,7H TIME =F8.3//) OUTEN 46
2217      KEL=KELEM(IGR)                                  OUTEN 47
2218      NEL=NELEM(IGR)                                  OUTEN 48
2219      NIN=NINF(IGR)                                    OUTEN 49
2220      C                                              OUTEN 50
2221      DO 160 IEL=1,NEL                                 OUTEN 51
2222      IC=IC+1                                          OUTEN 52
2223      IDD=IAD(IC)                                      OUTEN 53
2224      IF (NBLOK.EQ.1) GO TO 50                       OUTEN 54
2225      IF (IDD.EQ.1) CALL STORE (BL,NAVST,NF1,1)      OUTEN 55
2226      C                                              OUTEN 56
2227      50 GO TO (60,70), KEL                          OUTEN 57
2228      60 CALL OUT (BL(IDD),NIN,IC)                   OUTEN 58
2229      GO TO 160                                       OUTEN 59
2230      70 CALL OUT (BL(IDD),NIN,IC)                   OUTEN 60
2231      160 CONTINUE                                    OUTEN 61
2232      REWIND NF1                                      OUTEN 62
2233      C                                              OUTEN 63
2234      175 RETURN                                       OUTEN 64
2235      END                                             OUTEN 65
2236      SUBROUTINE OUTDAM (BL,IAD,ID,NJTS,NQKE,NELG,NELN,STDAM,STRDAM,NOD, OUTDA 1
2237      1DDAM)                                           OUTDA 2
2238      IMPLICIT REAL*8(A-H,O-Z)                        OUTDA 3
2239      C                                              OUTDA 4
2240      C      OUTPUT DAMAGE INDICES                    OUTDA 5

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2241	C		OUTDA	6
2242		COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY	OUTDA	7
2243		COMMON/GENINF/KCONT(10),KELEM(10),NELEM(10),NINF(10),NDOF(10)	OUTDA	8
2244	1	,FCONT(3),NUMEM(10)	OUTDA	9
2245		COMMON/PASS/ IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM,IDUM(2),	OUTDA	10
2246	1	ISYM,ISYMD	OUTDA	11
2247		COMMON/STOR/ NAVST,NF1,NF2,NF3,NF4,NTST,KODST,KDATA	OUTDA	12
2248		COMMON/DAMP/ ALPHA,BETA,DT,GAXCTE,CON1,CON2,CON3,CON4,CON5,	OUTDA	13
2249	1	C1,C2,C3,C4,C5,C6,C7,C8,C9,C10,C11,C12,BETA0,DELTA	OUTDA	14
2250		COMMON/WORK/POUT(1600)	OUTDA	15
2251		COMMON/DAMAGE/KDAMAGE,ITDAM,KIDAMT,NNSKIP,NSSKIP,NGSKIP,GLHYS,	OUTDA	16
2252		1GLDAM	OUTDA	17
2253		COMMON/AUTO/KAUTO,KAUTOD,KECO,KECOD,NDSGN,NDSGND,KFREQ,KFREQD,	OUTDA	18
2254	1	DBALL,DCALL,DBSTD,CONC,STEEL,IECO,BMAVG,BMDEV,ICONV	OUTDA	19
2255	C		OUTDA	20
2256		DIMENSION BL(1),IAD(1),ID(NJTS,1),DDAM(NQKE,1)	OUTDA	21
2257		DIMENSION STDAM(NELG,1),STRDAM(1),NOD(1)	OUTDA	22
2258	C		OUTDA	23
2259	C	DAMAGE INDICES	OUTDA	24
2260	C		OUTDA	25
2261		TIME=DT*DFLOAT(ISTEP)	OUTDA	26
2262		IC=0	OUTDA	27
2263		PRINT 40, TIME	OUTDA	28
2264	40	FORMAT(20H1NODAL DAMAGE INDICES,7H TIME =F8.3//)	OUTDA	29
2265		DO 160 IGR=1,NELGR	OUTDA	30
2266	C		OUTDA	31
2267		KEL=KELEM(IGR)	OUTDA	32
2268		NEL=NELEM(IGR)	OUTDA	33
2269		NIN=NINF(IGR)	OUTDA	34
2270	C		OUTDA	35
2271		DO 160 IEL=1,NEL	OUTDA	36
2272		IC=IC+1	OUTDA	37
2273		IDD=IAD(IC)	OUTDA	38
2274		IF (NBLOK.EQ.1) GO TO 50	OUTDA	39
2275		IF (IDD.EQ.1) CALL STORE (BL,NAVST,NF1,1)	OUTDA	40
2276	C		OUTDA	41
2277		50 GO TO (60,70), KEL	OUTDA	42
2278		60 CALL OUTD (BL(IDD),NIN,IC,NQKE,NELN,NOD,DDAM)	OUTDA	43
2279		GO TO 160	OUTDA	44
2280		70 CALL OUTD (BL(IDD),NIN,IC,NQKE,NELN,NOD,DDAM)	OUTDA	45
2281		160 CONTINUE	OUTDA	46
2282	C		OUTDA	47
2283		PRINT 820	OUTDA	48
2284	820	FORMAT(//2X,'*** STORY DAMAGE INDEX ***',	OUTDA	49
2285		1//10X,'FLOOR',9X,' BEAM ',9X,' COLUMN ',9X,' STORY '//)	OUTDA	50
2286		DO 830 II=1,NSTORY	OUTDA	51
2287		ISTORY=NSTORY+1-II	OUTDA	52
2288	830	PRINT 840, IISTORY,(STDAM(J,ISTORY),J=1,2),STRDAM(ISTORY)	OUTDA	53
2289	840	FORMAT(8X,I5,11X,F9.5,9X,F9.5,9X,F9.5)	OUTDA	54
2290	C		OUTDA	55
2291		PRINT 920, GLDAM	OUTDA	56
2292	920	FORMAT(///8X,'GLOBAL DAMAGE = ',F10.5)	OUTDA	57
2293	C		OUTDA	58
2294		REWIND NF1	OUTDA	59
2295	C		OUTDA	60
2296		RETURN	OUTDA	61
2297		END	OUTDA	62
2298		SUBROUTINE OUTD (COMS,NINF,IC,NQKE,NELN,NOD,DDAM)	OUTD	1
2299		IMPLICIT REAL*8(A-H,O-Z)	OUTD	2
2300	C		OUTD	3
2301		COMMON/INFEL/IMEM,IMEMD,KST,KSTD,LM(6),LMD(6),KGEOM,KGEOMD,PSH,	OUTD	4
2302	1	KHYST,KHYSTD,FL,COSA,SINA,EAL,A(2,6),ST(2,2),ECC(4),	OUTD	5
2303	2	KODY(2),XI(2),Q(2),ALPHAP(2,2),EI(5,2,2),PHF(2,2),PHY(2,2),	OUTD	6
2304	3	PHU(2,2),PHI(2),FM1(2,2),PH1(2,2),FMx(2,2),PHx(2,2),FMP(2,2),	OUTD	7
2305	4	PHp(2,2),PHr(2,2),RD3(2,2),RD4(2,2),RD5(2,2),FMxM(2,2),	OUTD	8
2306	5	PHxM(2,2),BMIY(2,2),BMEP(2),HYS(2),PPH1(2,2),BMMP,PHMX,	OUTD	9
2307	6	BMTOT(2),SFTOT(2),FTOT(2),PRTOT(2),SENP(8),SENN(8),TENP(8),	OUTD	10
2308	7	TENN(8),PRACP(2),PRACN(2),SDACT(3),NODI,NODJ,DUM(3),DAM(2),	OUTD	11
2309	8	REST(22)	OUTD	12
2310		COMMON/PASS/IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM,IEAR,NEAR,	OUTD	13
2311	1	ISYM,ISYMD	OUTD	14
2312		COMMON/DAMAGE/KDAMAGE,ITDAM,KIDAMT,NNSKIP,NSSKIP,NGSKIP,GLHYS,	OUTD	15
2313		1GLDAM	OUTD	16
2314		COMMON/AUTO/KAUTO,KAUTOD,KECO,KECOD,NDSGN,NDSGND,KFREQ,KFREQD,	OUTD	17
2315	1	DBALL,DCALL,DBSTD,CONC,STEEL,IECO,BMAVG,BMDEV,ICONV	OUTD	18
2316	C		OUTD	19
2317		DIMENSION COM(1),COMS(1)	OUTD	20
2318		EQUIVALENCE(IMEM,COM(1))	OUTD	21
2319	C		OUTD	22
2320		DIMENSION NOD(1),DDAM(NQKE,1)	OUTD	23

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2321 C OUTD 24
2322 C FINAL ENVELOPE OUTPUT, BEAM COLUMN ELEMENTS OUTD 25
2323 C OUTD 26
2324 DO 10 J=1,NINFC OUTD 27
2325 10 COM(J)=COMS(J) OUTD 28
2326 C OUTD 29
2327 C OUTPUT FOR DAMAGE INDICES OUTD 30
2328 C OUTD 31
2329 IF(IMEM.EQ.1) PRINT 40, IGR OUTD 32
2330 40 FORMAT(//48H DAMAGE INDICES FOR BEAM COLUMN ELEMENT : GROUP ,115/ OUTD 33
2331 1 7HELEMENT,6H NODE ,6H NODE ,18H DAMAGE INDICES ,/ OUTD 34
2332 2 7H NO. ,6H NO ,6H NO ,18H AT I AT J ,/) OUTD 35
2333 C OUTD 36
2334 PRINT 50, IMEM,NODI,DAM(1),NODJ,DAM(2) OUTD 37
2335 50 FORMAT(14,2(16,1X,F7.4,2X)) OUTD 38
2336 C OUTD 39
2337 C DATA FOR STATISITIC DAMAGE INDICES OUTD 40
2338 C OUTD 41
2339 IIC=2*(IC-1)+1 OUTD 42
2340 DDAM(IEAR,IIC)=DAM(1) OUTD 43
2341 DDAM(IEAR,IIC+1)=DAM(2) OUTD 44
2342 NOD(IIC)=NODI OUTD 45
2343 NOD(IIC+1)=NODJ OUTD 46
2344 C OUTD 47
2345 RETURN OUTD 48
2346 END OUTD 49
2347 SUBROUTINE DSTATIS (X,Y,NJTS,NELTOT,ICLK,IDSGN,NQKE,NELN,DDAM,NIBADSTAT 1
2348 1Y,DBAVG,DCAVG,DEDIF,PDEDIF,ICOR,IICLK,IECHK,NOD,DD1,DD2) DSTAT 2
2349 IMPLICIT REAL*8(A-H,O-Z) DSTAT 3
2350 C DSTAT 4
2351 COMMON/GENINF/KCONT(10),KELEM(10),NELEM(10),NINF(10),NDOF(10) DSTAT 5
2352 1 ,FCONT(3),NUMEM(10) DSTAT 6
2353 COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY DSTAT 7
2354 COMMON/PASS/IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM,IEAR,NEAR, DSTAT 8
2355 1 ISYM,ISYMD DSTAT 9
2356 COMMON/DAMAGE/KDAMAGE,ITDAM,KIDAMT,NNSKIP,NSSKIP,NGSKIP,GLHYS, DSTAT 10
2357 1GLDAM DSTAT 11
2358 COMMON/AUTO/KAUTO,KAUTOD,KECO,KECOD,NDSGN,NDSGND,KFREQ,KFREQD, DSTAT 12
2359 1 DBALL,DCALL,DBSTD,CONC,STEEL,IECO,BMAVG,BMDEV,ICONV DSTAT 13
2360 C DSTAT 14
2361 DIMENSION X(1),Y(1),DD1(1),DD2(1),DDAM(NQKE,1),NIBAY(1),DBAVG(1), DSTAT 15
2362 1DCAVG(1),DEDIF(1),PDEDIF(1),ICOR(1),IICLK(1),IECHK(1),NOD(1) DSTAT 16
2363 C DSTAT 17
2364 C WRITE ALL DAMAGE INDICES ON FIL'DAMAGE.DAT' FOR PLOTTING DSTAT 18
2365 C DSTAT 19
2366 ICHK=0 DSTAT 20
2367 NEQK=IABS(NEAR) DSTAT 21
2368 WRITE(33,*) NEQK DSTAT 22
2369 WRITE(33,*) NJTS,NELTOT DSTAT 23
2370 WRITE(34,*) NEQK DSTAT 24
2371 WRITE(34,*) NJTS,NELTOT DSTAT 25
2372 DO 10 I=1,NJTS DSTAT 26
2373 WRITE(33,*) X(I),Y(I) DSTAT 27
2374 10 WRITE(34,*) X(I),Y(I) DSTAT 28
2375 C DSTAT 29
2376 DO 11 I=1,NELTOT DSTAT 30
2377 IF(ICOR(I).EQ.1) PDEDIF(I)=DEDIF(I) DSTAT 31
2378 11 CONTINUE DSTAT 32
2379 C DSTAT 33
2380 PRINT 60 DSTAT 34
2381 60 FORMAT(//30H1STATISTICS FOR DAMAGE INDICES//) DSTAT 35
2382 C DSTAT 36
2383 JJ=0 DSTAT 37
2384 NB1=0 DSTAT 38
2385 NB2=0 DSTAT 39
2386 DBMEAN=0.0 DSTAT 40
2387 DSUM1=0.0 DSTAT 41
2388 DSUM2=0.0 DSTAT 42
2389 IF(ISYM .EQ. 0) GO TO 14 DSTAT 43
2390 NSYBAY=NBAY/2 DSTAT 44
2391 KBAY=NBAY-2*NSYBAY DSTAT 45
2392 IF(KBAY .NE. 0) NSYBAY=NSYBAY+1 DSTAT 46
2393 14 DO 15 IGR=1,NELGR DSTAT 47
2394 IF(IGR .EQ. 1) GO TO 72 DSTAT 48
2395 IF(ISYM .EQ. 0) NUCOL=NUMEM(IGR)-NBAY DSTAT 49
2396 IF(ISYM .NE. 0 .AND. KBAY .EQ. 0) NUCOL=NUMEM(IGR)-NSYBAY-1 DSTAT 50
2397 IF(ISYM .NE. 0 .AND. KBAY .NE. 0) NUCOL=NUMEM(IGR)-NSYBAY DSTAT 51
2398 72 PRINT 70, IGR DSTAT 52
2399 70 FORMAT(30H BEAM COLUMN ELEMENTS GROUP ,15,// DSTAT 53
2400 1 6H ELEM ,6H NODE ,6H NODE ,1X,10H NO. OF ,4X, DSTAT 54

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2401      2      12H MEAN VALUE,4X,12H STAND DEV ,4X,12H VARIANCE ,4X, DSTAT 55
2402      3      12H MAXIMUM ,4X,12H MINIMUM ,/ DSTAT 56
2403      4      6H NO. ,6H NO ,6H NO ,1X,10HEARTHQUAKE,2X, . DSTAT 57
2404      5      12H AT I AT J ,4X,12H AT I AT J ,4X,12H AT I AT J ,6X, DSTAT 58
2405      6      12H AT I AT J ,4X,12H AT I AT J ,/) DSTAT 59
2406      C      DSTAT 60
2407      DO 20 IEL=1,NUMEM(IGR) DSTAT 61
2408      IF(IGR .EQ. 2) GO TO 31 DSTAT 62
2409      IREM=1 DSTAT 63
2410      IF(ISYM.GT.0 .AND. KBAY.NE.0) IREM=IEL-NSYBAY*(IEL/NSYBAY) DSTAT 64
2411      IF(ISYM.LT.0 .AND. KBAY.NE.0) IIEL=(IEL-1)/NSYBAY DSTAT 65
2412      IF(ISYM.LT.0 .AND. KBAY.NE.0) IREM=IEL-1-NSYBAY*IIEL DSTAT 66
2413      31 JJ=JJ+2 DSTAT 67
2414      DO 30 K=1,NEQK DSTAT 68
2415      DD1(K)=DDAM(K, JJ-1) DSTAT 69
2416      30 DD2(K)=DDAM(K, JJ) DSTAT 70
2417      CALL STATIC(DD1,NEQK,DAVG1,DVAR1,DSTD1,DMX1,DMN1) DSTAT 71
2418      CALL STATIC(DD2,NEQK,DAVG2,DVAR2,DSTD2,DMX2,DMN2) DSTAT 72
2419      WRITE(33,85) IGR,NOD(JJ-1),DAVG1,NOD(JJ),DAVG2 DSTAT 73
2420      PRINT 80, IEL,NOD(JJ-1),NOD(JJ),NEQK,DAVG1,DAVG2,DSTD1, DSTAT 74
2421      1DSTD2,DVAR1,DVAR2,DMX1,DMX2,DMN1,DMN2 DSTAT 75
2422      IF(IGR.EQ.1 .AND. IREM.EQ.0) DBAVG(IEL)=DAVG1 DSTAT 76
2423      IF(IGR.EQ.1 .AND. IREM.EQ.1) DBAVG(IEL)=(DAVG1+DAVG2)/2.DO DSTAT 77
2424      IF(IGR.EQ.2 .AND. IEL.LT.NUCOL) DCAVG(IEL)=DMAX1(DAVG1,DAVG2) DSTAT 78
2425      IF(IGR.EQ.2 .AND. IEL.GE.NUCOL) DCAVG(IEL)=DAVG1 DSTAT 79
2426      IF(IGR.EQ.2) DEDIF(IEL+NUMEM(IGR))=DCAVG(IEL) DSTAT 80
2427      IF(IGR .EQ. 1) THEN DSTAT 81
2428      DSUM1=DSUM1+DAVG1**2 DSTAT 82
2429      DSUM2=DSUM2+DAVG2**2 DSTAT 83
2430      NB1=NB1+1 DSTAT 84
2431      NB2=NB2+1 DSTAT 85
2432      IF(IREM.EQ.0 .AND. ISYM.GT.0) NB2=NB2-1 DSTAT 86
2433      IF(IREM.EQ.0 .AND. ISYM.LT.0) NB1=NB1-1 DSTAT 87
2434      DBMEAN=DAVG1+DAVG2+DBMEAN DSTAT 88
2435      ENDIF DSTAT 89
2436      20 CONTINUE DSTAT 90
2437      IF(IGR .EQ. 2) GO TO 15 DSTAT 91
2438      DBMEAN=DBMEAN/(NB1+NB2) DSTAT 92
2439      DBVAR=(DSUM1+DSUM2)/(NB1+NB2)-DBMEAN**2 DSTAT 93
2440      DBSTD=DSQRT(DBVAR) DSTAT 94
2441      C      DSTAT 95
2442      DIFF=DABS(BMAVG-DBMEAN) DSTAT 96
2443      DO 21 I=1,NUMEM(IGR) DSTAT 97
2444      DEDIF(I)=DBAVG(I)-BMAVG DSTAT 98
2445      21 CONTINUE DSTAT 99
2446      IF(DIFF.LE.BMDEV) GO TO 22 DSTAT100
2447      C      GO TO 15 DSTAT101
2448      22 ICONV=1 DSTAT102
2449      C      DSTAT103
2450      15 CONTINUE DSTAT104
2451      80 FORMAT(14,216,4X,15,5X,5(2(F7.4,1X))) DSTAT105
2452      85 FORMAT(15,2(15,2X,F7.4)) DSTAT106
2453      C      DSTAT107
2454      C      PRINT STATISTICS FOR BEAM DAMAGE INDICES DSTAT108
2455      C      DSTAT109
2456      PRINT 71 DSTAT110
2457      71 FORMAT(///35H1STATISTICS FOR BEAM DAMAGE INDICES///) DSTAT111
2458      PRINT 74 DSTAT112
2459      74 FORMAT(3X,'ELEMENT NO.',4X,'1',8X,'2',8X,'3',8X,'4',8X,'5',8X,'6', DSTAT113
2460      18X,'7',8X,'8'//) DSTAT114
2461      NBM=NUMEM(1) DSTAT115
2462      PRINT 73, (NOD(2*NJ-1),NOD(2*NJ),NJ=1,NBM), DSTAT116
2463      1(DBAVG(IEL),IEL=1,NBM),(DEDIF(IEL),IEL=1,NBM),DBMEAN,DBSTD DSTAT117
2464      73 FORMAT(3X,'NODE I / J',14,'/',11,3X,14,'/',11,3X,14,'/',11,3X,14, DSTAT118
2465      11,'/',11,3X,14,'/',11,3X,14,'/',11,3X,14,'/',12,2X,14,'/',12,2X/ DSTAT119
2466      23X,' DAMAGE ',1X,8(F8.4,1X)// DSTAT120
2467      33X,' DIFFERENCE ',1X,8(F8.4,1X)// DSTAT121
2468      410X,' AVERAGE VALUE = ',F8.4,10X,' STANDARD DEVIATION = ',F8.5) DSTAT122
2469      C      DSTAT123
2470      C      CHECK ELEMENT DAMAGE INDICES FOR ALLOWABLE DAMAGE VALUES DSTAT124
2471      C      DSTAT125
2472      NNEL=0 DSTAT126
2473      DO 40 IGR=1,NELGR DSTAT127
2474      DO 41 IEL=1,NUMEM(IGR) DSTAT128
2475      IM=IEL+NNEL DSTAT129
2476      ICOR(IM)=0 DSTAT130
2477      ICHK(IM)=0 DSTAT131
2478      IF(ICONV .EQ. 0) GO TO 42 DSTAT132
2479      IF(IGR.EQ.1 .AND. DABS(DEDIF(IM)).LE.DBALL) GO TO 41 DSTAT133
2480      IF(IGR.EQ.2 .AND. DCAVG(IEL).LE.DCALL) GO TO 41 DSTAT134

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2481	42	IECHK(IM)=IECHK(IM)+1	DSTAT135
2482		ICLK=1	DSTAT136
2483		ICOR(IM)=1	DSTAT137
2484		IF(IECHK(IM) .EQ. 1) IICLK(IM)=1	DSTAT138
2485	41	CONTINUE	DSTAT139
2486		NNEL=NNEL+NUMEM(IGR)	DSTAT140
2487	40	CONTINUE	DSTAT141
2488		IF(IDSGN .GE. NDSGN) ICHK=0	DSTAT142
2489	C		DSTAT143
2490		WRITE(33,200) DS	DSTAT144
2491	C		DSTAT145
2492	C	PLOTING ALL THE DATA OF DAMAGE INDICES ON FILE 'DAMAGE.DAT'	DSTAT146
2493	C		DSTAT147
2494		DO 140 I=1,NEQK	DSTAT148
2495		KK=0	DSTAT149
2496		WRITE(34,*) I	DSTAT150
2497		DO 150 J=1,NELGR	DSTAT151
2498		DO 150 K=1,NUMEM(J)	DSTAT152
2499		KK=KK+2	DSTAT153
2500		WRITE(34,85) J,NOD(KK-1),DDAM(I,KK-1),NOD(KK),DDAM(I,KK)	DSTAT154
2501	150	CONTINUE	DSTAT155
2502	140	WRITE(34,200) DS	DSTAT156
2503	200	FORMAT(F10.4)	DSTAT157
2504	C		DSTAT158
2505		RETURN	DSTAT159
2506		END	DSTAT160
2507		SUBROUTINE STATIC(DD,N,DAVG,DVAR,DSTD,DMX,DMI)	STATI 1
2508		IMPLICIT REAL*8(A-H,O-Z)	STATI 2
2509		DIMENSION DD(1),DIF(1)	STATI 3
2510	C		STATI 4
2511	C	COMPUTE STATISTIC VALUES FOR DAMAGE INDICES	STATI 5
2512	C		STATI 6
2513	C	----- DAVG=MEAN VALUE OF DAMAGE INDICES AT EACH NODE	STATI 7
2514	C	----- DSUM =SUM OF DAMAGE INDICES AT EACH NODE	STATI 8
2515	C	----- DVAR =VARIANCE AT EACH NODE	STATI 9
2516	C	----- DSTD =STANDARD DEVIATION AT EACH NODE	STATI 10
2517	C		STATI 11
2518		EN=DFLOAT(N)	STATI 12
2519		DAVG=0.D0	STATI 13
2520		DSUM=0.D0	STATI 14
2521		DMX=DD(1)	STATI 15
2522		DMI=DD(1)	STATI 16
2523		DO 10 I=1,N	STATI 17
2524		DAVG=DAVG+DD(I)	STATI 18
2525		DMX=DMAX1(DD(I),DMX)	STATI 19
2526		DMI=DMIN1(DD(I),DMI)	STATI 20
2527	10	DSUM=DSUM+DD(I)**2	STATI 21
2528		DAVG=DAVG/EN	STATI 22
2529		DVAR=DSUM/EN-DAVG**2	STATI 23
2530		IF(DVAR .LT. 0.D0) DVAR=0.D0	STATI 24
2531		DSTD=DSQRT(DVAR)	STATI 25
2532	C		STATI 26
2533		DO 20 I=1,N	STATI 27
2534		DIF(I)=DD(I)-DAVG	STATI 28
2535	20	CONTINUE	STATI 29
2536		RETURN	STATI 30
2537		END	STATI 31
2538		SUBROUTINE THPRJ (LH,LV,LR,NF7)	THPRJ 1
2539		IMPLICIT REAL*8(A-H,O-Z)	THPRJ 2
2540	C		THPRJ 3
2541		COMMON /OUTN/ IPJ,IPE,KNTJ,KNTE,NHOUT,NVOUT,NROUT	THPRJ 4
2542		COMMON /THISTJ/ ITHPJ,NF5,NSTHJ,ISJ	THPRJ 5
2543	C		THPRJ 6
2544		DIMENSION LH(1), LV(1), LR(1)	THPRJ 7
2545		DIMENSION THJD(200)	THPRJ 8
2546	C		THPRJ 9
2547	C	OUTPUT REORGANISED JOINT DISPLACEMENT TIME HISTORIES	THPRJ 10
2548	C		THPRJ 11
2549		NTP=0	THPRJ 12
2550		IF (NHOUT.GT.0) NTP=NTP+1	THPRJ 13
2551		IF (NVOUT.GT.0) NTP=NTP+1	THPRJ 14
2552		IF (NROUT.GT.0) NTP=NTP+1	THPRJ 15
2553		NSTHJ=NSTHJ/NTP	THPRJ 16
2554		NSKJ=NTP-1	THPRJ 17
2555		NSKIP=0	THPRJ 18
2556		IF (NHOUT.EQ.0) GO TO 90	THPRJ 19
2557		NON=NHOUT+1	THPRJ 20
2558		N2=0	THPRJ 21
2559	10	N1=N2+1	THPRJ 22
2560		N2=N1+9	THPRJ 23

2561	IF (NHOUT.LT.N2) N2=NHOUT	THPRJ 24
2562	REWIND NF5	THPRJ 25
2563	DO 80 NS=1,NSTHJ	THPRJ 26
2564	CALL STORE (THJD,NON,NF5,1)	THPRJ 27
2565	IF (NS.GT.1) GO TO 40	THPRJ 28
2566	PRINT 20, (LH(I),I=N1,N2)	THPRJ 29
2567	20 FORMAT (42H1TIME HISTORY OF X-DISPLACEMENTS AT NODES, //13H	NODTHPRJ 30
2568	1E NO.,10I10)	THPRJ 31
2569	PRINT 30	THPRJ 32
2570	30 FORMAT (9H0 TIME)	THPRJ 33
2571	40 PRINT 50, THJD(NON),(THJD(I),I=N1,N2)	THPRJ 34
2572	50 FORMAT (1X,F8.3,6X,10F10.3)	THPRJ 35
2573	IF (ISJ.EQ.0) GO TO 60	THPRJ 36
2574	WRITE (99,50) THJD(NON),(THJD(I),I=N1,N2)	THPRJ 37
2575	60 CONTINUE	THPRJ 38
2576	IF (NS.EQ.NSTHJ.OR.NSKJ.EQ.0) GO TO 80	THPRJ 39
2577	DO 70 N=1,NSKJ	THPRJ 40
2578	70 READ (NF5)	THPRJ 41
2579	80 CONTINUE	THPRJ 42
2580	IF (N2.LT.NHOUT) GO TO 10	THPRJ 43
2581	NSKIP=NSKIP+1	THPRJ 44
2582	90 IF (NVOUT.EQ.0) GO TO 180	THPRJ 45
2583	NON=NVOUT+1	THPRJ 46
2584	N2=0	THPRJ 47
2585	100 N1=N2+1	THPRJ 48
2586	N2=N1+9	THPRJ 49
2587	IF (NVOUT.LT.N2) N2=NVOUT	THPRJ 50
2588	REWIND NF5	THPRJ 51
2589	IF (NSKIP.EQ.0) GO TO 120	THPRJ 52
2590	DO 110 N=1,NSKIP	THPRJ 53
2591	110 READ (NF5)	THPRJ 54
2592	120 DO 170 NS=1,NSTHJ	THPRJ 55
2593	CALL STORE (THJD,NON,NF5,1)	THPRJ 56
2594	IF (NS.GT.1) GO TO 140	THPRJ 57
2595	PRINT 130, (LV(I),I=N1,N2)	THPRJ 58
2596	130 FORMAT (42H1TIME HISTORY OF Y-DISPLACEMENTS AT NODES, //13H	NODTHPRJ 59
2597	1E NO.,10I10)	THPRJ 60
2598	PRINT 30	THPRJ 61
2599	140 PRINT 50, THJD(NON),(THJD(I),I=N1,N2)	THPRJ 62
2600	IF (ISJ.EQ.0) GO TO 150	THPRJ 63
2601	WRITE (99,50) THJD(NON),(THJD(I),I=N1,N2)	THPRJ 64
2602	150 CONTINUE	THPRJ 65
2603	IF (NS.EQ.NSTHJ.OR.NSKJ.EQ.0) GO TO 170	THPRJ 66
2604	DO 160 N=1,NSKJ	THPRJ 67
2605	160 READ (NF5)	THPRJ 68
2606	170 CONTINUE	THPRJ 69
2607	IF (N2.LT.NVOUT) GO TO 100	THPRJ 70
2608	NSKIP=NSKIP+1	THPRJ 71
2609	180 IF (NROUT.EQ.0) GO TO 280	THPRJ 72
2610	NON=NROUT+1	THPRJ 73
2611	N2=0	THPRJ 74
2612	190 N1=N2+1	THPRJ 75
2613	N2=N1+9	THPRJ 76
2614	IF (NROUT.LT.N2) N2=NROUT	THPRJ 77
2615	REWIND NF5	THPRJ 78
2616	IF (NSKIP.EQ.0) GO TO 210	THPRJ 79
2617	DO 200 N=1,NSKIP	THPRJ 80
2618	200 READ (NF5)	THPRJ 81
2619	210 DO 270 NS=1,NSTHJ	THPRJ 82
2620	CALL STORE (THJD,NON,NF5,1)	THPRJ 83
2621	IF (NS.GT.1) GO TO 230	THPRJ 84
2622	PRINT 220, (LR(I),I=N1,N2)	THPRJ 85
2623	220 FORMAT (36H1TIME HISTORY OF ROTATIONS AT NODES, //13H	NODE NO.,THPRJ 86
2624	110I10)	THPRJ 87
2625	PRINT 30	THPRJ 88
2626	230 PRINT 240, THJD(NON),(THJD(I),I=N1,N2)	THPRJ 89
2627	240 FORMAT (1X,F8.3,6X,10F10.5)	THPRJ 90
2628	IF (ISJ.EQ.0) GO TO 250	THPRJ 91
2629	WRITE (99,50) THJD(NON),(THJD(I),I=N1,N2)	THPRJ 92
2630	250 CONTINUE	THPRJ 93
2631	IF (NS.EQ.NSTHJ.OR.NSKJ.EQ.0) GO TO 270	THPRJ 94
2632	DO 260 N=1,NSKJ	THPRJ 95
2633	260 READ (NF5)	THPRJ 96
2634	IF (N2.LT.NROUT) GO TO 190	THPRJ 97
2635	270 CONTINUE	THPRJ 98
2636	280 CONTINUE	THPRJ 99
2637	RETURN	THPRJ100
2638	END	THPRJ101
2639	SUBROUTINE THPRR (NF7,ISJ)	THPRR 1
2640	IMPLICIT REAL*8(A-H,O-Z)	THPRR 2

2641	C		THPRR	3
2642		COMMON /THISTR/ ITHPR,NF6,NSTHR,NHR,NVR,LRH1(50),LRH2(50),LRV1(50)	THPRR	4
2643		1,LRV2(50)	THPRR	5
2644	C		THPRR	6
2645		DIMENSION REL(60)	THPRR	7
2646	C		THPRR	8
2647	C	OUTPUT REORGANISED RELATIVE DISPLACEMENT TIME HISTORIES	THPRR	9
2648	C		THPRR	10
2649		NTP=0	THPRR	11
2650		IF (NHR.GT.0) NTP=NTP+1	THPRR	12
2651		IF (NVR.GT.0) NTP=NTP+1	THPRR	13
2652		NSTHR=NSTHR/NTP	THPRR	14
2653		NSKJ=NTP-1	THPRR	15
2654		NSKIP=0	THPRR	16
2655		IF (NHR.EQ.0) GO TO 90	THPRR	17
2656		NON=NHR+1	THPRR	18
2657		N2=0	THPRR	19
2658	10	N1=N2+1	THPRR	20
2659		N2=N1+9	THPRR	21
2660		IF (NHR.LT.N2) N2=NHR	THPRR	22
2661		REWIND NF6	THPRR	23
2662		DO 80 NS=1,NSTHR	THPRR	24
2663		CALL STORE (REL,NON,NF6,1)	THPRR	25
2664		IF (NS.GT.1) GO TO 40	THPRR	26
2665		PRINT 20, (LRH1(I),LRH2(I),I=N1,N2)	THPRR	27
2666	20	FORMAT (56H1TIME HISTORY OF RELATIVE X-DISPLACEMENTS BETWEEN NODES	THPRR	28
2667		1, //13H NODE PAIRS,10(15,2H -,13))	THPRR	29
2668		PRINT 30	THPRR	30
2669	30	FORMAT (7H0 TIME)	THPRR	31
2670	40	PRINT 50, REL(NON),(REL(I),I=N1,N2)	THPRR	32
2671	50	FORMAT (1X,F8.3,3X,10F10.4)	THPRR	33
2672		IF (ISJ.EQ.0) GO TO 60	THPRR	34
2673		WRITE (NF7) REL(NON),(REL(I),I=N1,N2)	THPRR	35
2674	60	CONTINUE	THPRR	36
2675		IF (NS.EQ.NSTHR.OR.NSKJ.EQ.0) GO TO 80	THPRR	37
2676		DO 70 N=1,NSKJ	THPRR	38
2677	70	READ (NF6)	THPRR	39
2678	80	CONTINUE	THPRR	40
2679		IF (N2.LT.NHR) GO TO 10	THPRR	41
2680		NSKIP=NSKIP+1	THPRR	42
2681	90	IF (NVR.EQ.0) GO TO 170	THPRR	43
2682		NON=NVR+1	THPRR	44
2683		N2=0	THPRR	45
2684	100	N1=N2+1	THPRR	46
2685		N2=N1+9	THPRR	47
2686		IF (NVR.LT.N2) N2=NVR	THPRR	48
2687		REWIND NF6	THPRR	49
2688		IF (NSKIP.EQ.0) GO TO 110	THPRR	50
2689		READ (NF6)	THPRR	51
2690	110	DO 160 NS=1,NSTHR	THPRR	52
2691		CALL STORE (REL,NON,NF6,1)	THPRR	53
2692		IF (NS.GT.1) GO TO 130	THPRR	54
2693		PRINT 120, (LRV1(I),LRV2(I),I=N1,N2)	THPRR	55
2694	120	FORMAT (56H1TIME HISTORY OF RELATIVE Y-DISPLACEMENTS BETWEEN NODES	THPRR	56
2695		1, //13H NODE PAIRS,10(15,2H -,13))	THPRR	57
2696		PRINT 30	THPRR	58
2697	130	PRINT 50, REL(NON),(REL(I),I=N1,N2)	THPRR	59
2698		IF (ISJ.EQ.0) GO TO 140	THPRR	60
2699		WRITE (NF7) REL(NON),(REL(I),I=N1,N2)	THPRR	61
2700	140	CONTINUE	THPRR	62
2701		IF (NS.EQ.NSTHR.OR.NSKJ.EQ.0) GO TO 160	THPRR	63
2702		DO 150 N=1,NSKJ	THPRR	64
2703	150	READ (NF6)	THPRR	65
2704	160	CONTINUE	THPRR	66
2705		IF (N2.LT.NVR) GO TO 100	THPRR	67
2706	170	CONTINUE	THPRR	68
2707		RETURN	THPRR	69
2708		END	THPRR	70
2709		SUBROUTINE THPREL (NF4)	THPRE	1
2710		IMPLICIT REAL*8(A-H,O-Z)	THPRE	2
2711	C		THPRE	3
2712		COMMON /THIST/ ITHOUT(10),THOUT(20),ITHP,ISAVE,NELTH,NSTH,NF7,ISE	THPRE	4
2713	C		THPRE	5
2714	C	OUTPUT REORGANISED ELEMENT TIME HISTORIES	THPRE	6
2715	C		THPRE	7
2716		NSTH=NSTH/NELTH	THPRE	8
2717		NSKE=NELTH-1	THPRE	9
2718		DO 160 NE=1,NELTH	THPRE	10
2719		REWIND NF4	THPRE	11
2720		IF (NE.EQ.1) GO TO 20	THPRE	12


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2721      NSKIP=NE-1                                THPRE 13
2722      DO 10 N=1,NSKIP                            THPRE 14
2723      10 READ (NF4)                               THPRE 15
2724      20 DO 150 NS=1,NSTH                         THPRE 16
2725      CALL STORE (ITHOUT,25,NF4,1)               THPRE 17
2726      ITYP=ITHOUT(2)                             THPRE 18
2727      GO TO (30,40), ITYP                         THPRE 19
2728      30 CALL THPR (NS)                           THPRE 20
2729      GO TO 130                                    THPRE 21
2730      40 CALL THPR (NS)                           THPRE 22
2731      130 CONTINUE                                THPRE 23
2732      IF (NS.EQ.NSTH.OR.NSKE.EQ.0) GO TO 150     THPRE 24
2733      DO 140 N=1,NSKE                             THPRE 25
2734      140 READ (NF4)                              THPRE 26
2735      150 CONTINUE                                THPRE 27
2736      160 CONTINUE                                THPRE 28
2737      RETURN                                       THPRE 29
2738      END                                          THPRE 30
2739      SUBROUTINE THPR (NS)                          THPR 1
2740      IMPLICIT REAL*8(A-H,O-Z)                     THPR 2
2741      C                                             THPR 3
2742      COMMON /THIST/ ITHOUT(10),THOUT(20),ITHP,ISAVE,NELTH,NSTH,NF7,ISE THPR 4
2743      C                                             THPR 5
2744      C REORGANIZED TIME HISTORY OUTPUT, BEAM COLUMN ELEMENTS THPR 6
2745      C                                             THPR 7
2746      IF (NS.GT.1) GO TO 20                       THPR 8
2747      C                                             THPR 9
2748      PRINT 10, ITHOUT(1),ITHOUT(3)                THPR 10
2749      10 FORMAT(18H1RESULTS FOR GROUP,I3,          THPR 11
2750      1      35H, BEAM COLUMN ELEMENTS, ELEMENT NO.,I4//5X THPR 12
2751      2      5H TIME,4X,4HNODE,3X,5HYIELD,6X,7HBENDING,7X,5HSHEAR, THPR 13
2752      3      7X,5HAXIAL,12X,23HPLASTIC HINGE ROTATIONS/5X, THPR 14
2753      4      5H      ,4X,4H NO.,3X,5H CODE,6X,7H MOMENT,7X,5HFORCE, THPR 15
2754      5      7X,5HFORCE,8X,7HCURRENT,4X,9HACC. POS.,3X,9HACC. NEG./) THPR 16
2755      C                                             THPR 17
2756      20 PRINT 30, THOUT(13),ITHOUT(6),ITHOUT(4),(THOUT(I),I=1,11,2),ITHOUTTHPR 18
2757      1(7),ITHOUT(5),(THOUT(I),I=2,12,2)          THPR 19
2758      30 FORMAT (1H0,F8.3,I8,I7,3X,3F12.2,3X,3F12.5/9X,I8,I7,3X,3F12.2,3X,3THPR 20
2759      1F12.5)                                       THPR 21
2760      IF (ISE.EQ.0) GO TO 40                       THPR 22
2761      WRITE (NF7) THOUT(13),ITHOUT(6),ITHOUT(4),(THOUT(I),I=1,11,2),ITHOUTTHPR 23
2762      1UT(7),ITHOUT(8),(THOUT(I),I=2,12,2)        THPR 24
2763      40 CONTINUE                                  THPR 25
2764      C                                             THPR 26
2765      RETURN                                       THPR 27
2766      END                                          THPR 28
2767      SUBROUTINE OUT (COMS,NINFC,IC)                OUT 1
2768      IMPLICIT REAL*8(A-H,O-Z)                     OUT 2
2769      C                                             OUT 3
2770      COMMON/INFEL/IMEM,IMEMD,KST,KSTD,LM(6),LMD(6),KGEOM,KGEOMD,PSH, OUT 4
2771      1      KHYST,KHYSTD,FL,COSA,SINA,EAL,A(2,6),ST(2,2),ECC(4), OUT 5
2772      2      KODY(2),XI(2),Q(2),ALPHAP(2,2),EI(5,2,2),PHF(2,2),PHY(2,2), OUT 6
2773      3      PHU(2,2),PHI(2),FM1(2,2),PH1(2,2),FMx(2,2),PHx(2,2),FMP(2,2), OUT 7
2774      4      PHp(2,2),PHr(2,2),RD3(2,2),RD4(2,2),RD5(2,2),FMxM(2,2), OUT 8
2775      5      PHXM(2,2),BMIY(2,2),BMEP(2),HYS(2),PPH1(2,2),BMMP,PHMX, OUT 9
2776      6      BMTOT(2),SFTOT(2),FTOT(2),PRTOT(2),SENP(8),SENN(8),TENP(8), OUT 10
2777      7      TENN(8),PRACP(2),PRACN(2),SDACT(3),NODI,NODJ,DUM(3),DAM(2), OUT 11
2778      8      REST(22)                               OUT 12
2779      COMMON/PASS/IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM,IEAR,NEAR, OUT 13
2780      1      ISYM,ISYMD                             OUT 14
2781      C                                             OUT 15
2782      DIMENSION COM(1),COMS(1)                     OUT 16
2783      EQUIVALENCE(IMEM,COM(1))                     OUT 17
2784      C                                             OUT 18
2785      C FINAL ENVELOPE OUTPUT, BEAM COLUMN ELEMENTS OUT 19
2786      C                                             OUT 20
2787      DO 10 J=1,NINFC                               OUT 21
2788      10 COM(J)=COMS(J)                             OUT 22
2789      C                                             OUT 23
2790      IF (IMEM.EQ.1) PRINT 20                      OUT 24
2791      20 FORMAT(30H BEAM COLUMN ELEMENTS          /// OUT 25
2792      1      5H ELEM,3X,4HNODE,17X,7HBENDING,14X,5HSHEAR,14X,5HAXIAL, OUT 26
2793      2      13X,8HPL HINGE,12X,9H ACCUM /          OUT 27
2794      3      5H NO.,3X,4H NO.,17X,7H MOMENT,3X,4HTIME,7X,5HFORCE,3X, OUT 28
2795      4      4HTIME,7X,5HFORCE,3X,4HTIME,6X,8HROTATION,3X,4HTIME, OUT 29
2796      5      5X,9HROTATIONS/)                       OUT 30
2797      C                                             OUT 31
2798      PRINT 30, IMEM,NODI,(SENP(I),TENP(I),I=1,7,2),PRACP(1),(SENN(I),TEOUT 32
2799      1NN(I),I=1,7,2),PRACN(1),NODJ,(SENP(I),TENP(I),I=2,8,2),PRACP(2),(SOUT 33
2800      2ENN(I),TENN(I),I=2,8,2),PRACN(2)           OUT 34

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2801      30 FORMAT(14,17,5X,8HPOSITIVE,3(F12.2,F7.3),F14.5,F7.3,F14.5/      OUT 35
2802      1      16X,8HNEGATIVE,3(F12.2,F7.3),F14.5,F7.3,F14.5/      OUT 36
2803      2      7X,14,5X,8HPOSITIVE,3(F12.2,F7.3),F14.5,F7.3,F14.5/      OUT 37
2804      3      16X,8HNEGATIVE,3(F12.2,F7.3),F14.5,F7.3,F14.5/)      OUT 38
2805      C      RETURN      OUT 39
2806      END      OUT 40
2807      SUBROUTINE INELL (KCONT,FCONT,NDOF,NINFC,ID,X,Y,NN,VOL,STL,ITY,      INELL 1
2808      1STIN,CONIN,SECIN,DDIN,RHOM,YBM,NELG,NELN,IP,KIP,PR,PPR,DMY)      INELL 2
2809      IMPLICIT REAL*8(A-H,O-Z)      INELL 3
2810      C      INELL 4
2811      COMMON/GENINF/IIDUM(30),NINF(10),NDOFF(10),JJDUM(6),NUMEM(10)      INELL 5
2812      COMMON/PASS/IGR,KKDUM(11)      INELL 6
2813      COMMON/INFEL/IMEM,IMEMD,KST,KSTD,LM(6),LMD(6),KGEOM,KGEOMD,PSH,      INELL 7
2814      1      KHYST,KHYSTD,FL,COSA,SINA,EAL,A(2,6),ST(2,2),ECC(4),      INELL 8
2815      2      KODY(2),XI(2),Q(2),ALPHAP(2,2),EI(5,2,2),PHF(2,2),PHY(2,2),      INELL 9
2816      3      PHU(2,2),PHI(2),FM1(2,2),PH1(2,2),FMF(2,2),PHx(2,2),FMp(2,2),      INELL 10
2817      4      PHp(2,2),PHr(2,2),RD3(2,2),RD4(2,2),RD5(2,2),FMxM(2,2),      INELL 11
2818      5      PHxM(2,2),BMYI(2,2),BMEP(2),HYS(2),PPH1(2,2),BMMP,PHMX,      INELL 12
2819      6      BMTOT(2),SFTOT(2),FTOT(2),PRTOT(2),SENP(8),SENN(8),TENP(8),      INELL 13
2820      7      TENN(8),PRACP(2),PRACN(2),SDACT(3),NODI,NODJ,KOUTDT,KOUTD,      INELL 14
2821      8      INSLP(2,2),DAM(2),FMFI(2,2),FAC(2,2),FMDA(2,2),IDAM(2,2),      INELL 15
2822      9      PHDA(2,2),FMxM(2,2)      INELL 16
2823      COMMON/WORK/GA(6,6),SFF(8),SSFF(8),DD(6),FFEF(6),FF(6),      INELL 17
2824      1      FEF(35,7),KDFEF(36),FINIT(30,6),ECT(15,4),STYP(7,6),      INELL 18
2825      2      CONYP(7,9),SECYP(14,9),Wi(6),      INELL 19
2826      3      ES,PS,FSY,EPSSY,EPSSU,FSU,FC,RDD,EC,PC,FCY,EPSCY,EPSCU,FCU,      INELL 20
2827      4      EPSCM,PCP,F,FN,FN1,PS1,PC1,PH,FM,EPSS,EPSC,EPSSD,YY,PSP,W2(2),      INELL 21
2828      5      DPR(2),NPW(2),FACTOR,FMY(2),PY(2),PHUL(2),PHIF(2),FMU(2),      INELL 22
2829      6      FMIF(2),W3(740)      INELL 23
2830      COMMON/THIST/ITHOUT(10),THOUT(20),ITHP,ISAVE,NELTH,NSTH,NF7,ISE      INELL 24
2831      COMMON/AUTO/KAUTOD,KAUTOD,KECOD,KECOD,NDSGN,NDSGND,KFREQ,KFREQD,      INELL 25
2832      1      DBALL,DCALL,DBSTD,CONC,STEEL,IECO,BMAVG,BMDEV,ICONV      INELL 26
2833      C      INELL 27
2834      DIMENSION KCONT(1),ID(NN,1),X(1),Y(1),COM(1),STAR(2),YESNO(2),      INELL 28
2835      1KSF(2),STIN(6,1),CONIN(9,1),SECIN(9,1),DDIN(2,1),ITY(3,1),      INELL 29
2836      2YBM(2,NELG,1),RHOM(2,NELG,1),IP(1),KIP(1),PR(1),PPR(2,1),      INELL 30
2837      3DMY(NELG,1)      INELL 31
2838      C      INELL 32
2839      EQUIVALENCE (IMEM,COM(1))      INELL 33
2840      DATA STAR/2H ,2H */      INELL 34
2841      DATA YESNO/4H YES,4H NO /      INELL 35
2842      C      INELL 36
2843      DATA INPUT, BEAM COLUMN ELEMENTS      INELL 37
2844      C      INELL 38
2845      IF(IGR.EQ.1) NUM=0      INELL 39
2846      NDOF=6      INELL 40
2847      NINFC=215      INELL 41
2848      NDOFF(IGR)=NDOF      INELL 42
2849      NINF(IGR)=NINFC      INELL 43
2850      NMEM=KCONT(2)      INELL 44
2851      NUMEM(IGR)=NMEM      INELL 45
2852      NSTL=KCONT(3)      INELL 46
2853      NCON=KCONT(4)      INELL 47
2854      NSEC=KCONT(5)      INELL 48
2855      NECC=KCONT(6)      INELL 49
2856      NFEF=KCONT(7)      INELL 50
2857      NINT=KCONT(8)      INELL 51
2858      IF(IGR.EQ.2) PRINT 10, (KCONT(I),I=2,8)      INELL 52
2859      IF(IGR.EQ.1) PRINT 11, (KCONT(I),I=2,8)      INELL 53
2860      10 FORMAT(43H BEAM COLUMN ELEMENTS (REINFORCED CONCRETE)////      INELL 54
2861      1      34H NO. OF ELEMENTS      =14/      INELL 55
2862      2      34H NO. OF STEEL      TYPES      =14/      INELL 56
2863      3      34H NO. OF CONCRETE      TYPES      =14/      INELL 57
2864      4      34H NO. OF SECTION      TYPES      =14/      INELL 58
2865      5      34H NO. OF ECCENTRICITY      TYPES      =14/      INELL 59
2866      6      34H NO. OF FIXED END      FORCE PATTERNS =14/      INELL 60
2867      7      34H NO. OF INITIAL      FORCE PATTERNS =14      INELL 61
2868      11 FORMAT(36H BEAM ELEMENTS (REINFORCED CONCRETE)////      INELL 62
2869      1      34H NO. OF ELEMENTS      =14/      INELL 63
2870      2      34H NO. OF STEEL      TYPES      =14/      INELL 64
2871      3      34H NO. OF CONCRETE      TYPES      =14/      INELL 65
2872      4      34H NO. OF SECTION      TYPES      =14/      INELL 66
2873      5      34H NO. OF ECCENTRICITY      TYPES      =14/      INELL 67
2874      6      34H NO. OF FIXED END      FORCE PATTERNS =14/      INELL 68
2875      7      34H NO. OF INITIAL      FORCE PATTERNS =14      INELL 69
2876      C      INELL 70
2877      INPUT REINFORCING STEEL TYPES      INELL 71
2878      C      INELL 72
2879      PRINT 20      INELL 73
2880

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2881      20 FORMAT(////24H REINFORCING STEEL TYPES//                INELL 74
2882      1      20X,20H *** INPUT DATA *** ,35X,22H *** COMPUTED DATA ***//INELL 75
2883      2      5H TYPE,6X,7H YOUNGS,5X,9HHARDENING,6X,7H YIELD ,    INELL 76
2884      3      3X,9H ULTIMATE,16X,7H YIELD ,8X,9H ULTIMATE/        INELL 77
2885      4      5H NO. ,6X,7HMODULUS,5X,9H RATIO ,6X,7H STRESS,    INELL 78
2886      5      3X,9H STRAIN ,16X,7H STRAIN,8X,9H STRESS /)        INELL 79
2887      DO 30 N=1,NSTL                                             INELL 80
2888      READ 40,I,(STYP(N,J),J=1,4)                                INELL 81
2889      STYP(N,5)=STYP(N,3)/STYP(N,1)                              INELL 82
2890      STYP(N,6)=STYP(N,2)*STYP(N,1)*(STYP(N,4)-STYP(N,5))+STYP(N,3) INELL 83
2891      30 PRINT 50, N,(STYP(N,J),J=1,6)                            INELL 84
2892      40 FORMAT (I5,E15.4,E10.4,F10.2,F10.5)                      INELL 85
2893      50 FORMAT (I5,2X,E13.4,E13.4,F11.2,F12.4,13X,F11.4,1X,F14.2) INELL 86
2894      C                                                                 INELL 87
2895      C      INPUT CONCRETE TYPES                                  INELL 88
2896      C                                                                 INELL 89
2897      PRINT 60                                                    INELL 90
2898      60 FORMAT(////15H CONCRETE TYPES//                          INELL 91
2899      1      20X,20H *** INPUT DATA *** ,37X,22H *** COMPUTED DATA ***//INELL 92
2900      2      5H TYPE,6X,8HUNIAXIAL,4X,11H STRAIN AT ,6X,11HCONFINEMENT, INELL 93
2901      3      14X,7H YIELD,4X,7H YIELD,6X,8HULTIMATE,3X,8HULTIMATE,3X, INELL 94
2902      4      8HCritical/                                          INELL 95
2903      5      5H NO. ,6X,8HSTRENGTH,4X,11HMAX. STRESS,6X,11H RATIO , INELL 96
2904      6      14X,7H STRESS,4X,7H STRAIN,6X,8H STRESS ,3X,8H STRAIN ,3X, INELL 97
2905      7      8H STRAIN /)                                         INELL 98
2906      DO 70 N=1,NCON                                             INELL 99
2907      READ 80 ,I,(CONYP(N,J),J=1,3)                                INELL100
2908      ALPHAC=1.+10.*CONYP(N,3)                                    INELL101
2909      BETHAC=2.+600.*CONYP(N,3)                                   INELL102
2910      CONYP(N,6)=ALPHAC*CONYP(N,1)                                INELL103
2911      CONYP(N,7)=ALPHAC*CONYP(N,2)                                INELL104
2912      CONYP(N,4)=CONYP(N,6)*3./4.                                INELL105
2913      CONYP(N,5)=CONYP(N,7)*5./12.                                INELL106
2914      CONYP(N,8)=BETHAC*CONYP(N,7)                                INELL107
2915      70 PRINT 90, N,(CONYP(N,J),J=1,8)                            INELL108
2916      80 FORMAT (I5,7F10.4)                                       INELL109
2917      90 FORMAT (I5,F12.2,2X,E15.4,F15.4,12X,F10.2,F12.4,F12.2,1X,2(F11.4)) INELL110
2918      C                                                                 INELL111
2919      C      INPUT CONCRETE CROSS SECTION TYPES                    INELL112
2920      C                                                                 INELL113
2921      PRINT 100                                                    INELL114
2922      100 FORMAT(////29H CONCRETE CROSS SECTION TYPES//          INELL115
2923      1      5H TYPE,6X,8HSECTION ,4X,8H BOTTOM ,4X,8HDISTANCE,2X,  INELL116
2924      2      10H BOT. STEEL,4X,10HFACTOR FOR,4X,8H TOP ,4X,8HDISTANCE, INELL117
2925      3      3X,9HTOP STEEL,2X,10HSECTIONAL /                    INELL118
2926      4      5H NO. ,6X,8H HEIGHT ,4X,8H WIDTH ,4X,8H [DCB] ,2X,  INELL119
2927      5      10H AREA ,4X,10HMOM. DROP,4X,8H WIDTH ,4X,8H [DCT] , INELL120
2928      6      3X,9H AREA ,2X,10H AREA /)                            INELL121
2929      DO 110 N=1,NSEC                                             INELL122
2930      READ 120,I,(SECYP(N,J),J=1,8)                                INELL123
2931      120 FORMAT(I5,4F10.4,F5.2,3F10.4)                            INELL124
2932      IF(I.LE.0)THEN                                             INELL125
2933      SECYP(N,8)=SECYP(N,4)                                       INELL126
2934      SECYP(N,7)=SECYP(N,3)                                       INELL127
2935      SECYP(N,6)=SECYP(N,2)                                       INELL128
2936      ENDIF                                                       INELL129
2937      SECYP(N,9)=SECYP(N,1)*(SECYP(N,2)+SECYP(N,6))/2.           INELL130
2938      PRINT 125, N,(SECYP(N,J),J=1,9)                              INELL131
2939      110 IF(I.LE.0) SECYP(N,1)=-SECYP(N,1)                        INELL132
2940      125 FORMAT(I5,2X,F10.2,3X,F10.2,1X,2F10.2,4X,F10.4,4X,F10.2,1X,F10.2,2) INELL133
2941      1(2X,F10.2))                                                INELL134
2942      C                                                                 INELL135
2943      C      INPUT END ECCENTRICITIES                              INELL136
2944      C                                                                 INELL137
2945      IF (NECC.EQ.0) GO TO 170                                     INELL138
2946      PRINT 130                                                    INELL139
2947      130 FORMAT(////23H END ECCENTRICITY TYPES//                INELL140
2948      1      5H TYPE,6X,25HHORIZONTAL ECCENTRICITIES,5X,          INELL141
2949      2      25H VERTICAL ECCENTRICITIES /                        INELL142
2950      3      5H NO.,4X,25H END I END J ,5X,                      INELL143
2951      4      25H END I END J /)                                    INELL144
2952      DO 140 N=1,NECC                                             INELL145
2953      READ 150,I,(ECT(N,J),J=1,4)                                INELL146
2954      140 PRINT 160, N,(ECT(N,J),J=1,4)                            INELL147
2955      150 FORMAT (I5,4F10.4)                                       INELL148
2956      160 FORMAT (I5,2F14.2,2X,2F14.2)                            INELL149
2957      C                                                                 INELL150
2958      C      FIXED END FORCE PATTERNS                              INELL151
2959      C                                                                 INELL152
2960      170 IF (NFEF.EQ.0) GO TO 220                                INELL153

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2961      PRINT 180
2962      180 FORMAT(///25H FIXED END FORCE PATTERNS//
2963      1      8H PATTERN,3X,4HAXIS,7X,5HAXIAL,7X,5HSHEAR,6X,6HMOMENT,
2964      2      7X,5HAXIAL,7X,5HSHEAR,6X,6HMOMENT,5X,8HLL. RED./
2965      3      8H NO. ,3X,4HCODE,7X,5HAT I,7X,5HAT I,6X,6H AT I ,
2966      4      7X,5HAT J,7X,5HAT J,6X,6H AT J ,5X,8H FACTOR /)
2967      DO 190 N=1,NFEF
2968      READ 200, I,KDFEF(N),(FEF(N,J),J=1,7)
2969      190 PRINT 210, N,KDFEF(N),(FEF(N,J),J=1,7)
2970      200 FORMAT (2I5,7F10.0)
2971      210 FORMAT (I5,I9,F13.2,5F12.2,F12.3)
2972      C
2973      C      INITIAL FORCE PATTERNS
2974      C
2975      220 IF (NINT.EQ.0) GO TO 270
2976      PRINT 230
2977      230 FORMAT(///28H INITIAL END FORCE PATTERNS //
2978      1      8H PATTERN,7X,5HAXIAL,7X,5HSHEAR,6X,6HMOMENT,7X,5HAXIAL,
2979      2      7X,5HSHEAR,6X,6HMOMENT/
2980      3      8H NO. ,7X,5HAT I,7X,5HAT I,6X,6H AT I ,7X,5HAT J,
2981      4      7X,5HAT J,6X,6H AT J /)
2982      DO 240 N=1,NINT
2983      READ 250, I,(FINIT(N,J),J=1,6)
2984      240 PRINT 260, N,(FINIT(N,J),J=1,6)
2985      250 FORMAT (I5,6F10.0)
2986      260 FORMAT (I5,3X,6F12.2)
2987      C
2988      C      ELEMENT SPECIFICATION
2989      C
2990      270 PRINT 280
2991      280 FORMAT(///21H ELEMENT INPUT DATA//
2992      1      3X,4HELEM,2X,4HNODE,2X,4HNODE,2X,4HNODE,2X,4HCONC,2X,4HSTL
2993      2      ,2X,4HSECT,2X,4HECCY,2X,4HGEOM,2X,4HTIME,2X,5HHYST ,2X,
2994      3      11HFEF PATTERN,2X,17HFEF SCALE FACTORS,2X,
2995      4      15HINITIAL FORCES /
2996      5      3X,4H NO.,2X,4H I ,2X,4H J ,2X,4HDIFF,2X,4HTYPE,2X,4HTYPE
2997      6      ,2X,4HTYPE,2X,4HTYPE,2X,4HSTIF,2X,4HHIST,2X,5HCURVE,2X,
2998      7      11H DL LL ,2X,17H DL LL ,2X,
2999      8      15H NO. SCALE FAC./)
3000      C
3001      DO 290 J=1,215
3002      290 COM(J)=0.
3003      KST=0
3004      C
3005      IMEM=1
3006      310 READ 320, INNEL,INODI,INODJ,IINC,IICON,IISTL,IISEC,IECC,IKGM,IKDT
3007      1,IHYS,IKFDL,IKFLL,FFDL,FFLL,IINIT,FFINIT
3008      320 FORMAT (8I5,5I4,2F5.0,I5,F5.0)
3009      C
3010      IF (IABS(INNEL).GT.IMEM) GO TO 350
3011      330 INEL=INNEL
3012      NODI=INODI
3013      NODJ=INODJ
3014      INC=IINC
3015      IF (INC.EQ.0) INC=1
3016      ICON=IICON
3017      ISTL=IISTL
3018      ISEC=IISEC
3019      IECC=IECC
3020      KGEOM=IKGM
3021      KOUTDT=IKDT
3022      KHYST=IHYS
3023      YNG=YESNO(2)
3024      IF (KGEOM.NE.0) YNG=YESNO(1)
3025      YNT=YESNO(2)
3026      IF (KOUTDT.NE.0) YNT=YESNO(1)
3027      YNH=YESNO(2)
3028      IF (KHYST.NE.0) YNH=YESNO(1)
3029      KFDL=IKFDL
3030      KFLL=IKFLL
3031      FDL=FFDL
3032      FLLM=FFLL
3033      FLLF=1.
3034      IF (KFLL.EQ.0) GO TO 340
3035      FLLF=FEF(1KFLL,7)
3036      IF (FLLF.EQ.0.) FLLF=1.E-6
3037      340 INIT=IINIT
3038      FINI=FFINIT
3039      ASTT=STAR(1)
3040      IF (IABS(INEL).NE.NMEM) GO TO 310

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3041          GO TO 350                                INELL234
3042      C                                           INELL235
3043      360 NODI=NODI+INC                             INELL236
3044          NODJ=NODJ+INC                             INELL237
3045          ASTT=STAR(2)                              INELL238
3046      C                                           INELL239
3047      350 PRINT 370, ASTT,IMEM,NODI,NODJ,INC,ICON,ISTL,ISEC,IECC,YNG,YNT INELL240
3048          1,YNH,KFDL,KFLL,FDL,FLLM,INIT,FINT       INELL241
3049      370 FORMAT(A2,14,16,416,216,3X,A4,2X,A4,2X,A4,16,16,F10.2,F10.2,16,F10 INELL242
3050          1.2)                                       INELL243
3051      C                                           INELL244
3052      C      COUNT NUMBER OF ELEMENT TIME HISTORIES INELL245
3053      C                                           INELL246
3054          IF (KOUTDT.NE.0) NELTH=NELTH+1           INELL247
3055      C                                           INELL248
3056      C      LOCATION MATRIX                       INELL249
3057      C                                           INELL250
3058          DO 380 I=1,3                               INELL251
3059          LM(I)=ID(NODI,I)                           INELL252
3060      380 LM(I+3)=ID(NODJ,I)                       INELL253
3061          CALL BAND                                  INELL254
3062      C                                           INELL255
3063      C      ELEMENT PROPERTIES                    INELL256
3064      C                                           INELL257
3065          XL=X(NODJ)-X(NODI)                        INELL258
3066          YL=Y(NODJ)-Y(NODI)                        INELL259
3067          RFL=DSQRT(XL**2+YL**2)                   INELL260
3068          IF (IECC.EQ.0) GO TO 400                  INELL261
3069          DO 390 I=1,4                               INELL262
3070      390 ECC(I)=ECT(IECC,I)                       INELL263
3071          XL=XL-ECC(1)+ECC(2)                       INELL264
3072          YL=YL-ECC(3)+ECC(4)                       INELL265
3073      400 FL=DSQRT(XL**2+YL**2)                   INELL266
3074          COSA=XL/FL                                INELL267
3075          SINA=YL/FL                                INELL268
3076      C                                           INELL269
3077      C      DISPLACEMENT TRANSFORMATION           INELL270
3078      C                                           INELL271
3079          A(1,1)=-SINA/FL                           INELL272
3080          A(1,2)=COSA/FL                            INELL273
3081          A(1,3)=1.                                INELL274
3082          A(1,4)=-A(1,1)                           INELL275
3083          A(1,5)=-A(1,2)                           INELL276
3084          A(1,6)=0.                                INELL277
3085          A(2,1)=A(1,1)                             INELL278
3086          A(2,2)=A(1,2)                             INELL279
3087          A(2,3)=0.                                INELL280
3088          A(2,4)=A(1,4)                             INELL281
3089          A(2,5)=A(1,5)                             INELL282
3090          A(2,6)=1.                                INELL283
3091          IF (IECC.EQ.0) GO TO 410                  INELL284
3092          A(2,3)=(SINA*ECC(3)+COSA*ECC(1))/FL       INELL285
3093          A(1,3)=1.+A(2,3)                          INELL286
3094          A(1,6)=(-SINA*ECC(4)-COSA*ECC(2))/FL     INELL287
3095          A(2,6)=1.+A(1,6)                          INELL288
3096          GO TO 425                                  INELL289
3097      410 ECC(1)=1.23456E10                        INELL290
3098      425 CONTINUE                                  INELL291
3099      C                                           INELL292
3100      C      LOADS DUE TO FIXED END FORCES        INELL293
3101      C                                           INELL294
3102          DO 420 I=1,6                               INELL295
3103          SFF(I)=0.                                  INELL296
3104      420 SSFF(I)=0.                                INELL297
3105          IF (KFDL+KFLL.EQ.0) GO TO 510            INELL298
3106          DO 430 I=1,6                               INELL299
3107          DO 430 J=1,6                               INELL300
3108      430 GA(I,J)=0.                                 INELL301
3109          GA(1,1)=COSA                               INELL302
3110          GA(1,2)=SINA                               INELL303
3111          GA(2,1)=-SINA                             INELL304
3112          GA(2,2)=COSA                               INELL305
3113          GA(3,3)=1.                                INELL306
3114          GA(4,4)=COSA                               INELL307
3115          GA(4,5)=SINA                               INELL308
3116          GA(5,4)=-SINA                             INELL309
3117          GA(5,5)=COSA                               INELL310
3118          GA(6,6)=1.                                INELL311
3119      C                                           INELL312
3120          IF (KFLL.EQ.0) GO TO 470                  INELL313

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3121	DO 440 I=1,6	INELL314
3122	440 FFEF(I)=FEF(KFDL,I)*FDL	INELL315
3123	IF (KDFEF(KFDL).EQ.0) GO TO 450	INELL316
3124	CALL MULT (GA,FFEF,SFF,6,6,1)	INELL317
3125	GO TO 470	INELL318
3126	450 DO 460 I=1,6	INELL319
3127	460 SFF(I)=FFEF(I)	INELL320
3128	C	INELL321
3129	470 IF (KFLLEQ.0) GO TO 510	INELL322
3130	DO 480 I=1,6	INELL323
3131	FLL=FLLF*FLLM	INELL324
3132	IF (I.EQ.3.OR.I.EQ.6) FLL=FLLM	INELL325
3133	480 FFEF(I)=FEF(KFLL,I)*FLL	INELL326
3134	IF (KDFEF(KFLL).EQ.0) GO TO 490	INELL327
3135	CALL MULT (GA,FFEF,SSFF,6,6,1)	INELL328
3136	GO TO 510	INELL329
3137	490 DO 500 I=1,6	INELL330
3138	500 SSFF(I)=FFEF(I)	INELL331
3139	C	INELL332
3140	510 DO 520 I=1,6	INELL333
3141	520 FF(I)=SFF(I)+SSFF(I)	INELL334
3142	C	INELL335
3143	CALL MULTT (GA,FF,DD,6,6,1)	INELL336
3144	IF (IECC.EQ.0) GO TO 530	INELL337
3145	DD(3)=DD(3)-DD(1)*ECC(3)+DD(2)*ECC(1)	INELL338
3146	DD(6)=DD(6)-DD(4)*ECC(4)+DD(5)*ECC(2)	INELL339
3147	530 CALL SFORCE (DD)	INELL340
3148	C	INELL341
3149	C MODIFY TO GET INITIAL ELEMENT FORCES	INELL342
3150	C	INELL343
3151	DO 540 I=1,6	INELL344
3152	FLL=1./FLLF	INELL345
3153	IF (I.EQ.3.OR.I.EQ.6) FLL=1.	INELL346
3154	540 SFF(I)=SFF(I)+SSFF(I)*FLL	INELL347
3155	C	INELL348
3156	C INITIAL FORCES	INELL349
3157	C	INELL350
3158	550 DO 560 I=1,6	INELL351
3159	560 SSFF(I)=0.	INELL352
3160	IF (INIT.EQ.0) GO TO 580	INELL353
3161	DO 570 I=1,6	INELL354
3162	SSFF(I)=FINIT(INIT,I)*FINT	INELL355
3163	570 SFF(I)=SFF(I)+SSFF(I)	INELL356
3164	C	INELL357
3165	C INITIALIZE ELEMENT FORCES	INELL358
3166	C	INELL359
3167	580 BMEP(1)=SFF(3)	INELL360
3168	BMEP(2)=SFF(6)	INELL361
3169	FTOT(1)=SFF(1)	INELL362
3170	FTOT(2)=SFF(4)	INELL363
3171	SFTOT(1)=SFF(2)	INELL364
3172	SFTOT(2)=SFF(5)	INELL365
3173	BMTOT(1)=SFF(3)	INELL366
3174	BMTOT(2)=SFF(6)	INELL367
3175	C	INELL368
3176	C INITIALIZE ENVELOPES	INELL369
3177	C	INELL370
3178	FF(1)=SSFF(3)	INELL371
3179	FF(2)=SSFF(6)	INELL372
3180	FF(3)=SSFF(2)	INELL373
3181	FF(4)=SSFF(5)	INELL374
3182	FF(5)=SSFF(1)	INELL375
3183	FF(6)=SSFF(4)	INELL376
3184	DO 600 I=1,6	INELL377
3185	IF (FF(I).LT.0.) GO TO 590	INELL378
3186	SENP(I)=FF(I)	INELL379
3187	SENN(I)=0.	INELL380
3188	GO TO 600	INELL381
3189	590 SENN(I)=FF(I)	INELL382
3190	SENP(I)=0.	INELL383
3191	600 CONTINUE	INELL384
3192	C	INELL385
3193	C COMPUTE SECTION PROPERTIES	INELL386
3194	C	INELL387
3195	C 1) REINFORCING STEEL	INELL388
3196	ES=STYP(ISTL,1)	INELL389
3197	PS=STYP(ISTL,2)	INELL390
3198	FSY=STYP(ISTL,3)	INELL391
3199	EPSSU=STYP(ISTL,4)	INELL392
3200	EPSSY=STYP(ISTL,5)	INELL393

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3201      FSU=STYP(ISTL,6)                                INELL394
3202      C      2) CONCRETE PROPERTIES                    INELL395
3203      FC=CONYP(ICON,1)                                INELL396
3204      EPSCO=CONYP(ICON,2)                             INELL397
3205      RDD=CONYP(ICON,3)                               INELL398
3206      FCY=CONYP(ICON,4)                               INELL399
3207      EPSCY=CONYP(ICON,5)                             INELL400
3208      FCU=CONYP(ICON,6)                               INELL401
3209      EPSCU=CONYP(ICON,7)                             INELL402
3210      EPSCM=CONYP(ICON,8)                             INELL403
3211      SLR=CONYP(ICON,9)                               INELL404
3212      C      3) DIMENSION OF SECTION                  INELL405
3213      II=1                                             INELL406
3214      IF(SECYP(ISEC,1).LT.0.) II=-1                  INELL407
3215      HT=SECYP(ISEC,1)                                 INELL408
3216      IF(II.LE.0) HT=-HT                              INELL409
3217      BB=SECYP(ISEC,2)                                 INELL410
3218      DCB=SECYP(ISEC,3)                                INELL411
3219      ASB=SECYP(ISEC,4)                                INELL412
3220      OMEGA=SECYP(ISEC,5)                             INELL413
3221      BT=SECYP(ISEC,6)                                INELL414
3222      DCT=SECYP(ISEC,7)                               INELL415
3223      AST=SECYP(ISEC,8)                               INELL416
3224      AT=SECYP(ISEC,9)                                INELL417
3225      C                                               INELL418
3226      C      SAVE ELEMENT INPUT DATA                 INELL419
3227      C                                               INELL420
3228      IM=NUM+IMEM                                      INELL421
3229      DO 601 IN=1,6                                    INELL422
3230      601 STIN(IN,IM)=STYP(ISTL,IN)                   INELL423
3231      DO 602 IN=1,9                                    INELL424
3232      602 CONIN(IN,IM)=CONYP(ICON,IN)                 INELL425
3233      DO 603 IN=1,9                                    INELL426
3234      603 SECIN(IN,IM)=SECYP(ISEC,IN)                 INELL427
3235      DO 604 IN=1,2                                    INELL428
3236      604 DDIN(IN,IM)=DD(IN)                          INELL429
3237      ITY(1,IM)=ISTL                                  INELL430
3238      ITY(2,IM)=ICON                                  INELL431
3239      ITY(3,IM)=ISEC                                  INELL432
3240      C                                               INELL433
3241      VOL=VOL+RFL*AT                                  INELL434
3242      STL=STL+RFL*(AST+ASB)                            INELL435
3243      EC=FCY/EPSCY                                    INELL436
3244      PC=5./21.                                       INELL437
3245      C                                               INELL438
3246      PCP=(FCU-0.1*FC)/((EPSCM-EPSCU)*EC)           INELL439
3247      FN=ES/EC                                         INELL440
3248      FN1=FN-1                                         INELL441
3249      PS1=1.-PS                                        INELL442
3250      PC1=1.-PC                                        INELL443
3251      AS=AST+ASB                                       INELL444
3252      AC=AT-AS                                         INELL445
3253      DDT=HT-DCB                                       INELL446
3254      DDB=HT-DCT                                       INELL447
3255      AXF=DD(1)                                        INELL448
3256      IF(1GR.EQ.2) AXF=-DD(2)                         INELL449
3257      PSP=1.5*PS                                       INELL450
3258      CALL FMPHI(SLR,AXF,HT,BT,DCT,AST,DDT,ASB,FMY1,EI1,P1,PHIU1,PHIF1, INELL451
3259      1MF1,FMU1,YNX1)                                  INELL452
3260      IF(II.GE.0)THEN                                  INELL453
3261      CALL FMPHI(SLR,AXF,HT,BB,DCB,ASB,DDB,AST,FMY2,EI2,P2,PHIU2,PHIF2, INELL454
3262      1FMF2,FMU2,YNX2)                                  INELL455
3263      EII=.5*(EI1+EI2)                                  INELL456
3264      PP=.5*(P1*EI1+P2*EI2)/EII                       INELL457
3265      FMY1=FMY1*(1.-PP*EII/EI1)/(1.-PP)               INELL458
3266      FMY2=FMY2*(1.-PP*EII/EI2)/(1.-PP)               INELL459
3267      ELSE                                             INELL460
3268      EII=EI1                                           INELL461
3269      PP=P1                                              INELL462
3270      FMY2=FMY1                                         INELL463
3271      PHIU2=PHIU1                                       INELL464
3272      FMU2=FMU1                                         INELL465
3273      PHIF2=PHIF1                                       INELL466
3274      FMF2=FMF1                                         INELL467
3275      ENDIF                                             INELL468
3276      ATN=CONYP(ICON,4)*AT/HT*(DDT+DDB)/2.           INELL469
3277      EA=EC*.5*(BB+BT)*HT+ES*(ASB+AST)                INELL470
3278      STR=(AS/AT)*100.DO                                INELL471
3279      IF(STR.LT.0.75) STR=0.75                        INELL472
3280      CFR=RDD                                           INELL473

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3281	IF(CFR.GT.2.) CFR=2.	INELL474
3282	PHUL(1)=PHIU1	INELL475
3283	PHUL(2)=-PHIU2	INELL476
3284	FMU(1)=FMU1	INELL477
3285	FMU(2)=-FMU2	INELL478
3286	PHIF(1)=PHIF1	INELL479
3287	PHIF(2)=-PHIF2	INELL480
3288	FMIF(1)=FMF1	INELL481
3289	FMIF(2)=-FMF2	INELL482
3290	FMY(1)=FMY1	INELL483
3291	FMY(2)=-FMY2	INELL484
3292	PY(1)=AS*FSY+.85*FC*AC	INELL485
3293	PY(2)=-(.6.*AC*DSQRT(FC*1000.)/1000.+FSY*AS)	INELL486
3294	C	INELL487
3295	DO 610 I=1,2	INELL488
3296	KODY(I)=1	INELL489
3297	XI(I)=0.	INELL490
3298	Q(I)=1.	INELL491
3299	DO 610 J=1,2	INELL492
3300	EI(1,I,J)=EII	INELL493
3301	EI(2,I,J)=PP*EII	INELL494
3302	EI(3,I,J)=0.0	INELL495
3303	EI(4,I,J)=0.0	INELL496
3304	610 EI(5,I,J)=0.0	INELL497
3305	PSH=PP	INELL498
3306	EAL=EA/FL	INELL499
3307	IF(INEL.LT.0) GO TO 621	INELL500
3308	SSR=FL/(2.*DDT)	INELL501
3309	CCC=SSR	INELL502
3310	DO 620 I=1,4	INELL503
3311	IF(I.EQ.1.OR.I.EQ.4)CCC=FL/(2.*DDB)	INELL504
3312	ALPHA=.4*CCC-.6	INELL505
3313	IF(CCC.LT.1.50000025)ALPHA=.0000001	INELL506
3314	IF(CCC.GT.4.)ALPHA=1.	INELL507
3315	620 ALPHAP(I,1)=ALPHA	INELL508
3316	GO TO 625	INELL509
3317	621 SSR=FL/DDT	INELL510
3318	CCC=SSR	INELL511
3319	DO 622 I=1,4	INELL512
3320	IF(I.EQ.1.OR.I.EQ.4)CCC=FL/DDB	INELL513
3321	ALPHA=.4*CCC-.6	INELL514
3322	IF(CCC.LT.1.50000025)ALPHA=.0000001	INELL515
3323	IF(CCC.GT.4.)ALPHA=1.	INELL516
3324	622 ALPHAP(I,1)=ALPHA	INELL517
3325	C	INELL518
3326	625 DO 630 J=1,2	INELL519
3327	DAM(J)=0.0	INELL520
3328	DO 630 IE=1,2	INELL521
3329	PHF(IE,J)=PHIF(J)	INELL522
3330	FMF(IE,J)=FMIF(J)	INELL523
3331	PHU(IE,J)=PHUL(J)	INELL524
3332	BMIY(IE,J)=FMY(J)	INELL525
3333	630 PHY(IE,J)=FMY(J)/EII	INELL526
3334	DO 640 I=1,4	INELL527
3335	IDAM(I,1)=0	INELL528
3336	INSLP(I,1)=1	INELL529
3337	FAC(I,1)=OMEGA	INELL530
3338	FMDA(I,1)=BMIY(I,1)	INELL531
3339	FMxxM(I,1)=BMIY(I,1)	INELL532
3340	FMxM(I,1)=BMIY(I,1)	INELL533
3341	PHDA(I,1)=PHY(I,1)	INELL534
3342	PPH1(I,1)=PHY(I,1)	INELL535
3343	640 PHxM(I,1)=PHY(I,1)	INELL536
3344	C	INELL537
3345	SAVE DATA FOR DAMAGE ACCEPTANCE CRITERIA	INELL538
3346	C	INELL539
3347	CC=0.85*FC*YNX1*BT	INELL540
3348	CS=FSY*AST	INELL541
3349	RHOMAX=(CC+CS)/FSY	INELL542
3350	RHOMIN=200.0/FSY	INELL543
3351	IF(FSY .LE. 200.0) RHOMIN=RHOMIN/1000.	INELL544
3352	RHOM(1,IGR,IMEM)=RHOMIN	INELL545
3353	RHOM(2,IGR,IMEM)=RHOMAX	INELL546
3354	C	INELL547
3355	YBM(1,IGR,NODI)=FMY(1)	INELL548
3356	YBM(2,IGR,NODJ)=FMY(1)	INELL549
3357	C	INELL550
3358	CALL FINISH	INELL551
3359	C	INELL552
3360	C REARRANGE DATA FOR PRINTING INPUT DATA	INELL553


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3361 C INELL554
3362 IP(IMEM)=NODI INELL555
3363 IP(IMEM+NMEM)=NODJ INELL556
3364 IP(IMEM+2*NMEM)=ICON INELL557
3365 IP(IMEM+3*NMEM)=ISTL INELL558
3366 IP(IMEM+4*NMEM)=ISEC INELL559
3367 C INELL560
3368 PR(IMEM)=EII INELL561
3369 PR(IMEM+NMEM)=STR INELL562
3370 PR(IMEM+2*NMEM)=PP INELL563
3371 PR(IMEM+3*NMEM)=SSR INELL564
3372 PR(IMEM+4*NMEM)=CFR INELL565
3373 C INELL566
3374 PPR(1,IMEM)=FMY(1) INELL567
3375 PPR(1,IMEM+NMEM)=FMU(1) INELL568
3376 PPR(1,IMEM+2*NMEM)=FMIF(1) INELL569
3377 PPR(1,IMEM+3*NMEM)=PHUL(1) INELL570
3378 PPR(1,IMEM+4*NMEM)=PHIF(1) INELL571
3379 C INELL572
3380 PPR(2,IMEM)=FMY(2) INELL573
3381 PPR(2,IMEM+NMEM)=FMU(2) INELL574
3382 PPR(2,IMEM+2*NMEM)=FMIF(2) INELL575
3383 PPR(2,IMEM+3*NMEM)=PHUL(2) INELL576
3384 PPR(2,IMEM+4*NMEM)=PHIF(2) INELL577
3385 C INELL578
3386 IF(KAUTO.NE.1) GO TO 609 INELL579
3387 AST=1.1*AST INELL580
3388 ASB=1.1*ASB INELL581
3389 CALL FMPHI(SLR,AXF,HT,BT,DCT,AST,DDT,ASB,FMY1,EI1,P1,PHIU1,PHIF1,FINELL582
3390 1MF1,FMU1,YNX1) INELL583
3391 DMY(IGR,IMEM)=(FMY1-FMY(1))/10 INELL584
3392 C INELL585
3393 C GENERATE MISSING ELEMENTS INELL586
3394 C INELL587
3395 609 IF (IMEM.EQ.NMEM) GOTO 650 INELL588
3396 IMEM=IMEM+1 INELL589
3397 IF (IMEM.EQ.IABS(INNEL)) GOTO 330 INELL590
3398 GO TO 360 INELL591
3399 C INELL592
3400 C PRINT COMPUTED MEMBER PROPERTIES INELL593
3401 C INELL594
3402 650 DO 700 I=1,NMEM INELL595
3403 IF(I.EQ.1) PRINT 660 INELL596
3404 PRINT 680, I,IP(I),IP(I+NMEM),IP(I+2*NMEM),IP(I+3*NMEM), INELL597
3405 1P(I+4*NMEM),PR(I),PR(I+NMEM),PR(I+2*NMEM),PR(I+3*NMEM), INELL598
3406 2PR(I+4*NMEM),(PPR(J,I),J=1,2),(PPR(J,I+NMEM),J=1,2), INELL599
3407 3(PPR(J,I+2*NMEM),J=1,2),PPR(1,I)/PR(I),PPR(1,I+3*NMEM), INELL600
3408 4PPR(1,I+4*NMEM) INELL601
3409 700 CONTINUE INELL602
3410 660 FORMAT(///36H*** COMPUTED MEMBER PROPERTIES ***// INELL603
3411 1 3HEL.,1X,4HNODE,3X,4HMATL,1X,7HYOUNG'S,3X,5HLONG., INELL604
3412 2 1X,6HHARDEN,1X,6HS/SPAN,1X,6HCONFIN,2X,13H YIELD MOMENT,4X, INELL605
3413 3 13HULT. MOMENT,4X,11HFAIL MOMENT,9X,' CURVATURES'//, INELL606
3414 5 3HNO.,1X,4H I/J,1X,8HCO/ST/SE,1X,7HMODULUS,1X,5HSTL %, INELL607
3415 6 1X,6H RATIO,1X,6H RATIO,1X,6H RATIO,3X,13HPOSI. NEGA ,3X, INELL608
3416 7 13HPOSI. NEGA ,5X,11HPOSI. NEGA,5X,21HYIELD MAX MO. FAINELL609
3417 81L /) INELL610
3418 680 FORMAT(/I2,1X,I2,'/',I2,1X,I1,'/',I1,'/',I1,1X,E9.3,1X,F5.3,1X, INELL611
3419 1F6.4,1X,F5.2,1X,F6.3,2F9.2,2F9.2,1X,2F9.2,1X,2F7.4,1X,F7.4) INELL612
3420 IF(IGR.GE.1) NUM=NUM+NMEM INELL613
3421 RETURN INELL614
3422 END INELL615
3423 SUBROUTINE FMPHI(SLR,AXF,H,B,DD,ASD,D,AS,FMY,EI,P,PHIU,PHIF,FMF, FMPHI 1
3424 1FMU,YNX) FMPHI 2
3425 IMPLICIT REAL*8(A-H,O-Z) FMPHI 3
3426 COMMON/INFEL/IMEM,IMEMD,DUM(214) FMPHI 4
3427 COMMON/WORK/W1(810),ES,PS,FSY,EPSSU,EPSSU,FSU,FC,RDD,EC,PC,FCY, FMPHI 5
3428 1 EPSCY,EPSCU,FCU,EPSCM,PCP,F,FN,FN1,PS1,PC1,PHI,FM, FMPHI 6
3429 2 EPSS,EPSC,EPSSD,Y,PSP,W2(72) FMPHI 7
3430 C FMPHI 8
3431 DDD=.85*FCU*B*H FMPHI 9
3432 DDD=(DDD*H*.5+FSY*(ASD*DD+AS*D))/(DDD+(AS+ASD)*FSY) FMPHI 10
3433 DEPS=EPSSY/10. FMPHI 11
3434 ICODE=2 FMPHI 12
3435 DO 5 I=1,10 FMPHI 13
3436 EPS=DFLOAT(1)*DEPS FMPHI 14
3437 DO 10 KSD=1,2 FMPHI 15
3438 DO 10 KC=1,4 FMPHI 16
3439 CALL NUTAX(AXF,ICODE,KC,KS,KSD,IOK,EPS,H,B,DD,ASD,D,AS,DDD) FMPHI 17
3440 IF(IOK.EQ.0)GO TO 20 FMPHI 18

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3441	10 CONTINUE	FMPHI 19
3442	20 CALL MOMENT(AXF,KC,KS,KSD,H,B,DD,ASD,D,AS,DDD,ICODE)	FMPHI 20
3443	5 CONTINUE	FMPHI 21
3444	c 5 WRITE(30,100) Y,EPSC,EPSSD,EPSS,PHI,FM	FMPHI 22
3445	PHIY=PHI	FMPHI 23
3446	FMY=FM	FMPHI 24
3447	FMP=FMY	FMPHI 25
3448	FMU=FMY	FMPHI 26
3449	YNX=Y	FMPHI 27
3450	PEPSC=EPSC	FMPHI 28
3451	PEPSS=EPSS	FMPHI 29
3452	C	FMPHI 30
3453	IULT=0	FMPHI 31
3454	ICODE=1	FMPHI 32
3455	EPS=EPSCM	FMPHI 33
3456	DO 24 KSD=1,2	FMPHI 34
3457	DO 24 KS=2,3	FMPHI 35
3458	CALL NUTAX(AXF,ICODE,KC,KS,KSD,IOK,EPS,H,B,DD,ASD,D,AS,DDD)	FMPHI 36
3459	IF(IOK.EQ.0)GO TO 26	FMPHI 37
3460	24 CONTINUE	FMPHI 38
3461	26 IF(EPSS.GT. EPSSU) GO TO 51	FMPHI 39
3462	PEPS=PEPSC	FMPHI 40
3463	DEPS=(EPSCM-PEPSC)/20.	FMPHI 41
3464	DO 25 I=1,20	FMPHI 42
3465	EPS=PEPS+DFLOAT(I)*DEPS	FMPHI 43
3466	IF(EPS.GT.EPSCM)EPS=EPSCM	FMPHI 44
3467	DO 30 KSD=1,2	FMPHI 45
3468	DO 30 KS=2,3	FMPHI 46
3469	CALL NUTAX(AXF,ICODE,KC,KS,KSD,IOK,EPS,H,B,DD,ASD,D,AS,DDD)	FMPHI 47
3470	IF(IOK.EQ.0)GO TO 35	FMPHI 48
3471	30 CONTINUE	FMPHI 49
3472	35 CALL MOMENT(AXF,KC,KS,KSD,H,B,DD,ASD,D,AS,DDD,ICODE)	FMPHI 50
3473	IF(IULT.EQ.1) GO TO 33	FMPHI 51
3474	IF(FMP.GT.FM) THEN	FMPHI 52
3475	FMU=FMP	FMPHI 53
3476	PHIU=PHIP	FMPHI 54
3477	IULT=1	FMPHI 55
3478	ELSE	FMPHI 56
3479	ENDIF	FMPHI 57
3480	33 PEPSS=EPSS	FMPHI 58
3481	IF(EPSSD.GE.EPSCM) GO TO 200	FMPHI 59
3482	IF(FM.LE.0.75*FMY) GO TO 300	FMPHI 60
3483	IF(EPSS.GE.1.5*EPSSU) GO TO 400	FMPHI 61
3484	EPSSDP=EPSSD	FMPHI 62
3485	EPSSP=EPSS	FMPHI 63
3486	FMP=FM	FMPHI 64
3487	PHIP=PHI	FMPHI 65
3488	IF(EPS.GE.EPSCM) GO TO 51	FMPHI 66
3489	25 CONTINUE	FMPHI 67
3490	c WRITE(30,100) Y,EPSC,EPSSD,EPSS,PHI,FM	FMPHI 68
3491	C	FMPHI 69
3492	51 PEPSS=PEPSS	FMPHI 70
3493	IEPS=0	FMPHI 71
3494	DEPS=(EPSSU-PEPS)/20.	FMPHI 72
3495	EI=1.5*(PEPS/DEPS)+1	FMPHI 73
3496	NN=30+IDINT(EI)+1	FMPHI 74
3497	IF(DEPS.LE. 0.0) DEPS=EPSSU/20.	FMPHI 75
3498	IF(DEPS.LE. 0.0) NN=100	FMPHI 76
3499	ICODE=2	FMPHI 77
3500	DO 40 I=1,NN	FMPHI 78
3501	EPS=PEPS+DEPS*DFLOAT(I)	FMPHI 79
3502	IF(IEPS.EQ.1) GO TO 52	FMPHI 80
3503	IF(EPS.GT.EPSSU) THEN	FMPHI 81
3504	EPS=EPSSU	FMPHI 82
3505	IEPS=1	FMPHI 83
3506	ELSE	FMPHI 84
3507	ENDIF	FMPHI 85
3508	52 DO 50 KC=1,4	FMPHI 86
3509	DO 50 KSD=1,2	FMPHI 87
3510	CALL NUTAX(AXF,ICODE,KC,KS,KSD,IOK,EPS,H,B,DD,ASD,D,AS,DDD)	FMPHI 88
3511	IF(IOK.EQ.0)GO TO 45	FMPHI 89
3512	50 CONTINUE	FMPHI 90
3513	45 CALL MOMENT(AXF,KC,KS,KSD,H,B,DD,ASD,D,AS,DDD,ICODE)	FMPHI 91
3514	IF(IULT.EQ.1) GO TO 43	FMPHI 92
3515	IF(FMP.GT.FM) THEN	FMPHI 93
3516	FMU=FMP	FMPHI 94
3517	PHIU=PHIP	FMPHI 95
3518	IULT=1	FMPHI 96
3519	ELSE	FMPHI 97
3520	ENDIF	FMPHI 98

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3521      43 IF(EPSSD.GE.EPSCM) GO TO 200      FMPHI 99
3522      IF(FM.LE.0.75*FMY) GO TO 300      FMPHI100
3523      IF(EPSS.GT.1.5*EPSSU) GO TO 400      FMPHI101
3524      EPSSDP=EPSSD      FMPHI102
3525      EPSSP=EPSS      FMPHI103
3526      FMP=FM      FMPHI104
3527      PHIP=PHI      FMPHI105
3528      40 CONTINUE      FMPHI106
3529      c 40 if(imem.eq.8)WRITE(30,100) Y,EPSC,EPSSD,EPSS,PHI,FM      FMPHI107
3530      200 DRA=(EPSCM-EPSSDP)/(EPSSD-EPSSDP)      FMPHI108
3531      GO TO 500      FMPHI109
3532      300 DRA=(0.75*FMY-FMP)/(FM-FMP)      FMPHI110
3533      GO TO 500      FMPHI111
3534      400 DRA=(1.5*EPSSU-EPSSP)/(EPSS-EPSSP)      FMPHI112
3535      500 FMF=FMP+DRA*(FM-FMP)      FMPHI113
3536      PHIF=PHIP+DRA*(PHI-PHIP)      FMPHI114
3537      IF(IULT.EQ.0) THEN      FMPHI115
3538      FMU=FMF      FMPHI116
3539      PHIU=PHIF      FMPHI117
3540      FMF=0.75*FMF      FMPHI118
3541      PHIF=PHIF*1.5      FMPHI119
3542      ELSE      FMPHI120
3543      ENDIF      FMPHI121
3544      c WRITE(30,100) Y,EPSC,EPSSD,EPSS,PHIF,FMF      FMPHI122
3545      EI=FMY/PHIY      FMPHI123
3546      P=(FMU-FMY)/((PHIU-PHIY)*EI)      FMPHI124
3547      100 FORMAT(5(F10.5,2X),F12.5)      FMPHI125
3548      RETURN      FMPHI126
3549      END      FMPHI127
3550      SUBROUTINE NUTAX(AXF, ICODE, KC, KS, KSD, IOK, EPS, H, B, DD, ASD, D, AS, DDD)      NUTAX 1
3551      IMPLICIT REAL*8(A-H, O-Z)      NUTAX 2
3552      COMMON/WORK/W1(810), ES, PS, FSY, EPSSY, EPSSU, FSU, FC, RDD, EC, PC, FCY,      NUTAX 3
3553      1 EPSCY, EPSCU, FCU, EPSCM, PCP, F, FN, FN1, PS1, PC1, PHI, FM,      NUTAX 4
3554      2 EPSS, EPSC, EPSSD, Y, PSP, W2(72)      NUTAX 5
3555      IOK=0      NUTAX 6
3556      RCY=EPSCY/EPS      NUTAX 7
3557      RCU=EPSCU/EPS      NUTAX 8
3558      RCM=EPSCM/EPS      NUTAX 9
3559      RSY=EPSSY/EPS      NUTAX 10
3560      RSU=EPSSU/EPS      NUTAX 11
3561      GO TO(10,20), ICODE      NUTAX 12
3562      10 EPSC=EPS      NUTAX 13
3563      BETA=-AXF/(EC*EPS)      NUTAX 14
3564      GAMA=0.      NUTAX 15
3565      KC=1      NUTAX 16
3566      IF(EPS.GT.EPSCY)KC=2      NUTAX 17
3567      IF(EPS.GT.EPSCU)KC=3      NUTAX 18
3568      IF(EPS.GT.EPSCM) KC=4      NUTAX 19
3569      GO TO 30      NUTAX 20
3570      20 EPSS=EPS      NUTAX 21
3571      BETA=AXF/(EC*EPS)      NUTAX 22
3572      GAMA=AXF*D/(EC*EPS)      NUTAX 23
3573      KS=1      NUTAX 24
3574      IF(EPS.GT.EPSSY)KS=2      NUTAX 25
3575      IF(EPS.GT.EPSSU)KS=3      NUTAX 26
3576      30 KC1=KC+4*(ICODE-1)      NUTAX 27
3577      KS1=KS+3*(ICODE-1)      NUTAX 28
3578      KSD1=KSD+2*(ICODE-1)      NUTAX 29
3579      GO TO (40,50,66,68,40,60,67,69), KC1      NUTAX 30
3580      40 ALFA=0.5*B      NUTAX 31
3581      GO TO 70      NUTAX 32
3582      50 ALFA=0.5*B*(1-PC1*(1-RCY)**2)      NUTAX 33
3583      GO TO 70      NUTAX 34
3584      60 ALFA=0.5*B*(1-PC1*(1+RCY)**2)      NUTAX 35
3585      BETA=BETA+B*D*PC1*(RCY+RCY*RCY)      NUTAX 36
3586      GAMA=GAMA+B*.5*PC1*(D*RCY)**2      NUTAX 37
3587      GO TO 70      NUTAX 38
3588      66 ALFA=0.5*B*(1-PC1*(1-RCY)**2-(PC+PCP)*(1-RCU)**2)      NUTAX 39
3589      GO TO 70      NUTAX 40
3590      67 ALFA=0.5*B*(1-PC1*(1+RCY)**2-(PC+PCP)*(1+RCU)**2)      NUTAX 41
3591      BETA=BETA+B*D*(PC1*(RCY+RCY*RCY)+(PC+PCP)*(RCU+RCU*RCU))      NUTAX 42
3592      GAMA=GAMA+.5*B*D*(PC1*RCY*RCY+(PC+PCP)*RCU*RCU)      NUTAX 43
3593      GO TO 70      NUTAX 44
3594      68 ALFA=0.5*B*(1-PC1*(1-RCY)**2-(PC+PCP)*(1-RCU)**2+      NUTAX 45
3595      1PCP*(1-RCM)**2)      NUTAX 46
3596      GO TO 70      NUTAX 47
3597      69 ALFA=0.5*B*(1-PC1*(1+RCY)**2-(PC+PCP)*(1+RCU)**2+      NUTAX 48
3598      1PCP*(1+RCM)**2)      NUTAX 49
3599      BETA=BETA+B*D*(PC1*(RCY+RCY*RCY)+(PC+PCP)*(RCU+RCU*RCU)-      NUTAX 50
3600      1PCP*(RCM+RCM*RCM))      NUTAX 51

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3601      GAMA=GAMA+.5*B*D*D*(PC1*RCY*RCY+(PC+PCP)*RCU*RCU-PCP*
3602      1RCM*RCM)
3603      70 GO TO (80,90,80,100),KSD1
3604      80 BETA=BETA+FN1*ASD
3605      GAMA=GAMA+FN1*ASD*DD
3606      GO TO 110
3607      90 BETA=BETA+FN1*RSY*ASD
3608      GO TO 110
3609      100 BETA=BETA-FN1*RSY*ASD
3610      GAMA=GAMA-FN1*RSY*ASD*D
3611      110 GO TO(120,130,135,120,140,145),KS1
3612      120 BETA=BETA+FN*AS
3613      GAMA=GAMA+FN*AS*D
3614      GO TO 150
3615      130 BETA=BETA+FN*AS*(1-PS1*(1+RSY))
3616      GAMA=GAMA+FN*AS*PS*D
3617      GO TO 150
3618      135 BETA=BETA+FN*AS*(1-PS1*(1+RSY)-(PS+PSP)*(1+RSU))
3619      GAMA=GAMA-FN*AS*PSP*D
3620      GO TO 150
3621      140 BETA=BETA+FN*AS*(1-PS1*(1-RSY))
3622      GAMA=GAMA+FN*AS*(PS+PS1*RSY)*D
3623      GO TO 150
3624      145 BETA=BETA+FN*AS*(1-PS1*(1-RSY)-(PS+PSP)*(1-RSU))
3625      GAMA=GAMA+FN*AS*(-PSP+PS1*RSY+(PS+PSP)*RSU)*D
3626      150 DM=BETA**2+.4.*ALFA*GAMA
3627      IF(DM.LE.0.)GO TO 1000
3628      Y=(-BETA+DSQRT(DM))/(2.*ALFA)
3629      IF((Y.LE.0).OR.(Y.GT.H))GO TO 1000
3630      GO TO(160,170),ICODE
3631      160 PHI=EPSC/Y
3632      EPSS=PHI*(D-Y)
3633      GO TO 180
3634      170 PHI=EPSS/(D-Y)
3635      EPSC=PHI*Y
3636      180 EPSSD=PHI*(Y-DD)
3637      GO TO(190,200,205),KS
3638      190 IF(EPSS.GT.EPSSY)GO TO 1000
3639      GO TO 210
3640      200 IF(EPSS.LT.EPSSY.OR.EPSS.GT.EPSSU)GO TO 1000
3641      GO TO 210
3642      205 IF(EPSS.LT.EPSSU)GO TO 1000
3643      210 GO TO(220,230,231,232),KC
3644      220 IF(EPSC.GT.EPSCY)GO TO 1000
3645      GO TO 240
3646      230 IF((EPSC.LT.EPSCY).OR.(EPSC.GT.EPSCU))GO TO 1000
3647      GO TO 240
3648      231 IF((EPSC.LT.EPSCU).OR.(EPSC.GT.EPSCM))GO TO 1000
3649      GO TO 240
3650      232 IF(EPSC.LT.EPSCM)GO TO 1000
3651      240 GO TO (250,260), KSD
3652      250 IF(DABS(EPSSD).GT.EPSCM) GO TO 1000
3653      GO TO 270
3654      260 IF(DABS(EPSSD).LT.EPSCM) GO TO 1000
3655      270 RETURN
3656      1000 IOK=-1
3657      RETURN
3658      END
3659      SUBROUTINE MOMENT(AXF,KC,KS,KSD,H,B,DD,ASD,D,AS,DDD,ICODE)
3660      IMPLICIT REAL*8(A-H,O-Z)
3661      COMMON/WORK/W1(810),ES,PS,FSY,EPSSY,EPSSU,FSU,FC,RDD,EC,PC,FCY,
3662      1 EPSCY,EPSCU,FCU,EPSCM,PCP,F,FN,FN1,PS1,PC1,PHI,FM,
3663      2 EPSS,EPSC,EPSSD,Y,PSP,W2(72)
3664      C
3665      GO TO (10,20,21,22),KC
3666      10 CC=EC*.5*B*Y*EPSC
3667      FM=CC*(DDD-Y/3.)
3668      GO TO 30
3669      20 Y1=(EPSC-EPSCY)/PHI
3670      CC1=Y*Y
3671      CC2=Y1*Y1*PC1
3672      CON=EC*PHI*B*.5
3673      CC=CON*(CC1-CC2)
3674      FM=CON*(CC1*(DDD-Y/3.)-CC2*(DDD-Y1/3.))
3675      GO TO 30
3676      21 Y1=(EPSC-EPSCY)/PHI
3677      Y2=(EPSC-EPSCU)/PHI
3678      CC1=Y**2
3679      CC2=PC1*Y1**2
3680      CC3=(PC+PCP)*Y2**2

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MOMEN 22

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3681      CON=EC*PHI*B*.5                                MOMEN 23
3682      CC=CON*(CC1-CC2-CC3)                            MOMEN 24
3683      FM=CON*(CC1*(DDD-Y/3.)-CC2*(DDD-Y1/3.)-CC3*(DDD-Y2/3.)) MOMEN 25
3684      GO TO 30                                          MOMEN 26
3685      22 Y1=(EPSC-EPSCY)/PHI                          MOMEN 27
3686      Y2=(EPSC-EPSCU)/PHI                            MOMEN 28
3687      Y3=(EPSC-EPSCM)/PHI                          MOMEN 29
3688      CC1=Y**2                                          MOMEN 30
3689      CC2=PC1*Y1**2                                    MOMEN 31
3690      CC3=(PC+PCP)*Y2**2                              MOMEN 32
3691      CC4=PCP*Y3**2                                    MOMEN 33
3692      CON=EC*PHI*B*.5                                MOMEN 34
3693      CC=CON*(CC1-CC2-CC3+CC4)                      MOMEN 35
3694      FM=CON*(CC1*(DDD-Y/3.)-CC2*(DDD-Y1/3.)-CC3*(DDD-Y2/3.)+ MOMEN 36
3695      1CC4*(DDD-Y3/3.))                               MOMEN 37
3696      30 FMC=FM                                        MOMEN 38
3697      GO TO(40,50,55),KS                             MOMEN 39
3698      40 T=ES*AS*EPSS                                 MOMEN 40
3699      GO TO 60                                         MOMEN 41
3700      50 T=ES*AS*(PS1*EPSSY+PS*EPSS)                MOMEN 42
3701      GO TO 60                                         MOMEN 43
3702      55 T=ES*AS*(-PSP*EPSS+PS1*EPSSY+(PS+PSP)*EPSSU) MOMEN 44
3703      60 FMST=T*(D-DDD)                               MOMEN 45
3704      FM=FM+T*(D-DDD)                                 MOMEN 46
3705      GO TO (80,85), KSD                             MOMEN 47
3706      80 CS=FN1*EC*ASD*EPSSD                        MOMEN 48
3707      GO TO 90                                         MOMEN 49
3708      85 CS=FN1*EC*ASD*EPSSY                        MOMEN 50
3709      90 FMSC=CS*(DDD-DD)                            MOMEN 51
3710      FM=FM+CS*(DDD-DD)                             MOMEN 52
3711      P1=CC+CS-T                                     MOMEN 53
3712      C WRITE(50,300) PHI,FMC,FMST,FMSC             MOMEN 54
3713      TOL=.001                                        MOMEN 55
3714      ERR=DABS(AXF-P1)                               MOMEN 56
3715      IF(ERR.LE.TOL)RETURN                          MOMEN 57
3716      PRINT 200                                       MOMEN 58
3717      200 FORMAT('SOMETHING WRONG')                 MOMEN 59
3718      C 300 FORMAT(4F10.4)                          MOMEN 60
3719      STOP                                            MOMEN 61
3720      END                                             MOMEN 62
3721      SUBROUTINE STIF (MSTEP,NDOF,NINFC,COMS,FK,DFAC) STIF 1
3722      IMPLICIT REAL*8(A-H,O-Z)                       STIF 2
3723      C                                               STIF 3
3724      COMMON/INFEL/IMEM,IMEMD,KST,KSTD,LM(6),LMD(6),KGEOM,KGEOMD,PSH, STIF 4
3725      1 KHYST,KHYSTD,FL,COSA,SINA,EAL,A(2,6),ST(2,2),ECC(4), STIF 5
3726      2 KODY(2),XI(2),Q(2),ALPHAP(2,2),EI(5,2,2),PHF(2,2),PHY(2,2), STIF 6
3727      3 PHU(2,2),PHI(2),FM1(2,2),PH1(2,2),FMx(2,2),PHx(2,2),FMp(2,2), STIF 7
3728      4 PHp(2,2),PHr(2,2),RD3(2,2),RD4(2,2),RD5(2,2),FMxM(2,2), STIF 8
3729      5 PHxM(2,2),BMY(2,2),BMEP(2),HYS(2),PPH1(2,2),BMMP,PHMX, STIF 9
3730      6 BMTOT(2),SFTOT(2),FTOT(2),PRTOT(2),SENP(8),SENN(8),TENP(8), STIF 10
3731      7 TENN(8),PRACP(2),PRACN(2),SDACT(3),NOD1,NODJ,KOUTDT,KOUTDTD, STIF 11
3732      8 REST(26)                                       STIF 12
3733      COMMON/WORK/ GA(6,6),PST(2,2),DST(2,2),ATK(6,2),AA(2,6),PFL,AXK, STIF 13
3734      1 FFK(6,6),ELS(6,6),FAC,W(1457)                STIF 14
3735      C                                               STIF 15
3736      DIMENSION COM(1),COMS(1),FK(6,6)              STIF 16
3737      EQUIVALENCE(IMEM,COM(1))                       STIF 17
3738      C                                               STIF 18
3739      C STIFFNESS FORMULATION, BEAM COLUMN ELEMENTS STIF 19
3740      C                                               STIF 20
3741      DO 10 J=3,NINFC                                  STIF 21
3742      10 COM(J)=COMS(J)                                STIF 22
3743      C                                               STIF 23
3744      C SAVE PREVIOUS FLEXURAL STIFFNESS             STIF 24
3745      C                                               STIF 25
3746      DO 20 I=1,4                                     STIF 26
3747      20 PST(I,1)=ST(I,1)                             STIF 27
3748      C                                               STIF 28
3749      C CURRENT FLEXURAL STIFFNESS, ELASTO-PLASTIC PART STIF 29
3750      C                                               STIF 30
3751      CALL FSTF(mstep)                                STIF 31
3752      C                                               STIF 32
3753      IF (MSTEP.LT.2) GO TO 50                       STIF 33
3754      DO 30 I=1,4                                     STIF 34
3755      30 DST(I,1)=ST(I,1)-PST(I,1)                   STIF 35
3756      CALL MULTST (A,DST,ATK,FK,6,2)                 STIF 36
3757      C                                               STIF 37
3758      C SAVE CURRENT FLEXURAL STIFFNESS FOR NEXT STEP STIF 38
3759      C                                               STIF 39
3760      120 DO 40 I=28,31                              STIF 40

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3761	40	COMS(1)=COM(1)	STIF	41
3762		DO 45 I=37,38	STIF	42
3763	45	COMS(1)=COM(1)	STIF	43
3764		RETURN	STIF	44
3765	C		STIF	45
3766	C	ORIGINAL STIFFNESS AT STEP 0, BETA-0 CORR AT STEP 1	STIF	46
3767	C		STIF	47
3768	50	FAC=1.	STIF	48
3769		IF (MSTEP.EQ.1) FAC=DFAC	STIF	49
3770		DO 60 I=1,4	STIF	50
3771	60	DST(I,1)=ST(I,1)*FAC	STIF	51
3772		CALL MULTST (A,DST,ATK,FK,6,2)	STIF	52
3773		IF (FAC.EQ.0.) GO TO 90	STIF	53
3774		EAL=EAL*FAC	STIF	54
3775		AXK=EAL*COA**2	STIF	55
3776		FK(1,1)=FK(1,1)+AXK	STIF	56
3777		FK(1,4)=FK(1,4)-AXK	STIF	57
3778		FK(4,4)=FK(4,4)+AXK	STIF	58
3779		AXK=EAL*SINA**2	STIF	59
3780		FK(2,2)=FK(2,2)+AXK	STIF	60
3781		FK(2,5)=FK(2,5)-AXK	STIF	61
3782		FK(5,5)=FK(5,5)+AXK	STIF	62
3783		AXK=EAL*SINA*COA	STIF	63
3784		FK(1,2)=FK(1,2)+AXK	STIF	64
3785		FK(1,5)=FK(1,5)-AXK	STIF	65
3786		FK(2,4)=FK(2,4)-AXK	STIF	66
3787		FK(4,5)=FK(4,5)+AXK	STIF	67
3788		IF (ECC(1).EQ.1.23456E10) GO TO 70	STIF	68
3789		EC3=COA*ECC(3)-SINA*ECC(1)	STIF	69
3790		EC4=SINA*ECC(2)-COA*ECC(4)	STIF	70
3791		AXK=COA*EC3*EAL	STIF	71
3792		FK(1,3)=FK(1,3)-AXK	STIF	72
3793		FK(3,4)=FK(3,4)+AXK	STIF	73
3794		AXK=SINA*EC3*EAL	STIF	74
3795		FK(2,3)=FK(2,3)-AXK	STIF	75
3796		FK(3,5)=FK(3,5)+AXK	STIF	76
3797		FK(3,3)=FK(3,3)+EAL*EC3**2	STIF	77
3798		AXK=COA*EC4*EAL	STIF	78
3799		FK(1,6)=FK(1,6)-AXK	STIF	79
3800		FK(4,6)=FK(4,6)+AXK	STIF	80
3801		AXK=SINA*EC4*EAL	STIF	81
3802		FK(2,6)=FK(2,6)-AXK	STIF	82
3803		FK(5,6)=FK(5,6)+AXK	STIF	83
3804		FK(3,6)=FK(3,6)+EC3*EC4*EAL	STIF	84
3805		FK(6,6)=FK(6,6)+EC4**2*EAL	STIF	85
3806		EAL=EAL/FAC	STIF	86
3807	70	DO 80 I=1,6	STIF	87
3808		DO 80 J=I,6	STIF	88
3809	80	FK(J,I)=FK(I,J)	STIF	89
3810	C		STIF	90
3811	C	ADD GEOMETRIC STIFFNESS	STIF	91
3812	C		STIF	92
3813	90	IF (MSTEP.EQ.0.OR.KGEOM.EQ.0) GO TO 120	STIF	93
3814		PFL=(COMS(134)-COMS(133))/2.	STIF	94
3815		DO 100 I=1,36	STIF	95
3816		GA(I,1)=0.	STIF	96
3817	100	ELS(I,1)=0.	STIF	97
3818		CN=PFL/30.	STIF	98
3819		C1=CN*36./FL	STIF	99
3820		C2=CN*3.	STIF	100
3821		C3=CN*4.*FL	STIF	101
3822		C4=CN*FL	STIF	102
3823		ELS(2,2)=C1	STIF	103
3824		ELS(2,3)=C2	STIF	104
3825		ELS(2,5)=-C1	STIF	105
3826		ELS(2,6)=C2	STIF	106
3827		ELS(3,3)=C3	STIF	107
3828		ELS(3,5)=-C2	STIF	108
3829		ELS(3,6)=-C4	STIF	109
3830		ELS(5,5)=C1	STIF	110
3831		ELS(5,6)=-C2	STIF	111
3832		ELS(6,6)=C3	STIF	112
3833	C		STIF	113
3834		GA(1,1)=COA	STIF	114
3835		GA(1,2)=SINA	STIF	115
3836		GA(2,1)=-SINA	STIF	116
3837		GA(2,2)=COA	STIF	117
3838		GA(3,3)=1.	STIF	118
3839		GA(4,4)=COA	STIF	119
3840		GA(4,5)=SINA	STIF	120

3841		GA(5,4)=-SINA	STIF 121
3842		GA(5,5)=COSA	STIF 122
3843		GA(6,6)=1.	STIF 123
3844	C		STIF 124
3845		DO 105 I=1,6	STIF 125
3846		DO 105 J=1,6	STIF 126
3847	105	ELS(J,I)=ELS(I,J)	STIF 127
3848	C		STIF 128
3849		CALL MULTST (GA,ELS,ATK,FFK,6,6)	STIF 129
3850		DO 110 I=1,36	STIF 130
3851	110	FK(I,1)=FK(I,1)+FFK(I,1)	STIF 131
3852		GO TO 120	STIF 132
3853		END	STIF 133
3854		SUBROUTINE FSTF (MSTEP)	FSTF 1
3855		IMPLICIT REAL*8 (A-H,O-Z)	FSTF 2
3856	C		FSTF 3
3857	C	FORM 2*2 FLEXURAL STIFFNESS	FSTF 4
3858	C		FSTF 5
3859		COMMON/INFEL/IMEM, IMEMD, KST, KSTD, LM(6), LMD(6), KGEOM, KGEOMD, PSH,	FSTF 6
3860	1	KHYST, KHYSTD, FL, COSA, SINA, EAL, A(2,6), ST(2,2), ECC(4),	FSTF 7
3861	2	KODY(2), X(2), Q(2), ALPHAP(2,2), EI(5,2,2), PHF(2,2), PHY(2,2),	FSTF 8
3862	3	PHU(2,2), PHI(2), FM1(2,2), PH1(2,2), FMX(2,2), PHX(2,2), FMP(2,2),	FSTF 9
3863	4	PHp(2,2), PHr(2,2), RD3(2,2), RD4(2,2), RD5(2,2), FMXM(2,2),	FSTF 10
3864	5	PHXM(2,2), BMYI(2,2), BMEP(2), HYS(2), PPH1(2,2), BMMP, PHMX,	FSTF 11
3865	6	BMTOT(2), SFTOT(2), FTOT(2), PRTOT(2), SENP(8), SENN(8), TENP(8),	FSTF 12
3866	7	TENN(8), PRACP(2), PRACN(2), SDACT(3), NODI, NODJ, KOUTDT, KOUTDTD,	FSTF 13
3867	8	REST(26)	FSTF 14
3868	C		FSTF 15
3869	C	USE THE CONCEPT OF PLASTIC HINGES	FSTF 16
3870	C		FSTF 17
3871		FL2=FL**2	FSTF 18
3872		FL3=FL**3	FSTF 19
3873		ICI=KODY(1)	FSTF 20
3874		ICJ=KODY(2)	FSTF 21
3875		QI=DABS(Q(1))	FSTF 22
3876		QJ=DABS(Q(2))	FSTF 23
3877		FMI=BMEP(1)	FSTF 24
3878		FMJ=BMEP(2)	FSTF 25
3879		FMT=DABS(FMI-FMJ)	FSTF 26
3880		GO TO (10,20,30,30,30), ICI	FSTF 27
3881	10	XI=0.	FSTF 28
3882		GO TO 40	FSTF 29
3883	20	IF(FMI)22,21,21	FSTF 30
3884	21	FMYI=BMIY(1,1)	FSTF 31
3885		XI=DABS(FMI-FMYI)/FMT*FL	FSTF 32
3886		IF(XI.GT.X(1))X(1)=XI	FSTF 33
3887		IF(XI.LE.X(1))X(1)=X(1)	FSTF 34
3888		GO TO 40	FSTF 35
3889	22	FMYI=BMIY(2,1)	FSTF 36
3890		XI=DABS(FMI-FMYI)/FMT*FL	FSTF 37
3891		IF(XI.GT.X(1))X(1)=XI	FSTF 38
3892		IF(XI.LE.X(1))X(1)=X(1)	FSTF 39
3893		GO TO 40	FSTF 40
3894	30	XI=X(1)	FSTF 41
3895	40	GO TO (50,70,60,60,60), ICJ	FSTF 42
3896	50	XJ=0.	FSTF 43
3897		GO TO 75	FSTF 44
3898	60	XJ=X(2)	FSTF 45
3899		GO TO 75	FSTF 46
3900	70	IF(FMJ)72,71,71	FSTF 47
3901	71	FMYJ=BMIY(1,2)	FSTF 48
3902		XJ=DABS(FMJ-FMYJ)/FMT*FL	FSTF 49
3903		IF(XJ.GE.X(2))X(2)=XJ	FSTF 50
3904		IF(XJ.LT.X(2))XJ=X(2)	FSTF 51
3905		GO TO 75	FSTF 52
3906	72	FMYJ=BMIY(2,2)	FSTF 53
3907		XJ=DABS(FMJ-FMYJ)/FMT*FL	FSTF 54
3908		IF(XJ.GE.X(2))X(2)=XJ	FSTF 55
3909		IF(XJ.LT.X(2))XJ=X(2)	FSTF 56
3910	C	75 IF((XI+XJ).LE.FL)GO TO 80	FSTF 57
3911	75	IF(XI.GT.FL/2.) XI=FL/2.	FSTF 58
3912		IF(XJ.GT.FL/2.) XJ=FL/2.	FSTF 59
3913	C	XI=FL/2.	FSTF 60
3914	C	XJ=XI	FSTF 61
3915		X(1)=XI	FSTF 62
3916		X(2)=XJ	FSTF 63
3917	80	EIE=EI(1,1,1)	FSTF 64
3918		XX=1./(3.*EIE*FL2)	FSTF 65
3919		QI1=QI-1.	FSTF 66
3920		QJ1=QJ-1.	FSTF 67

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3921      F11=(QJ1*XJ**3-QI1*(FL-XI)**3+QI*FL3)*XX      FSTF 68
3922      F22=(QI1*XI**3-QJ1*(FL-XJ)**3+QJ*FL3)*XX      FSTF 69
3923      F12=-(QJ1*XJ**2*(1.5*FL-XJ)+                  FSTF 70
3924      1      QI1*XI**2*(1.5*FL-XI)+FL3*0.5)*XX      FSTF 71
3925      DET=F11*F22-F12*F12                            FSTF 72
3926      ST(1,1)=F22/DET                                 FSTF 73
3927      ST(2,2)=F11/DET                                 FSTF 74
3928      ST(1,2)=-F12/DET                               FSTF 75
3929      ST(2,1)=ST(1,2)                               FSTF 76
3930      RETURN                                          FSTF 77
3931      END                                              FSTF 78
3932      SUBROUTINE RESP (NDOF,NINFC,KBAL,KPR,COMS,DDISM,DD,TIME,VELM,DFAC,RESP 1
3933      1DELTA,ELDAM,ELHYS)                               RESP 2
3934      IMPLICIT REAL*8(A-H,O-Z)                          RESP 3
3935      C                                                 RESP 4
3936      C      STATE DETERMINATION, BEAM COLUMN ELEMENTS  RESP 5
3937      C                                                 RESP 6
3938      COMMON/INFEL/IMEM,IMEMD,KST,KSTD,LM(6),LMD(6),KGEOM,KGEOMD,PSH,RESP 7
3939      1      KHYST,KHYSTD,FL,COSA,SINA,EAL,A(2,6),ST(2,2),ECC(4),RESP 8
3940      2      KODY(2),XI(2),Q(2),ALPHAP(2,2),EI(5,2,2),PHF(2,2),PHY(2,2),RESP 9
3941      3      PHU(2,2),PHI(2),FM1(2,2),PH1(2,2),FMx(2,2),PHx(2,2),FMp(2,2),RESP 10
3942      4      PHp(2,2),PHr(2,2),RD3(2,2),RD4(2,2),RD5(2,2),FMxm(2,2),RESP 11
3943      5      PHxm(2,2),BMIY(2,2),BMEP(2),HYS(2),PPH1(2,2),BMMP,PHMX,RESP 12
3944      6      BMTOT(2),SFTOT(2),FTOT(2),PRTOT(2),SENP(8),SENN(8),TENP(8),RESP 13
3945      7      TENN(8),PRACP(2),PRACN(2),SDACT(3),NODI,NODJ,KOUTDT,KOUTDTD,RESP 14
3946      8      INSLP(2,2),DAM(2),FMFI(2,2),RAC(2,2),FMDA(2,2),IDAM(2,2),RESP 15
3947      9      PHDA(2,2),FMxm(2,2)                        RESP 16
3948      COMMON/WORK/GA(6,6),DVR(2),DBM(2),BBMTOT(2),BML(2),DUM(6),RESP 17
3949      1      BMEL(2),DVAX,DFAX,FACAC,FAC,DSF,BMIUB,BMJUB,SFUB,KT(2),RESP 18
3950      2      KBL(2),W1(778),DPR(2),NPW(2),FACTOR,islop(2,2),W2(64)RESP 19
3951      COMMON/THIST/ITHOUT(10),THOUT(20),ITHP,ISAVE,NELTH,NSTH,NF7,ISERESP 20
3952      COMMON/PASS/IGR,ISTEP,DUMP(4),ISYM,ISYMD        RESP 21
3953      COMMON/DAMAGE/KDAMAGE,ITDAM,KIDAMT,NNSKIP,NGSKIP,GLHYS,RESP 22
3954      1GLDAM                                           RESP 23
3955      COMMON/AUTO/KAUTO,KAUTOD,KECO,KECOD,NDSGN,NDSGND,KFREQ,KFREQD,RESP 24
3956      1      DBALL,DCALL,DBSTD,CONC,STEEL,IECO,BMAVG,BMDEV,ICONVRESP 25
3957      C                                                 RESP 26
3958      DIMENSION COM(1),COMS(1),DDISM(1),DD(1),VELM(1),NOD(2),ISLP(2)RESP 27
3959      EQUIVALENCE (IMEM,COM(1)),(NODI,NOD(1))          RESP 28
3960      C                                                 RESP 29
3961      DO 10 J=1,NINFC                                   RESP 30
3962      10      COM(J)=COMS(J)                             RESP 31
3963      IF (IMEM.EQ.1) IHED=0                            RESP 32
3964      C                                                 RESP 33
3965      C      DEFORMATION INCREMENTS                     RESP 34
3966      C                                                 RESP 35
3967      IF (ECC(1).EQ.1.23456E10) GO TO 20              RESP 36
3968      DDISM(1)=DDISM(1)-ECC(3)*DDISM(3)                RESP 37
3969      DDISM(2)=DDISM(2)+ECC(1)*DDISM(3)                RESP 38
3970      DDISM(4)=DDISM(4)-ECC(4)*DDISM(6)                RESP 39
3971      DDISM(5)=DDISM(5)+ECC(2)*DDISM(6)                RESP 40
3972      20      DVAX=COSA*(DDISM(4)-DDISM(1))+SINA*(DDISM(5)-DDISM(2))RESP 41
3973      ROT=(SINA*(DDISM(4)-DDISM(1))+COSA*(DDISM(2)-DDISM(5)))/FLRESP 42
3974      DVR(1)=DDISM(3)+ROT                               RESP 43
3975      DVR(2)=DDISM(6)+ROT                               RESP 44
3976      C                                                 RESP 45
3977      C      AXIAL FORCE INCREMENT                     RESP 46
3978      C                                                 RESP 47
3979      DFAX=EAL*DVAX                                     RESP 48
3980      FTOT(1)=FTOT(1)-DFAX                             RESP 49
3981      FTOT(2)=FTOT(2)+DFAX                             RESP 50
3982      C                                                 RESP 51
3983      C      LINEAR MOMENT INCREMENTS                 RESP 52
3984      C                                                 RESP 53
3985      DBM(1)=ST(1,1)*DVR(1)+ST(1,2)*DVR(2)            RESP 54
3986      DBM(2)=ST(1,2)*DVR(1)+ST(2,2)*DVR(2)            RESP 55
3987      BML(1)=BMEP(1)+DBM(1)                            RESP 56
3988      BML(2)=BMEP(2)+DBM(2)                            RESP 57
3989      BMEL(1)=BMTOT(1)-BMEP(1)                         RESP 58
3990      BMEL(2)=BMTOT(2)-BMEP(2)                         RESP 59
3991      C                                                 RESP 60
3992      do 31 j=1,2                                       RESP 61
3993      do 31 i=1,2                                       RESP 62
3994      31      islop(i,j)=inslp(i,j)                    RESP 63
3995      C                                                 RESP 64
3996      DO 30 I=1,2                                       RESP 65
3997      30      CALL STATE(DBM(I),I,KT(I),KBL(I))         RESP 66
3998      C                                                 RESP 67
3999      do 32 j=1,2                                       RESP 68
4000      do 32 i=1,2                                       RESP 69

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4001	32 inslp(i,j)=islop(i,j)	RESP	70
4002	C	RESP	71
4003	IF(KDAMAGE.LT.1) GO TO 37	RESP	72
4004	IF(ISLP(1).NE.1.AND.ISLP(2).NE.1) IISLP=1	RESP	73
4005	ELHYS=HYS(1)+HYS(2)	RESP	74
4006	ELDAM=DAM(1)*HYS(1)+DAM(2)*HYS(2)	RESP	75
4007	C	RESP	76
4008	37 KBAL=0	RESP	77
4009	IF(KBL(1).NE.0.AND.KBL(2).NE.0)KBAL=1	RESP	78
4010	C	RESP	79
4011	C PLASTIC HINGE ROTATION	RESP	80
4012	C	RESP	81
4013	IF(KODY(1)-2) 40,50,60	RESP	82
4014	50 DPR(1)=DVR(1)+DVR(2)*ST(1,2)/ST(1,1)	RESP	83
4015	60 DPR(1)=DVR(1)	RESP	84
4016	40 IF(KODY(2)-2) 45,55,65	RESP	85
4017	55 DPR(2)=DVR(2)+DVR(1)*ST(1,2)/ST(2,2)	RESP	86
4018	65 DPR(2)=DVR(2)	RESP	87
4019	45 CONTINUE	RESP	88
4020	C	RESP	89
4021	C UPDATE ACCUMULATED PLASTIC HINGE ROTATION	RESP	90
4022	C	RESP	91
4023	DO 80 IEND=1,2	RESP	92
4024	IF(NPW(IEND).EQ.0)GO TO 80	RESP	93
4025	DPPR=FACTOR*DPR(IEND)	RESP	94
4026	PRTOT(IEND)=PRTOT(IEND)+DPPR	RESP	95
4027	IF(DPPR.LT.0) GO TO 90	RESP	96
4028	PRACP(IEND)=PRACP(IEND)+DPPR	RESP	97
4029	GO TO 80	RESP	98
4030	90 PRACN(IEND)=PRACN(IEND)+DPPR	RESP	99
4031	GO TO 80	RESP	100
4032	80 CONTINUE	RESP	101
4033	C	RESP	102
4034	C ELASTIC AND TOTAL FORCES	RESP	103
4035	C	RESP	104
4036	BBMTOT(1)=BMTOT(1)	RESP	105
4037	BBMTOT(2)=BMTOT(2)	RESP	106
4038	BMTOT(1)=BMEP(1)+BMEL(1)	RESP	107
4039	BMTOT(2)=BMEP(2)+BMEL(2)	RESP	108
4040	DSF=(BMTOT(1)-BBMTOT(1)+BMTOT(2)-BBMTOT(2))/FL	RESP	109
4041	SFTOT(1)=SFTOT(1)+DSF	RESP	110
4042	SFTOT(2)=SFTOT(2)-DSF	RESP	111
4043	C	RESP	112
4044	C UNBALANCED LOADS DUE TO YIELD	RESP	113
4045	C	RESP	114
4046	BMIUB=0.	RESP	115
4047	BMJUB=0.	RESP	116
4048	FOUB=0.	RESP	117
4049	IF (KBAL.EQ.0) GO TO 210	RESP	118
4050	BMIUB=BML(1)-BMEP(1)	RESP	119
4051	BMJUB=BML(2)-BMEP(2)	RESP	120
4052	C	RESP	121
4053	C DEFORMATION RATES FOR DAMPING	RESP	122
4054	C	RESP	123
4055	210 IF (DFAC.EQ.0.0.AND.DELTA.EQ.0.0) GO TO 240	RESP	124
4056	IF (TIME.EQ.0.) GO TO 250	RESP	125
4057	KBAL=1	RESP	126
4058	IF (ECC(1).EQ.1.23456E10) GO TO 220	RESP	127
4059	VELM(1)=VELM(1)-ECC(3)*VELM(3)	RESP	128
4060	VELM(2)=VELM(2)+ECC(1)*VELM(3)	RESP	129
4061	VELM(4)=VELM(4)-ECC(4)*VELM(6)	RESP	130
4062	VELM(5)=VELM(5)+ECC(2)*VELM(6)	RESP	131
4063	220 DVAX=COSA*(VELM(4)-VELM(1))+SINA*(VELM(5)-VELM(2))	RESP	132
4064	ROT=(SINA*(VELM(4)-VELM(1))+COSA*(VELM(2)-VELM(5)))/FL	RESP	133
4065	DVR(1)=VELM(3)+ROT	RESP	134
4066	DVR(2)=VELM(6)+ROT	RESP	135
4067	C	RESP	136
4068	C BETA-O DAMPING	RESP	137
4069	C	RESP	138
4070	IF (DFAC.EQ.0.) GO TO 230	RESP	139
4071	FAC=DFAC*(1./(1.-PSH))	RESP	140
4072	BMIUB=BMIUB+(ST(1,1)*DVR(1)+ST(1,2)*DVR(2))*FAC	RESP	141
4073	BMJUB=BMJUB+(ST(1,2)*DVR(1)+ST(2,2)*DVR(2))*FAC	RESP	142
4074	FOUB=EAL*DVAX*DFAC	RESP	143
4075	C	RESP	144
4076	C STRUCTURAL DAMPING LOAD	RESP	145
4077	C	RESP	146
4078	230 IF (DELTA.EQ.0.) GO TO 240	RESP	147
4079	SDMI=DELTA*DABS(BMTOT(1))*DSIGN(1.D0,DVR(1))	RESP	148
4080	SDMJ=DELTA*DABS(BMTOT(2))*DSIGN(1.D0,DVR(2))	RESP	149

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4081      SDFO=DELTA*DABS((FTOT(1)+FTOT(2))/2.D0)*DSIGN(1.D0,DVAX)      RESP 150
4082      BMIUB=BMIUB-SDMI+SDACT(1)      RESP 151
4083      BMJUB=BMJUB-SDMJ+SDACT(2)      RESP 152
4084      FOUB=FOUB-SDFO+SDACT(3)      RESP 153
4085      SDACT(1)=SDMI      RESP 154
4086      SDACT(2)=SDMJ      RESP 155
4087      SDACT(3)=SDFO      RESP 156
4088      C      RESP 157
4089      C      SET UP UNBALANCED LOAD VECTOR      RESP 158
4090      C      RESP 159
4091      240 IF (KBAL.EQ.0) GO TO 250      RESP 160
4092      SFUB=(BMIUB+BMJUB)/FL      RESP 161
4093      DD(1)=-SFUB*SINA-FOUB*COSA      RESP 162
4094      DD(2)=SFUB*COSA-FOUB*SINA      RESP 163
4095      DD(3)=BMIUB      RESP 164
4096      DD(4)=-DD(1)      RESP 165
4097      DD(5)=-DD(2)      RESP 166
4098      DD(6)=BMJUB      RESP 167
4099      IF (ECC(1).EQ.1.23456E10) GO TO 250      RESP 168
4100      DD(3)=DD(3)-DD(1)*ECC(3)+DD(2)*ECC(1)      RESP 169
4101      DD(6)=DD(6)-DD(4)*ECC(4)+DD(5)*ECC(2)      RESP 170
4102      C      RESP 171
4103      C      EXTRACT ENVELOPES      RESP 172
4104      C      RESP 173
4105      250 DO 270 I=1,8      RESP 174
4106      S=BMTOT(I)      RESP 175
4107      IF (S.LE.SENP(I)) GO TO 260      RESP 176
4108      SENP(I)=S      RESP 177
4109      TENP(I)=TIME      RESP 178
4110      260 IF (S.GE.SENN(I)) GO TO 270      RESP 179
4111      SENN(I)=S      RESP 180
4112      TENN(I)=TIME      RESP 181
4113      270 CONTINUE      RESP 182
4114      C      RESP 183
4115      C      PRINT TIME HISTORY      RESP 184
4116      C      RESP 185
4117      ISAVE=0      RESP 186
4118      IF (KPR.LT.0) GO TO 280      RESP 187
4119      IF (KPR.EQ.0.OR.KOUTDT.EQ.0) GO TO 350      RESP 188
4120      IF (ITHP.GT.1) GO TO 320      RESP 189
4121      280 IF (IHED.NE.0) GO TO 300      RESP 190
4122      KKPR=IABS(KPR)      RESP 191
4123      PRINT 290, KKPR, TIME      RESP 192
4124      290 FORMAT(///18H RESULTS FOR GROUP, I3,      RESP 193
4125      1      30H, BEAM COLUMN ELEMENTS, TIME =, F8.3///5X,      RESP 194
4126      2      5H ELEM, 4X, 4H NODE, 3X, 5HYIELD, 6X, 7HBENDING, 7X, 5HSHEAR,      RESP 195
4127      3      7X, 5HAXIAL, 12X, 23HPLASTIC HINGE ROTATIONS/5X,      RESP 196
4128      4      5H NO., 4X, 4H NO., 3X, 5H CODE, 6X, 7H MOMENT, 7X, 5HFORCE,      RESP 197
4129      5      7X, 5HFORCE, 8X, 7HCURRENT, 4X, 9HACC. POS., 3X, 9HACC. NEG./)      RESP 198
4130      IHED=1      RESP 199
4131      300 PRINT 310, IMEM, (NOD(I), KODY(I), BMTOT(I), SFTOT(I), FTOT(I), PRTOT(I)      RESP 200
4132      1, PRACP(I), PRACN(I), I=1, 2)      RESP 201
4133      310 FORMAT (I9, I8, I7, 3X, 3F12.2, 3X, 3F12.5/9X, I8, I7, 3X, 3F12.2, 3X, 3F12.5)      RESP 202
4134      C      RESP 203
4135      C      SET TIME HISTORY DATA IN /THIST/      RESP 204
4136      C      RESP 205
4137      320 IF (ITHP.LT.1.OR.KOUTDT.EQ.0) GO TO 350      RESP 206
4138      KKPR=IABS(KPR)      RESP 207
4139      ITHOUT(1)=KKPR      RESP 208
4140      ITHOUT(2)=2      RESP 209
4141      ITHOUT(3)=IMEM      RESP 210
4142      ITHOUT(4)=KODY(1)      RESP 211
4143      ITHOUT(5)=KODY(2)      RESP 212
4144      ITHOUT(6)=NODI      RESP 213
4145      ITHOUT(7)=NODJ      RESP 214
4146      DO 330 I=1, 8      RESP 215
4147      330 THOUT(I)=BMTOT(I)      RESP 216
4148      DO 340 I=1, 4      RESP 217
4149      340 THOUT(I+8)=PRACP(I)      RESP 218
4150      THOUT(13)=TIME      RESP 219
4151      ISAVE=1      RESP 220
4152      C      RESP 221
4153      C      SET INDICATOR FOR STIFFNESS CHANGE      RESP 222
4154      C      RESP 223
4155      350 KST=0      RESP 224
4156      IF(KT(1).NE.0.OR.KT(2).NE.0)KST=1      RESP 225
4157      C      RESP 226
4158      C      UPDATE INFORMATION IN COMS      RESP 227
4159      C      RESP 228
4160      COMS(2)=COM(2)      RESP 229

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4161      DO 360 J=36,215                                RESP 230
4162      360 COMS(J)=COM(J)                              RESP 231
4163      RETURN                                          RESP 232
4164      END                                             RESP 233
4165      SUBROUTINE STATE (DM,IE,KSTT,KBAL)              STATE 1
4166      IMPLICIT REAL*8(A-H,O-Z)                       STATE 2
4167      C                                               STATE 3
4168      C   FIND THE CORRESPONDING STATE OF A HYSTERETIC CURVE STATE 4
4169      C                                               STATE 5
4170      COMMON/INFEL/IMEM,IMEMD,KST,KSTD,LM(6),LMD(6),KGEOM,KGEOMD,PSH, STATE 6
4171      1  KHYST,KHYSTD,FL,COSA,SINA,EAL,A(2,6),ST(2,2),ECC(4), STATE 7
4172      2  KODY(2),XI(2),Q(2),ALPHAP(2,2),EI(5,2,2),PHF(2,2),PHY(2,2), STATE 8
4173      3  PHU(2,2),PHI(2,2),FM1(2,2),PH1(2,2),FMF(2,2),PHx(2,2),FMp(2,2), STATE 9
4174      4  PHp(2,2),PHr(2,2),RD3(2,2),RD4(2,2),RD5(2,2),FMxM(2,2), STATE 10
4175      5  PHxM(2,2),BMIY(2,2),BMEP(2),HYS(2),PPH1(2,2),BMMP,PHMX, STATE 11
4176      6  BMTOT(2),SFTOT(2),FTOT(2),PRTOT(2),SENP(8),SENN(8),TENP(8), STATE 12
4177      7  TENN(8),PRACP(2),PRACN(2),SDACT(3),NODI,NODJ,KOUTDT,KOUTDTD, STATE 13
4178      8  INLP(2,2),DAM(2),FMFI(2,2),FAC(2,2),FMDA(2,2),IDAM(2,2), STATE 14
4179      9  PHDA(2,2),FMxM(2,2)                          STATE 15
4180      COMMON/WORK/W1(840),DPR(2),NPW(2),FACTOR,INSLP(2,2),DUM(2),ISHT, STATE 16
4181      1KISHT,DFM,W(59),OMEGA                          STATE 17
4182      COMMON/PASS/IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM,IDUM(2), STATE 18
4183      1ISYM,ISYMD                                     STATE 19
4184      COMMON/DAMAGE/KDAMAGE,ITDAM,KIDAMT,NNSKIP,NSSKIP,NGSKIP,GLHYS, STATE 20
4185      1GLDAM                                          STATE 21
4186      COMMON/AUTO/KAUTO,KAUTOD,KECO,KECOD,NDSGN,NDSGND,KFREQ,KFREQD, STATE 22
4187      1 DBALL,DCALL,DBSTD,CONC,STEEL,IECO,BMAVG,BMDEV,ICONV STATE 23
4188      C                                               STATE 24
4189      ISHT=0                                           STATE 25
4190      ISLP=0                                           STATE 26
4191      NPW(IE)=0                                       STATE 27
4192      FACTOR=1.0                                       STATE 28
4193      DHYS=0.0                                         STATE 29
4194      ICIE=KODY(IE)                                    STATE 30
4195      IT=2                                             STATE 31
4196      IY=1                                             STATE 32
4197      IF(BMEP(IE).LT.0.)IY=2                          STATE 33
4198      IF(IY.EQ.2)IT=1                                  STATE 34
4199      DPHI=DM/EI(KODY(IE),IY,IE)                      STATE 35
4200      FMDM=BMEP(IE)+DM                               STATE 36
4201      C                                               STATE 37
4202      GO TO(100,200,300,400,500),ICIE                STATE 38
4203      100 IF(FMDM.LT.BMIY(IE,1).AND.FMDM.GT.BMIY(IE,2)) GO TO 710 STATE 39
4204      NPW(IE)=1                                       STATE 40
4205      KODY(IE)=2                                       STATE 41
4206      IY=1                                             STATE 42
4207      IT=2                                             STATE 43
4208      IF(FMDM.LT.0.)IY=2                              STATE 44
4209      IF(IY.EQ.2)IT=1                                  STATE 45
4210      CALL OVRSHI(IMEM,IE,KODY(IE),BMEP(IE),BMIY(IE,IY),FMDM,PHI(IE),DPH STATE 46
4211      1I,EI(1,IY,IE),EI(2,IY,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE 47
4212      GO TO 700                                       STATE 48
4213      C                                               STATE 49
4214      200 IF(BMEP(IE)*DPR(IE).LE.0.0) NPW(IE)=0      STATE 50
4215      IF(BMEP(IE))202,201,201                        STATE 51
4216      201 IF(DM)203,710,710                          STATE 52
4217      202 IF(DM)710,710,205                          STATE 53
4218      203 CALL SLOPE(IE)                              STATE 54
4219      IF(FMDM)230,211,211                            STATE 55
4220      205 CALL SLOPE(IE)                              STATE 56
4221      IF(FMDM)211,211,230                            STATE 57
4222      211 KODY(IE)=3                                  STATE 58
4223      DPHI=DM/EI(KODY(IE),IY,IE)                    STATE 59
4224      GO TO 700                                       STATE 60
4225      230 IY=1                                         STATE 61
4226      IT=2                                             STATE 62
4227      IF(BMEP(IE).LT.0.)IY=2                          STATE 63
4228      IF(IY.EQ.2)IT=1                                  STATE 64
4229      C                                               STATE 65
4230      250 IF(INSLP(IE,IY).EQ.1) GO TO 260            STATE 66
4231      KODY(IE)=4                                       STATE 67
4232      IDAM(IE,IY)=2                                    STATE 68
4233      CALL OVRSHI(IMEM,IE,KODY(IE),BMEP(IE),0.,FMDM,PHI(IE),DPHI,EI(3,IY STATE 69
4234      1,IE),EI(4,IT,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE 70
4235      IF(DABS(FMDM).LT.DABS(FMp(IE,IT))) GO TO 700   STATE 71
4236      KODY(IE)=5                                       STATE 72
4237      CALL OVRSHI(IMEM,IE,KODY(IE),0.,FMp(IE,IT),FMDM,PHI(IE),DPHI,EI(4, STATE 73
4238      1IT,IE),EI(5,IT,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE 74
4239      IF(DABS(FMDM).LT.DABS(FMxM(IE,IT))) GO TO 700 STATE 75
4240      KODY(IE)=2                                       STATE 76

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4241 CALL OVRSHI(IMEM,IE,KODY(IE),FMp(IE,IT),FMxxM(IE,IT),FMDM,PHI(IE),STATE 77
4242 1DPHI,EI(5,IT,IE),EI(2,IT,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE 78
4243 GO TO 700 STATE 79
4244 260 KODY(IE)=4 STATE 80
4245 IDAM(IE,IY)=2 STATE 81
4246 CALL OVRSHI(IMEM,IE,KODY(IE),BMEP(IE),O.,FMDM,PHI(IE),DPHI,EI(3,IYSTATE 82
4247 1,IE),EI(4,IT,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE 83
4248 IF(DABS(FMDM).LT.DABS(BMIY(IE,IT))) GO TO 700 STATE 84
4249 KODY(IE)=2 STATE 85
4250 CALL OVRSHI(IMEM,IE,KODY(IE),O.,BMIY(IE,IT),FMDM,PHI(IE),DPHI,EI(4STATE 86
4251 1,IT,IE),EI(2,IT,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE 87
4252 GO TO 700 STATE 88
4253 C STATE 89
4254 300 IF(BMEP(IE)*DPR(IE).LE.O.O) NPW(IE)=0 STATE 90
4255 IF(BMEP(IE))302,301,301 STATE 91
4256 301 IF(DM)303,710,304 STATE 92
4257 302 IF(DM)304,710,306 STATE 93
4258 303 IF(FMDM)320,710,710 STATE 94
4259 306 IF(FMDM)710,710,320 STATE 95
4260 320 IY=1 STATE 96
4261 IT=2 STATE 97
4262 IF(BMEP(IE).LT.O.)IY=2 STATE 98
4263 IF(IY.EQ.2)IT=1 STATE 99
4264 IF(BMEP(IE)*PHr(IE,IY).GE.O.) GO TO 250 STATE100
4265 KODY(IE)=5 STATE101
4266 IDAM(IE,IY)=2 STATE102
4267 CALL OVRSHI(IMEM,IE,KODY(IE),BMEP(IE),O.,FMDM,PHI(IE),DPHI,EI(3,IYSTATE103
4268 1,IE),EI(5,IT,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE104
4269 IF(DABS(FMDM).LT.DABS(FMxxM(IE,IT))) GO TO 700 STATE105
4270 KODY(IE)=2 STATE106
4271 CALL OVRSHI(IMEM,IE,KODY(IE),O.,FMxxM(IE,IT),FMDM,PHI(IE),DPHI,EI(STATE107
4272 15,IT,IE),EI(2,IT,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE108
4273 GO TO 700 STATE109
4274 304 IF(DABS(FMDM).LT.DABS(FM1(IE,IY))) GO TO 710 STATE110
4275 IF(BMEP(IE)*PHr(IE,IY).LT.O.) GO TO 360 STATE111
4276 FF=(PH1(IE,IY)-PHY(IE,IY))*EI(2,IY,IE)+BMIY(IE,IY) STATE112
4277 IF(DABS(FMDM).GE.DABS(FF)) GO TO 350 STATE113
4278 KODY(IE)=5 STATE114
4279 CALL OVRSHI(IMEM,IE,KODY(IE),BMEP(IE),FM1(IE,IY),FMDM,PHI(IE),DPHI STATE115
4280 1,EI(3,IY,IE),EI(5,IY,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE116
4281 IF(DABS(FMDM).LT.DABS(FMxxM(IE,IY))) GO TO 700 STATE117
4282 KODY(IE)=2 STATE118
4283 CALL OVRSHI(IMEM,IE,KODY(IE),FM1(IE,IY),FMxxM(IE,IY),FMDM,PHI(IE),STATE119
4284 1DPHI,EI(5,IY,IE),EI(2,IY,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE120
4285 GO TO 700 STATE121
4286 350 KODY(IE)=2 STATE122
4287 CALL OVRSHI(IMEM,IE,KODY(IE),BMEP(IE),FM1(IE,IY),FMDM,PHI(IE),DPHI STATE123
4288 1,EI(3,IY,IE),EI(2,IY,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE124
4289 GO TO 700 STATE125
4290 360 KODY(IE)=4 STATE126
4291 CALL OVRSHI(IMEM,IE,KODY(IE),BMEP(IE),FM1(IE,IY),FMDM,PHI(IE),DPHI STATE127
4292 1,EI(3,IY,IE),EI(4,IY,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE128
4293 IF(INSLP(IE,IT).EQ.1) GO TO 370 STATE129
4294 IF(DABS(FMDM).LE.DABS(FMp(IE,IY))) GO TO 700 STATE130
4295 KODY(IE)=5 STATE131
4296 CALL OVRSHI(IMEM,IE,KODY(IE),FM1(IE,IY),FMp(IE,IY),FMDM,PHI(IE),DPSTATE132
4297 1HI,EI(4,IY,IE),EI(5,IY,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE133
4298 IF(DABS(FMDM).LT.DABS(FMxxM(IE,IY))) GO TO 700 STATE134
4299 KODY(IE)=2 STATE135
4300 CALL OVRSHI(IMEM,IE,KODY(IE),FMp(IE,IY),FMxxM(IE,IY),FMDM,PHI(IE),STATE136
4301 1DPHI,EI(5,IY,IE),EI(2,IY,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE137
4302 GO TO 700 STATE138
4303 370 IF(DABS(FMDM).LE.DABS(BMIY(IE,IY))) GO TO 700 STATE139
4304 KODY(IE)=2 STATE140
4305 CALL OVRSHI(IMEM,IE,KODY(IE),FM1(IE,IY),BMIY(IE,IY),FMDM,PHI(IE),DSTATE141
4306 1PHI,EI(4,IY,IE),EI(2,IY,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE142
4307 GO TO 700 STATE143
4308 C STATE144
4309 400 IF(BMEP(IE)*DPR(IE).LE.O.O) NPW(IE)=0 STATE145
4310 IF(BMEP(IE))402,401,401 STATE146
4311 401 IF(DM)403,710,404 STATE147
4312 402 IF(DM)404,710,405 STATE148
4313 403 CALL SLOPE(IE) STATE149
4314 IF(FMDM)450,411,411 STATE150
4315 405 CALL SLOPE(IE) STATE151
4316 IF(FMDM)411,411,450 STATE152
4317 411 KODY(IE)=3 STATE153
4318 DPHI=DM/EI(KODY(IE),IY,IE) STATE154
4319 GO TO 700 STATE155
4320 450 IY=1 STATE156

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4321      IT=2                                STATE157
4322      IF(BMEP(IE).LT.0.)IY=2              STATE158
4323      IF(IY.EQ.2) IT=1                    STATE159
4324      KODY(IE)=5                          STATE160
4325      IDAM(IE,IY)=2                      STATE161
4326      CALL OVRSH(T(IMEM,IE,KODY(IE),BMEP(IE),0.,FMDM,PHI(IE),DPHI,EI(3,IYSTATE162
4327      1,IE),EI(5,IT,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE163
4328      IF(DABS(FMDM).LT.DABS(FMxxM(IE,IT))) GO TO 700 STATE164
4329      KODY(IE)=2                          STATE165
4330      CALL OVRSH(T(IMEM,IE,KODY(IE),0.,FMxxM(IE,IT),FMDM,PHI(IE),DPHI,EI(STATE166
4331      15,IT,IE),EI(2,IT,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE167
4332      GO TO 700                            STATE168
4333      404 IF(INSPL(IE,IT).EQ.1) GO TO 460  STATE169
4334      IF(DABS(FMDM).LT.DABS(FMp(IE,IY))) GO TO 710 STATE170
4335      KODY(IE)=5                          STATE171
4336      CALL OVRSH(T(IMEM,IE,KODY(IE),BMEP(IE),FMp(IE,IY),FMDM,PHI(IE),DPHI STATE172
4337      1,EI(4,IY,IE),EI(5,IY,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE173
4338      IF(DABS(FMDM).LT.DABS(FMxxM(IE,IY))) GO TO 700 STATE174
4339      KODY(IE)=2                          STATE175
4340      CALL OVRSH(T(IMEM,IE,KODY(IE),FMp(IE,IY),FMxxM(IE,IY),FMDM,PHI(IE), STATE176
4341      1DPHI,EI(5,IY,IE),EI(2,IY,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE177
4342      GO TO 700                            STATE178
4343      460 IF(DABS(FMDM).LT.DABS(BMIY(IE,IY))) GO TO 710 STATE179
4344      KODY(IE)=2                          STATE180
4345      CALL OVRSH(T(IMEM,IE,KODY(IE),BMEP(IE),BMIY(IE,IY),FMDM,PHI(IE),DPH STATE181
4346      1I,EI(4,IY,IE),EI(2,IY,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE182
4347      GO TO 700                            STATE183
4348      C                                     STATE184
4349      500 IF(BMEP(IE)*DPR(IE).LE.0.0) NPW(IE)=0 STATE185
4350      IF(BMEP(IE))502,501,501             STATE186
4351      501 IF(DM)503,710,505                STATE187
4352      502 IF(DM)505,710,504                STATE188
4353      503 CALL SLOPE(IE)                   STATE189
4354      IF(FMDM)530,511,511                 STATE190
4355      504 CALL SLOPE(IE)                   STATE191
4356      IF(FMDM)511,511,530                 STATE192
4357      511 KODY(IE)=3                       STATE193
4358      DPHI=DM/EI(KODY(IE),IY,IE)          STATE194
4359      GO TO 700                            STATE195
4360      530 IY=1                             STATE196
4361      IT=2                                 STATE197
4362      IF(BMEP(IE).LT.0.) IY=2             STATE198
4363      IF(IY.EQ.2)IT=1                     STATE199
4364      GO TO 250                            STATE200
4365      505 IY=1                             STATE201
4366      IT=2                                 STATE202
4367      IF(BMEP(IE).LT.0.) IY=2             STATE203
4368      IF(IY.EQ.2)IT=1                     STATE204
4369      IF(DABS(FMDM).LT.DABS(FMxxM(IE,IY))) GO TO 710 STATE205
4370      KODY(IE)=2                          STATE206
4371      CALL OVRSH(T(IMEM,IE,KODY(IE),BMEP(IE),FMxxM(IE,IY),FMDM,PHI(IE),DP STATE207
4372      1HI,EI(5,IY,IE),EI(2,IY,IE),DHYS,NODI,NODJ,KHYST,ICIE) STATE208
4373      GO TO 700                            STATE209
4374      C                                     STATE210
4375      700 KSTT=1                            STATE211
4376      710 PHI(IE)=PHI(IE)+DPHI            STATE212
4377      C                                     STATE213
4378      C COMPUTE THE ACCUMULATED DAMAGE INDEX STATE214
4379      C                                     STATE215
4380      IF(KDAMAGE.LT.1) GO TO 800           STATE216
4381      IY=1                                  STATE217
4382      IT=2                                  STATE218
4383      IF(BMEP(IE).LT.0.) IY=2             STATE219
4384      IF(IY.EQ.2) IT=1                     STATE220
4385      C ISHT= CHECK OVERSHOOTING, I.E. IF ISHT=1, PASSED SUBROUTINE STATE221
4386      C "OVRSH(T".                          STATE222
4387      IF(ISHT.NE.1) DHYS=DPHI*(FMDM+BMEP(IE))/2. STATE223
4388      IF(ISHT.EQ.1) DHYS=DHYS+DPHI*DFM/2. STATE224
4389      HYS(IE)=HYS(IE)+DHYS                STATE225
4390      C                                     STATE226
4391      IF(ISTEP.EQ.NSTEPS) GO TO 801         STATE227
4392      IF(DABS(FMDA(IE,IY)) .LE. DABS(FMcr)) GO TO 802 STATE228
4393      IF(IDAM(IE,IY).NE.2) GO TO 800       STATE229
4394      801 OMEGA=FAC(IE,IY)                  STATE230
4395      RPHI=PHDA(IE,IY)/PHF(IE,IY)          STATE231
4396      IF((IY.EQ.1).AND.(PHDA(IE,IY).LE.PHY(IE,IY))) GO TO 802 STATE232
4397      IF((IY.EQ.2).AND.(PHDA(IE,IY).GE.PHY(IE,IY))) GO TO 802 STATE233
4398      FMcr=EI(2,IY,IE)*PHDA(IE,IY)         STATE234
4399      FMFi(IE,IY)=FMF(IE,IY)*DSQRT(2.*RPHI/(RPHI+1.0)) STATE235
4400      AA=PHDA(IE,IY)-PHY(IE,IY)           STATE236

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4401      AF=PHF(IE,IY)-PHY(IE,IY)                                STATE237
4402      DDFM=(AF*EI(2,IY,IE)+BMIY(IE,IY)-FMF(IE,IY))*(AA/AF)**OMEGA STATE238
4403      FMI1=BMIY(IE,IY)+AA*EI(2,IY,IE)                        STATE239
4404      FNI=(FMI1-FMFI(IE,IY))/DDFM+1                          STATE240
4405      RATIO=(PHr(IE,IY)-PHr(IE,IT))/(2.*PHr(IE,IY))         STATE241
4406      IF(RATIO.EQ.0.0) GO TO 802                               STATE242
4407      DENOM=1.DO/(FMI1-(FNI-1)*DDFM/2.)*DABS(RATIO)         STATE243
4408      ALPHA=FMDA(IE,IY)*DENOM                                 STATE244
4409      DAM(IE)=DAM(IE)+ALPHA/DINT(FNI)                         STATE245
4410      802 IDAM(IE,IY)=0                                       STATE246
4411      C                                                         STATE247
4412      IF(DABS(PHDA(IE,IY)) .LE. DABS(PHF(IE,IY))) GO TO 800  STATE248
4413      PRINT 930, IGR, IMEM, IE, PHDA(IE,IY), PHF(IE,IY)     STATE249
4414      930 FORMAT(///'1CURVATURE EXCEEDED THE FAILURE CURVATURE'/ STATE250
4415      * 10X, 'GROUP', 5X, 'MEMBER', 5X, 'CURVATURE', 5X, 'FAIL CURVATURE', / STATE251
4416      * 10X, I3, 7X, I3, '/', I1, 6X, F9.5, 5X, F10.3)      STATE252
4417      C                                                         STATE253
4418      800 BMEP(IE)=FMDM                                       STATE254
4419      IY=1                                                       STATE255
4420      IF(BMEP(IE).LT.0.0)IY=2                                  STATE256
4421      GO TO(811,812,813,814,815),KODY(IE)                   STATE257
4422      811 Q(IE)=1.                                             STATE258
4423      GO TO 900                                                STATE259
4424      812 Q(IE)=EI(1,IY,IE)/EI(2,IY,IE)                       STATE260
4425      GO TO 900                                                STATE261
4426      813 Q(IE)=EI(1,IY,IE)/(EI(3,IY,IE)*RD3(IY,IE))        STATE262
4427      GO TO 900                                                STATE263
4428      814 Q(IE)=EI(1,IY,IE)/(EI(4,IY,IE)*RD4(IY,IE))        STATE264
4429      GO TO 900                                                STATE265
4430      815 Q(IE)=EI(1,IY,IE)/(EI(5,IY,IE)*RD5(IY,IE))        STATE266
4431      900 IF(NODI.NE.KHYST.AND.NODJ.NE.KHYST) GO TO 910     STATE267
4432      WRITE(16,1111)IMEM, ISTEP, ICIE, IE, KODY(IE), BMEP(IE), STATE268
4433      1PHI(IE), EI(KODY(IE), IY, IE)                          STATE269
4434      1111 FORMAT(5I5, 3E12.4)                                STATE270
4435      C                                                         STATE271
4436      910 RETURN                                               STATE272
4437      END                                                         STATE273
4438      SUBROUTINE OVRSHT (IMEM, IE, KODY, BMEP, BMY, FM, PHI, DPHI, EI1, EI2, DHYS, OVRSH 1
4439      1NODI, NODJ, KHYST, ICIE)                                OVRSH 2
4440      IMPLICIT REAL*8(A-H, O-Z)                                  OVRSH 3
4441      COMMON/DAMAGE/KDAMAGE, ITDAM, KIDAMT, NNSKIP, NSSKIP, NGSKIP, GLHYS, OVRSH 4
4442      1GLDAM                                                    OVRSH 5
4443      COMMON/AUTO/KAUTO, KAUTOD, KECO, KECOD, NDSGN, NDSGND, KFREQ, KFREQD, OVRSH 6
4444      1 DBALL, DCALL, DBSTD, CONC, STEEL, IECD, BMAVG, BMDEV, ICONV OVRSH 7
4445      COMMON/WORK/W1(840), DPR(2), NPW(2), FACTOR, DUM(4), ISHT, KISHT, DFM, OVRSH 8
4446      1W(60)                                                    OVRSH 9
4447      C                                                         OVRSH 10
4448      C CONSIDER OVERSHOOTING PROBLEMS                          OVRSH 11
4449      C                                                         OVRSH 12
4450      ISHT=1                                                    OVRSH 13
4451      DFM=FM-BMY                                                OVRSH 14
4452      DPHI=DFM/EI1                                              OVRSH 15
4453      DDPHI=(BMY-BMEP)/EI1                                     OVRSH 16
4454      PHI=PHI+DDPHI                                            OVRSH 17
4455      FM=BMY+DPHI*EI2                                          OVRSH 18
4456      IF(KDAMAGE.GE.1)DHYS=DHYS+(BMY+BMEP)*DDPHI/2.         OVRSH 19
4457      IF(NODI.EQ.KHYST.OR.NODJ.EQ.KHYST)WRITE(16,100)IMEM, ISTEP, ICIE, OVRSH 20
4458      1IE, KODY, BMY, PHI, EI2                                  OVRSH 21
4459      100 FORMAT(5I5, 3E12.4)                                   OVRSH 22
4460      RETURN                                                    OVRSH 23
4461      END                                                         OVRSH 24
4462      SUBROUTINE SLOPE(IE)                                       SLOPE 1
4463      IMPLICIT REAL*8(A-H, O-Z)                                   SLOPE 2
4464      C                                                         SLOPE 3
4465      C COMPUTE SLOPE OF HYSTERETIC CURVE AT EACH TIME STEP    SLOPE 4
4466      C                                                         SLOPE 5
4467      COMMON/INFEL/IMEM, IMEMD, KST, KSTD, LM(6), LMD(6), KGEOM, KGEOMD, PSH, SLOPE 6
4468      1 KHYST, KHYSTD, FL, COSA, SINA, EAL, A(2,6), ST(2,2), ECC(4), SLOPE 7
4469      2 KODY(2), XI(2), Q(2), ALPHAP(2,2), EI(5,2,2), PHF(2,2), PHY(2,2), SLOPE 8
4470      3 PHU(2,2), PHI(2), FM1(2,2), PH1(2,2), BMF(2,2), PH(2,2), FMP(2,2), SLOPE 9
4471      4 PHp(2,2), PHr(2,2), RD3(2,2), RD4(2,2), RD5(2,2), FMxM(2,2), SLOPE 10
4472      5 PHxM(2,2), BMIY(2,2), BMEP(2), HYS(2), PPH1(2,2), BMMP, PHMX, SLOPE 11
4473      6 BMTOT(2), SFTOT(2), FTOT(2), PRTOT(2), SENP(8), SENN(8), TENP(8), SLOPE 12
4474      7 TENN(8), PRACP(2), PRACN(2), SDACT(3), NOD1, NODJ, KOUTDT, KOUTDTD, SLOPE 13
4475      8 INLP(2,2), DAM(2), FMFI(2,2), FAC(2,2), FMDA(2,2), IDAM(2,2), SLOPE 14
4476      9 PHDA(2,2), FMxM(2,2)                                     SLOPE 15
4477      COMMON/WORK/W1(840), DPR(2), NPW(2), FACTOR, inslp(2,2), W2(63) SLOPE 16
4478      COMMON/PASS/IGR, ISTEP, NSTEPS, KVARY, NBLOK, KSTAT, KDDS, KM, DUM(1), SLOPE 17
4479      1ISYM, ISYMD                                              SLOPE 18
4480      COMMON/DAMAGE/KDAMAGE, ITDAM, KIDAMT, NNSKIP, NSSKIP, NGSKIP, GLHYS, SLOPE 19

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4481      1GLDAM                                SLOPE 20
4482      COMMON/AUTO/KAUTO,KAUTOD,KECO,KECOD,NDSGN,NDSGND,KFREQ,KFREQD, SLOPE 21
4483      1 DBALL,DCALL,DBSTD,CONC,STEEL,IECO,BMAVG,BMDEV,ICONV SLOPE 22
4484      C                                        SLOPE 23
4485      CC=.5                                    SLOPE 24
4486      CC1=1.-CC                               SLOPE 25
4487      IULT=0                                   SLOPE 26
4488      IF(BMEP(IE))20,10,10                   SLOPE 27
4489      10 I1=1                                  SLOPE 28
4490      I2=2                                    SLOPE 29
4491      GO TO 30                                SLOPE 30
4492      20 I1=2                                  SLOPE 31
4493      I2=1                                    SLOPE 32
4494      30 FM1(IE,I1)=BMEP(IE)                 SLOPE 33
4495      PH1(IE,I1)=PHI(IE)                    SLOPE 34
4496      FMx=FMxM(IE,I2)                       SLOPE 35
4497      PHx=PHxM(IE,I2)                      SLOPE 36
4498      C                                        SLOPE 37
4499      omega=fac(ie,i2)                       SLOPE 38
4500      Ps=EI(2,1,IE)/EI(1,1,IE)              SLOPE 39
4501      FMo=Ps/(1.-Ps)*(PH1(IE,I1)*EI(1,1,IE)-FM1(IE,I1)) SLOPE 40
4502      PHo=1./(1.-Ps)*(PH1(IE,I1)-FM1(IE,I1)/EI(1,1,IE)) SLOPE 41
4503      C                                        SLOPE 42
4504      IF(IDAM(IE,I1).EQ.1) GO TO 40          SLOPE 43
4505      IF(DABS(PHx).EQ.DABS(PHY(IE,I2))) GO TO 40 SLOPE 44
4506      IF(DABS(FMDA(IE,I2)).LE.DABS(PHDA(IE,I2)*EI(2,1,IE))) GO TO 40 SLOPE 45
4507      C                                        SLOPE 46
4508      AA=PHx-PHY(IE,I2)                      SLOPE 47
4509      AF=PHF(IE,I2)-PHY(IE,I2)              SLOPE 48
4510      DFM=(AF*EI(2,1,IE)+BMIY(IE,I2)-BMF(IE,I2))*(AA/AF)**omega SLOPE 49
4511      FMxM(IE,I2)=FMx-DFM                   SLOPE 50
4512      C                                        SLOPE 51
4513      STF=FMxM(IE,I2)/PHx                   SLOPE 52
4514      IF(STF.LE.EI(2,1,IE)) THEN            SLOPE 53
4515      FMx=PHx*EI(2,1,IE)*1.0005            SLOPE 54
4516      FMxM(IE,I2)=FMx                      SLOPE 55
4517      IULT=1                                 SLOPE 56
4518      ELSE                                    SLOPE 57
4519      EIp=(FMxM(IE,I2)-FMo)/(PHx-PHo)        SLOPE 58
4520      EII=1/(EIp-EI(2,1,IE))                SLOPE 59
4521      PHx=EII*(BMIY(IE,I2)-FMo-PHY(IE,I2)*EI(2,1,IE)+PHo*EIp) SLOPE 60
4522      FMx=EII*(BMIY(IE,I2)-FMo+(PHo-PHY(IE,I2))*EI(2,1,IE))+FMo SLOPE 61
4523      ENDIF                                  SLOPE 62
4524      C                                        SLOPE 63
4525      40 C1=(FMx-FMo)/(PHx-PHo)              SLOPE 64
4526      C                                        SLOPE 65
4527      IF(DABS(FMo).GE.DABS(FM1(IE,I1)).AND.KODY(IE).EQ.5) GO TO 45 SLOPE 66
4528      IF((BMEP(IE)*PHo.LT.0.).OR.(IULT.EQ.1)) THEN SLOPE 67
4529      45 EI(3,I1,IE)=EI(3,I1,IE)            SLOPE 68
4530      IF(EI(3,I1,IE).EQ.0.) EI(3,I1,IE)=EI(1,I1,IE) SLOPE 69
4531      PHr(IE,I1)=PH1(IE,I1)-(FM1(IE,I1)/EI(3,I1,IE)) SLOPE 70
4532      IF(PHr(IE,I1).EQ.0.0) PHr(IE,I1)=DSIGN(1.D0,PH1(IE,I1))*0.0005 SLOPE 71
4533      ELSE                                    SLOPE 72
4534      PHr(IE,I1)=PHo-FMo/C1                  SLOPE 73
4535      IF(DABS(PHr(IE,I1)).LE.DABS(PHr(IE,I2)).AND.(PHr(IE,I1)*PHr(IE,I2) SLOPE 74
4536      1).GT.0.0) PHr(IE,I1)=PHr(IE,I2)      SLOPE 75
4537      IF(PHr(IE,I1).EQ.0.0) PHr(IE,I1)=DSIGN(1.D0,PHo)*0.0005 SLOPE 76
4538      IF(FM1(IE,I1).EQ.0.0) THEN            SLOPE 77
4539      EI(3,I1,IE)=EI(3,I1,IE)              SLOPE 78
4540      PHr(IE,I1)=PH1(IE,I1)                 SLOPE 79
4541      IF(PHr(IE,I1).EQ.0.0) PHr(IE,I1)=DSIGN(1.D0,PH1(IE,I1))*0.0005 SLOPE 80
4542      ELSE                                    SLOPE 81
4543      EI(3,I1,IE)=FM1(IE,I1)/(PH1(IE,I1)-PHr(IE,I1)) SLOPE 82
4544      ENDIF                                  SLOPE 83
4545      ENDIF                                  SLOPE 84
4546      C                                        SLOPE 85
4547      RD3(I1,IE)=EI(1,I1,IE)/(CC1*EI(3,I1,IE)+CC*EI(1,I1,IE)) SLOPE 86
4548      IF(DABS(FMx).EQ.DABS(BMIY(IE,I2))) GO TO 60 SLOPE 87
4549      C                                        SLOPE 88
4550      IF(KODY(IE).EQ.4) GO TO 70            SLOPE 89
4551      C                                        SLOPE 90
4552      EEI=FMx/(PHx-PHr(IE,I1))              SLOPE 91
4553      PHIn=Phr(IE,i1)*EEI/(EEI-EI(1,I2,IE)) SLOPE 92
4554      FMn=EI(1,I2,IE)*PHIn                   SLOPE 93
4555      ALFA=ALPHAP(IE,I2)                    SLOPE 94
4556      FMp(IE,I2)=ALFA*FMn                   SLOPE 95
4557      PHp(IE,I2)=ALFA*PHIn                  SLOPE 96
4558      C                                        SLOPE 97
4559      EI(4,I2,IE)=FMp(IE,I2)/(PHp(IE,I2)-PHr(IE,I1)) SLOPE 98
4560      RD4(I2,IE)=EI(1,I2,IE)/(CC1*EI(4,I2,IE)+CC*EI(1,I2,IE)) SLOPE 99

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4561      EI(5, I2, IE)=(FMx-FMp(IE, I2))/(PHx-PHp(IE, I2))          SLOPE100
4562      RD5(I2, IE)=EI(1, I1, IE)/(CC1*EI(5, I2, IE)+CC*EI(1, I1, IE)) SLOPE101
4563      GO TO 80                                                    SLOPE102
4564      60 EI(4, I2, IE)=FMx/(PHx-PHr(IE, I1))                      SLOPE103
4565      RD4(I2, IE)=EI(1, I1, IE)/(CC1*EI(4, I2, IE)+CC*EI(1, I1, IE)) SLOPE104
4566      GO TO 80                                                    SLOPE105
4567      70 EI(5, I2, IE)=FMx/(PHx-PHr(IE, I1))                      SLOPE106
4568      RD5(I2, IE)=EI(1, I1, IE)/(CC1*EI(5, I2, IE)+CC*EI(1, I1, IE)) SLOPE107
4569      80 IF(DABS(PH1(IE, I1)).GE.DABS(PHxM(IE, I1)).AND.(PH1(IE, I1)*PHxM(IE, I1)).GT.0.0) THEN SLOPE108
4570      11)).GT.0.0) THEN                                          SLOPE109
4571      FMxM(IE, I1)=FM1(IE, I1)                                    SLOPE110
4572      PHxM(IE, I1)=PH1(IE, I1)                                    SLOPE111
4573      ELSE                                                         SLOPE112
4574      ENDIF                                                       SLOPE113
4575      IF(IDAM(IE, I1) .EQ. 0) FMxxM(IE, I2)=FMx                  SLOPE114
4576      FMdA(IE, I1)=FM1(IE, I1)                                    SLOPE115
4577      PHdA(IE, I1)=PH1(IE, I1)                                    SLOPE116
4578      IDAM(IE, I1)=1                                              SLOPE117
4579      IF(DABS(FMxM(IE, I1)).GT.DABS(BMIY(IE, I1))) INSLP(IE, I2)=0 SLOPE118
4580      RETURN                                                       SLOPE119
4581      END                                                           SLOPE120
4582      SUBROUTINE FNFQ (NEQ, ST, FM, M, W1, SHP1)                   FNFQ 1
4583      IMPLICIT REAL*8(A-H, O-Z)                                     FNFQ 2
4584      COMMON/WORK/A(2500)                                          FNFQ 3
4585      DIMENSION ST(1), FM(1), M(1), SHP1(1)                       FNFQ 4
4586      IWKAR=2500                                                  FNFQ 5
4587      MBAND=1                                                      FNFQ 6
4588      DO 10 I=2, NEQ                                              FNFQ 7
4589      NBAND=M(I)-M(I-1)                                           FNFQ 8
4590      IF(NBAND .GT. MBAND) MBAND=NBAND                            FNFQ 9
4591      10 CONTINUE                                                 FNFQ 10
4592      NBAND=NEQ*MBAND-IWKAR                                       FNFQ 11
4593      IF(NBAND.LE.0)GO TO 15                                       FNFQ 12
4594      PRINT 100, NBAND                                             FNFQ 13
4595      100 FORMAT(// ' ** EXECUTION TERMINATED IN SUBROUTINE FRFQ ***', /, FNFQ 14
4596      . ' WORK AREA EXECEDED BY ', I5)                             FNFQ 15
4597      STOP                                                         FNFQ 16
4598      15 NBAND=NBAND+IWKAR                                         FNFQ 17
4599      DO 20 I=1, NBAND                                            FNFQ 18
4600      20 A(I)=0.                                                  FNFQ 19
4601      A(1)=ST(1)                                                  FNFQ 20
4602      NEQ1=NEQ-1                                                  FNFQ 21
4603      DO 30 I=2, NEQ                                              FNFQ 22
4604      NN=M(I)-M(I-1)                                             FNFQ 23
4605      DO 30 J=1, NN                                               FNFQ 24
4606      JJ=M(I-1)+J                                                FNFQ 25
4607      KK=I+(NN-J)*NEQ1                                           FNFQ 26
4608      30 A(KK)=ST(JJ)                                             FNFQ 27
4609      NSTIF=12                                                    FNFQ 28
4610      NMASS=13                                                    FNFQ 29
4611      NT=14                                                       FNFQ 30
4612      NF=1                                                         FNFQ 31
4613      COFQ=100.                                                  FNFQ 32
4614      IFPR=0                                                       FNFQ 33
4615      SCALE=1.E-8                                                 FNFQ 34
4616      ANORM=0.                                                    FNFQ 35
4617      DO 40 I=1, NEQ                                             FNFQ 36
4618      40 ANORM=ANORM+ST(M(I))*SCALE                               FNFQ 37
4619      ANORM=ANORM/NEQ                                             FNFQ 38
4620      REWIND NSTIF                                               FNFQ 39
4621      REWIND NMASS                                               FNFQ 40
4622      WRITE(NSTIF)(A(I), I=1, NBAND)                              FNFQ 41
4623      WRITE(NMASS)(FM(I), I=1, NEQ)                               FNFQ 42
4624      CALL FREQS(NEQ, MBAND, NF, COFQ, IFPR, ANORM, NSTIF, NMASS, NT, A, IWKAR) FNFQ 43
4625      TPI=8.*ATAN(1.0)                                           FNFQ 44
4626      REWIND NT                                                  FNFQ 45
4627      READ(NT)W1                                                  FNFQ 46
4628      READ(NT)(SHP1(I), I=1, NEQ)                                 FNFQ 47
4629      W1=W1/TPI                                                  FNFQ 48
4630      DO 50 I=2, NEQ                                             FNFQ 49
4631      50 SHP1(I)=SHP1(I)/SHP1(1)                                  FNFQ 50
4632      SHP1(1)=1.                                                 FNFQ 51
4633      RETURN                                                       FNFQ 52
4634      END                                                           FNFQ 53
4635      SUBROUTINE FREQS(NEQ, MBAND, NF, COFQ, IFPR, ANORM, NSTIF, NMASS, NT, A, FREQS 1
4636      1IWKAR)                                                       FREQS 2
4637      IMPLICIT REAL*8(A-H, O-Z)                                     FREQS 3
4638      DIMENSION A(1)                                               FREQS 4
4639      TPI=8.*ATAN(1.0)                                           FREQS 5
4640      COFQ=COFQ*TPI                                               FREQS 6

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4641	COFQ=COFQ*COFQ	FREQS 7
4642	NIM=3	FREQS 8
4643	NVM=6	FREQS 9
4644	NC=NF+NIM	FREQS 10
4645	NCA=NEQ*MAX0(MBAND,NC)	FREQS 11
4646	N2=1+NCA	FREQS 12
4647	N3=N2+NEQ	FREQS 13
4648	N4=N3+NEQ	FREQS 14
4649	N5=N4+NEQ	FREQS 15
4650	N6=N5+NEQ	FREQS 16
4651	N7=N6+NEQ*NVM	FREQS 17
4652	N8=N7+NEQ*NVM	FREQS 18
4653	N9=N8+NC	FREQS 19
4654	N10=N9+NC	FREQS 20
4655	N11=N10+NC	FREQS 21
4656	N12=N11+NC	FREQS 22
4657	NNNN=N12+NC-IWKAR	FREQS 23
4658	IF(NNNN.LE.0)GO TO10	FREQS 24
4659	PRINT 150,NNNN	FREQS 25
4660	150 FORMAT(//' ** EXECUTION TERMINATED IN SUBROUTINE FREQS **',/,	FREQS 26
4661	.' WORK AREA EXCEEDED BY ',15)	FREQS 27
4662	STOP	FREQS 28
4663	10 CALL SECNTD(A(1),A(N2),A(N3),A(N4),A(N5),A(N6),A(N7),	FREQS 29
4664	.A(N8),A(N9),A(N10),A(N11),A(N12),NEQ,MBAND,NF,NC,IFPR,	FREQS 30
4665	.ANORM,COFQ,NSTIF,NMASS,NT)	FREQS 31
4666	RETURN	FREQS 32
4667	END	FREQS 33
4668	SUBROUTINE SECNTD (A,B,V,MAXA,W,VV,WW,ROOT,TIM,ERRVL,ERRVR,	SECNT 1
4669	1NITE,N,MA,NROOT,NC,IFPR,ANORM,COFQ,NSTIF,NMASS,NT)	SECNT 2
4670	IMPLICIT REAL*8(A-H,O-Z)	SECNT 3
4671	DIMENSION A(N,NC),B(N),V(1),W(1),VV(N,1),WW(N,1),ROOT(1),	SECNT 4
4672	1TIM(1),ERRVL(1),ERRVR(1),NITE(1),MAXA(1)	SECNT 5
4673	C	SECNT 6
4674	C THE FOLLOWING TOLERANCES ARE SET FOR THE IBM 370	SECNT 7
4675	ACTOL=1.0D-04	SECNT 8
4676	RCBTOL=1.D-05	SECNT 9
4677	RTOL=1.0D-10	SECNT 10
4678	RQTOL=1.0D-12	SECNT 11
4679	C	SECNT 12
4680	NTF=5	SECNT 13
4681	IITEM=10	SECNT 14
4682	NITEM=60	SECNT 15
4683	NVM=6	SECNT 16
4684	C	SECNT 17
4685	REWIND NT	SECNT 18
4686	REWIND NMASS	SECNT 19
4687	READ (NMASS) B	SECNT 20
4688	C	SECNT 21
4689	ETA=2.0	SECNT 22
4690	NOV=0	SECNT 23
4691	JR=1	SECNT 24
4692	NSK=0	SECNT 25
4693	NWA=N*MA	SECNT 26
4694	C	SECNT 27
4695	C CHECK FOR SINGLE DEGREE-OF-FREEDOM SYSTEM	SECNT 28
4696	C	SECNT 29
4697	IF (N.GT.1) GO TO 5	SECNT 30
4698	IF(B(1).GT.0.) GO TO 7	SECNT 31
4699	WRITE(6,3000)	SECNT 32
4700	STOP	SECNT 33
4701	7 REWIND NSTIF	SECNT 34
4702	READ(NSTIF) A(1,1)	SECNT 35
4703	ROOT(1)=A(1,1)/B(1)	SECNT 36
4704	NSCH=1	SECNT 37
4705	A(1,1)=1.0D0/DSQRT(B(1))	SECNT 38
4706	GO TO 950	SECNT 39
4707	C	SECNT 40
4708	C FIRST STARTING VALUE	SECNT 41
4709	C	SECNT 42
4710	5 CONTINUE	SECNT 43
4711	RA=0.0	SECNT 44
4712	RR=0.0	SECNT 45
4713	CALL BANDET (A,B,V,MAXA,N,NWA,RA,NSCH,DETA,ISCA,1,NSTIF)	SECNT 46
4714	FA=DETA	SECNT 47
4715	IA=ISCA	SECNT 48
4716	IR=ISCA	SECNT 49
4717	ISCR=ISCA	SECNT 50
4718	FR=FA	SECNT 51
4719	DETR=DETA	SECNT 52
4720	C	SECNT 53

4721	C	CHECK FOR ZERO EIGENVALUE(S)	SECNT 54
4722	C		SECNT 55
4723		IF (A(N,1) .GT. ANORM) GO TO 10	SECNT 56
4724		WRITE (6,3001)	SECNT 57
4725		STOP	SECNT 58
4726	C		SECNT 59
4727	C	INVERSE ITERATION FOR LOWER BOUND ON SMALLEST ROOT	SECNT 60
4728	C		SECNT 61
4729	10	IF (IFPR.EQ.1)	SECNT 62
4730		* WRITE(6,2000)	SECNT 63
4731		DO 100 I=1,N	SECNT 64
4732	100	W(I)=B(I)	SECNT 65
4733		RT=0.0	SECNT 66
4734		IITE=0	SECNT 67
4735		KK=2	SECNT 68
4736	110	IITE=IITE+1	SECNT 69
4737		DO 120 I=1,N	SECNT 70
4738	120	V(I)=W(I)	SECNT 71
4739		CALL BANDET (A,B,V,MAXA,N,NWA,RA,NSCH,DETA,ISCA,KK,NSTIF)	SECNT 72
4740		KK=2	SECNT 73
4741		RQT=0.0	SECNT 74
4742		DO 130 I=1,N	SECNT 75
4743	130	RQT=RQT+W(I)*V(I)	SECNT 76
4744		DO 180 I=1,N	SECNT 77
4745	180	W(I)=B(I)*V(I)	SECNT 78
4746		RQB=0.0	SECNT 79
4747		DO 140 I=1,N	SECNT 80
4748	140	RQB=RQB+W(I)*V(I)	SECNT 81
4749		RQ=RQT/RQB	SECNT 82
4750		IF (IFPR.EQ.1)	SECNT 83
4751		* WRITE (6,2001) RQ	SECNT 84
4752		BS=DSQRT(RQB)	SECNT 85
4753		TOL=DABS(RQ-RT)/RQ	SECNT 86
4754		IF (TOL.LT.RCBTOL) GO TO 150	SECNT 87
4755		DO 160 I=1,N	SECNT 88
4756	160	W(I)=W(I)/BS	SECNT 89
4757		RT=RQ	SECNT 90
4758		IF (IITE.LT.IITEM) GO TO 110	SECNT 91
4759	C		SECNT 92
4760	150	DO 170 I=1,N	SECNT 93
4761	170	V(I)=V(I)/BS	SECNT 94
4762		RB=RQ*(1.000-DMIN1(1.0D-1,1.0D2*TOL))	SECNT 95
4763		IS=0	SECNT 96
4764	230	CALL BANDET (A,B,V,MAXA,N,NWA,RB,NSCH,DETB,ISCB,1,NSTIF)	SECNT 97
4765		IF (IFPR.EQ.1)	SECNT 98
4766		* WRITE (6,2002) RB,NSCH	SECNT 99
4767		FB=DETB	SECNT100
4768		IB=ISCB	SECNT101
4769		IF (NSCH.EQ.0) GO TO 300	SECNT102
4770		IS=IS+1	SECNT103
4771		IF (IS.LE.NTF) GO TO 240	SECNT104
4772		WRITE (6,3002) NTF	SECNT105
4773		STOP	SECNT106
4774	240	RB=RB/(NSCH+1)	SECNT107
4775		GO TO 230	SECNT108
4776	C		SECNT109
4777	C	ITERATION FOR INDIVIDUAL ROOTS	SECNT110
4778	C		SECNT111
4779	300	IF (IFPR.EQ.1)	SECNT112
4780		* WRITE (6,2003)	SECNT113
4781		NITE(JR)=-1	SECNT114
4782		IF (IFPR.EQ.1)	SECNT115
4783		* WRITE (6,2004) JR,NITE(JR),RA,DETA,FA,ETA,ISCA	SECNT116
4784		NITE(JR)=0	SECNT117
4785		IF (IFPR.EQ.1)	SECNT118
4786		* WRITE (6,2004) JR,NITE(JR),RB,DETB,FB,ETA,ISCB	SECNT119
4787	C		SECNT120
4788	C	WE STOP WHEN WE HAVE THE REQUIRED NO OF ROOTS SMALLER THAN RC AND	SECNT121
4789	C	NOV=0	SECNT122
4790	C		SECNT123
4791	310	IF (NSCH.GE.NROOT) GO TO 900	SECNT124
4792		IF (RB.GT.COFQ) GO TO 900	SECNT125
4793	C		SECNT126
4794		DIF=FB-FA	SECNT127
4795		IDIF=IA-IB	SECNT128
4796		IF (DIF.NE.0.0) GO TO 320	SECNT129
4797		IF(IDIF.NE.0)GO TO 320	SECNT130
4798		WRITE (6,3003)	SECNT131
4799		GO TO 900	SECNT132
4800	320	DIF=FB-FA*10.**IDIF	SECNT133

4801		DEL=FB*(RB-RA)/DIF	SECNT134
4802		RC=RB-ETA*DEL	SECNT135
4803		TOL=RCBTOL*RC	SECNT136
4804		IF (DABS(RC-RB) .GT. TOL) GO TO 330	SECNT137
4805		IF (IFPR.EQ.1)	SECNT138
4806		* WRITE (6,2005)	SECNT139
4807		ROOT(JR)=RB	SECNT140
4808		GO TO 400	SECNT141
4809	C		SECNT142
4810	330	CALL BANDET (A,B,V,MAXA,N,NWA,RC,NSCH,DETC,ISCC,1,NSTIF)	SECNT143
4811		FC=DETC	SECNT144
4812		IC=ISCC	SECNT145
4813		NITE(JR)=NITE(JR)+1	SECNT146
4814		IF (JR.EQ.1) GO TO 340	SECNT147
4815		JJ=JR-1	SECNT148
4816		DO 350 K=1,JJ	SECNT149
4817		FC=FC/(RC-ROOT(K))	SECNT150
4818	350	CALL EXPO(FC,IC)	SECNT151
4819	340	IF (IFPR.EQ.1)	SECNT152
4820		* WRITE (6,2004) JR,NITE(JR),RC,DETC,FC,ETA,ISCC	SECNT153
4821	C		SECNT154
4822	C	IF WE HAVE MORE SIGN CHANGES THAN EIGENVALUES SMALLER THAN RC WE	SECNT155
4823	C	START INV. ITERATION	SECNT156
4824	C		SECNT157
4825		NES=0	SECNT158
4826		IF (JR.EQ.1) GO TO 380	SECNT159
4827		DO 360 I=1,JJ	SECNT160
4828	360	IF (ROOT(I).LT.RC) NES=NES+1	SECNT161
4829	380	NOV=NSCH-NES	SECNT162
4830		IF (NOV.EQ.0) GO TO 370	SECNT163
4831		IF (IFPR.EQ.1)	SECNT164
4832		* WRITE (6,2006) NOV	SECNT165
4833		ROOT(JR)=RC	SECNT166
4834		IF (NOV.GT.1) NSK=1	SECNT167
4835	C		SECNT168
4836		GO TO 400	SECNT169
4837	370	RR=RA	SECNT170
4838		FR=FA	SECNT171
4839		IR=IA	SECNT172
4840		DETR=DETA	SECNT173
4841		ISCR=ISCA	SECNT174
4842		RA=RB	SECNT175
4843		FA=FB	SECNT176
4844		IA=IB	SECNT177
4845		DETA=DETB	SECNT178
4846		ISCA=ISCB	SECNT179
4847		RB=RC	SECNT180
4848		FB=FC	SECNT181
4849		IB=IC	SECNT182
4850		DETB=DETC	SECNT183
4851		ISCB=ISCC	SECNT184
4852	C		SECNT185
4853	C	WE RESET ETA IF NECESSARY	SECNT186
4854	C		SECNT187
4855		TOL=RB*ACTOL	SECNT188
4856		IF (DABS(RA-RB) .LT. TOL) ETA=ETA*2.0D0	SECNT189
4857		IF (NITE(JR).LE.NITEM) GO TO 310	SECNT190
4858		WRITE (6,3004) JR,NITE(JR)	SECNT191
4859		GO TO 900	SECNT192
4860	C		SECNT193
4861	C	CHECK FOR STORAGE	SECNT194
4862	400	IF (JR.LE.NC) GO TO 405	SECNT195
4863		WRITE (6,3005)	SECNT196
4864		GO TO 900	SECNT197
4865	C		SECNT198
4866	405	NOR=JR-1	SECNT199
4867		IF (NOR.GT.NVM) NOR=NVM	SECNT200
4868		IF (IFPR.EQ.1)	SECNT201
4869		* WRITE (6,2007) NOR	SECNT202
4870		IF (JR.EQ.1) GO TO 410	SECNT203
4871		DO 420 I=1,N	SECNT204
4872	420	V(I)=1.0	SECNT205
4873		KK=2	SECNT206
4874	410	DO 430 I=1,N	SECNT207
4875	430	W(I)=B(I)*V(I)	SECNT208
4876		IS=0	SECNT209
4877		GO TO 510	SECNT210
4878	C		SECNT211
4879	C	INVERSE ITERN	SECNT212
4880	C		SECNT213

4881	440	NITE(JR)=NITE(JR)+1	SECNT214
4882		DO 450 I=1,N	SECNT215
4883	450	V(I)=W(I)	SECNT216
4884		CALL BANDET (A,B,V,MAXA,N,NWA,RC,NSCH,DETC,ISCC,KK,NSTIF)	SECNT217
4885		IF (IS.EQ.1) GO TO 460	SECNT218
4886		KK=2	SECNT219
4887		RQT=0.0	SECNT220
4888		DO 470 I=1,N	SECNT221
4889	470	RQT=RQT+W(I)*V(I)	SECNT222
4890		DO 475 I=1,N	SECNT223
4891	475	W(I)=B(I)*V(I)	SECNT224
4892		RQB=0.0	SECNT225
4893		DO 480 I=1,N	SECNT226
4894	480	RQB=RQB+W(I)*V(I)	SECNT227
4895		RQ=RQT/RQB	SECNT228
4896		RT=ROOT(JR)+RQ	SECNT229
4897		IF (IFPR.EQ.1)	SECNT230
4898		* WRITE (6,2004) JR,NITE(JR),RT,RQ	SECNT231
4899		TOL=RT*RQTOL	SECNT232
4900		IF (DABS(RT-RTA) .GT. TOL) GO TO 510	SECNT233
4901		IS=1	SECNT234
4902		GO TO 440	SECNT235
4903	C		SECNT236
4904	510	RTA=RT	SECNT237
4905		BS=DSQRT(RQB)	SECNT238
4906		DO 490 I=1,N	SECNT239
4907	490	W(I)=W(I)/BS	SECNT240
4908		IF (NOR.EQ.0) GO TO 550	SECNT241
4909		DO 520 K=1,NOR	SECNT242
4910		AL=0.0	SECNT243
4911		DO 530 I=1,N	SECNT244
4912	530	AL=AL+VW(I,K)*W(I)	SECNT245
4913		DO 540 I=1,N	SECNT246
4914	540	W(I)=W(I)-AL*W(I,K)	SECNT247
4915	520	CONTINUE	SECNT248
4916	C		SECNT249
4917	550	IF (NITE(JR).LE.NITEM) GO TO 440	SECNT250
4918		WRITE (6,3004) JR,NITE(JR)	SECNT251
4919		GO TO 900	SECNT252
4920	C		SECNT253
4921	460	RQT=0.0	SECNT254
4922		ERRT=RQB	SECNT255
4923		DO 570 I=1,N	SECNT256
4924	570	RQT=RQT+V(I)*W(I)	SECNT257
4925		DO 560 I=1,N	SECNT258
4926	560	W(I)=B(I)*V(I)	SECNT259
4927		RQB=0.0	SECNT260
4928		DO 580 I=1,N	SECNT261
4929	580	RQB=RQB+V(I)*W(I)	SECNT262
4930	C		SECNT263
4931	C	OBTAIN A RATHER LARGE ERROR BOUND	SECNT264
4932	C		SECNT265
4933		RQ=RQT/RQB	SECNT266
4934		ROOT(JR)=ROOT(JR)+RQ	SECNT267
4935		ERR=DSQRT(ERRT/RQB)	SECNT268
4936		ERRVL(JR)=ROOT(JR)-ERR	SECNT269
4937		ERRVR(JR)=ROOT(JR)+ERR	SECNT270
4938	C		SECNT271
4939		BS=DSQRT(RQB)	SECNT272
4940		DO 590 I=1,N	SECNT273
4941		W(I)=W(I)/BS	SECNT274
4942	590	V(I)=V(I)/BS	SECNT275
4943		JJ=JR	SECNT276
4944		IF (JJ.LE.NVM) GO TO 610	SECNT277
4945		WRITE (NT) (VV(J,1),J=1,N)	SECNT278
4946		DO 600 K=1,N	SECNT279
4947		DO 600 L=2,NVM	SECNT280
4948		WM(K,L-1)=WM(K,L)	SECNT281
4949	600	VV(K,L-1)=VV(K,L)	SECNT282
4950		JJ=NVM	SECNT283
4951	610	DO 620 K=1,N	SECNT284
4952		WM(K,JJ)=W(K)	SECNT285
4953	620	VV(K,JJ)=V(K)	SECNT286
4954	C		SECNT287
4955	C	DECIDE STRATEGY FOR ITERN TOWARDS NEXT ROOT	SECNT288
4956	C		SECNT289
4957		TOL=RTOL*ROOT(JR)	SECNT290
4958		IF (NOV.GT.0) GO TO 700	SECNT291
4959		IF (DABS(ROOT(JR)-RB) .GT. TOL) GO TO 710	SECNT292
4960		IF (RA.GT.0.0) GO TO 720	SECNT293

4961		RA=RB/2.	SECNT294
4962		CALL BANDET (A,B,V,MAXA,N,NWA,RA,NSCH,DETA,ISCA,1,NSTIF)	SECNT295
4963		FA=DETA	SECNT296
4964		IA=ISCA	SECNT297
4965	720	RB=RA	SECNT298
4966		FB=FA	SECNT299
4967		IB=IA	SECNT300
4968		DETB=DETA	SECNT301
4969		ISCB=ISCA	SECNT302
4970		RA=RR	SECNT303
4971		FA=FR	SECNT304
4972		IA=IR	SECNT305
4973		DETA=DETR	SECNT306
4974		ISCA=ISCR	SECNT307
4975		GO TO 710	SECNT308
4976	C		SECNT309
4977	700	IF (ROOT(JR).GT.RC) NSK=1	SECNT310
4978		IF (NSK.EQ.1) GO TO 730	SECNT311
4979		IF (DABS(RC-ROOT(JR)) .LT. TOL) GO TO 740	SECNT312
4980		IF (DABS(ROOT(JR)-RB) .LT. TOL) GO TO 750	SECNT313
4981		RA=RB	SECNT314
4982		FA=FB	SECNT315
4983		IA=IB	SECNT316
4984		DETA=DETB	SECNT317
4985		ISCA=ISCB	SECNT318
4986	750	RB=RC	SECNT319
4987		FB=FC	SECNT320
4988		IB=IC	SECNT321
4989		DETB=DETC	SECNT322
4990		ISCB=ISCA	SECNT323
4991		GO TO 710	SECNT324
4992	740	IF (DABS(ROOT(JR)-RB) .GT. TOL) GO TO 710	SECNT325
4993		IF (RA.GT.0.0) GO TO 760	SECNT326
4994		RA=RB/2.	SECNT327
4995		CALL BANDET (A,B,V,MAXA,N,NWA,RA,NSCH,DETA,ISCA,1,NSTIF)	SECNT328
4996		FA=DETA	SECNT329
4997		IA=ISCA	SECNT330
4998	760	RB=RA	SECNT331
4999		FB=FA	SECNT332
5000		IB=IA	SECNT333
5001		DETB=DETA	SECNT334
5002		ISCB=ISCA	SECNT335
5003		RA=RR	SECNT336
5004		FA=FR	SECNT337
5005		IA=IR	SECNT338
5006		DETA=DETR	SECNT339
5007		ISCA=ISCR	SECNT340
5008	710	FA=FA/(RA-ROOT(JR))	SECNT341
5009		CALL EXPO(FA,IA)	SECNT342
5010		FB=FB/(RB-ROOT(JR))	SECNT343
5011		CALL EXPO(FB,IB)	SECNT344
5012		JR=JR+1	SECNT345
5013	C		SECNT346
5014		ETA=2.0	SECNT347
5015		GO TO 300	SECNT348
5016	C		SECNT349
5017	730	IF (RA.GT.0.0) GO TO 780	SECNT350
5018		RA=RB/2.	SECNT351
5019		CALL BANDET (A,B,V,MAXA,N,NWA,RA,NSCH,DETA,ISCA,1,NSTIF)	SECNT352
5020		FA=DETA	SECNT353
5021		IA=ISCA	SECNT354
5022	780	IF (DABS(ROOT(JR)-RB).GT.TOL) GO TO 770	SECNT355
5023		RB=RA	SECNT356
5024		FB=FA	SECNT357
5025		IB=IA	SECNT358
5026		DETB=DETA	SECNT359
5027		ISCB=ISCA	SECNT360
5028		RA=RR	SECNT361
5029		FA=FR	SECNT362
5030		IA=IR	SECNT363
5031		DETA=DETR	SECNT364
5032		ISCA=ISCR	SECNT365
5033	770	FA=FA/(RA-ROOT(JR))	SECNT366
5034		CALL EXPO(FA,IA)	SECNT367
5035		FB=FB/(RB-ROOT(JR))	SECNT368
5036		CALL EXPO(FB,IB)	SECNT369
5037		FR=FR/(RR-ROOT(JR))	SECNT370
5038		CALL EXPO(FR,IR)	SECNT371
5039		IF (ROOT(JR).LE.RC)NOV=NOV-1	SECNT372
5040		JR=JR+1	SECNT373

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5041      NITE(JR)=0
5042      ROOT(JR)=RC
5043      IF (NOV.GT.0) GO TO 400
5044      NSK=0
5045      ETA=2.0
5046      GO TO 300
5047      C
5048      900  NROOT=JR-1
5049      IF(NROOT.GT.0) GO TO 902
5050      WRITE (6,3006)
5051      STOP
5052      902  CONTINUE
5053      IF (IFPR.EQ.0) GO TO 905
5054      WRITE (6,2009) (NITE(J),J=1,NROOT)
5055      WRITE (6,2010) (TIM(J),J=1,NROOT)
5056      WRITE (6,2011)
5057      WRITE (6,2001) (ERRVL(J),J=1,NROOT)
5058      WRITE (6,2001) (ERRVR(J),J=1,NROOT)
5059      C
5060      C  READ EIGENVECTORS INTO CORE
5061      C
5062      905  IF (NROOT.LE.NVM) GO TO 906
5063      NDIF=NROOT - NVM
5064      REWIND NT
5065      DO 904 L=1,NDIF
5066      READ (NT) (A(I,L),I=1,N)
5067      904  CONTINUE
5068      GO TO 908
5069      906  NDIF=0
5070      908  NROOT=NROOT - NDIF
5071      DO 912 L=1,NROOT
5072      DO 912 I=1,N
5073      912  A(I,L+NDIF)=VV(I,L)
5074      C
5075      C  ARRANGE EIGENVALUES AND VECTORS IN ASCENDING ORDER
5076      C
5077      IF (JR.EQ.2) GO TO 950
5078      JR=JR-2
5079      910  IS=0
5080      DO 920 I=1,JR
5081      IF (ROOT(I+1).GE.ROOT(I)) GO TO 920
5082      IS=IS+1
5083      RT=ROOT(I+1)
5084      ROOT(I+1)=ROOT(I)
5085      ROOT(I)=RT
5086      C
5087      C  FORMAT
5088      C
5089      2000 FORMAT('1INVERSE ITERATION GIVES FOLLOWING APPROXIMATION TO',
5090      1' LOWEST EIGENVALUE')
5091      2001 FORMAT(1H0,6E20.12)
5092      2002 FORMAT('ORB =',E20.12,' NSC=',I4)
5093      2003 FORMAT('////5X,'ROOT',18X,'RC',15X,'DER?????')
5094      2004 FORMAT(1H0,2(4X,I4),8X,3E22.14,F7.2,2I6)
5095      2005 FORMAT('OR(RC-RB) IS SMALLER THAN TOL')
5096      2006 FORMAT('OWE JUMPED OVER ',I4,'UNKNOWN ROOT(S)')
5097      2007 FORMAT('1',34X,'ROOT',18X,'RQ',18X,'NOR=',I2)
5098      2008 FORMAT('OTIME FOR INVERSE ITERATION =',F5.2)
5099      2009 FORMAT('ONUMBER OF ITERATIONS FOR EACH EIGENVALUE',(/1X,6I20))
5100      2010 FORMAT('////OTIME USED FOR EACH EIGENVALUE',(/1X,6F20.2))
5101      2011 FORMAT('FOLLOWING ARE ERROR BOUNDS ON EIGENVALUES')
5102      2012 FORMAT('////OWE SOLVED FOR THE FOLLOWING EIGENVALUES')
5103      C
5104      3000 FORMAT('////*** FATAL ERROR IN SECNTD',/,
5105      1' ZERO MASS FOR SDOF SYSTEM')
5106      3001 FORMAT('//// *** FATAL ERROR IN SECNTD',/,
5107      1' RIGID BODY MODE FOUND')
5108      3002 FORMAT('//// FATAL ERROR IN SECNTD',/,1X,I3,
5109      1' FACTORIZATION PERFORMED TO FIND LOWER BOUND ON FIRST EIG.')
5110      3003 FORMAT(' DEFLATED POLYNOMIAL HAS NO MORE ROOTS')
5111      3004 FORMAT('//' PREMATURE EXIT FROM SECNTD',/,
5112      1' ITERATION FOR ROOT NO.',I4,' ABANDONED AFTER',I4,
5113      2' ITERATIONS')
5114      3005 FORMAT('//' PREMATURE EXIT FROM SECNTD')
5115      3006 FORMAT('////' FATAL ERROR IN SECNTD',/, ' NO EIGN. COMPUTED')
5116      DO 930 K=1,N
5117      RT=A(K,I+1)
5118      A(K,I+1)=A(K,I)
5119      930  A(K,I)=RT
5120      920  CONTINUE

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5121		IF (IS.GT.0) GO TO 910	SECNT454
5122	C		SECNT455
5123	950	CONTINUE	SECNT456
5124		NROOT=NSCH	SECNT457
5125		IF(IFPR.EQ.1)WRITE(6,2012)	SECNT458
5126		IF(IFPR.EQ.1)WRITE(6,2001)(ROOT(J),J=1,NROOT)	SECNT459
5127	C		SECNT460
5128		REWIND NT	SECNT461
5129		DO 970 I=1,NROOT	SECNT462
5130	970	ROOT(I)=DSQRT(ROOT(I))	SECNT463
5131		WRITE (NT) (ROOT(I),I=1,NROOT)	SECNT464
5132		NWA=N*NROOT	SECNT465
5133		WRITE (NT) (A(I,1),I=1,NWA)	SECNT466
5134		RETURN	SECNT467
5135		END	SECNT468
5136		SUBROUTINE BANDET (A,B,V,MAXA,NN,NWA,RA,NSCH,DET,ISCALE,KK,NSTIF)	BANDE 1
5137	C	ROUTINE TO PERFORM TRIANGULAR FACTORIZATION AT A SHIFT, DETERMINAT	BANDE 2
5138	C	CALCULATION AND VECTOR ITERATION	BANDE 3
5139		IMPLICIT REAL*8(A-H,O-Z)	BANDE 4
5140		DIMENSION A(NWA),B(1),V(1),MAXA(1)	BANDE 5
5141	C		BANDE 6
5142		NR=NN-1	BANDE 7
5143		IF (KK-2) 100,700,800	BANDE 8
5144	C		BANDE 9
5145	100	TOL=1.0E+10	BANDE 10
5146		RTOL=1.0E-07	BANDE 11
5147		NTF=3	BANDE 12
5148		IS=1	BANDE 13
5149	120	REWIND NSTIF	BANDE 14
5150		READ (NSTIF) A	BANDE 15
5151		DO 140 I=1,NN	BANDE 16
5152	140	A(I)=A(I)-RA*B(I)	BANDE 17
5153		IF(NWA.EQ.NN)GO TO 230	BANDE 18
5154		DO 200 N=1,NR	BANDE 19
5155		IH=N+NWA-NN	BANDE 20
5156	210	IF (A(IH)) 220,215,220	BANDE 21
5157	215	IH=IH-NN	BANDE 22
5158		GO TO 210	BANDE 23
5159	220	MAXA(N)=IH	BANDE 24
5160		PIV=A(N)	BANDE 25
5161		IF(PIV) 221,222,221	BANDE 26
5162	222	IS = IS+1	BANDE 27
5163		IF(IS.GT.NTF) GO TO 1000	BANDE 28
5164		RT=RTOL*10**(IS-2)	BANDE 29
5165		RA=RA*(1.0D0-RT)	BANDE 30
5166		GO TO 120	BANDE 31
5167	221	IL=N+NN	BANDE 32
5168		L=N	BANDE 33
5169		DO 240 I=IL,IH,NN	BANDE 34
5170		L=L+1	BANDE 35
5171		C=A(I)	BANDE 36
5172		IF (C) 225,240,225	BANDE 37
5173	225	C=C/PIV	BANDE 38
5174		IF (DABS(C) .LT. TOL) GO TO 235	BANDE 39
5175		GO TO 222	BANDE 40
5176	235	J=L-I	BANDE 41
5177		DO 260 K=I,IH,NN	BANDE 42
5178	260	A(K+J)=A(K+J)-C*A(K)	BANDE 43
5179		A(I)=C	BANDE 44
5180	240	CONTINUE	BANDE 45
5181	200	CONTINUE	BANDE 46
5182	230	IF (A(NN).NE.0.0) GO TO 280	BANDE 47
5183		AA=DABS(A(1))	BANDE 48
5184		DO 290 I=2,NR	BANDE 49
5185	290	AA=AA+DABS(A(I))	BANDE 50
5186		A(NN)=- (AA/NR)*1.0E-16	BANDE 51
5187	C		BANDE 52
5188	C	COMPUTE CHARACTERISTIC POLYNOMIAL	BANDE 53
5189	C	DET(A-RA*B)=DET*10**ISCALE	BANDE 54
5190	C		BANDE 55
5191	280	NSCH=0	BANDE 56
5192		DET=1.0	BANDE 57
5193		ISCALE=0	BANDE 58
5194		DO 300 I=1,NN	BANDE 59
5195	320	DET=DET*A(I)	BANDE 60
5196		CALL EXPO(DET,ISCALE)	BANDE 61
5197	300	IF (A(I).LT.0.) NSCH=NSCH+1	BANDE 62
5198		GO TO 900	BANDE 63
5199	C		BANDE 64
5200	700	IL=NN	BANDE 65

5201	DO 400 N=1,NR	BANDE 66
5202	C=V(N)	BANDE 67
5203	V(N)=C/A(N)	BANDE 68
5204	IF (NWA-NN) 410,400,410	BANDE 69
5205	410 IL=IL+1	BANDE 70
5206	IH=MAXA(N)	BANDE 71
5207	K=N	BANDE 72
5208	DO 420 I=IL,IH,NN	BANDE 73
5209	K=K+1	BANDE 74
5210	420 V(K)=V(K)-C*A(I)	BANDE 75
5211	400 CONTINUE	BANDE 76
5212	V(NN)=V(NN)/A(NN)	BANDE 77
5213	C	BANDE 78
5214	800 IF (NWA-NN) 430,900,430	BANDE 79
5215	430 N=NN	BANDE 80
5216	DO 440 L=2,NN	BANDE 81
5217	N=N-1	BANDE 82
5218	IL=N+NN	BANDE 83
5219	IH=MAXA(N)	BANDE 84
5220	K=N	BANDE 85
5221	DO 460 I=IL,IH,NN	BANDE 86
5222	K=K+1	BANDE 87
5223	460 V(N)=V(N)-A(I)*V(K)	BANDE 88
5224	440 CONTINUE	BANDE 89
5225	900 RETURN	BANDE 90
5226	C	BANDE 91
5227	1000 WRITE(6,1001)NTF,RA	BANDE 92
5228	1001 FORMAT (37H0***ERROR SOLUTION STOP IN *BANDET*, / 12X,	BANDE 93
5229	1 1H(,I3,37H) TRIANGULAR FACTORIZATIONS ATTEMPTED, / 12X,	BANDE 94
5230	2 16HCURRENT SHIFT = ,E20.14 / 1X)	BANDE 95
5231	STOP	BANDE 96
5232	END	BANDE 97
5233	SUBROUTINE EXPO (A,IX)	EXPO 1
5234	IMPLICIT REAL*8(A-H,O-Z)	EXPO 2
5235	XM=DABS(A)	EXPO 3
5236	10 IF(XM.LE.1.0D0)GO TO 20	EXPO 4
5237	XM=XM*0.1D0	EXPO 5
5238	IX=IX+1	EXPO 6
5239	GO TO 10	EXPO 7
5240	20 IF(XM.GE.0.1D0)GO TO 30	EXPO 8
5241	XM=XM*10.D0	EXPO 9
5242	IX=IX-1	EXPO 10
5243	GO TO 20	EXPO 11
5244	30 A=DSIGN(XM,A)	EXPO 12
5245	RETURN	EXPO 13
5246	END	EXPO 14
5247	SUBROUTINE PRTFQ(WE,S,NJTS,NEQ,ID)	PRTFQ 1
5248	IMPLICIT REAL*8(A-H,O-Z)	PRTFQ 2
5249	DIMENSION S(1),ID(NJTS,1),W(3)	PRTFQ 3
5250	PRINT 10,WE	PRTFQ 4
5251	10 FORMAT('OFIRST NATUARAL FREQUENCY = ',E14.5, '//,	PRTFQ 5
5252	1' FIRST MODE SHAPE:',/,	PRTFQ 6
5253	2' NODE X Y R')	PRTFQ 7
5254	DO 30 I=1,NJTS	PRTFQ 8
5255	DO 20 J=1,3	PRTFQ 9
5256	20 W(J)=S(ID(I,J))	PRTFQ 10
5257	PRINT 40,I,W	PRTFQ 11
5258	30 CONTINUE	PRTFQ 12
5259	40 FORMAT(18,3E20.4)	PRTFQ 13
5260	RETURN	PRTFQ 14
5261	END	PRTFQ 15
5262	SUBROUTINE REINT(IEAR,IDSGN,NELG,NELN,ICOR,DEDIF,PDEDIF,IICLK,DA,SREINT 1	
5263	1ECIN,STIN,CONIN,YBM,RHOM,DDIN,ITY,DMY)	REINT 2
5264	IMPLICIT REAL*8(A-H,O-Z)	REINT 3
5265	C	REINT 4
5266	C REINITIALIZE ALL THE ELEMENT DATA FOR SUBSEQUENT INPUT MOTIONS	REINT 5
5267	C	REINT 6
5268	COMMON/CONTR/ NELGR,NEQ,MBAND,NPTH,NPTV,NSTO,JCOL,NSTORY,NBAY	REINT 7
5269	COMMON/GENINF/KCONT(10),KELEM(10),NELEM(10),NINF(10),NDOF(10)	REINT 8
5270	1 ,FCONT(3),NUMEM(10)	REINT 9
5271	COMMON/PASS/ IGR,ISTEP,NSTEPS,KVARY,NBLOK,KSTAT,KDDS,KM,IDUM(2),	REINT 10
5272	1 ISYM,ISYMD	REINT 11
5273	COMMON/STOR/ NAVST,NF1,NF2,NF3,NF4,NTST,KODST,KDATA,NF17	REINT 12
5274	COMMON/INFEL/IMEM,IMEMD,KST,KSTD,LM(6),LMD(6),KGEOM,KGEOMD,PSH,	REINT 13
5275	1 KHYST,KHYSTD,FL,COSA,SINA,EAL,A(2,6),ST(2,2),ECC(4),	REINT 14
5276	2 KODY(2),XI(2),Q(2),ALPHAP(2,2),EI(5,2,2),PHF(2,2),PHY(2,2),	REINT 15
5277	3 PHU(2,2),PHI(2),FM1(2,2),PH1(2,2),FMF(2,2),PHX(2,2),FMp(2,2),	REINT 16
5278	4 PHp(2,2),PHr(2,2),RD3(2,2),RD4(2,2),RD5(2,2),FMxM(2,2),	REINT 17
5279	5 PHxM(2,2),BMIY(2,2),BMEP(2),HYS(2),PPH1(2,2),BMMP,PHMX,	REINT 18
5280	6 BMTOT(2),SFTOT(2),FTOT(2),PRTOT(2),SENP(8),SENN(8),TENP(8),	REINT 19


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5281      7   TENN(8),PRACP(2),PRACN(2),SDACT(3),NODI,NODJ,KOUTDT,KOUTDTD, REINT 20
5282      8   INSLP(2,2),DAM(2),FMFI(2,2),FAC(2,2),FMDA(2,2),IDAM(2,2), REINT 21
5283      9   PHDA(2,2),FMxxM(2,2) REINT 22
5284      C REINT 23
5285      DIMENSION ICOR(1),DEDIF(1),PDEDIF(1),IICLK(1),DA(1),SECIN(9,1), REINT 24
5286      1STIN(6,1),CONIN(9,1),YBM(2,NELG,1),RHOM(2,NELG,1),DDIN(2,1), REINT 25
5287      2ITY(3,1),DMY(NELG,1) REINT 26
5288      C REINT 27
5289      DIMENSION COM(1) REINT 28
5290      EQUIVALENCE(IMEM,COM(1)) REINT 29
5291      C REINT 30
5292      NUM=0 REINT 31
5293      DO 10 IGR=1,NELGR REINT 32
5294      NEL=NELEM(IGR) REINT 33
5295      NDATA=NINF(IGR) REINT 34
5296      DO 20 IEL=1,NEL REINT 35
5297      CALL STORE(COM,NDATA,NF17,1) REINT 36
5298      IF(IEAR.EQ.1 .AND. IDSGN.GE.1)CALL REINELL(IEL,NUM,IDSGN,IGR,NELG,REINT 37
5299      1NELN,ICOR,DEDIF,PDEDIF,IICLK,DA,SECIN,STIN,CONIN,YBM,RHOM,DDIN,ITYREINT 38
5300      2,DMY) REINT 39
5301      CALL STORE(COM,NDATA,NF2,2) REINT 40
5302      20 CONTINUE REINT 41
5303      IF(IGR.GE.1) NUM=NUM+NEL REINT 42
5304      10 CONTINUE REINT 43
5305      REWIND NF17 REINT 44
5306      REWIND NF2 REINT 45
5307      DO 30 IGR=1,NELGR REINT 46
5308      NEL=NELEM(IGR) REINT 47
5309      NDATA=NINF(IGR) REINT 48
5310      DO 30 IEL=1,NEL REINT 49
5311      CALL STORE(COM,NDATA,NF2,1) REINT 50
5312      CALL STORE(COM,NDATA,NF17,2) REINT 51
5313      30 CONTINUE REINT 52
5314      REWIND NF17 REINT 53
5315      REWIND NF2 REINT 54
5316      RETURN REINT 55
5317      END REINT 56
5318      SUBROUTINE REINELL (IEL,NUM,IDSGN,IGR,NELG,NELN,ICOR,DEDIF,PDEDIF,REINE 1
5319      1IICLK,DA,SECIN,STIN,CONIN,YBM,RHOM,DDIN,ITY,DMY) REINE 2
5320      IMPLICIT REAL*8(A-H,O-Z) REINE 3
5321      C REINE 4
5322      COMMON/GENINF/IIDUM(30),NINF(10),NDOFF(10),JJDUM(6),NUMEM(10) REINE 5
5323      COMMON/INFEL/IMEM,IMEMD,KST,KSTD,LM(6),LMD(6),KGEOM,KGEOMD,PSH, REINE 6
5324      1 KHYST,KHYSTD,FL,COSA,SINA,EAL,A(2,6),ST(2,2),ECC(4), REINE 7
5325      2 KODY(2),XI(2),Q(2),ALPHAP(2,2),EI(5,2,2),PHF(2,2),PHY(2,2), REINE 8
5326      3 PHU(2,2),PHI(2),FM1(2,2),PH1(2,2),FMF(2,2),PHx(2,2),Fmp(2,2), REINE 9
5327      4 PHp(2,2),PHr(2,2),RD3(2,2),RD4(2,2),RD5(2,2),FMxm(2,2), REINE 10
5328      5 PHxM(2,2),BMY(2,2),BMEP(2),HYS(2),PPH1(2,2),BMMP,PHMX, REINE 11
5329      6 BMTOT(2),SFTOT(2),FTOT(2),PRTOT(2),SENP(8),SENN(8),TENP(8), REINE 12
5330      7 TENN(8),PRACP(2),PRACN(2),SDACT(3),NODI,NODJ,KOUTDT,KOUTDTD, REINE 13
5331      8 INSLP(2,2),DAM(2),FMFI(2,2),FAC(2,2),FMDA(2,2),IDAM(2,2), REINE 14
5332      9 PHDA(2,2),FMxxM(2,2) REINE 15
5333      COMMON/WORK/GA(6,6),SFF(8),SSFF(8),DD(6),FFEF(6),FF(6), REINE 16
5334      1 FEF(35,7),KDFEF(36),FINIT(30,6),ECT(15,4),STYP(7,6), REINE 17
5335      2 CONYP(7,9),SECYP(14,9),W1(6), REINE 18
5336      3 ES,PS,FSY,EPSSY,EPSSU,FSU,FC,RDD,EC,PC,FCY,EPSCY,EPSCU,FCU, REINE 19
5337      4 EPSCM,PCP,F,FN,FN1,PS1,PC1,PH,FM,EPSS,EPSC,EPSSD,YY,PSP,W2(2), REINE 20
5338      5 DPR(2),NPW(2),FACTOR,FMY(2),PY(2),PHUL(2),PHIF(2),FMU(2), REINE 21
5339      6 FMIF(2),W3(744) REINE 22
5340      COMMON/AUTO/KAUTO,KAUTOD,KECO,KECOD,NDSGN,NDSGND,KFREQ,KFREQD, REINE 23
5341      1 DBALL,DCALL,DBSTD,CONC,STEEL,IECO,BMAVG,BMDEV REINE 24
5342      COMMON/THIST/ITHOUT(10),THOUT(20),ITHP,ISAVE,NELTH,NSTH,NF7,ISE REINE 25
5343      C REINE 26
5344      DIMENSION ICOR(1),DEDIF(1),PDEDIF(1),IICLK(1),DA(1), REINE 27
5345      1SECIN(9,1),STIN(6,1),CONIN(9,1),YBM(2,NELG,1),DDIN(2,1),ITY(3,1), REINE 28
5346      2RHOM(2,NELG,1),DMY(NELG,1) REINE 29
5347      C REINE 30
5348      IM=NUM+IEL REINE 31
5349      C REINE 32
5350      C CORRECTIVE DESIGN FOR BEAMS EXCEEDING ALLOWABLE DAMAGE REINE 33
5351      C REINE 34
5352      IF(IGR .EQ. 2) GO TO 10 REINE 35
5353      IF(IBMOK.EQ.0) IBMOK=0 REINE 36
5354      IF(ICOR(IM) .EQ. 0) GO TO 30 REINE 37
5355      DAL=DBALL REINE 38
5356      DDD=DSIGN(1.DO,DEDIF(IM))*DABS(DEDIF(IM)) REINE 39
5357      PDD=DSIGN(1.DO,PDEDIF(IM))*DABS(PDEDIF(IM)) REINE 40
5358      IF(IDSGN.GE.2) SIGNA=DDD/PDD REINE 41
5359      IF(IICLK(IM).EQ.1) DA(IM)=DSIGN(1.DO,DDD)*0.05*SECIN(4,IM) REINE 42
5360      IF(IICLK(IM).EQ.0)DA(IM)=DA(IM)*DSIGN(1.DO,SIGNA)*DABS(SIGNA)**1.5REINE 43

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5361      SECIN(4,IM)=SECIN(4,IM)+DA(IM)          REINE 44
5362      SECIN(8,IM)=SECIN(4,IM)                REINE 45
5363      IBMOK=1                                REINE 46
5364      GO TO 30                               REINE 47
5365      10 IF(IBMOK .EQ. 1) GO TO 20           REINE 48
5366      IF(ICOR(IM) .EQ. 0) GO TO 30          REINE 49
5367      DAL=DCALL                              REINE 50
5368      DDD=DSIGN(1.DO,DEDIF(IM))*DABS(DEDIF(IM)) REINE 51
5369      PDD=DSIGN(1.DO,PDEDIF(IM))*DABS(PDEDIF(IM)) REINE 52
5370      IF(IDSGN.GE.2) SIGNA=DDD/PDD           REINE 53
5371      IF(IICLK(IM).EQ.1) DA(IM)=DSIGN(1.DO,DDD)*0.05*SECIN(4,IM) REINE 54
5372      IF(IICLK(IM).EQ.0)DA(IM)=DA(IM)*DSIGN(1.DO,SIGNA)*DABS(SIGNA)**1.5 REINE 55
5373      SECIN(4,IM)=SECIN(4,IM)+DA(IM)        REINE 56
5374      SECIN(8,IM)=SECIN(4,IM)              REINE 57
5375      GO TO 30                               REINE 58
5376      20 YBI1=YBM(1,1,NODI)                  REINE 59
5377      YBI2=YBM(2,1,NODI)                  REINE 60
5378      YBJ1=YBM(1,1,NODJ)                  REINE 61
5379      YBJ2=YBM(2,1,NODJ)                  REINE 62
5380      YCI1=YBM(1,2,NODI)                  REINE 63
5381      YCI2=YBM(2,2,NODI)                  REINE 64
5382      YCJ1=YBM(1,2,NODJ)                  REINE 65
5383      YCJ2=YBM(2,2,NODJ)                  REINE 66
5384      C                                     REINE 67
5385      DM1=(YBI1+YBI2)/2.0                  REINE 68
5386      IF(YBI1*YBI2 .EQ. 0.0) DM1=DMAX1(YBI1,YBI2) REINE 69
5387      DM2=(YBJ1+YBJ2)/2.0                  REINE 70
5388      IF(YBJ1*YBJ2 .EQ. 0.0) DM2=DMAX1(YBJ1,YBJ2) REINE 71
5389      DBMY=DMAX1(DM1,DM2)                  REINE 72
5390      YC=DMAX1(YCI1,YCJ2)                  REINE 73
5391      DMRATIO=(1.25*DBMY-YC)/DMY(IGR,IEL)   REINE 74
5392      SECIN(4,IM)=SECIN(4,IM)*(1+DMRATIO/100.) REINE 75
5393      SECIN(8,IM)=SECIN(8,IM)*(1+DMRATIO/100.) REINE 76
5394      30 IF(SECIN(4,IM).LE.RHOM(1,IGR,IMEM))SECIN(4,IM)=RHOM(1,IGR,IMEM) REINE 77
5395      IF(SECIN(8,IM).LE.RHOM(1,IGR,IMEM))SECIN(8,IM)=RHOM(1,IGR,IMEM) REINE 78
5396      IF(SECIN(4,IM).GE.RHOM(2,IGR,IMEM))SECIN(4,IM)=RHOM(2,IGR,IMEM) REINE 79
5397      IF(SECIN(8,IM).GE.RHOM(2,IGR,IMEM))SECIN(8,IM)=RHOM(2,IGR,IMEM) REINE 80
5398      C                                     REINE 81
5399      C COMPUTE SECTION PROPERTIES          REINE 82
5400      C                                     REINE 83
5401      C 1) REINFORCING STEEL                REINE 84
5402      ES=STIN(1,IM)                        REINE 85
5403      PS=STIN(2,IM)                        REINE 86
5404      FSU=STIN(3,IM)                       REINE 87
5405      EPSSU=STIN(4,IM)                     REINE 88
5406      EPSSY=STIN(5,IM)                     REINE 89
5407      FSU=STIN(6,IM)                       REINE 90
5408      C 2) CONCRETE PROPERTIES              REINE 91
5409      FC=CONIN(1,IM)                       REINE 92
5410      EPSCO=CONIN(2,IM)                     REINE 93
5411      RDD=CONIN(3,IM)                       REINE 94
5412      FCY=CONIN(4,IM)                       REINE 95
5413      EPSCY=CONIN(5,IM)                     REINE 96
5414      FCU=CONIN(6,IM)                       REINE 97
5415      EPSCU=CONIN(7,IM)                     REINE 98
5416      EPSCM=CONIN(8,IM)                     REINE 99
5417      SLR=CONIN(9,IM)                       REINE100
5418      C 3) DIMENSION OF SECTION            REINE101
5419      II=1                                  REINE102
5420      IF((SECIN(1,IM)) .LT. 0.) II=-1      REINE103
5421      HT=SECIN(1,IM)                        REINE104
5422      IF(II.LE.0) HT=-HT                    REINE105
5423      BB=SECIN(2,IM)                        REINE106
5424      DCB=SECIN(3,IM)                       REINE107
5425      ASB=SECIN(4,IM)                       REINE108
5426      OMEGA=SECIN(5,IM)                     REINE109
5427      BT=SECIN(6,IM)                        REINE110
5428      DCT=SECIN(7,IM)                       REINE111
5429      AST=SECIN(8,IM)                       REINE112
5430      AT=SECIN(9,IM)                        REINE113
5431      C                                     REINE114
5432      C EXAMINE ACI-CODE FOR MINIMUM AND MAXIMUM STEEL REINE115
5433      C                                     REINE116
5434      RHO=ASB/(BT*(HT-DCB))                 REINE117
5435      DELD=0.0                               REINE118
5436      RR=ASB/BT                              REINE119
5437      IF(RHO .LT. RHOM(1,IGR,IEL)) DELD=RR/(RHOM(1,IGR,IEL)-RHO) REINE120
5438      IF(RHO .GT. RHOM(2,IGR,IEL)) DELD=RR/(RHOM(2,IGR,IEL)-RHO) REINE121
5439      HT=HT-DELD                             REINE122
5440      C                                     REINE123

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5441 C COMPUTE SECTION PROPERTIES REINE124
5442 C REINE125
5443 VOL=VOL+RFL*AT REINE126
5444 STL=STL+RFL*(AST+ASB) REINE127
5445 EC=FCY/EPSCY REINE128
5446 PC=5./21. REINE129
5447 C REINE130
5448 PCP=(FCU-0.1*FC)/((EPSCM-EPSCU)*EC) REINE131
5449 FN=ES/EC REINE132
5450 FN1=FN-1 REINE133
5451 PS1=1.-PS REINE134
5452 PC1=1.-PC REINE135
5453 AS=AST+ASB REINE136
5454 AC=AT-AS REINE137
5455 DDT=HT-DCB REINE138
5456 DDB=HT-DCT REINE139
5457 AXF=DDIN(1,IM) REINE140
5458 IF(IGR.EQ.2) AXF=-DDIN(2,IM) REINE141
5459 PSP=1.5*PS REINE142
5460 CALL FMPHI(SLR,AXF,HT,BT,DCT,AST,DDT,ASB,FMY1,E11,P1,PHIU1,PHIF1,FREINE143
5461 1MF1,FMU1,YNX1) REINE144
5462 IF(I1.GE.0)THEN REINE145
5463 CALL FMPHI(SLR,AXF,HT,BB,DCB,ASB,DDB,AST,FMY2,E12,P2,PHIU2,PHIF2, REINE146
5464 1FMF2,FMU2,YNX2) REINE147
5465 EII=.5*(E11+E12) REINE148
5466 PP=.5*(P1*E11+P2*E12)/EII REINE149
5467 FMY1=FMY1*(1.-PP*EII/E11)/(1.-PP) REINE150
5468 FMY2=FMY2*(1.-PP*EII/E12)/(1.-PP) REINE151
5469 ELSE REINE152
5470 EII=E11 REINE153
5471 PP=P1 REINE154
5472 FMY2=FMY1 REINE155
5473 PHIU2=PHIU1 REINE156
5474 FMU2=FMU1 REINE157
5475 PHIF2=PHIF1 REINE158
5476 FMF2=FMF1 REINE159
5477 ENDIF REINE160
5478 ATN=CONIN(4,IM)*AT/HT*(DDT+DDB)/2. REINE161
5479 EA=EC*.5*(BB+BT)*HT+ES*(ASB+AST) REINE162
5480 STR=(AS/AT)*100.DO REINE163
5481 IF(STR.LT.0.75) STR=0.75 REINE164
5482 CFR=RDD REINE165
5483 IF(CFR.GT.2.) CFR=2. REINE166
5484 PHUL(1)=PHIU1 REINE167
5485 PHUL(2)=-PHIU2 REINE168
5486 FMU(1)=FMU1 REINE169
5487 FMU(2)=-FMU2 REINE170
5488 PHIF(1)=PHIF1 REINE171
5489 PHIF(2)=-PHIF2 REINE172
5490 FMIF(1)=FMF1 REINE173
5491 FMIF(2)=-FMF2 REINE174
5492 FMY(1)=FMY1 REINE175
5493 FMY(2)=-FMY2 REINE176
5494 PY(1)=AS*FSY+.85*FC*AC REINE177
5495 PY(2)=-(.6.*AC*DSQRT(FC*1000.)/1000.+FSY*AS) REINE178
5496 C REINE179
5497 DO 610 I=1,2 REINE180
5498 KODY(I)=1 REINE181
5499 XI(I)=0. REINE182
5500 Q(I)=1. REINE183
5501 DO 610 J=1,2 REINE184
5502 EI(1,I,J)=EII REINE185
5503 EI(2,I,J)=PP*EII REINE186
5504 EI(3,I,J)=0.0 REINE187
5505 EI(4,I,J)=0.0 REINE188
5506 610 EI(5,I,J)=0.0 REINE189
5507 PSH=PP REINE190
5508 EAL=EA/FL REINE191
5509 C REINE192
5510 DO 630 J=1,2 REINE193
5511 DO 630 IE=1,2 REINE194
5512 PHF(IE,J)=PHIF(J) REINE195
5513 FMF(IE,J)=FMIF(J) REINE196
5514 PHU(IE,J)=PHUL(J) REINE197
5515 BMIY(IE,J)=FMY(J) REINE198
5516 630 PHY(IE,J)=FMY(J)/EII REINE199
5517 DO 640 I=1,4 REINE200
5518 IDAM(I,1)=0 REINE201
5519 INSLP(I,1)=1 REINE202
5520 FAC(I,1)=OMEGA REINE203

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5521      FMDA(I,1)=BMIY(I,1)                REINE204
5522      FMxxM(I,1)=BMIY(I,1)              REINE205
5523      FMxM(I,1)=BMIY(I,1)              REINE206
5524      PHDA(I,1)=PHY(I,1)               REINE207
5525      PPH1(I,1)=PHY(I,1)              REINE208
5526      640 PHxM(I,1)=PHY(I,1)           REINE209
5527      C                                  REINE210
5528      C  RESAVE DATA FOR DAMAGE ACCEPTANCE CRITERIA REINE211
5529      C                                  REINE212
5530      CC=0.85*FC*YXN1*BT                REINE213
5531      CS=FSY*AST                        REINE214
5532      RHOMAX=(CC+CS)/FSY                REINE215
5533      RHOMIN=200.0/FSY                  REINE216
5534      IF(FSY .LE. 200.0) RHOMIN=RHOMIN/1000.0 REINE217
5535      RHOM(1,IGR,IMEM)=RHOMIN           REINE218
5536      RHOM(2,IGR,IMEM)=RHOMAX          REINE219
5537      C                                  REINE220
5538      YBM(1,IGR,NODI)=FMY(1)            REINE221
5539      YBM(2,IGR,NODJ)=FMY(1)            REINE222
5540      C                                  REINE223
5541      IF(IMEM.EQ.1) PRINT 660            REINE224
5542      PRINT 680,IMEM,NODI,NODJ,ITY(2,IM),ITY(1,IM),ITY(3,IM),EII,STR, REINE225
5543      1PP,SSR,CFR,(FMY(J),J=1,2),(FMU(J),J=1,2),(FMIF(J),J=1,2), REINE226
5544      2FMY(1)/EII,PHUL(1),PHIF(1)       REINE227
5545      660 FORMAT(///36H*** COMPUTED MEMBER PROPERTIES ***// REINE228
5546      1      3HEL.,1X,4HNODE,3X,4HMATL,1X,7HYOUNG'S,3X,5HLONG., REINE229
5547      2      1X,6HHARDEN,1X,6HS/SPAN,1X,6HCONFIN,2X,13H YIELD MOMENT,4X, REINE230
5548      3      13HULT. MOMENT,4X,11HFAIL MOMENT,9X,' CURVATURES'//, REINE231
5549      5      3HNO.,1X,4H I/J,1X,8HCO/ST/SE,1X,7HMODULUS,1X,5HSTL %, REINE232
5550      6      1X,6H RATIO,1X,6H RATIO,1X,6H RATIO,3X,13HPOSI. NEGA,3X, REINE233
5551      7      13HPOSI. NEGA,5X,11HPOSI. NEGA,5X,21HYIELD MAX MO. FAREINE234
5552      8IL /) REINE235
5553      680 FORMAT(/I2,1X,I2,'/',I2,1X,I1,'/',I1,'/',I1,1X,E9.3,1X,F5.3,1X, REINE236
5554      1F6.4,1X,F5.2,1X,F6.3,2F9.2,2F9.2,1X,2F9.2,1X,2F7.4,1X,F7.4) REINE237
5555      RETURN REINE238
5556      END REINE239
5557

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- NCEER-88-0041 "Small-Scale Modeling Techniques for Reinforced Concrete Structures Subjected to Seismic Loads," by W. Kim, A. El-Attar and R.N. White, 11/22/88, to be published.
- NCEER-88-0042 "Modeling Strong Ground Motion from Multiple Event Earthquakes," by G.W. Ellis and A.S. Cakmak, 10/15/88.
- NCEER-88-0043 "Nonstationary Models of Seismic Ground Acceleration," by M. Grigoriu, S.E. Ruiz and E. Rosenblueth, 7/15/88, to be published.
- NCEER-88-0044 "SARCF User's Guide: Seismic Analysis of Reinforced Concrete Frames," by Y.S. Chung, C. Meyer and M. Shinozuka, 11/9/88.