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Civil Engineering Study Structural Series 96-4

**INRESB-3D-SUPII**  
**Program Listing for Supercomputer**

**GENERAL PURPOSE PROGRAM FOR INELASTIC ANALYSIS  
OF RC AND STEEL BUILDING SYSTEMS FOR 3D STATIC  
AND DYNAMIC LOADS AND SEISMIC EXCITATIONS**

by

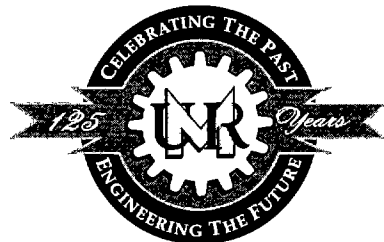
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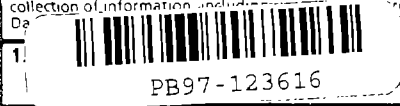
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13. ABSTRACT (Maximum 200 words)  
INRESB-3D-SUPII (supercomputer version) is written in FORTRAN and contains about 32,500 statements. It operates in the VM/SP CMS environment on an IBM 4381 computer or in the AIX/370 environment on an IBM 3090S supercomputer. INRESB-3D-SUPII is a general purpose computer program for analyzing elastic and inelastic building systems subject to static loads, dynamic forces, multicomponent earthquake motion, and pseudo-static cyclic loads. Also the program is capable of calculating inelastic post-buckling behavior of steel members and systems, natural frequency, and free vibration. A joint-based system is used to define the geometry of a structure. Structural members may be elastic 3D prismatic beams, nonlinear RC shear walls, nonlinear springs, inelastic 3D-beam-column elements, finite-segment elements, and nonlinear bracing elements. INRESB-3D-SUP II had five main functions. They are 1) elastic static analysis with multiple load cases based on control program SOL01; 2) elastic or inelastic analysis of a structure subject to 3D ground acceleration based on SOL02; 3) calculation of either natural frequencies and mode shape or buckling load and mode shape of an elastic structure based on SOL03; 4) calculation of nonlinear static cyclic response of a given loading pattern which consists of joint loads, imposed displacements, or element loads based on SOL04; and 5) calculation of maximum response of an elastic structure subject to pseudo-dynamic force obtained from the response spectrum based on SOL05. INRESB-3D-SUPII pertains to analysis of building systems above ground level. Any connection with basements is treated as a boundary condition fixed, hinged, or with a spring.

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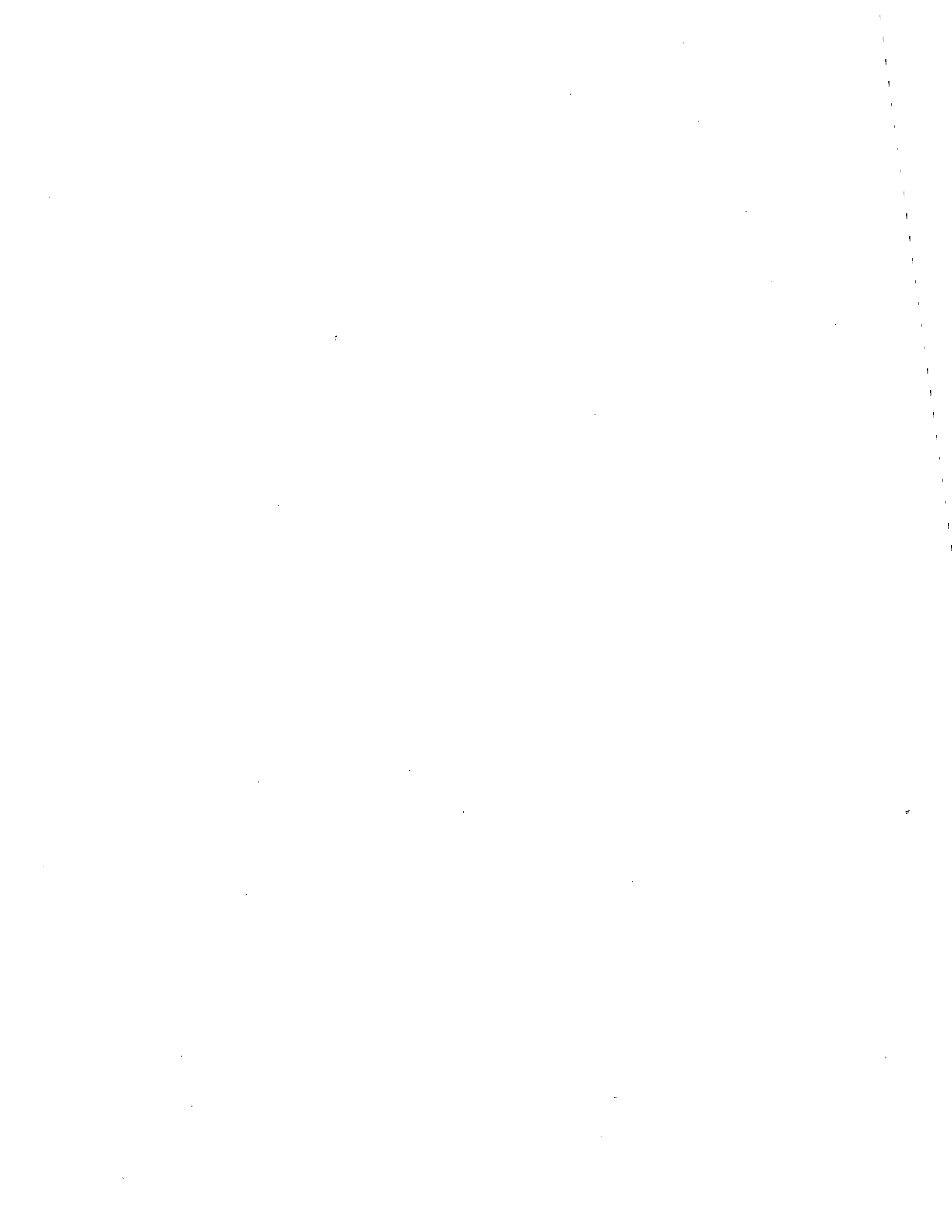
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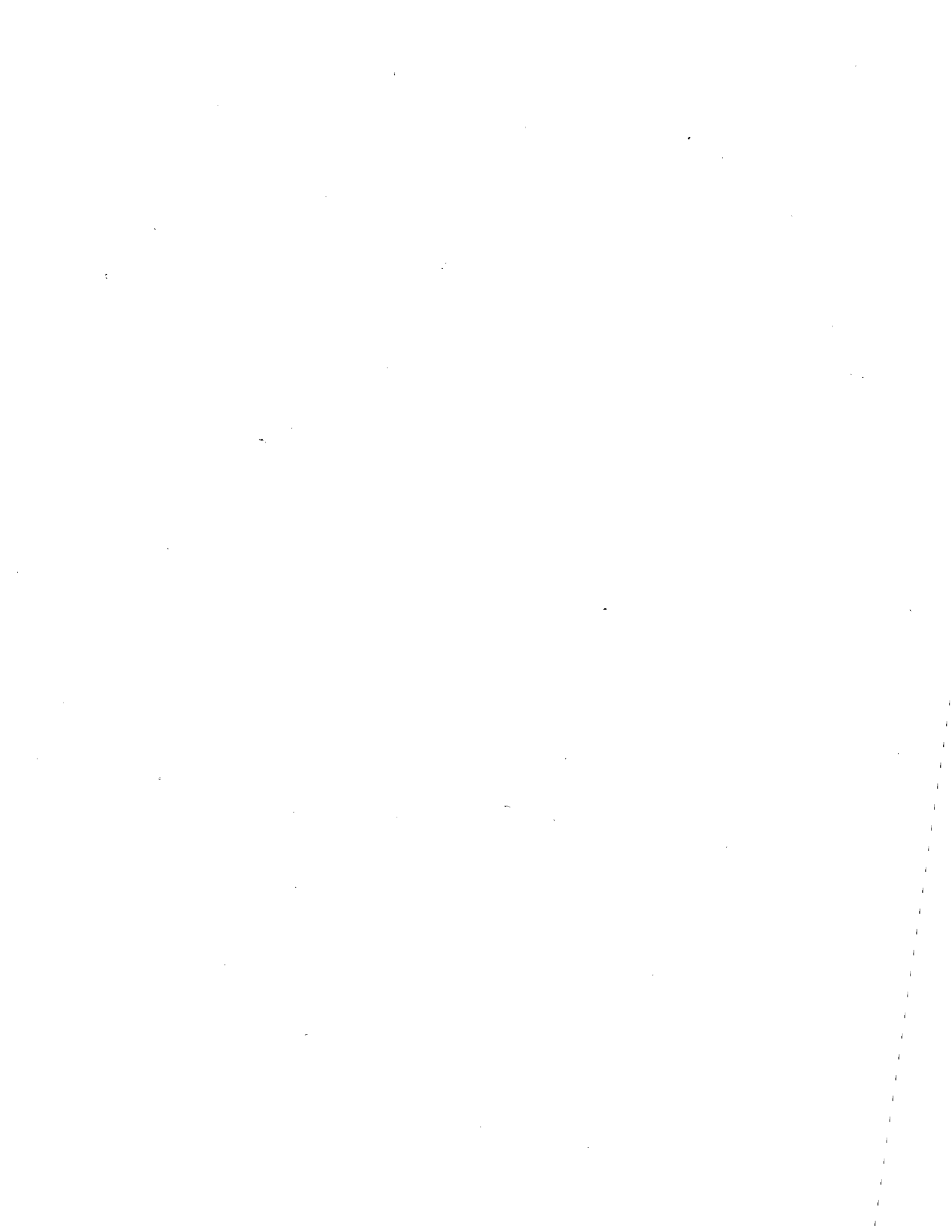
## ABSTRACT

INRESB-3D-SUPII (supercomputer version) is a general purpose program written in FORTRAN and containing about 32,500 statements. Neither the mathematics subroutine DG2CSP (an IMSL eigenvalue routine) nor subroutines SDUMP, CPUTIM, TIMEIT, TIME and DATE are included in this program. Mathematics library IMSL contains DG2CSP. Absence of the other five subroutines (SDUMP, CPUTIM, TIMEIT, TIME and DATE) does not affect operation of the program.

INRESB-3D-SUPII operates in the VM/SP CMS environment on an IBM 4381 computer or in the AIX/370 environment on an IBM 3090S supercomputer. Operation of this program in both the former and the latter consists of three steps. First, a macro library is generated that contains the common blocks. Second, the main program and each subroutine are compiled. Text files are stored in a disk on the user's account. Third, the program is executed. The first and second steps are done only once to install the program.

INRESB-3D-SUPII is suitable for analyzing elastic and inelastic building systems subject to static loads, dynamic forces, multi-component earthquake motion, and pseudo-static cyclic loads. Also, the program is capable of calculating inelastic post-buckling behavior of steel members and systems, natural frequency, and spectral response analysis. A joint-based system is used to define the geometry of a structure. Structural members may be elastic 3D prismatic beams, nonlinear reinforced concrete shear walls, nonlinear springs, inelastic 3D-beam-column elements, finite-segment elements, and nonlinear bracing elements.

INRESB-3D-SUPII has five main functions. They are 1) elastic static analysis with multiple load cases based on control program SOL01; 2) elastic or inelastic analysis of a structure subjected to multiple ground acceleration (one-, two-, or three-dimensional seismic input) based on SOL02; 3) calculation of either natural frequencies and mode shape or buckling load and mode shape for an elastic structure based on SOL03; 4) calculation of nonlinear static cyclic response for a given loading pattern which consists of joint loads, imposed displacements or element loads for based on SOL04; and 5) calculation of maximum response for an elastic



structure subject to pseudo-dynamic force obtained from the response spectrum based on SOL05.

INRESB-3D-SUPII pertains to analysis of building systems above ground level. Any connection with basements is treated as a boundary condition fixed, hinged, or with a spring.

Detail instruction of using the program is published in Civil Engineering Study Structural Series 96-3, INRESB-3D--SUPII, User's Manual.

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```

C----- BEGIN COMMON -----
INCLUDE (ZCOMN1)
INCLUDE (ZCOMN2)
END COMMON
C----- BEGIN BLOCK - ZCOMN1 -----
CHARACTER*112 TITLE(2)
CHARACTER*40 STEPID
LOGICAL BUG, MCOND, KGCOND, DOUBLE, NEWK, NEWKG, ELSTIC, ABORT
INTEGER FMASS, FDAMP, CPUREM, CPUELA
DOUBLE PRECISION B1S, B2S, B3S, B4S, EDD, EDUMMT
COMMON /ZDATA/ TITLE, BUG, IBUG, MAXEDF, MD(25), STEPID, GRAV
COMMON /ZDATA/ IZ, IZREL, CPUREM, CPUELA, ITCPU, INCCPU,
& NEWK, KSAME, NEWKG, MCOND, KGCOND, DOUBLE, ELSTIC, ABORT,
& NCOND, NDOF, MCOND, NFREE, NREST, MAXNOD, NCOB,
& NMAT, NEMLT, FMASS,
& NLOAD, NSOLN,
& MAXELO, NELD,
C
& IZID, IZIDOF, IZICORD, IZICOS, IZICNST, IZIFLG, IZICOS,
& IZMAT, IZELE,
& IZELD, IZELD,
& IZKE, IZLM, IZSTIF, IZMD, IZKEKG, IZLMKG,
& IZMDS, IZMDSG, IZMDAT, IZMDAT, IZKGT, IZKG, IZMDKG,
& IZLOAD, IZDISP, IZVEL, IZACC,
& IZDLOA, IZDOSP, IZDVEL, IZDACC,
& IZAGTX, IZAGTY, IZAGTZ,
& IZDUMP, IZSUB, IZAN, IZAR,
C
& FMASS, FDAMP, IUNIT,
& B1S, B2S, B3S, B4S, EDD, EDUMMT,
& SUMCT(5), GROUND(9), INC,
& EDUMMT(50)
C----- END BLOCK - ZCOMN1 -----
C----- BEGIN BLOCK - ZCOMN2 -----
PARAMETER (MAXZ=50000, MAXDS=MAXZ/2)
COMMON /DATA/ Z(MAXZ)
DIMENSION NZ(MAXZ), DZ(MAXDS)
DOUBLE PRECISION DZ
EQUIVALENCE (Z(1), NZ(1), DZ(1))
C----- END BLOCK - ZCOMN2 -----
C----- BEGIN BLOCK - ZCOMN3 -----
C----- VARIABLES IN COMMON -----
C TITLE = USER INPUT TITLES DESCRIBING STRUCTURE AND SOLUTION
C IZ = NEXT AVAILABLE LOCATION IN LINEAR ARRAY Z
C IZREL = VALUE OF IZ IF STORAGE FOR A SOLUTION IS RELEASED
C BUG = FLAG, IF TRUE, DETAILED OUTPUT IS PRINTED FOR DEBUGGING
C MCOND = FLAG, IF TRUE, MASS HAS 'NOT' BEEN CONDENSED OUT
C NNODE = NUMBER OF NODES
C NDOF = TOTAL NUMBER OF DEGREES OF FREEDOM
C NCOND = NUMBER OF DOF TO BE CONDENSED OUT
C NFREE = NUMBER OF FREE DOF
C NREST = NUMBER OF RESTRAINED DOF
C MAXNOD = MAXIMUM NUMBER OF NODES INPUT
C NMAT = NUMBER OF MATERIAL PROPERTIES
C NEMLT = NUMBER OF ELEMENTS
C NLOAD = NUMBER OF LOADING CASES
C NSOLN = SOLUTION NUMBER
C MAXEDF = MAXIMUM NUMBER OF ELEMENT DOF
C MD = MAIN DIAGONAL ADDRESSES FOR AN UPPER TRIANGULAR STIFFNESS
C NCOS = NUMBER OF JOINT DIRECTION COSINES
C MAXELO = MAXIMUM NUMBER OF ELEMENT LOADS
C NELD = NUMBER OF ELEMENT LOADS
C----- ALL VARIABLES BEGINNING WITH IZ ARE THE ABSOLUTE ADDRESS
C----- IN THE LINEAR ARRAY FOR THE DATA. -----
C IZID = EXTERNAL JOINT NUMBERS
C IZIDOF = DEGREE OF FREEDOM MATRIX FOR JOINT DOF'S
C IZICORD = MATRIX OF JOINT COORD'S
C IZICOS = MATRIX OF JOINT DIRECTION COSINES
C IZICNST = MATRIX OF JOINT CONSTRAINT COEFF
C IZMAT = MATERIAL DATA
C IZELE = ELEMENT DATA
C IZIFLG = MATRIX OF JOINT DOF FLAGS (RELEASE CONSTRAIN CONDENSE ELIM.)
C IZICOS = MATRIX OF JOINT COSINE ID NUMBERS
C IZKE = ADDRESS OF ELEMENT STIFFNESS
C IZLM = ADDRESS OF ELEMENT GLOBAL TO LOCAL MAPPING MATRIX
C IZKEKG = ADDRESS OF ELEMENT GEOMETRIC STIFFNESS
C IZLMKG = ADDRESS OF ELEMENT KG GLOBAL TO LOCAL MAPPING MATRIX
C IZSTIF = GLOBAL STIFFNESS
C IZMD = ADDRESS OF MAIN DIAGONAL STIFFNESS TERMS
C IZLOAD = LOAD MATRIX
C IZDISP = DISPLACEMENT
C IZELD = INTEGER ELEMENT LOAD DATA
C IZIELD = REAL ELEMENT LOAD DATA
C IZELoad = TOTAL LOAD
C IZDISP = TOTAL DISPLACEMENT
C IZVEL = TOTAL VELOCITY
C IZACC = TOTAL ACCELERATION
C IZDLOA = INCREMENTAL LOAD
C IZDOSP = INCREMENTAL DISPLACEMENT
C IZDVEL = INCREMENTAL VELOCITY
C IZDACC = INCREMENTAL ACCELERATION
C IZAGTX = TRANSLATIONAL GROUND ACCELERATION - X DIRECTION
C IZAGTY = TRANSLATIONAL GROUND ACCELERATION - Y DIRECTION
C IZAGTZ = TRANSLATIONAL GROUND ACCELERATION - Z DIRECTION
C IZSUB = UNBALANCED FORCE VECTOR
C IZQ = TEMPORARY VECTOR
C IZR = TEMPORARY VECTOR
C FDAMP = DAMPING TYPE
C FMASS = MASS TYPE
C IUNIT = UNIT # FOR PRINTING DATA TO AN OUTPUT FILE
C----- END COMMON BLOCK - ZCOMN3 -----
C----- PROGRAM FEM - ANALYSIS OF STRUCTURES -----
NOTE: USE IBM COMPILER OPTION AUTODOB(DOUBLE)
TO COMPILE THIS PROGRAM IN DOUBLE PRECISION
NOTE: USE IBM COMPILER OPTION OPT(2)
TO OPTIMIZE THE CODE FOR FASTER EXECUTION
C----- CHARACTER*80 OPTION, INPUT, BLANK
CHARACTER*8 TIME1, DATE1
CHARACTER*6 PLACE
CHARACTER*1 CHR(0:25), NAME
LOGICAL TEST, BTEST, HEAD, AXIAL
C----- INCLUDE (ZCOMN) -----
INCLUDE (ZCOMN3)
DATA CHR/'A', 'B', 'C', 'D', 'E', 'F', 'G', 'H', 'I', 'J', 'K', 'L', 'M',
& 'N', 'O', 'P', 'Q', 'R', 'S', 'T', 'U', 'V', 'W', 'X', 'Y', 'Z',
C
C== INITIALIZE THE UMRX TIMING SUBROUTINE ==
CPUREM=0
CPUELA=0
CALL CPUIMP(CPUREM, CPUELA)
ITCPU=CPUREM
C----- INITIALIZE THE CPU CLOCK -----
CALL TIMCON
TCPU=0
C----- ECHO THE INPUT -----
L=5
9995 READ (5, '(A)', END=9996) INPUT
IF (INDEX(INPUT, 'NOBCO') .NE. 0) GO TO 9996
L=L+1
IF ( L.GT.55 ) THEN
WRITE (6, 9994) (KK, KK=1, 8)
L=0
ENDIF
WRITE (6, 9997) L, INPUT
IF (INDEX(INPUT, 'STOP') .EQ. 0) GO TO 9995
9994 FORMAT ('1. ECHO OF INPUT DATA //
& I=0 1X,3X, LINE ', 8('...', '11, '0'))
9997 FORMAT (1X,7, 'A)
9996 REWIND(5)
C== INITIALIZE DATA -----
TITLE(1)= '
TITLE(2)= '
BLANK= '
BUG=.FALSE.
DOUBLE=.FALSE.
IBUG=0
MAXEDF=24
MD(1)=1
DO 1 I=2, (MAXEDF+1)
1 MD(I)=MD(I-1)+1
C----- INITIALIZE MEMORY -----
DO 10 I=1, MAXZ
10 Z(I)=0
20 READ(5, *, END=1000) OPTION
RATIZ=100.*REAL(IZ)/REAL(MAXZ)
C----- DEFINE JOB RUNNING PLACE -----
IF (INDEX(OPTION, 'UMR') .NE. 0) THEN
PLACE = 'UMR'
READ(5, *, END=1000) REQCPU
ELSE IF (INDEX(OPTION, 'COR') .NE. 0) THEN
PLACE = 'CORNEL'
READ(5, *, END=1000) REQCPU
C----- DEFINE THE STRUCTURE -----
ELSE IF (INDEX(OPTION, 'STR') .NE. 0) THEN
ABORT=.FALSE.
CALL COMPT(1, ZDATA)
GRAV=386.4
READ (5, *, END=1000) TITLE(1)
CALL TIME(TIME1)
CALL DATE(DATE1)
WRITE (TITLE(1)(80), 45) TIME1, DATE1
RATIZ=100.*REAL(IZ)/REAL(MAXZ)
WRITE (1, 52) ' BUILDING STRUCTURAL MODEL -----',
& IZ, RATIZ
WRITE (1, 51) TITLE(1)(1:70)
ABORT=.FALSE.
CALL STRUCT
IZREL=IZ
RATIZ=100.*REAL(IZ)/REAL(MAXZ)
CALL TIMEIT(CPU)
TCPU=CPU*TCPU
WRITE (6, 53) IZ, RATIZ, CPU, TCPU
C----- DEFINE THE LOADING AND SOLVE -----
ELSE IF (INDEX(OPTION, 'SOL') .NE. 0) THEN
READ (5, *, END=1000) TITLE(2)
CALL TIME(TIME1)
CALL DATE(DATE1)
WRITE (TITLE(2)(80), 45) TIME1, DATE1
WRITE (6, 50) '1', TITLE(1), TITLE(2)
WRITE (1, 52) ' SOLVING STRUCTURAL MODEL -----',
& IZ, RATIZ
WRITE (1, 51) TITLE(1)(1:70)
ABORT=.FALSE.
CALL SOLN(OPTION, PLACE, CPU, TCPU, REQCPU)
RATIZ=100.*REAL(IZ)/REAL(MAXZ)
CALL TIMEIT(CPU)
TCPU=CPU*TCPU
WRITE (6, 53) IZ, RATIZ, CPU, TCPU
IF (ABORT) GO TO 1000
C----- DUMP THE MEMORY -----
ELSE IF (INDEX(OPTION, 'DUMP') .NE. 0) THEN
WRITE (1, 52) ' DUMPING MEMORY -----',
& IZ, RATIZ
WRITE (1, 51) TITLE(1)(1:70)
CALL COMPT(2, NZ, DZ)
RATIZ=100.*REAL(IZ)/REAL(MAXZ)
CALL TIMEIT(CPU)
TCPU=CPU*TCPU
WRITE (6, 53) IZ, RATIZ, CPU, TCPU
C== STOP... END OF PROGRAM ==
ELSE IF (INDEX(OPTION, 'STOP') .NE. 0) THEN
WRITE (1, 52) ' NORMAL STOP -----',
& IZ, RATIZ
STOP
C== READ RESULTS FROM INPUT FILE ==

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ELSE IF (INDEX(OPTION,'READ').NE.0) THEN
140 CALL GETINT(OPTION,'UNIT',IUNIT,0,TRUE,...,TRUE.)
CALL GETINT(OPTION,'INC',INC,1,TRUE,...,FALSE.)
IF (IUNIT.GT.0) THEN
WRITE (1,52)
6 READING DATA FROM FILE -----,I2,RATIZ
CALL DMPDAT(IOPT,WRITE,TO,DT)
GO TO 140
ENDIF
RATIZ=100.*REAL(I2)/REAL(MAX2)
CALL TIMEIT(CPU)
TCPU=CPU*TCPU
WRITE (6,53) I2,RATIZ,CPU,TCPU
C-----
C== SET BUG OPTION ==
C-----
ELSE IF (INDEX(OPTION,'BUG',FALSE.) THEN
CALL GETCHR(OPTION,'BUG',INPUT,TRUS,,FALSE.)
IBUG=0
WRITE (OPTION,160) INPUT(1:70)
DO 150 I=0,23
IF (TEST(INPUT,CHR(I),FALSE.) THEN
IBUG=IBSET(1,BUG,1)
ENDIF
150 CONTINUE
160 FORMAT ('BUG',A)
WRITE (1,52) SET BUG OPTION:-----
I2,RATIZ
WRITE (1,51) OPTION(1:170)
WRITE (6,51) SET BUG OPTION:
WRITE (6,51) OPTION(1:170)
C-----
C== SET BUG OPTION ==
C-----
ELSE IF (INDEX(OPTION,'BUG').NE.0) THEN
WRITE (1,51) ('SET BUG OPTION: //OPTION(1:154) )
BUG=.TRUE.
IF (INDEX(OPTION,'NOBUG').NE.0) BUG=.FALSE.
C-----
C== RELEASE MEMORY FROM LAST SOLUTION ==
C-----
ELSE IF (INDEX(OPTION,'RELEASE').NE.0) THEN
WRITE (6,52) '-----'
WRITE (1,52) 'RELEASING MEMORY'
WRITE (6,52) 'RELEASING MEMORY'
IF (INDEX(OPTION,'ELEMENT').NE.0) THEN
WRITE (6,52) 'RESETTING ELEMENT FORCES'
DO 170 IELND=1,NELMT
LSTTYP=0
HEAD=.FALSE.
CALL ELELIB
170 (I2,LSTTYP,,FALSE,,I2,EL,INPUT,EESE,EPSE,DAMAGE,
6 DUCFAG,
6 IELNO,IELDOP,KGDOF,PGBOM,2,AXIAL,I2DDSP,I2DLOA,MSTOR)
ELSE
WRITE (6,52) '-----'
I2,I2ZREL
I2ACC=0
I2DACC=0
I2DOSP=0
I2DISP=0
I2DLOA=0
I2DVEL=0
I2FUP=0
I2LOAD=0
I2VEL=0
RATIZ=100.*REAL(I2)/REAL(MAX2)
CALL TIMEIT(CPU)
TCPU=CPU*TCPU
WRITE (6,53) I2,RATIZ,CPU,TCPU
C-----
C== SAVE MEMORY FOR A RESTART ==
C-----
ELSE IF (INDEX(OPTION,'SAVE').NE.0) THEN
WRITE (1,52) 'SAVING DATA -----'
I2,RATIZ
WRITE (1,51) TITLE(1):(1:70)
WRITE (1,51) TITLE(2):(1:70)
WRITE (6,200)
200 FORMAT (5X, '***** WRITING DATA TO UNIT FT09 *****')
OPEN(UNIT=9,ACCESS='SEQUENTIAL',STATUS='UNKNOWN',
6 FORM='UNFORMATTED')
WRITE (9) TITLE
6 I2,I2ZREL
6 NDOF,NCORD,NFREQ,NREST,MAXNOD,NMAT,NELMT,NNODE,
6 NLOAD,NCOLN,MAXEFD,MD,NCOS,MAXELD,NELD,
6 I2ID,I2IDOP,I2CORD,I2COS,I2CNST,I2MAT,I2ELE,
6 I2JFG,I2COS,
6 I2KE,I2LM,I2STIF,I2MD,I2LOAD,I2DISP,I2ELD,I2ELD
DO 210 J=1,12,256
K=J*255
K=MIN(K,I2)
210 WRITE (9) (2(I),I=J,K)
CLOSE (UNIT=9)
RATIZ=100.*REAL(I2)/REAL(MAX2)
CALL TIMEIT(CPU)
TCPU=CPU*TCPU
WRITE (6,53) I2,RATIZ,CPU,TCPU
C-----
C== RESTART PROGRAM ==
C-----
WRITE (1,52) 'RETRIVING DATA -----'
I2,RATIZ
ELSE IF (INDEX(OPTION,'RESTART').NE.0) THEN
300 WRITE (6,300)
FORMAT (5X, '***** READING DATA FROM UNIT FT09 *****')
OPEN(UNIT=9,ACCESS='SEQUENTIAL',STATUS='OLD',
6 FORM='UNFORMATTED')
READ (9) TITLE
6 I2,I2ZREL
6 NDOF,NCORD,NFREQ,NREST,MAXNOD,NMAT,NELMT,NNODE,
6 NLOAD,NCOLN,MAXEFD,MD,NCOS,MAXELD,NELD,
6 I2JFG,I2COS,
6 I2KE,I2LM,I2STIF,I2MD,I2LOAD,I2DISP,I2ELD,I2ELD
DO 310 J=1,12,256
K=J*255
K=MIN(K,I2)
310 READ (9) (2(I),I=J,K)
CLOSE (UNIT=9)
WRITE (6,50) 'TITLE(1),TITLE(2)'
WRITE (1,52) 'DATA RETRIEVED -----'
I2,RATIZ
WRITE (1,51) TITLE(1):(1:70)
WRITE (1,51) TITLE(2):(1:70)
RATIZ=100.*REAL(I2)/REAL(MAX2)
CALL TIMEIT(CPU)
TCPU=CPU*TCPU
WRITE (6,53) I2,RATIZ,CPU,TCPU
C-----
C== DUMPT STATEMENTS DURING ABORT ==
C-----
ELSE IF (ABORT) THEN

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DO 10 J=1,NLOAD
FEM(I,J)=0
FLAG=.TRUE.
ENDIF
C..... ZERO FORCE
DO 15 I=1,12
FORCE(I)=0
C..... DETERMINE LOAD CODES...
KODE=ELD(5,N)
KDOF=MOD(KODE,100)
KGRP=KODE-KDOF
LOAD=ELD(1,N)
C..... CONCENTRATED LOADING
IF (KGRP.EQ.100) THEN
P=ELD(1,N)
R=ELD(2,N)
IF (KDOF.EQ.1) THEN
F1=(1-R)*P
F2=-R*P
ELSE IF (KDOF.EQ.2) THEN
F6=-P*R*(1-R)**2
F12=P*(1-R)*L*(R)**2
CALL FEMREL(F5 ,F12,B3 ,B0 ,.0,IELNO)
F8=(F5-F12)*R/L
F2=-P-F8
ELSE IF (KDOF.EQ.3) THEN
F5 = P*R*(1-R)**2
F11=-P*(1-R)**2
CALL FEMREL(F5 ,F11,B4 ,B1 ,.0,IELNO)
F9 =(F5-F11)*R/L
F3 =-P-F9
ELSE IF (KDOF.EQ.4) THEN
F4 =-P*(1-R)
F10=-P*R
CALL FEMREL(F4 ,F10,B5 ,B2 ,.1,IELNO)
ELSE IF (KDOF.EQ.5) THEN
F5 =-P*(1-4*R+3*R**2)
F11 =P*(2*R-3*R**2)
CALL FEMREL(F5 ,F11,B4 ,B1 ,.0,IELNO)
F9 =(F5-F11)*R/L
F3 =-P-F9
ELSE IF (KDOF.EQ.6) THEN
F6 =-P*(1-4*R+3*R**2)
F12 =P*(2*R-3*R**2)
CALL FEMREL(F6 ,F12,B3 ,B0 ,.0,IELNO)
F8 =-(F6-F12)*R/L
F2 =-F8
ENDIF
C..... UNIFORM LOADING
ELSE IF (KGRP.EQ.200) THEN
W=ELD(1,N)
IF (KDOF.EQ.1) THEN
F1=-W*L/2
F2=-W*L/2
ELSE IF (KDOF.EQ.2) THEN
F6 =-W*L/12
F12 =W*L/12
CALL FEMREL(F6 ,F12,B3 ,B0 ,.0,IELNO)
F8 =-(F6-F12)/L *-W*L/2
F2 =-W-L-F8
ELSE IF (KDOF.EQ.3) THEN
F5 = W*L/12
F11=-W*L/12
CALL FEMREL(F5 ,F11,B4 ,B1 ,.0,IELNO)
F9 =(F5-F11)/L *-W*L/2
F3 =-W-L-F9
ELSE IF (KDOF.EQ.4) THEN
F4 =-W*L/2
F10=-W*L/2
CALL FEMREL(F4 ,F10,B5 ,B2 ,.1,IELNO)
ELSE
WRITE (6,900) IELNO,' UNIFORM MT OR UNIFORM M2'
ENDIF
C..... USER INPUT FIXED END FORCE
ELSE IF (KGRP.EQ.400) THEN
DO 400 I=1,12
FORCE(I)= ELD(I,N)
C..... INVALID LOAD GROUP
ELSE
WRITE (6,900) IELNO,' INVALID LOAD GROUP'
ENDIF
C..... TRANSFER BEAM LOADS TO FIXED ENDS.....
DO 800 I=1,12
FEM(I,LOAD)=FEM(I,LOAD)+FORCE(I)
C.....
1000 CONTINUE
900 FORMAT(' ER, 'ROR CALCULATING FIXED END FORCES FOR ELEMENT #',
& I5, 'A, ' IS NOT AVAILABLE')
RETURN
END
C-----
C..... DEBUG UNIT(6),SUBCHK,SUBTRACE
END DEBUG
SUBROUTINE CKRNG(V,A,B,NAME)
CHARACTER(*) NAME
IF (A.LT.V.AND.V.LT.B) RETURN
WRITE (6,10) NAME,A,V,B
10 FORMAT (' >>> A, ' IS OUT OF RANGE, SET ',IP,G13.5, ' LE INPUT',
& G13.5, ' LE ',G13.5)
V=MIN(V,B)
V=MAX(V,A)
RETURN
END
C-----
C..... DEBUG UNIT(6),SUBCHK,SUBTRACE,INIT
END DEBUG
SUBROUTINE CKSTOR(ITEMP,IW)
INCLUDE (ZCOMM)
C-----
C= CHECK STORAGE REQUIREMENTS ==
N=MAX(12,IW)+ITEMP
IF (N.LE.MAX2) RETURN
WRITE (6,10) N,MAX2
10 FORMAT ('//IX,48') /
& ' *** AVAILABLE STORAGE IS EXCEEDED' ,T48,'****/'
& ' *** REQUIRED STORAGE =',I10 ,T48,'****/'
& ' *** AVAILABLE STORAGE =',I10 ,T48,'****/'
& ' *** EXECUTION IS TERMINATED, ' ,T48,'****/'
& ' *** RECOMPILE PROGRAM WITH INCREASED MEMORY ,T48,'****/'
& ' 1X,49(') )
C-----
C----- INITIATE IBM ER_ROR TRACEBACK ROUTINE
CALL ERRTA
C-----
C----- ABORT RUN
STOP 'REQUIRED MEMORY > AVAILABLE MEMORY '
END
PROCESS DUMP
C..... DEBUG UNIT(6),SUBCHK,SUBTRACE

```

```

END DEBUG
SUBROUTINE COMDMP(2,NZ,DB)
DIMENSION Z(1),N4(1),D2(1)
DOUBLE PRECISION DB
LOGICAL BTEST
INCLUDE (ZCOMM)
C.....
WRITE (6,50) TITLE(1),TITLE(2)
WRITE (6,51) 'DUMPING MEMORY'
CALL DUMP
IF (.NOT. BTEST(1BUG,13) ) RETURN
C.....
I=0
DO 100 J=1,I2,2
K=0
I=I+1
K=K+1
IF (N4(I).NE.0) WRITE(6,110) I,N4(I),2(I),D2(K)
100 IF (N4(I).NE.0) WRITE(6,110) I,N4(I),2(I)
C.....
50 FORMAT ('1 STRUCTURE.....',A, /
& ' SOLUTION.....',A, /)
51 FORMAT (1X, /)
110 FORMAT (' ZMATRIX(' ,I6,')',I15,IP,G20.10, / ,G20.10)
RETURN
END
C-----
C..... DEBUG UNIT(6),SUBCHK,SUBTRACE
END DEBUG
SUBROUTINE COMPT(IOPTR,LDATA)
CHARACTER*12 TITLE(2)
COMMON /DATA/ TITLE
COMMON /DATA/ I2 ,IDATA(1)
C.....
IF (IOPTR.EQ.1) THEN
DO 10 I=1,LDATA-1
10 IDATA(I)=0
I2=1
TITLE(1)=' '
TITLE(2)=' '
ENDIF
END
C-----
C..... DEBUG UNIT(6),SUBCHK,SUBTRACE
END DEBUG
SUBROUTINE CROSS(VZ,VX,VT,NCRM)
DIMENSION VX(3),VT(3),VZ(3)
LOGICAL BTEST
CROSSI(A1,B1,C1,A2,B2,C2)=B1*C2-B2*C1
CROSSJ(A1,B1,C1,A2,B2,C2)=C1*A2-C2*A1
CROSSK(A1,B1,C1,A2,B2,C2)=A1*B2-A2*B1
VALUE (A1,A2,A3)=SQRT(A1**2+A2**2+A3**2)
C-----
C----- CALCULATE VZ, IF VT WAS CORRECTLY INPUT
VZ(1)=CROSSI(VX(1),VX(2),VX(3),VT(1),VT(2),VT(3))
VZ(2)=CROSSJ(VX(1),VX(2),VX(3),VT(1),VT(2),VT(3))
VZ(3)=CROSSK(VX(1),VX(2),VX(3),VT(1),VT(2),VT(3))
C-----
C----- NORMALISE VX,VT,VZ
IF (NORM.NE.0) THEN
AVZ=VALUE(VZ(1),VZ(2),VZ(3))
AVT=VALUE(VT(1),VT(2),VT(3))
AVV=VALUE(VX(1),VX(2),VX(3))
DO 10 I=1,3
VZ(I)=VZ(I)/AVZ
VT(I)=VT(I)/AVT
VX(I)=VX(I)/AVV
10 CONTINUE
ENDIF
RETURN
END
C-----
C----- DIRECTION OF LOADING .....
C..... DEBUG UNIT(6),SUBCHK,SUBTRACE
END DEBUG
C.....
P = CURRENT LOAD
PL = LAST LOAD
DIR = CURRENT DISPLACEMENT
DIR = CURRENT DIRECTION
DIR=1, POSITIVE LOADING
DIR=2, POSITIVE UNLOADING
DIR=3, NEGATIVE LOADING
DIR=4, NEGATIVE UNLOADING
DIRL = LAST DIRECTION
VEL = PRODUCT OF LAST AND CURRENT VELOCITIES
C-----
SUBROUTINE DIRECT(DIR,DIRL,P,PL,VEL)
IMPLICIT REAL(A-H,O-S)
IMPLICIT INTEGER(I-N)
INTEGER DIR,DIRL
LOGICAL BTEST
IF (P.GT.0) THEN
IF (P.GT.PL.AND.PL.GT.0) THEN
DIRL=DIR
ELSE IF (P.GT.PL.AND.PL.LT.0) THEN
DIRL=4
ELSE IF (P.LT.PL) THEN
DIRL=3
ELSE
DIRL=DIR
ENDIF
ELSE IF (P.LT.0) THEN
IF (P.LT.PL.AND.PL.LT.0) THEN
DIRL=3
ELSE IF (P.LT.PL.AND.PL.GT.0) THEN
DIRL=2
ELSE IF (P.GT.PL) THEN
DIRL=4
ELSE
DIRL=DIR
ENDIF
ELSE
IF (PL.LT.0) THEN
DIRL=4
ELSE IF (PL.GT.0) THEN
DIRL=2
ELSE
DIRL=DIR
ENDIF
ENDIF
ENDIF
IF (DIRL.EQ.1.OR.DIRL.EQ.0) THEN
IF (DIRL.GT.0) THEN
ELSE
DIR=2
ENDIF
ELSE IF (DIRL.EQ.2) THEN
IF (VEL.LT.0) THEN
IF (P.GT.0) THEN
DIR=2
ELSE
DIR=3
ENDIF
ENDIF
ELSE
DIR=1
ENDIF
ENDIF

```

```

COM0030
COM0040
COM0050
COM0060
COM0070
COM0080
COM0090
COM0100
COM0110
COM0120
COM0130
COM0140
COM0150
COM0160
COM0170
COM0180
COM0190
COM0200
COM0210
COM0220
COM0230
COM0240
COM0250
COM0260
COM0270
COM0280
COM0290
COM0300
COM0310
COM0320
COM0330
COM0340
COM0350
COM0360
COM0370
COM0380
COM0390
COM0400
COM0410
COM0420
COM0430
COM0440
COM0450
COM0460
COM0470
COM0480
COM0490
COM0500
COM0510
COM0520
COM0530
COM0540
COM0550
COM0560
COM0570
COM0580
COM0590
COM0600
COM0610
COM0620
COM0630
COM0640
COM0650
COM0660
COM0670
COM0680

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ELSE IF (DIRL.EQ.3) THEN
  IF (VEL.GT.0) THEN
    DIR=3
  ELSE
    DIR=4
  ENDF
ELSE IF (DIRL.EQ.4) THEN
  IF (VEL.GT.0) THEN
    IF (P.LT.0) THEN
      DIR=4
    ELSE
      DIR=1
    ENDF
  ELSE
    DIR=3
  ENDF
ELSE
  WRITE (6,5)
  5 FORMAT(5X,'ERROR IN SUBROUTINE DIRECT, DIRL=NE. 1,2,3,OR 4')
  WRITE (6,10) DIR,DIRL,P,PL,VEL
  ENDF
  IF (DIR.EQ.0) DIR=1
C
  WRITE (6,10) DIR,DIRL,P,PL,VEL
  10 FORMAT(5X,'*** IN DIRECT 1X,
  &' DIR=',14,' DIRL=',14,' P=',F8.3,' PL=',F8.3,' VEL=',F10.3)
RETURN
END
-----
C-----
C      DEBUG UNIT(6),SUBCHK,SUBTRACE,INIT
C      END DEBUG
C-----
SUBROUTINE DMPDAT(IOPT,IWRITE,TO,DT)
LOGICAL TEST,OPEND,FALSE,AXIAL
LOGICAL TEST,FIRST
CHARACTER*40 OPTION
CHARACTER*80 NAME, DIR*2
CHARACTER*112 TITL(2)
CHARACTER*12 INAME
DIMENSION RINPUT(100)
DOUBLE PRECISION TEI,TSE,DSE
SAVE FIRST
INCLUDE (ZCOMM)
C
GO TO (100,200,300,400),IOPT
C-----
C= READ DATA ID TO BE SAVED, AND INITIALIZE FILES ---
C-----
100 FIRST=.TRUE.
  IZDUMP=12
  IZ=I2-1
  NWRITE=0
  WRITE (6,109)
  109 FORMAT (/,5X,' DATA WRITTEN TO FILES /')
  READ (5,*) OPTION,IDOF,IUNIT,IGEN,IDOF,IUNIT
  102 CONTINUE
C-----
C-----SET UP DOF DATA
  IF (TEST(OPTION,'DOF',.FALSE.)) THEN
    WRITE (6,110) 'DEGREE OF FREEDOM',IDOF,IUNIT
  C
  C      CHECK FILE STATUS
  C      INQUIRE (UNIT=IUNIT,OPENED=OPEND)
  C      IF (OPEND) THEN
  C        WRITE (6,105)
  C      ELSE
  C        OPEN (UNIT=IUNIT)
  C
  C      INITIALIZE DATA AND FILE
  C      NWRITE=NWRITE+1
  C      N2(I2)=1
  C      N2(I2+1)=IDOF
  C      N2(I2+2)=IUNIT
  C      IZ=I2-3
  C      WRITE (IUNIT,120) TITLE(1)(1:80),TITLE(1)(81:)
  C      WRITE (IUNIT,120) TITLE(2)(1:80),TITLE(2)(81:)
  C      WRITE (IUNIT,130) 1,IDOF,IWRITE,TO,DT
C-----
  C-----SET UP JOINT
  ELSE IF (TEST(OPTION,'JOINT',.FALSE.)) THEN
    JOINT=IDOF
    JOINTI=IQUICK(IDOF,N2(I2D),NMODE)-1
    IF (TEST(OPTION,'FX',.FALSE.)) THEN
      IDOFJ=N2(I2IDOF+JOINTI)
      DIR='FX'
    ELSE IF (TEST(OPTION,'FY',.FALSE.)) THEN
      IDOFJ=N2(I2IDOF+JOINTI+NAXNOD*1)
      DIR='FY'
    ELSE IF (TEST(OPTION,'FZ',.FALSE.)) THEN
      IDOFJ=N2(I2IDOF+JOINTI+NAXNOD*2)
      DIR='FZ'
    ELSE IF (TEST(OPTION,'MX',.FALSE.)) THEN
      IDOFJ=N2(I2IDOF+JOINTI+NAXNOD*3)
      DIR='MX'
    ELSE IF (TEST(OPTION,'MY',.FALSE.)) THEN
      IDOFJ=N2(I2IDOF+JOINTI+NAXNOD*4)
      DIR='MY'
    ELSE IF (TEST(OPTION,'M3',.FALSE.)) THEN
      IDOFJ=N2(I2IDOF+JOINTI+NAXNOD*5)
      DIR='M3'
    ENDF
  C
  WRITE (6,110) 'DEGREE OF FREEDOM',IDOFJ,IUNIT,JOINT,DIR
C
  C      CHECK FILE STATUS
  C      INQUIRE (UNIT=IUNIT,OPENED=OPEND)
  C      IF (OPEND) THEN
  C        WRITE (6,105)
  C      ELSE
  C        OPEN (UNIT=IUNIT)
  C
  C      INITIALIZE DATA AND FILE
  C      NWRITE=NWRITE+1
  C      N2(I2)=1
  C      N2(I2+1)=IDOFJ
  C      N2(I2+2)=IUNIT
  C      IZ=I2-3
  C      WRITE (IUNIT,120) TITLE(1)(1:80),TITLE(1)(81:)
  C      WRITE (IUNIT,120) TITLE(2)(1:80),TITLE(2)(81:)
  C      WRITE (IUNIT,130) 1,IDOF,IWRITE,TO,DT,JOINT,DIR
C-----
  C-----SET UP ELEMENT
  ELSE IF (TEST(OPTION,'ELE',.FALSE.)) THEN
    WRITE (6,110) 'ELEMENT',IDOF,IUNIT
  C
  C      CHECK FILE STATUS
  C      INQUIRE (UNIT=IUNIT,OPENED=OPEND)
  C      IF (OPEND) THEN
  C        WRITE (6,105)
  C      ELSE
  C        OPEN (UNIT=IUNIT)
  C
  C      INITIALIZE DATA AND FILE
  C      NWRITE=NWRITE+1
  C      N2(I2)=2
  C      N2(I2+1)=IDOF
  C      N2(I2+2)=IUNIT
  C      IZ=I2-3
  C      WRITE (IUNIT,120) TITLE(1)(1:80),TITLE(1)(81:)
  C      WRITE (IUNIT,120) TITLE(2)(1:80),TITLE(2)(81:)
  C      WRITE (IUNIT,130) 1,IDOF,IWRITE,TO,DT,JOINT,DIR
C-----
  C-----SET UP ENERGY
  ELSE IF (TEST(OPTION,'ENERGY',.FALSE.)) THEN
    WRITE (6,111) 'ENERGY BALANCE',IUNIT
  C
  C      CHECK FILE STATUS
  C      INQUIRE (UNIT=IUNIT,OPENED=OPEND)
  C      IF (OPEND) THEN
  C        WRITE (6,105)
  C      ELSE
  C        OPEN (UNIT=IUNIT)
  C
  C      INITIALIZE DATA AND FILE
  C      NWRITE=NWRITE+1
  C      N2(I2)=3
  C      N2(I2+1)=0
  C      N2(I2+2)=IUNIT
  C      IZ=I2-3
  C      WRITE (IUNIT,120) TITLE(1)(1:80),TITLE(1)(81:)
  C      WRITE (IUNIT,120) TITLE(2)(1:80),TITLE(2)(81:)
  C      WRITE (IUNIT,130) 3,IDOF,IWRITE,TO,DT,0,'ENERGY'
C-----
  C-----SET UP SUM OF REACTIONS
  ELSE IF (TEST(OPTION,'SUMRCT',.FALSE.)) THEN
    WRITE (6,111) 'SUMMATION OF REACTIONS',IUNIT
  C
  C      CHECK FILE STATUS
  C      INQUIRE (UNIT=IUNIT,OPENED=OPEND)
  C      IF (OPEND) THEN
  C        WRITE (6,105)
  C      ELSE
  C        OPEN (UNIT=IUNIT)
  C
  C      INITIALIZE DATA AND FILE
  C      NWRITE=NWRITE+1
  C      N2(I2)=4
  C      N2(I2+1)=0
  C      N2(I2+2)=IUNIT
  C      IZ=I2-3
  C      WRITE (IUNIT,120) TITLE(1)(1:80),TITLE(1)(81:)
  C      WRITE (IUNIT,120) TITLE(2)(1:80),TITLE(2)(81:)
  C      WRITE (IUNIT,130) 4,IDOF,IWRITE,TO,DT,0,'SUMRCT'
C-----
  C-----SET UP SUM OF REACTIONS
  ELSE IF (TEST(OPTION,'GROUND',.FALSE.)) THEN
    WRITE (6,111) 'GROUND MOTION',IUNIT
  C
  C      CHECK FILE STATUS
  C      INQUIRE (UNIT=IUNIT,OPENED=OPEND)
  C      IF (OPEND) THEN
  C        WRITE (6,105)
  C      ELSE
  C        OPEN (UNIT=IUNIT)
  C
  C      INITIALIZE DATA AND FILE
  C      NWRITE=NWRITE+1
  C      N2(I2)=5
  C      N2(I2+1)=0
  C      N2(I2+2)=IUNIT
  C      IZ=I2-3
  C      WRITE (IUNIT,120) TITLE(1)(1:80),TITLE(1)(81:)
  C      WRITE (IUNIT,120) TITLE(2)(1:80),TITLE(2)(81:)
  C      WRITE (IUNIT,130) 5,IDOF,IWRITE,TO,DT,0,'GROUND'
C-----
  ELSE IF (TEST(OPTION,'END',.FALSE.)) THEN
    N2(IZDUMP)=NWRITE
    RETURN
  ELSE
    WRITE (6,140) OPTION
    GO TO 101
  ENDF
C-----
C-----GENERATE LOADS
  IF (IGEN.NE.0) THEN
    IGEN=IGEN-1
    IDOF=IDOF + IDOF
    IUNIT=IUNIT + IUNIT
    GO TO 102
  ENDF
  GO TO 101
C-----
  105 FORMAT (5X,'FILE IS ALREADY OPENED, WRITE IS ABORTED',10(' '))
  110 FORMAT (5X,A,' #',15,' IS WRITTEN TO UNIT #',15,',',15,' JOINT:',16)
  111 FORMAT (5X,A,' IS WRITTEN TO UNIT #',15)
  120 FORMAT (A)
  130 FORMAT (3110,1P,2G15.5,OP,1X,16,A)
  140 FORMAT (5X,'INVALID INPUT: ',A,',',15,' INPUT IS SKIPPED...')
C-----
C= WRITE DATA TO FILES ---
C-----
200 NUMB=N2(IZDUMP)
  DO 250 I=1,NUMB
    IZ=IZDUMP+3*(I-1)+1
    ITYPE=N2(I1)
    IDOF=N2(I1+1)
    IUNIT=N2(I1+2)
    IF (ITYPE.EQ.1) THEN
      IF (I2=IDOF-1) THEN
        IF (I2VEL.EQ.0 .OR. I2ACC.EQ.0) THEN
          WRITE (IUNIT,210) 2(I2LOAD+J),2(I2DISP+J),0
        ELSE
          WRITE (IUNIT,210) 2(I2LOAD+J),2(I2DISP+J),
          & 2(I2VEL+J),2(I2ACC+J)
        ENDF
      ELSE IF (ITYPE.EQ.2) THEN
        I7=I7
        IELNO=IDOF
        LASTYP=0
        FALSE=.FALSE.
        CALL ESELB
        & I7,LETTYP,FIRST,IREL,FIRST,NAME,ESSE,EPSE,DAMAGE,
        & DUCFAG,
        & IELNO,IELDOF,KGDOF,PGEOM,RINPUT,AXIAL,IZDDSP,IZDLOG,MSTCR
      ELSE IF (ITYPE.EQ.3) THEN
        DIF=ESSE-ESSE-EKE-PSE-EDD
        IF (DIF.NE.0) THEN
          RE=DIF/EIE*100
        ELSE
          IF (DIF.NE.0) RE=100.00
          IF (DIF.EQ.0) RE=0.00
        ENDF
        WRITE (IUNIT,210) EIE,ESSE,EKE,PSE,EDD,RE
      ELSE IF (ITYPE.EQ.4) THEN
        WRITE (IUNIT,210) SUMRCT
      ELSE IF (ITYPE.EQ.5) THEN

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WRITE (IUNIT,210) GROUND
ENDIF
250 CONTINUE
210 FORMAT(1P,6G13.5)
FIRST=.FALSE.
RETURN
C
C-----
C= CLOSE DATA FILES ---
C-----
C
300 NUNB=N2(12DUMP)
DO 350 I=1,NUNB
  I1=12DUMP+I*(I-1)+1
  ITYPE=N2(I1)
  IDOF=N2(I1+1)
  IUNIT=N2(I1+2)
  CLOSE(UNIT=IUNIT)
350 CONTINUE
RETURN
C
C-----
C= READ DATA FROM FILE AND WRITE TO FTO6 ---
C-----
C
400 OPEN (UNIT=IUNIT)
  REWIND(UNIT=IUNIT)
  READ (IUNIT,120,END=499) TITL(1)(1:80),TITL(1)(81:)
  READ (IUNIT,120,END=499) TITL(2)(1:80),TITL(2)(81:)
  WRITE (6,405) TITL(1),TITL(2)
405 FORMAT ('1' STRUCTURE:.....',A,/)
  SOLUTION.....',A,/)
  READ (IUNIT,131,END=499) ITYPE,DOF,IWRITE,TO,DT,ITEMP,INAME
131 FORMAT (3I10,OP,2G15.5,OP,1X,16,A)
C
C-----
C= DGF DATA -----
C-----
IF (ITYPE.EQ.1) THEN
  WRITE (6,450) IDOF,IUNIT,ITEMP,INAME
  ISTEP=IWRITE
410 READ (IUNIT,*,END=440) RLOAD,DISP,VEL,ACC
  ISTEP=ISTEP+IWRITE
  T=TO+DT*ISTEP
  IF (MOD(ISTEP,INC).EQ.0)
    & WRITE (6,460) ISTEP,T,RLOAD,DISP,VEL,ACC
  GO TO 410
ELSE IF (ITYPE.EQ.2) THEN
  IELNO=IDOF
  IELTYP=ITEMP
  WRITE (6,445) IELNO,IUNIT
C
C GET ELEMENT TYPE AND COMPARE WITH INPUT TYPE.....
IF (IC.EQ.0) THEN
  IC= N2(12ELE+IELNO)
ELSE
  IC=-1
ENDIF
IF (IC.GT.0 .AND. IC.LE.NMAX2) THEN
  IELTPO = N2(IC)
ELSE
  IELTPO = -100
ENDIF
CREATE DUMM ELEMENT DIRECTORY IF INPUT TYPE IS DIFF
12LELE=12ELE
IF (IELTPO.NE.IELTPO) THEN
  CALL CKETOR(12LELE,2,D)
  12ELE = 12+2
  N2(12ELE+IELNO) = 12ELE+IELNO+1
  N2(12ELE+IELNO+1) = IELTPO
ENDIF
19=0
LASTYP=0
FALSE=.FALSE.
CALL EELIIB
  (18 ,LETTYP ,FALSE,IREL,FALSE, NAME ,EISE,EPSE,DAMAGE,
  & DUCFAG,
  & IELNO,IELODF,KGDOF,PGEOM,RINPUT,AXIAL,12DDSP,12DLOA,MSTOR)
C
RESTORE OLD ELEMENT DIRECTORY
12LELE=12LELE
ELSE IF (ITYPE.EQ.3) THEN
  TEI=0.00
  TSE=0.00
  DSE=0.00
  EISE=0
  ESEM=0
  EKEM=0
  PSEM=0
  EDDM=0
  WRITE (6,455) IUNIT
  ISTEP=IWRITE
430 READ (IUNIT,*,END=431) EIE,ESE,EKE, PSE,EDD, RE
  TEI=TEI+EIE
  TSE=TSE+ESE+EKE+PSE+EDD
  DSE=DSE+ABS(EIE-ESE-EKE-PSE-EDD)
  EISE=AMAX1(SNGL(EIE),EISE)
  ESEM=AMAX1(SNGL(ESE),ESEM)
  EKEM=AMAX1(SNGL(EKE),EKEM)
  PSEM=AMAX1(SNGL(PSE),PSEM)
  EDDM=AMAX1(SNGL(EDD),EDDM)
  ISTEP=ISTEP+IWRITE
  T=TO+DT*ISTEP
  IF (MOD(ISTEP,INC).EQ.0)
    & WRITE (6,459) ISTEP,T, EIE, ESE,EKE, PSE,EDD, RE
  GO TO 430
CC431 RE = 100*(TEI-TSE)/TEI
431 RE = 200*(DSE/(TEI+TSE))
  WRITE (6,458) EISE,ESEM,EKEM,PSEM,EDDM,RE
  GO TO 440
ELSE IF (ITYPE.EQ.4) THEN
  WRITE (6,456) IUNIT
  ISTEP=IWRITE
435 READ (IUNIT,*,END=440) SUMRCT
  ISTEP=ISTEP+IWRITE
  T=TO+DT*ISTEP
  IF (MOD(ISTEP,INC).EQ.0)
    & WRITE (6,460) ISTEP,T, SUMRCT
  GO TO 435
ELSE IF (ITYPE.EQ.5) THEN
  WRITE (6,457) IUNIT
  ISTEP=IWRITE
436 READ (IUNIT,*,END=440) GROUND
  ISTEP=ISTEP+IWRITE
  T=TO+DT*ISTEP
  IF (MOD(ISTEP,INC).EQ.0)
    & WRITE (6,461) ISTEP,T, GROUND
  GO TO 436
ENDIF
C
440 CLOSE (UNIT=IUNIT)
RETURN
C
499 CLOSE (UNIT=IUNIT)
WRITE (6,500) IUNIT
500 FORMAT (////10X,'PREMATURE END OF FILE, UNIT',I,/,
  & 10X,'READ COMMAND IS ABORTED'///)
RETURN
C
445 FORMAT (5X,'ELEMENT #',I5,' IS READ FROM UNIT #',I5)
450 FORMAT (5X,'DEGREE OF FREEDOM #',I5,' IS READ FROM UNIT #',I5,/,
  & 6X,'STEP', TIME , DIRECTION, , LOAD ,

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DMP02950 6' DISPLACEMENT VELOCITY ACCELERATION )
DMP02951 455 FORMAT (5X,'ENERGY DATA IS READ FROM UNIT #',I5//
DMP02952 6' 6X,'STEP' TIME INPUT
DMP02953 6' ELASTIC STRAIN KINETIC PLASTIC STRAIN
DMP02954 6' DAMPED RELATIVE ERROR )
DMP02955 456 FORMAT (5X,'SUMMATION OF REACTIONS ARE READ FROM UNIT #',I5//
DMP02956 6' 6X,'STEP' TIME FX
DMP02957 6' 6X,'STEP' TIME FY
DMP02958 6' 6X,'STEP' TIME FZ
DMP02959 457 FORMAT (5X,'GROUND MOTION IS READ FROM UNIT #',I5//
DMP02960 6' 2X,
DMP02961 6' ACCELERATION
DMP02962 6' /-----
DMP02963 6' /----- DISPLACEMENT -----/
DMP02964 1234567890AB1234567890AB1234567890AB
DMP02965 C 2X,'STEP' TIME
DMP02966 6' GLOBAL X GLOBAL Y GLOBAL Z
DMP02967 6' GLOBAL X GLOBAL Y GLOBAL Z
DMP02968 6' GLOBAL X GLOBAL Y GLOBAL Z
DMP02969 6' GLOBAL X GLOBAL Y GLOBAL Z
DMP02970 1234567890AB 1234567890AB 1234567890AB
DMP02971 458 FORMAT(25X,30(' ')/10X,' MAXIMUM 'IP,5G15.5,/,
DMP02972 6' /25X,'GROSS RE1',OP,F15.4,' '
DMP02973 6' /25X,'GROSS RE2',OP,F15.4,' '
DMP02974 459 FORMAT(110,1P,6G15.5,OP,F15.4)
DMP02975 460 FORMAT(110,1P,7G15.5)
DMP02976 461 FORMAT(16,1P,10G12.4)
DMP02977 C
DMP02978 END
DMP02979 C@PROCESS SDUMP OPT(0) GOSTMT XREF MAP
DMP02980 C-----
DMP02981 C DEBUG UNIT(6),TRACE.SUBCHK,INIT,SUBTRACE
DMP02982 C END DEBUG
DMP02983 C-----
DMP02984 C DUCTILITIES AND EXCURSION RATIOS
DMP02985 C-----
DMP02986 SUBROUTINE DNE(IOPT,U,E,DT,D,ESE,PSE,PSEOLD,CSE)
DMP02987 DIMENSION U(3),E(6),UC(3)
DMP02988 C WRITE (6,*) U=,U
DMP02989 C WRITE (6,*) E=,E
DMP02990 C WRITE (6,*) DT=,DT
DMP02991 C WRITE (6,*) D=,D
DMP02992 C WRITE (6,*) ESE=,ESE
DMP02993 C WRITE (6,*) PSE=,PSE
DMP02994 C WRITE (6,*) PSEOLD=,PSEOLD
DMP02995 C WRITE (6,*) CSE=,CSE
DMP02996 C
DMP02997 C-----
DMP02998 C VARIABLES -----
DMP02999 C U(I) = DUCTILITY (I)
DMP03000 C E(I) = EXCURSION RATIO (I)
DMP03001 C E(I+3) = DUCTILITY (I) FOR THE CURRENT HALF CYCLE
DMP03002 C DT = YIELD DISPLACEMENT
DMP03003 C D = CURRENT DISPL
DMP03004 C TSE = TOTAL STRAIN ENERGY
DMP03005 C PSE = PLASTIC STRAIN ENERGY
DMP03006 C ESE = ELASTIC STRAIN ENERGY
DMP03007 C CSE = CONSTANT STRAIN ENERGY AT YIELD.....
DMP03008 C UC = CURRENT DUCTILITY
DMP03009 C-----
DMP03010 C IF (IOPT.EQ.1) THEN
DMP03011 C ..... CALCULATE CURRENT DUCTILITIES .....
DMP03012 PSE0=PSE-PSEOLD
DMP03013 IF (DT.NE.0) UC(1)=D/DT
DMP03014 IF (E(1).GT.0) UC(2)=(PSE0/ESE + 1)
DMP03015 IF (CSE.NE.0) UC(3)=(PSE0/CSE + 1)
DMP03016 DO 10 I=1,3
DMP03017 J=I+3
DMP03018 IF ( UC(I).GE.U(J) ) U(I)=UC(I)
DMP03019 C 10 U(1)=MAX( U(1),UC(1) )
DMP03020 U(2)=MAX( U(2),UC(2) )
DMP03021 U(3)=MAX( U(3),UC(3) )
DMP03022 E(4)=MAX( E(4),UC(1) )
DMP03023 E(5)=MAX( E(5),UC(2) )
DMP03024 E(6)=MAX( E(6),UC(3) )
DMP03025 C
DMP03026 ELSE
DMP03027 C ..... CALCULATE EXCURSION RATIOS .....
DMP03028 DO 20 I=1,3
DMP03029 J=I+3
DMP03030 IF (E(J).GT.1) E(I) = E(I) + E(J) - 1
DMP03031 20 E(J)=0
DMP03032 C
DMP03033 ENDIF
DMP03034 C WRITE (6,*) U=,U
DMP03035 C WRITE (6,*) E=,E
DMP03036 C WRITE (6,*) DT=,DT
DMP03037 C WRITE (6,*) D=,D
DMP03038 C WRITE (6,*) ESE=,ESE
DMP03039 C WRITE (6,*) PSE=,PSE
DMP03040 C WRITE (6,*) PSE0=,PSE0
DMP03041 RETURN
DMP03042 END
DMP03043 C@PROCESS SDUMP OPT(0) GOSTMT XREF
DMP03044 C-----
DMP03045 C DEBUG UNIT(6),SUBCHK,SUBTRACE,INIT
DMP03046 C END DEBUG
DMP03047 C-----
DMP03048 C-----
DMP03049 C MAINMOD,NODE,COSINE,DOF,ITFLG,JTCOS,NCOS,
DMP03050 & TX,TY,TZ,MASS,MD,IMD,A,MC,P,GA,GAT)
DMP03051 DIMENSION COSINE(3,3,NCOS),IDOF(MAINMOD,6)
DMP03052 DIMENSION ITFLG(MAINMOD),JTCOS(MAINMOD)
DMP03053 DIMENSION TX(N),TY(N),TZ(N),A(NAIN),P(NDOF)
DMP03054 DIMENSION V1(3),V2(3),V3(3),MD(NDOF+1),GA(3),GAT(3)
DMP03055 REAL MASS(IMD),MC(NDOF,3)
DMP03056 CHARACTER*80 FORMAT(2),TITLE(2)
DMP03057 LOGICAL FMT,ETCH,BUG,PRINT,REWIND
DMP03058 ACCELZ=0
DMP03059 C
DMP03060 GO TO (100,200) IOPT
DMP03061 C
DMP03062 C= INPUT ACCELERATIONS ---
DMP03063 C-----
DMP03064 100 CONTINUE
DMP03065 C----- ZERO GLOBAL BASE ACCELERATIONS
DMP03066 DO 101 I=1,N
DMP03067 TX(I)=0
DMP03068 TY(I)=0
DMP03069 TZ(I)=0
DMP03070 101 TX(I)=0
DMP03071 C
DMP03072 C----- INPUT NUMBER OF ACCEL, AMP, FACTOR SCALE, AND TIME INCREMENT
DMP03073 READ (5,*) NA,ASCALE,TO,DT,PRINT
DMP03074 C
DMP03075 C----- INPUT DIRECTION VECTORS V1 AND V2
DMP03076 READ (5,*) V1,V2
DMP03077 IF (BUG) WRITE (6,29) 'INPUT= V1:',V1,' V2:',V2
DMP03078 C----- CALCULATE V3 = V1 X V2
DMP03079 CALL CROSS(V3,V1,V2,0)
DMP03080 IF (BUG) WRITE (6,28) 'V3=V1XV2 V1:',V1,' V2:',V2,' V3:',V3
DMP03081 C-----CALCULATE V2 = V3 X V1 AND NORMALIZE ALL VECTORS...
DMP03082 CALL CROSS(V2,V3,V1,1)
DMP03083 IF (BUG) WRITE (6,29) 'V2=V3XV1 V1:',V1,' V2:',V2,' V3:',V3
DMP03084 C
DMP03085 IF (BUG) WRITE (6,29) ' V1:',V1,' V2:',V2,' V3:',V3
DMP03086 29 FORMAT(5X,3(1A,5E,9.5,' '),SP,F9.5,' J',F9.5,' K',SS)
DMP03087 C
DMP03088 C----- INPUT INDIVIDUAL ACCELEROGRAMS

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DO 30 I=1,NA
  READ (5,*) IN,NPTS,IDIR,FMT,ECHO,REWIND
  NPTS=MIN(NPTS,N)
  IF (REWIND .AND. IN.NE.5) REWIND (IN)
  IF (FMT) THEN
    READ (5,91) FORMAT(1),FORMAT(2)
    READ (IN,FORMAT(1)) ATITLE(1),ATITLE(2)
  ELSE
    READ (IN,91) ATITLE(1),ATITLE(2)
  ENDDIF
  IF (IDIR.GE.1 .AND. IDIR.LE.3) THEN
    WRITE (6,10) I,IN,ATITLE(1),ATITLE(2),NPTS,TO,DT
    FORMAT (/5X,' GROUND ACCELERATION RECORD '//
    & 5X,' INPUTTING TRANSLATIONAL ACCELERATION RECORD #',
    & I2,' FROM UNIT:',4,'/5X,A/5X,A//
    & 5X,' THE RECORD CONTAINS',16,' POINTS, ',
    & ' BEGINNING AT TIME:',1P,G16.6,
    & ' WITH A TIME INCREMENT OF:',1P,G15.6)
  ELSE
    WRITE (6,*) 'INVALID DIRECTION',IDIR,' SET Q < IDIR < 4'
    GO TO 30
  ENDDIF
  IF (IDIR.EQ.1) THEN
    CX=V1(1)
    CY=V1(2)
    CZ=V1(3)
    WRITE (6,92) V1
  ELSE IF (IDIR.EQ.2) THEN
    CX=V2(1)
    CY=V2(2)
    CZ=V2(3)
    WRITE (6,92) V2
  ELSE IF (IDIR.EQ.3) THEN
    CX=V3(1)
    CY=V3(2)
    CZ=V3(3)
    WRITE (6,92) V3
  ENDDIF
  IF (FMT) THEN
    READ (IN,FORMAT(2),END=25) (A(J),J=1,NPTS)
  ELSE
    READ (IN,*,END=25) (A(J),J=1,NPTS)
  ENDDIF
  GO TO 27
  ----- END OF FILE WAS ENCOUNTERED BEFORE ALL INPUT WAS READ
  25 NPTS=J
  WRITE (6,26) J
  FORMAT (5X,' ONLY ',I6,
  & ' POINTS WERE READ BEFORE END OF FILE WAS ENCOUNTERED')
  27 IF (ECHO) WRITE (6,94) (A(J),J=1,NPTS)
  WRITE (6,9280)
  CALL SMAX (NPTS,DT,A,TKX,TMY,T1,T2,NPTS,AVE,STDEV,RMS)
  T1=T1-DT*TO
  T2=T2-DT*TO
  T1=T1+TO
  T2=T2+TO
  WRITE (6,9290) 'INPUT ACCELERATION ',
  & TKX,T1,TMY,T2,AVE,STDEV,RMS
  DO 20 J=1,NPTS
    ACC=A(J)*ASCALF
    TX(J)=TX(J) + ACC*CX
    TY(J)=TY(J) + ACC*CY
    TZ(J)=TZ(J) + ACC*CZ
  20 CONTINUE
  30 CONTINUE
  IF (PRINT) THEN
    WRITE (6,31) 'X'
    WRITE (6,93) (TX(J),J=1,N)
    WRITE (6,31) 'Y'
    WRITE (6,93) (TY(J),J=1,N)
    WRITE (6,31) 'Z'
    WRITE (6,93) (TZ(J),J=1,N)
  31 FORMAT (/5X,' GLOBAL BASE ACCELERATION IN THE ',A,
  & ' DIRECTION (G)'S)
  ENDDIF
  WRITE (6,9280)
  CALL SMAX (N,DT,TKX,TMY,T1,T2,N,AVE,STDEV,RMS)
  T1=T1-DT*TO
  T2=T2-DT*TO
  T1=T1+TO
  T2=T2+TO
  WRITE (6,9290) 'X-AXIS ACCELERATION ',TKX,T1,TMY,T2,AVE,STDEV,RMS
  CALL SMAX (N,DT,TKY,TMY,T1,T2,N,AVE,STDEV,RMS)
  T1=T1-DT*TO
  T2=T2-DT*TO
  T1=T1+TO
  T2=T2+TO
  WRITE (6,9290) 'Y-AXIS ACCELERATION ',TKY,T1,TMY,T2,AVE,STDEV,RMS
  CALL SMAX (N,DT,TKZ,TMY,T1,T2,N,AVE,STDEV,RMS)
  T1=T1-DT*TO
  T2=T2-DT*TO
  T1=T1+TO
  T2=T2+TO
  WRITE (6,9290) 'Z-AXIS ACCELERATION ',TKZ,T1,TMY,T2,AVE,STDEV,RMS
  ----- CALCULATE MASS * COSINE MATRIX FOR EACH DOF...
  C..... LOOP FOR DIRECTION = JDIR
  DO 90 JDIR=1,3
  C..... ZERO MC MATRIX AND TEMP COSINE MATRIX FOR JDIR
  DO 40 I=1,NDOP
    MC(I,JDIR)=0
    A(I)=0
  40 CONTINUE
  C..... CONSTRUCT COSINE MATRIX FOR JDIR
  CMT ALL DOF THAT ARE CONSTRAINED TO A MASTER DOF...
  DO 50 NODES=1,NNODE
    ICOS=JTCOS(NODE)
    IF (.NOT. STREST(JFLG(NODE),K=1)) THEN
      II=IDOF(NODE,K)
      A(II)=A(II)+COSINE(JDIR,K,ICOS)
    ENDIF
  50 CONTINUE
  C..... MULTIPLY MASS * COSINE MATRIX FOR JDIR
  STORE IN MC(____,JDIR)
  DO 60 J=1,NDOP
    MDJ=MD(J)+J
    M = J-(MD(J+1)-MD(J))+1
    DO 60 I=M,J
      DO 70 I=2,NDOP
        MDI=MD(I)+I
        M = I-(MD(I+1)-MD(I))+1
        DO 70 J=M,I-1
          MC(I,JDIR)+MC(I,JDIR) + MASS(MDI-J) * A(J)
      70 CONTINUE
  60 CONTINUE
  IF (BUG) CALL WMATRIX(MC,NDOP,3,'MASS*COSINE')
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DL00500 C
DL00510 91 FORMAT (A)
DL00520 92 FORMAT (5X,' DIRECTION OF ACCELERATION:',
DL00530 & ' 4X,SS,F9.5, ', ' ',SP,F9.5, ' ',J',F9.5, ' ',K',SS)
DL00540 93 FORMAT (/5X,' INPUT ACCELERATION:',
DL00550 & ' (G)'S)
DL00560 & ' (/1X,IP,10G12.4)
DL00570
DL00580 C-----
DL00590 C= INITIAL GROUND ACCELERATION ---
DL00600
DL00610 IF (TL.GE.TO .AND. TL.LE.N*DT) THEN
DL00620 I=(TL-TO)/DT +1
DL00630 R=(I*DT-TO-T)/DT
DL00640 GAT(1)=TX(I)*R
DL00650 GAT(2)=TY(I)*R
DL00660 GAT(3)=TZ(I)*R
DL00670 IF (J.LE.N .AND. R.NE.1) THEN
DL00680 GAT(1)=GAT(1) + (1-R)*TX(J)
DL00690 GAT(2)=GAT(2) + (1-R)*TY(J)
DL00700 GAT(3)=GAT(3) + (1-R)*TZ(J)
DL00710 ENDDIF
DL00720 ELSE
DL00730 GAT(1)=0
DL00740 GAT(2)=0
DL00750 GAT(3)=0
DL00760 ENDDIF
DL00770 GAT(1)= GAT(1)+GRAV
DL00780 GAT(2)= GAT(2)+GRAV
DL00790 GAT(3)= GAT(3)+GRAV
DL00800 IF (BUG) WRITE (6,9279) T,TL,ACCELS,GAT(1),GAT(2),GAT(3)
DL00810 RETURN
DL00820
DL00830 C-----
DL00840 C= DETERMIN INCREMENTAL ACCELERATIONS ---
DL00850 C-----
DL00860 200 CONTINUE
DL00870 C----- DETERMINE THE INCREMENTAL ACCELERATION BETWEEN TIME T AND TL
DL00880 DO 201 I=1,3
DL00890 201 GA(I)=0
DL00900 T=T
DL00910 DT=DT
DL00920 C..... CURRENT ACCELERATION
DL00930 IF (TL.GE.TO .AND. TL.LE.N*DT) THEN
DL00940 I=(TL-TO)/DT +1
DL00950 R=(I*DT-TO-T)/DT
DL00960 GA(1)=TX(I)*R
DL00970 GA(2)=TY(I)*R
DL00980 GA(3)=TZ(I)*R
DL00990 IF (J.LE.N .AND. R.NE.1) THEN
DL01000 GA(1)=GA(1) + (1-R)*TX(J)
DL01010 GA(2)=GA(2) + (1-R)*TY(J)
DL01020 GA(3)=GA(3) + (1-R)*TZ(J)
DL01030 ENDDIF
DL01040 TXI=TX(I)
DL01050 TXJ=TX(J)
DL01060 ENDDIF
DL01070 C..... PREVIOUS ACCELERATION
DL01080 IF (TL.GE.TO .AND. TL.LE.N*DT) THEN
DL01090 I=(TL-TO)/DT +1
DL01100 R=(I*DT-TO-T)/DT
DL01110 GA(1)=GA(1)-TX(I)*R
DL01120 GA(2)=GA(2)-TY(I)*R
DL01130 GA(3)=GA(3)-TZ(I)*R
DL01140 IF (J.LE.N .AND. R.NE.1) THEN
DL01150 GA(1)=GA(1) - (1-R)*TX(J)
DL01160 GA(2)=GA(2) - (1-R)*TY(J)
DL01170 GA(3)=GA(3) - (1-R)*TZ(J)
DL01180 ENDDIF
DL01190 TXI=TX(I)
DL01200 TXJ=TX(J)
DL01210 ENDDIF
DL01220 C----- MULTIPLY B BY GRAV.
DL01230 DO 215 K=1,3
DL01240 215 GA(K)= GA(K)+GRAV
DL01250 C----- CALC. ACCELERATION VECTOR = -MASS * COSINE * GROUND ACCELERATION
DL01260 DO 220 I=1,NDOP
DL01270 220 P(I)=P(I) - MC(I,K)*GA(K)
DL01280 IF (BUG) WRITE (6,9279) T,TL,ACCELS,GA(1),GA(2),GA(3)
DL01290 9279 FORMAT (' DLOAD: ',T1,IP,G12.4,' T1:',G12.4,' ACCELZ:',G12.4,'
DL01300 & ' DFLD: ',X',IP,G12.4,' T1:',G12.4,'
DL01310 & ' IF (BUG) CALL WMATRIX(P,NDOP,1,'INCREMENTAL LOAD')
DL01320 RETURN
DL01330 9280 FORMAT (/3X,20X,' MAXIMUM AT TIME ',1X,
DL01340 & ' RMS ' AT TIME AVERAGE STANDARD DEV.',1X,
DL01350 & ' RMS ')
DL01360 9290 FORMAT (3X,A,IP,G15.5)
DL01370 C END
DL01380 C-----
DL01390 C DEBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
DL01400 C END DEBUG
DL01410 C-----
DL01420 SUBROUTINE ELEM8
DL01430 & (IOPT,LESTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,
DL01440 & DUCPAG,
DL01450 & IELNO,ISELDOF,KDOF,PGEOM,RINPUB,AXIAL,DISP,ILCAD,MSTOR)
DL01460 LOGICAL KNAME,ERR,FIRST,PRINT,HEAD,AXIAL,STEST
DL01470 CHARACTER*80 NAME
DL01480 DIMENSION RINPUB(100)
DL01490 DIMENSION ASAT(12,12),SAT(12,12),STIFF(78)
DL01500 C
DL01510 INCLUDE (COMMON)
DL01520 IOPT=IOPT
DL01530 I# = I#
DL01540 IC = N2(ISELE+IELNO)
DL01550 ISELE= ISELE
DL01560 ISELNO= IELNO
DL01570 IF (.NOT. (IOPT.EQ.1 .OR. IOPT.EQ.9)) ISELTP = N2(IC)
DL01580 ISELTP = N2(IC)
DL01590 IF (LESTYP.EQ.ISELTP) THEN
DL01600 FIRST=.FALSE.
DL01610 ELSE
DL01620 FIRST=.TRUE.
DL01630 IF (HEAD) WRITE (6,305) TITLE(1),TITLE(2)
DL01640 305 IF (HEAD) WRITE (6,305) TITLE(1),TITLE(2)
DL01650 & ' SOLUTION.....',A,/'
DL01660 ENDDIF
DL01670 C
DL01680 MSTOR=0
DL01690 C
DL01700 IF (ISELTP.EQ.1) THEN
DL01710 IZL=IC-64
DL01720 IZKE=IC-281-N2(IC-133)
DL01730 IZLMKG=IC-121
DL01740 IF (MAXELD.LE.0) MAXELD=1
DL01750 IF (IOPT.NE.3) CALL ELEM1
DL01760 & (IOPT,IUNIT,INC,FIRST,PRINT,
DL01770 & NDOP,NLOAD,MAXNOD,NNODE,NCOS,
DL01780 & N2(12ID),N2(13IDOF),3(12CORD),2(12COS),N2(12JCOS),
DL01790 & 2(12CNST),2(12DISP),2(LOAD),BUG,
DL01800 & IBUG,2(12KGD),N2(12KGD+1),GRAV,PGEOM)
DL01810
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DL04800
DL04810
DL04820
DL04830
DL04840
DL04850
DL04860
DL04870
DL04880
DL04890
DL04900
DL04910
DL04920
DL04930
DL04940
DL04950
DL04960
DL04970
DL04980
DL04990
DL05000
```

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4 MAXELD ,NELD ,N2(I3IELD) ,Z(I3IELD) ,NAME ,ELED0440
4 STEPID ,AXIAL ,IELNO ,RINPUT ,Z(I3FUB) ,ELED0450
4 ESESE ,EPSE ,DAMAGE ,DUCFAG ,ELED0460
4 N2(IC ) ,N2(IC+1) ,Z(IC+2) ,Z(IC+3) ,Z(IC+4) ,ELED0470
4 Z(IC+5) ,Z(IC+6) ,Z(IC+7) ,Z(IC+8) ,Z(IC+9) ,ELED0480
4 Z(IC+10) ,Z(IC+11) ,Z(IC+12) ,Z(IC+13) ,Z(IC+14) ,ELED0490
4 Z(IC+15) ,Z(IC+16) ,Z(IC+17) ,Z(IC+18) ,Z(IC+19) ,ELED0500
4 N2(I2LM ) ,Z(IC+16) ,Z(IC+18) ,Z(IC+20) ,Z(IC+21) ,ELED0510
4 Z(IC+12) ,N2(IC+11) ,N2(IC+14) ,N2(IC+15) ,N2(IC+16) ,ELED0520
4 N2(IC+17) ,Z(IC+18) ,N2(IC+19) ,N2(IC+20) ,N2(I2LMKG) ,ELED0530
4 N2(IC+133) ,Z(IC+134) ,Z(IC+136) ,N2(IC+280) ,Z(IC+281) ,ELED0540
4 Z(I2KE ) ,ELED0550
4 Z(I2W ) ,ELED0560
C MSTAR =N2(IC+133) ELED0570
4 IELDOF=N2(IC+1) ELED0580
4 I2LMKG=IC+121 ELED0590
4 I2KKEG=I2KE+N2(IC+120) ELED0600
4 KGDOF =N2(IC+119) ELED0610
C ELSE IF (IELTYP.EQ.2) THEN ELED0620
4 CALL ELED2 ELED0630
4 (IOPT ,IUNIT ,NEWK ,FIRST ,PRINT ,ELED0640
4 NDOP ,NLOAD ,MAXNO ,NNODE ,NCOS ,ELED0650
4 N2(I2ID) ,N2(I2IDOF) ,Z(I2ICORD) ,Z(I2COS) ,N2(I2JCOS) ,ELED0660
4 Z(I2ICNST) ,Z(I2IDISP) ,Z(I2IDVEL) ,Z(I2LOAD) ,Z(I2IFUB) ,ELED0670
4 IREL ,BUG ,IBUG ,ACCEL ,N2(I2KGD+1) ,ELED0680
4 GRAV ,PGDOM ,ELSTIC ,NAME ,STEPID ,ELED0690
4 IELNO ,RINPUT ,AXIAL ,ESESE ,EPSE ,INC ,ELED0700
4 DAMAGE ,DUCFAG ,ELED0710
4 N2(IC ) ,N2(IC+1) ,N2(IC+2) ,N2(IC+3) ,N2(IC+4) ,ELED0720
4 N2(IC+5) ,Z(IC+6) ,Z(IC+7) ,Z(IC+8) ,Z(IC+9) ,ELED0730
4 Z(IC+10) ,Z(IC+11) ,N2(IC+12) ,Z(IC+13) ,Z(IC+14) ,ELED0740
4 N2(IC+15) ,Z(IC+16) ,N2(IC+40) ,N2(IC+52) ,Z(IC+53) ,ELED0750
4 Z(IC+55) ,Z(I2KE ) ,ELED0760
4 IOPP=IOPT ELED0770
4 MSTAR =N2(IC+12) ELED0780
4 IELDOF=N2(IC+1) ELED0790
4 I2LMKG=IC+58 ELED0800
4 I2KKEG=IC+167+N2(IC+5) +N2(IC+33)-1 ELED0810
4 KGDOF =N2(IC+159) ELED0820
C ELSE ELED0830
4 WRITE (6,10) IELNO ELED0840
4 ERR=TRUE ELED0850
10 FORMAT('ELEMENT ',16,' IS NOT AVAILABLE') ELED0860
4 ENDF ELED0870
4 LSTTYP=IELTYP ELED0880
4 IOPT=IOPT ELED0890
4 RETURN ELED0900
4 END ELED0910
C ----- ELED0920
4 DEBUB UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE ELED0930
4 END DEBUB ELED0940
C ----- ELED0950
4 SUBROUTINE ELED0A(BUG,NF,NELMT,MAXELD,NELD,NDOP,NLOAD,FORCE,IELD, ELED0960
4 ELD,I2DISP,ILOAD,NAME,TITLE,HEAD) ELED0970
C NOTE ON ARGUMENTS: THE LOAD MATRIX FORCE IS PASSED FROM THE CALLING ELED0980
C ROUTINE. FORCE IS EQUIVALENCED TO THE 2 MATRIX BY THE CALLING ROUTINE ELED0990
C I.E. Z(ILOAD)=FORCE(1,I), WHERE ILOAD IS THE OFFSET IN THE 2 MATRIX. ELED1000
C BOTH THE CALLING ROUTINE AND SUBROUTINE ELED1010 USE THE 2 MATRIX ELED1020
4 DIMENSION FORCE(NDOP,NLOAD),ELD(5,MAXELD),ELD(12,MAXELD) ELED1030
4 DIMENSION RINPUT(100) ELED1040
4 LOGICAL FIRST,HEAD,FALSE ELED1050
4 LOGICAL AXIAL,BUG ELED1060
4 LOGICAL BTSET ELED1070
4 CHARACTER*(*) TITLE(2) ELED1080
4 CHARACTER*(*) NAME ELED1090
4 CHARACTER*20 TYPE,DIRI ELED1100
4 CHARACTER*40 DIR ELED1110
4 CHARACTER*1 CX(6) ELED1120
4 FIRST=.TRUE. ELED1130
4 5 FORMAT('1 STRUCTURE.....',A,/) ELED1140
4 SOLUTION.....',A,/) ELED1150
4 6 FORMAT('/', ELEMENT LOADS ,// ELED1160
4 '-----',// ELED1170
4 4 IX,' LOAD ELEM GEN INC GROUP & DIRECTION VALUES....') ELED1180
C 123451234512345123451234512345678901234567890 ELED1190
C ----- ELED1200
C INPUT LOADING DATA == ELED1210
C ===== ELED1220
4 N=0 ELED1230
4 10 N=N+1 ELED1240
4 IF (N.GT.MAXELD) GO TO 100 ELED1250
4 READ (5,*) (ELD(I,N),I=1,4),TYPE,DIRI,ELD(1,N) ELED1260
4 DIR=TYPE/DIRI ELED1270
4 IF (INDEX(DIR,'END').NE.0) GO TO 100 ELED1280
4 NELD=N ELED1290
4 IF (FIRST) THEN ELED1300
4 WRITE (6,5) TITLE(1),TITLE(2) ELED1310
4 FIRST=.FALSE. ELED1320
4 ENDF ELED1330
C ----- PROCESS LOAD GROUPS AND READ ADDITIONAL INFO... ELED1340
4 IF (INDEX(DIR,'CONC').NE.0) THEN ELED1350
4 KODE=100 ELED1360
4 1 BACKSPACE(5) ELED1370
4 READ (5,*) (ELD(I,N),I=1,4),TYPE,DIRI,(ELD(I,N),I=1,J) ELED1380
4 DIR=TYPE/DIRI ELED1390
4 CALL CRNG(ELD(2,N),0,1,'DISTANCE') ELED1400
4 ELSE IF (INDEX(DIR,'UNIF').NE.0) THEN ELED1410
4 KODE=200 ELED1420
4 ELSE IF (INDEX(DIR,'VARR').NE.0) THEN ELED1430
4 J=4 ELED1440
4 BACKSPACE(5) ELED1450
4 READ (5,*) (ELD(I,N),I=1,4),TYPE,DIRI,(ELD(I,N),I=1,J) ELED1460
4 CALL CRNG(ELD(3,N),0,1,'DISTANCE') ELED1470
4 ELSE IF (INDEX(DIR,'FEM').NE.0) THEN ELED1480
4 J=12 ELED1490
4 BACKSPACE(5) ELED1500
4 READ (5,*) (ELD(I,N),I=1,4),TYPE,DIRI,(ELD(I,N),I=1,J) ELED1510
4 DIR=TYPE/DIRI ELED1520
4 ELSE ELED1530
4 WRITE(6,30) 'INVALID LOAD TYPE' ELED1540
4 GO TO 20 (ELD(I,N),I=1,4),DIR,ELD(1,N) ELED1550
4 ENDF ELED1560
4 IF (INDEX(DIR,'FX').NE.0) THEN ELED1570
4 KODE=KODE+1 ELED1580
4 ELSE IF (INDEX(DIR,'FY').NE.0) THEN ELED1590
4 KODE=KODE+2 ELED1600
4 ELSE IF (INDEX(DIR,'FZ').NE.0) THEN ELED1610
4 KODE=KODE+3 ELED1620
4 ELSE IF (INDEX(DIR,'MX').NE.0) THEN ELED1630
4 KODE=KODE+4 ELED1640
4 ELSE IF (INDEX(DIR,'MY').NE.0) THEN ELED1650
4 KODE=KODE+5 ELED1660
4 ELSE IF (INDEX(DIR,'MZ').NE.0) THEN ELED1670
4 KODE=KODE+6 ELED1680
4 ELSE IF (KODE.NE.400) THEN ELED1690
4 WRITE(6,30) 'INVALID LOAD DIRECTION' ELED1700
4 (ELD(I,N),I=1,4),DIR,(ELD(1,N),I=1,J) ELED1710
4 ENDF ELED1720
4 IELD(5,N)=KODE ELED1730
4 WRITE(6,40) (ELD(I,N),I=1,4),DIR,(ELD(1,N),I=1,J) ELED1740

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GO TO 10  
C 30 FORMAT (' .A. ' --- LOAD IS SKIPPED '  
 & IX,4I5,IX,A20,IP,6G14.6:/(42X,6G14.6) )  
 40 FORMAT (IX,4I5,IX,A20,IP,6G14.6:/(42X,6G14.6) )  
C----- RETURN IF NO ELEMENT LOADS WERE INPUT  
 100 IF (FIRST) RETURN  
C----- LOOP TO GENERATE MEMBER LOADS FOR EACH ELEMENT  
 101 IOPT=6  
 DO 101 IELNO=1,NELMT  
 FALSE=.FALSE.  
 101 CALL EBLE1B  
 (IOPT,'LSTTYP',FALSE,'IREL',FALSE,'NAME',EBSE,EPSE,DAMAGE,  
 6 DUCFAL, 6 IELNO, IELDOF,KGDOF,PGEOM,RINPUT,AXIAL,IDISP,ILOAD,MSFOR)  
C----- PRINT LOADS AT EACH DOF  
 IF (BUG) THEN  
 N5=NDOP-5  
 DO 120 L=1,NLOAD  
 IF (HEAD) WRITE (6,5) TITLE(1),TITLE(2)  
 DO 120 I=1,N5,5  
 IS=MIN(I+4,NDOP)  
 DO 110 J=I,IS  
 K=J-I  
 CX(K)=''  
 IF (J.GT.NF) CX(K)=''  
 WRITE (6,140) (J,FORCE(J,L),CX(J-I-1),J=1,15)  
 120 CONTINUE  
 ENDP  
C 130 FORMAT (' GLOBAL JOINT FORCES DUE TO ELEMENT LOADS ',3X,  
 & ' LOADING #',I5,I0X,  
 & ' NO.1 DENOTES DISPLACEMENT '  
 & '-----'  
 & 5X,5(' DOF',JX,'LOAD ' // )  
C 140 FORMAT (5X,5(O'P',I5,IP,6G14.6,A) // )  
RETURN  
END  
#PROCESS SUMP OPT(3) GOSTMT XREP  
C-----  
C 6 DEBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE  
C 6 END DEBUG  
C-----  
SUBROUTINE EBLE1  
 & IOPT, 6 UNIT, 'INC', 'FIRST', 'PRINT'  
 & NDOP, 'NLOAD', 'MAXNOD', 'NNODE', 'NCOS'  
 & ID, 'IDOF', 'COORD', 'COSINE', 'JTCOS'  
 & CONST, 'DISPL', 'FORCE', 'IREL', 'BUG'  
 & IBUG, 'ADSEL', 'KDATA', 'GRAV', 'PGEON'  
 & MAXELD, 'IELD', 'ELD', 'NAME'  
 & STEPID, 'AXIAL', 'IELNO', 'RINPUT', 'FUB'  
 & EBSE, 'EPSE', 'DAMAGE', 'DUCFAG'  
 & IELTYP, 'IELDOF', 'PKG', 'H', 'O'  
 & S1, 'S', 'S21', 'S31', 'S33', 'S35'  
 & S22, 'S2', 'S12', 'S13', 'S35'  
 & S311, 'R', 'RT', 'PT'  
 & LM, 'CTS', 'BE', 'SE'  
 & ID, 'MODEI', 'NODEJ', 'JOINTI', 'JOINTJ'  
 & REL, 'LENGTH', 'KGDOF', 'LKG', 'LMKG'  
 & ISE, 'ENGDAT', 'FMAT', 'MAT', 'SE'  
 & STIFF  
 & FEM  
C  
C 1 IMPLICIT REAL(A-H,O-3)  
C  
COMMON /RSA/ICOMN,RSAP(12)  
REAL ICOMN  
C  
LOGICAL ERR,FIRST,PRINT,BUG,FEMFLG,FALSE,AXIAL,BTEST  
CHARACTER\*80 NAME  
CHARACTER\*(\*) STPID  
CHARACTER\*6 IREL  
DIMENSION KGDATA(3)  
DIMENSION IDOF(MAXNOD,6),COORD(MAXNOD,6),COSINE(3,3,NCOS)  
DIMENSION CONST(MAXNOD,6),ID(MAXNOD),JTCOS(MAXNOD)  
DIMENSION ASAT(12,12),SAT(12,12),STIFF(156)  
DIMENSION A(12,12), S(12,12),LM(12),LMT(12),LMKG(12),FMAT(12)  
DIMENSION M(12,12), S(12,12),LM(12),LMT(12),LMKG(12),FMAT(144)  
DIMENSION CTS( 3, 3),CTE( 3, 3),COSLOC(3,3),VK(3),VT(3),VZ(3)  
DIMENSION BS( 3, 3),BE( 3, 3),SE(15),ENGDA(2)  
DIMENSION R(12),DISP(NDOP,NLOAD),F(12),RT(12),PT(12),FUB(NDOP)  
DIMENSION FORCE(NDOP,NLOAD),IELD(5,MAXELD),ELD(12,MAXELD)  
DIMENSION ASA(12,12),RINPUT(100),FEM(12,NLOAD),GFEM(12)  
DIMENSION DUCT(3),EXCR(6)  
DIMENSION V(12),VL(12)  
REAL LENGTH  
REAL IX,IY,IZ,K112,KJ22,KI22,KI11,KJ11,KI11,KT1,TYJ  
INTEGER REL,REL2  
DIMENSION MD(13)  
DATA MD/1,2,4,7,11,16,22,29,37,46,56,67,79/  
C  
C VARIABLES:  
C-----  
C GLOBAL VARIABLES  
C IOPT = INTERNAL BEAM COLUMN ELEMENT  
C = 2. CALCULATE GEOMETRIC STIFFNESS  
C = 3. FORM STIFFNESS  
C = 4. CALCULATE INCREMENTAL FORCES  
C = 5. CALCULATE TOTAL FORCES AND ENERGIES  
C IUNIT = OUTPUT FILE UNIT # FOR PRINTING OUTPUT  
C ERR = ERR OR FLAG, IF AN ERR OR HAS OCCURED, ERR=.TRUE.  
C FIRST = FLAG, FIRST=.TRUE., PRINT HEADERS  
C PRINT = FLAG, PRINT=.TRUE., PRINT DATA  
C NDOP = # OF GLOBAL DOF  
C NLOAD = # OF LOAD COMBINATIONS  
C MAXNOD = # ROW DIMENSION OF NODE ARRAY  
C NNODE = # OF NODES  
C ID = ARRAY OF EXTERNAL NODE NUMBERS  
C IDOF = ARRAY OF DEGREES OF FREEDOM  
C COORD = ARRAY OF COORDINATES  
C COSINE = ARRAY OF DIRECTION COSINES OF NODES  
C CONST = ARRAY OF CONSTRAINT TRANSFORMATIONS  
C DISPL = GLOBAL DISPLACEMENT MATRIX  
C ISMAT = LOCATION OF MATERIAL DATA IN GLOBAL STORAGE VECTOR  
C NAME = ELEMENT NAME  
C IELNO = ELEMENT NUMBER  
C RINPUT = INPUT DATA  
C STIFF = OUTPUT ELEMENT STIFFNESS (UPPER TRIANGULAR FORM)  
C LM = OUTPUT LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX  
C-----  
C ELEMENT VARIABLES  
C K,Q,AI,AJ = 8,3,52,6,6,212,S33,S35,S311,D  
C = INDIVIDUAL TERMS IN THE ELEMENT STIFFNESS (S) MATRIX  
C R = INPUT LOAD FOR FORMING GEOMETRIC STIFFNESS MATRIX...  
C INCR = INCREMENTAL DISPLACEMENTS  
C RT = TOTAL DISPLACEMENTS  
C F = INCREMENTAL FORCES  
C FT = TOTAL FORCES  
C LM = LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX  
C CLM = LOCAL TO GLOBAL ROTATION MATRIX START  
C CTE = LOCAL TO GLOBAL ROTATION MATRIX END  
C BS = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT  
C BE = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT  
C CTE = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT  
C STIFF = ELEMENT STIFFNESS  
C IELTYP = ELEMENT TYPE = 10  
C NODEI = EXTERNAL JOINT NUMBER, END I  
C NODEJ = EXTERNAL JOINT NUMBER, END J

ELE00950 C JOINTI = INTERNAL JOINT NUMBER, END I  
ELE00960 C JOINTJ = INTERNAL JOINT NUMBER, END J  
ELE00970 C----- TEMPORARY VARIABLES  
ELE00980 C VX = VECTOR DEFINING THE ELEMENT X AXIS  
ELE00990 C VY = VECTOR DEFINING THE ELEMENT Y AXIS  
ELE01000 C COSLOC = LOCAL COSINE MATRIX  
ELE01010 C A = ELEMENT STIFFNESS TRANSFORMATION MATRIX  
ELE01020 C S = ELEMENT STIFFNESS AT LOCAL COORD. AT ENDS OF FLEXIBLE PART  
ELE01030 C SAT = PRODUCT OF S\*A  
ELE01040 C ASAT = PRODUCT OF A\*SAT  
ELE01050  
ELE01060  
ELE01070 C----- CHOOSE OPTION  
ELE01080 IF (IOPT.LT.1 .OR. IOPT.GT.14)  
ELE01090 & WRITE (6,\*) 'INVALID OPTION IN EBLE-1. IOPT=' ,IOPT  
ELE01100  
ELE01110 GO TO (100,200,300,400,500,600,700,800,900,1000,1100,1200,  
ELE01120 & 1300,1400),IOPT  
ELE01130  
ELE01140 C 100 CONTINUE  
ELE01150  
ELE01160 C----- ZERO STRAIN ENERGY  
ELE01170 ENGDAT(1) = 0  
ELE01180 ENGDAT(2) = 0  
ELE01190  
ELE01200 C  
ELE01210 IF ( BTEST( IBUG,7 ) ) THEN  
ELE01220 WRITE (6,\*) NAME  
ELE01230 CALL WHMATRX(RINPUT,10,1,'RINPUT')  
ELE01240 ENDP  
ELE01250  
ELE01260 C----- INTERPRETATE INPUT DATA  
ELE01270 IELTYP = 1  
ELE01280 MAT = RINPUT( 1 )  
ELE01290 NODEI = RINPUT( 2 )  
ELE01300 NODEJ = RINPUT( 3 )  
ELE01310 VT(1) = RINPUT( 4 )  
ELE01320 VT(2) = RINPUT( 5 )  
ELE01330 VT(3) = RINPUT( 6 )  
ELE01340 XS = RINPUT( 7 )  
ELE01350 XE = RINPUT( 8 )  
ELE01360 PKG = RINPUT( 9 )  
ELE01370 REL2 = RINPUT(10)  
ELE01380  
ELE01390 C  
ELE01400 C----- GET INTERNAL NODE # ASSOCIATED WITH EXTERNAL NODE #'S  
ELE01410 JOINTI = QUICK(NODEI, ID, NNODE)  
ELE01420 JOINTJ = QUICK(NODEJ, ID, NNODE)  
ELE01430 WRITE (6,\*) 'ID', ID  
ELE01440  
ELE01450 C  
ELE01460 C----- PRINT DATA FOR PLOTTING  
ELE01470 IF ( BTEST( IBUG,2 ) ) THEN  
ELE01480 WRITE(7,10) NODEJ, NODEI,  
ELE01490 & {COORD(JOINTJ,I),I=1,3}, {COORD(JOINTJ,J),J=1,3}  
ELE01500 10 FORMAT (15,I5,15,IP,6G12.4)  
ELE01510 ENDP  
ELE01520  
ELE01530 C  
ELE01540 C GET GLOBAL TO LOCAL TRANSFORMATION MATRIX  
ELE01550  
ELE01560 C----- DEFINE VECTOR X, LENGTH  
ELE01570 VX(1)=COORD(JOINTJ,1)-COORD(JOINTI,1)  
ELE01580 VX(2)=COORD(JOINTJ,2)-COORD(JOINTI,2)  
ELE01590 VX(3)=COORD(JOINTJ,3)-COORD(JOINTI,3)  
ELE01600 LENGTH=SQRT(VX(1)\*\*2+VX(2)\*\*2+VX(3)\*\*2)-XS-XE  
ELE01610 IF (LENGTH.LE.0) THEN  
ELE01620 WRITE(6,101) IELNO,JOINTI,JOINTJ,LENGTH  
ELE01630 101 FORMAT(IX,20(' '),ERR,RCR - LENGTH IS LE 0,/  
ELE01640 & 'X', 'Y', 'Z', 'ID', 'JOINTI', 'I', '5, 'X', 'JOINTJ', 'J', '5,  
ELE01650 & 5X, 'LENGTH', 'IP, 615.6/  
ELE01660 & 5X, 'REVISE INPUT', .....// )  
ELE01670 LENGTH=1.0  
ELE01680 ENDP  
ELE01690 IF ( BTEST( IBUG,7 ) ) THEN  
ELE01700 WRITE (6,\*) 'JOINTS',JOINTI,JOINTJ  
ELE01710 WRITE (6,\*) 'VX', VX  
ELE01720 WRITE (6,\*) 'VT', VT  
ELE01730 WRITE (6,\*) 'LENGTH', LENGTH  
ELE01740 ENDP  
ELE01750 C----- GET LOCAL TO GLOBAL TRANSFORMATION MATRICES CT AND B  
ELE01760 ICS=JTCOS(JOINTI)  
ELE01770 CALL ROTXYZ(VX,VT,COSINE(1,1,ICS),CTS, XS, 0.,0.,BS,  
ELE01780 & CONST(JOINTI,1),MAXNOD)  
ELE01790 CALL ROTXYZ(VX,VT,COSINE(1,1,ICE),CTE,-XE,0.,0.,BE,  
ELE01800 & CONST(JOINTJ,1),MAXNOD)  
ELE01810 C  
ELE01820 C IF ( BTEST( IBUG,7 ) ) THEN  
ELE01830 WRITE (6,\*) 'LENGTH', LENGTH  
ELE01840 IF ( BTEST( IBUG,7 ) ) THEN  
ELE01850 CALL WHMATRX ( CTS , 3, 3, 'CTS '  
ELE01860 CALL WHMATRX ( CTE , 3, 3, 'CTE '  
ELE01870 ENDP  
ELE01880  
ELE01890 C  
ELE01900 C----- GET GLOBAL CONSTRAINT MATRIX BS, BE  
ELE01910 BS(1,1)=CONST(JOINTI,1)\*CTS(2,1)+CONST(JOINTI,2)\*CTS(3,1)+BS(1,1)  
ELE01920 BS(2,1)=CONST(JOINTI,1)\*CTS(2,2)+CONST(JOINTI,2)\*CTS(3,2)+BS(2,1)  
ELE01930 BS(3,1)=CONST(JOINTI,1)\*CTS(2,3)+CONST(JOINTI,2)\*CTS(3,3)+BS(3,1)  
ELE01940 BS(2,2)=CONST(JOINTI,3)\*CTS(1,1)+CONST(JOINTI,4)\*CTS(3,1)+BS(2,2)  
ELE01950 BS(3,2)=CONST(JOINTI,3)\*CTS(1,2)+CONST(JOINTI,4)\*CTS(3,2)+BS(2,3)  
ELE01960 BS(3,3)=CONST(JOINTI,5)\*CTS(1,1)+CONST(JOINTI,6)\*CTS(2,1)+BS(3,3)  
ELE01970 BS(3,2)=CONST(JOINTI,5)\*CTS(1,2)+CONST(JOINTI,6)\*CTS(2,2)+BS(3,2)  
ELE01980 BS(3,3)=CONST(JOINTI,5)\*CTS(1,3)+CONST(JOINTI,6)\*CTS(2,3)+BS(3,3)  
ELE01990 C  
ELE02000 BS(1,2)=CONST(JOINTJ,1)\*CTE(2,1)+CONST(JOINTJ,2)\*CTE(3,1)+BE(1,1)  
ELE02010 BE(1,2)=CONST(JOINTJ,1)\*CTE(2,2)+CONST(JOINTJ,2)\*CTE(3,2)+BE(1,2)  
ELE02020 BE(1,3)=CONST(JOINTJ,1)\*CTE(2,3)+CONST(JOINTJ,2)\*CTE(3,3)+BE(1,3)  
ELE02030 BE(2,2)=CONST(JOINTJ,3)\*CTE(1,1)+CONST(JOINTJ,4)\*CTE(3,1)+BE(2,2)  
ELE02040 BE(2,3)=CONST(JOINTJ,3)\*CTE(1,2)+CONST(JOINTJ,4)\*CTE(3,2)+BE(2,3)  
ELE02050 BE(2,3)=CONST(JOINTJ,3)\*CTE(1,3)+CONST(JOINTJ,4)\*CTE(3,3)+BE(2,3)  
ELE02060 BE(3,1)=CONST(JOINTJ,5)\*CTE(1,1)+CONST(JOINTJ,6)\*CTE(2,1)+BE(3,1)  
ELE02070 BE(3,2)=CONST(JOINTJ,5)\*CTE(1,2)+CONST(JOINTJ,6)\*CTE(2,2)+BE(3,2)  
ELE02080 BE(3,3)=CONST(JOINTJ,5)\*CTE(1,3)+CONST(JOINTJ,6)\*CTE(2,3)+BE(3,3)  
ELE02090  
ELE02100 C  
ELE02110 IF ( BTEST( IBUG,7 ) ) THEN  
ELE02120 CALL WHMATRX(BS, 3, 3, 'BS '  
ELE02130 CALL WHMATRX(BE, 3, 3, 'BE '  
ELE02140 ENDP  
ELE02150  
ELE02160 C----- ZERO MATRICES  
ELE02170 DO 40 I=1,12  
ELE02180 FMAT(I)=0  
ELE02190 F(I)=0  
ELE02200 FT(I)=0  
ELE02210 R(I)=0  
ELE02220 RT(I)=0  
ELE02230 DO 40 J=1,12  
ELE02240 S(I,J)=0.  
ELE02250 A(I,J)=0.  
ELE02260  
ELE02270 C  
ELE02280 C----- ASSEMBLE TRANSFORMATION MATRIX A  
ELE02290 DO 50 I=1,3  
ELE02300 DO 50 J=1,3  
ELE02310 A(I+J,3)=CTS(I,J)  
ELE02320 A(I+J,6)=CTE(I,J)  
ELE02330 A(I+J,9)=CTE(I,J)  
ELE02340 A(I+J,3)+BE(I,J)  
ELE02350 A(I+J,6)+BE(I,J)  
ELE02360 IF ( BTEST( IBUG,7 ) ) CALL WHMATRX ( A , 12,12, ' A '  
ELE02370  
ELE02380 C  
ELE02390 C----- GET MATERIAL PROPERTIES  
ELE02400 FALSE=.FALSE.  
ELE02410 LSTTYP=0  
ELE02420 CALL MATLIB  
ELE02430

```

4 ( 2 ,LSTTYP ,FALSE ,IREL ,FALSE ,NAME ,SESE ,EPSE ,DUCT ,EXCR. ELE02420
4 FT ,F ,RT ,R ,V ,VL ,LELEM ,LMAT , ELE02430
4 MAT ,MATTP ,SE ,IELNO ,DAMAGE) ELE02440
C
IF (MATTP,NE.1) THEN ELE02450
WRITE (6,190) IELNO,MATTP,NODEI,NODEJ ELE02460
190 FORMAT (' ELEMENT',I6,' INCOMPATIBLE MATERIAL PROP.#',I5, ELE02470
4 ' START JOINT',I5,' END JOINT',I5, ELE02480
4 ' THE ELEMENT IS REJECTED AND ER, FOR CHECKING CONTINUES') ELE02490
ERR=.TRUE. ELE02510
GO TO 191 ELE02520
ENDIF ELE02530
C EAX =SE( 1) ELE02540
GAY =SE( 2) ELE02550
GAZ =SE( 3) ELE02560
GIX =SE( 4) ELE02570
GIT =SE( 5) ELE02580
GIJ =SE( 6) ELE02590
ELE02600
C-----
C DETERMINE THE MEMBER RELEASE CODES
C-----
RELT IS AN INPUT MEMBER RELEASE CODE ELE02610
REL IS THE INTERNAL MEMBER RELEASE CODE ELE02620
BIT 6 AXIAL TRUE = AXIAL DEFORMATION CONSIDERED ELE02630
BIT 5 NX AT END I ELE02640
BIT 4 MY AT END I ELE02650
BIT 3 MZ AT END I ELE02660
BIT 2 NX AT END J ELE02670
BIT 1 MY AT END J ELE02680
BIT 0 MZ AT END J ELE02690
C-----
IF (BTST(IBUG,7)) WRITE (6,*) 'RELT',RELT ELE02700
REL=0 ELE02710
IF (MOD(RELT,10),NE.0) THEN ELE02720
REL=IBSET(REL,0) ELE02730
IRELC(5:6)='2' ELE02740
ENDIF ELE02750
RELT=RELT/10 ELE02760
IF (MOD(RELT,10),NE.0) THEN ELE02770
REL=IBSET(REL,1) ELE02780
IRELC(5:5)='Y' ELE02790
ENDIF ELE02800
RELT=RELT/10 ELE02810
IF (MOD(RELT,10),NE.0) THEN ELE02820
REL=IBSET(REL,2) ELE02830
IRELC(4:4)='X' ELE02840
ENDIF ELE02850
RELT=RELT/10 ELE02860
IF (MOD(RELT,10),NE.0) THEN ELE02870
REL=IBSET(REL,3) ELE02880
IRELC(3:3)='Z' ELE02890
ENDIF ELE02900
RELT=RELT/10 ELE02910
IF (MOD(RELT,10),NE.0) THEN ELE02920
REL=IBSET(REL,4) ELE02930
IRELC(2:2)='Y' ELE02940
ENDIF ELE02950
RELT=RELT/10 ELE02960
IF (MOD(RELT,10),NE.0) THEN ELE02970
REL=IBSET(REL,5) ELE02980
IRELC(1:1)='X' ELE02990
ENDIF ELE03000
C----- SET STIFFNESS COEFFICIENTS...
C----- SET CONSISTENT MASS GEOMETRIC STIFFNESS COEFFICIENTS...
A2=0 ELE03010
B2=0 ELE03020
C2=0 ELE03030
D2=0 ELE03040
E2=0 ELE03050
F2=1.0 / LENGTH ELE03060
C----- BOTH ENDS RELEASED, Z-AXIS
IF (BTST(REL,0) .AND. BTST(REL,3)) THEN
KI2=0 ELE03070
KJ2=0 ELE03080
KJ3=0 ELE03090
B2= 0.20 * LENGTH ELE03100
E2= 0.20 ELE03110
F2= 1.20 / LENGTH ELE03120
C----- END END RELEASED, Z-AXIS
ELSE IF (BTST(REL,3)) THEN
KI2=0 ELE03130
KJ2=0 ELE03140
KJ3=0 ELE03150
B2= 0.20 * LENGTH ELE03160
E2= 0.20 ELE03170
F2= 1.20 / LENGTH ELE03180
C----- BOTH ENDS RELEASED, Z-AXIS
ELSE IF (BTST(REL,0)) THEN
KI2=3 ELE03190
KJ2=0 ELE03200
KJ3=0 ELE03210
B2= 0.20 * LENGTH ELE03220
E2= 0.20 ELE03230
F2= 1.20 / LENGTH ELE03240
C----- BOTH ENDS RELEASED, Z-AXIS
ELSE IF (BTST(REL,4)) THEN
KI2=4 ELE03250
KJ2=4 ELE03260
KJ3=2 ELE03270
B2= 2 * LENGTH / 15. ELE03280
B3= A2 ELE03290
C2= LENGTH / 30. ELE03300
D2= 0.10 ELE03310
E2= 0.10 ELE03320
F2= 1.2 / LENGTH ELE03330
ENDIF ELE03340
C
AT=0 ELE03350
BT=0 ELE03360
CT=0 ELE03370
DT=0 ELE03380
ET=0 ELE03390
FT=1.0 / LENGTH ELE03400
C----- BOTH ENDS RELEASED, Y-AXIS
IF (BTST(REL,1) .AND. BTST(REL,4)) THEN
KI1=0 ELE03410
KJ1=0 ELE03420
KJ2=0 ELE03430
B1= 0.20 * LENGTH ELE03440
E1= 0.20 ELE03450
F1= 1.20 / LENGTH ELE03460
C----- END END RELEASED, Y-AXIS
ELSE IF (BTST(REL,1)) THEN
KI1=0 ELE03470
KJ1=0 ELE03480
KJ2=0 ELE03490
B1= 0.20 * LENGTH ELE03500
E1= 0.20 ELE03510
F1= 1.20 / LENGTH ELE03520
C----- BOTH ENDS RELEASED, Z-AXIS
ELSE
KI1=4 ELE03530
KJ1=4 ELE03540
KJ2=2 ELE03550
B1= 2 * LENGTH / 15. ELE03560
B2= A1 ELE03570
C1= LENGTH / 30. ELE03580
D1= 0.10 ELE03590
E1= 0.10 ELE03600
F1= 1.2 / LENGTH ELE03610
ENDIF ELE03620
C
AT=0 ELE03630
BT=0 ELE03640
CT=0 ELE03650
DT=0 ELE03660
ET=0 ELE03670
FT=1.0 / LENGTH ELE03680
C----- BOTH ENDS RELEASED, Z-AXIS
ELSE
KI1=4 ELE03690
KJ1=4 ELE03700
KJ2=2 ELE03710
B1= 2 * LENGTH / 15. ELE03720
B2= A1 ELE03730
C1= LENGTH / 30. ELE03740
D1= 0.10 ELE03750
E1= 0.10 ELE03760
F1= 1.2 / LENGTH ELE03770
ENDIF ELE03780
C
AT=0 ELE03790
BT=0 ELE03800
CT=0 ELE03810
DT=0 ELE03820
ET=0 ELE03830
FT=1.2 / LENGTH ELE03840
C-----
DY= 0.10 ELE03840
DX= 0.10 ELE03850
BT= 1.2 / LENGTH ELE03860
ENDIF ELE03870
C
IF (BTST(REL,2) .AND. BTST(REL,5)) THEN ELE03880
KTI =0 ELE03890
KTJ =0 ELE03900
ELSE IF (BTST(REL,5)) THEN ELE03910
KTI =0 ELE03920
KTJ =1 ELE03930
ELSE IF (BTST(REL,2)) THEN ELE03940
KTI =1 ELE03950
KTJ =0 ELE03960
ELSE ELE03970
KTI =1 ELE03980
KTJ =1 ELE03990
ENDIF ELE04000
C
IF ( BTST(IBUG,7) ) WRITE (6,*) 'IRELC',IRELC,'REL',REL ELE04010
C
C ASSEMBLE LOCAL STIFFNESS MATRIX
C-----
H=EAX/LENGTH ELE04020
H1=C14/LENGTH ELE04030
AJ=K1J2*E13/LENGTH ELE04040
AJ=KJ2*E13/LENGTH ELE04050
C=K1J2*E12/LENGTH ELE04060
B1=K1T1*E17/LENGTH ELE04070
B1=K1T1*E17/LENGTH ELE04080
D=K1J1*E17/LENGTH ELE04090
S22=(A1+2*C*AJ)/(LENGTH**2) ELE04100
S26=(A1+C)/(LENGTH) ELE04110
S212=(S22-C*AJ)/(LENGTH) ELE04120
S33=(B1+2*D*BJ)/(LENGTH**2) ELE04130
S35=(B1+D)/(LENGTH) ELE04140
S311=(D*BJ)/(LENGTH) ELE04150
S( 1, 1) =H ELE04160
S( 2, 2) =S22 ELE04170
S( 2, 6) =S26 ELE04180
S( 2, 9) =S22 ELE04190
S( 2, 12) =S212 ELE04200
S( 3, 3) =S33 ELE04210
S( 3, 5) =S35 ELE04220
S( 3, 9) =S33 ELE04230
S( 3, 11) =S( 1, 11) ELE04240
S( 4, 4) =Q * KTI ELE04250
S( 4, 10) =-Q ELE04260
S( 5, 5) =B1 ELE04270
S( 5, 9) =S35 ELE04280
S( 5, 11) =D ELE04290
S( 6, 6) =A1 ELE04300
S( 6, 8) =-S26 ELE04310
S( 6, 12) =C ELE04320
S( 7, 7) =H ELE04330
S( 8, 8) =S22 ELE04340
S( 8, 12) =-S212 ELE04350
S( 9, 9) =S33 ELE04360
S( 9, 11) =S311 ELE04370
S( 10, 10) =Q * KTJ ELE04380
S( 11, 11) =B1 ELE04390
S( 12, 12) =AJ ELE04400
S( 5, 3) =S( 3, 5) ELE04410
S( 6, 2) =S( 2, 6) ELE04420
S( 7, 1) =S( 1, 7) ELE04430
S( 8, 2) =S( 2, 8) ELE04440
S( 8, 9) =S( 9, 8) ELE04450
S( 9, 3) =S( 3, 9) ELE04460
S( 9, 5) =S( 5, 9) ELE04470
S( 10, 4) =S( 4, 10) ELE04480
S( 11, 3) =S( 3, 11) ELE04490
S( 11, 5) =S( 5, 11) ELE04500
S( 11, 9) =S( 9, 11) ELE04510
S( 12, 2) =S( 2, 12) ELE04520
S( 12, 6) =S( 6, 12) ELE04530
S( 12, 8) =S( 8, 12) ELE04540
C IF (FIRST) CALL WMATRIX ( S ,12,12,' S ') ELE04550
C----- CALCULATE SAT ELE04560
DO 60 I=1,12 ELE04570
DO 60 J=1,12 ELE04580
SAT(I,J)=0 ELE04590
DO 60 K=1,12 ELE04600
SAT(I,J)=SAT(I,J)+S(I,K)*A(J,K) ELE04610
C----- CALCULATE ASAT ELE04620
DO 70 I=1,12 ELE04630
DO 70 J=1,12 ELE04640
ASAT(I,J)=0 ELE04650
DO 70 K=1,12 ELE04660
ASAT(I,J)=ASAT(I,J)+A(I,K)*SAT(K,J) ELE04670
C IF ( BTST(IBUG,8) ) CALL WMATRIX(ASAT ,12,12,' ASAT ') ELE04680
C----- DEGREES OF FREEDOM ELE04690
C REMOVE DOP'S THAT ARE CONSTRAINED TO THE SAME DOP... ELE04700
DO 75 I=1,6 ELE04710
IF ( IDOF(JOINTI,I),NE.IDOF(JOINTJ,I) ) THEN ELE04720
LMT(I+6)=IDOF(JOINTI,I) ELE04730
LMT(I+6)=IDOF(JOINTJ,I) ELE04740
ELSE ELE04750
LMT(I) =0 ELE04760
LMT(I+6) =0 ELE04770
ENDIF ELE04780
75 CONTINUE ELE04790
C----- CONVERT TO HALF STORAGE MODE BY COLUMNS ELE04800
C
1 3 6 10 15 21 28 36 45 55 66 78 NUMBER INDICATES ELE04810
2 5 9 14 20 27 35 44 54 65 77 LOCATION OF DATA ELE04820
4 8 13 19 26 34 43 53 64 76 IN ARRAY STIFF ELE04830
7 12 18 25 33 42 52 63 75 ELE04840
11 17 24 32 41 51 62 74 ELE04850
16 23 31 40 50 61 73 ELE04860
22 30 39 49 60 72 ELE04870
29 38 48 59 71 ELE04880
37 47 58 70 ELE04890
46 57 69 ELE04900
56 68 ELE04910
67 ELE04920
C
L=0 ELE04930
IELDOF=0 ELE04940
DO I=1,12 ELE04950
IF (LMT(I),EQ.0 .OR. ASAT(I,I),LE.0) GO TO 90 ELE04960
IELDOF=IELDOF+1 ELE04970
LMT(IELDOF)=LMT(I) ELE04980
DO 80 J=1,12 ELE04990
IF (LMT(J),EQ.0 .OR. ASAT(J,J),LE.0) GO TO 80 ELE05000
L=L+1 ELE05010
STIFF(L)=ASAT(I,J) ELE05020
80 CONTINUE ELE05030
90 CONTINUE ELE05040
IF ( BTST(IBUG,7) ) THEN ELE05050
WRITE (6,*) 'LMT=' ,LMT ELE05060
WRITE (6,*) 'IELDOF',IELDOF,'LM =',(LMT(I),I=1,IELDOF) ELE05070
CALL LMATRIX(STIFF,MD,IELDOF,(L+1),'ELEMENT STIFFNESS') ELE05080
WRITE (6,*) 'KGDATA1',KGDATA,' PKG',PKG ELE05090
ENDIF ELE05100
C-----
C ASSEMBLE GEOMETRIC STIFFNESS MATRIX FOR A UNIT LOAD... ELE05110
ELE05120
ELE05130
ELE05140
ELE05150
ELE05160
ELE05170
ELE05180
ELE05190
ELE05200
ELE05210
ELE05220
ELE05230
ELE05240
ELE05250

```

C-----  
LKG=L  
IF ( PKG.EQ.0 .AND. KGDAT(1).NE.2 ) THEN  
AXIAL=.FALSE.  
REL=IBCLR(REL,6)  
GO TO 180  
ENDIF  
C-----  
C----- DETERMINE IF AXIAL DEFORMATION IS CONSIDERED  
BIT #6 OF REL IS TRUE IF AXIAL DEFORMATION IS CONSIDERED  
DO 999 I=1,3  
CS=CTS(I,1)  
CE=CTE(I,1)  
IF (ABS(CE).LE..0001 .AND. ABS(CE).LE..0001 ) GO TO 999  
IS=IDOF(JOINT,I)  
IE=IDOF(JOINT,J)  
IF (IS.EQ.IE) GO TO 999  
REL=IBSET(REL,6)  
999 CONTINUE  
C-----  
AXIAL=BTEST(REL,6)  
IF ( BTEST(IBUG,7) ) WRITE (6,\*) 'AXIAL',AXIAL  
C-----  
IF (KGDAT(1).NE.0 .AND. KGDAT(2).NE.0 .AND. BTEST(REL,6) ) THEN  
C----- DETERMINE THE AXIAL LOAD TO BE USED FOR KG...  
IF (KGDAT(1).EQ.1) THEN  
PGEOM = PKG  
ELSE IF (KGDAT(1).EQ.2) THEN  
PGEOM = 0.50\*(F(1)-F(7))  
ENDIF  
C-----ASSEMBLE LOCAL GEOMETRIC STIFFNESS MATRIX FOR A UNIT LOAD...  
ZERO LOCAL KG MATRIX  
DO 998 I=1,12  
DO 998 J=1,12  
S(I,J)=0  
998 CONTINUE  
C----- LUMPED MASS FORM OF GEOMETRIC STIFFNESS  
IF (KGDAT(2).EQ.1) THEN  
F=1/LENGTH  
S( 2, 2)= F1  
S( 2, 8)= -F1  
S( 8, 2)= -F1  
S( 8, 8)= F1  
S( 3, 3)= F1  
S( 3, 9)= -F1  
S( 9, 3)= -F1  
S( 9, 9)= F1  
C----- CONSISTENT MASS FORM OF GEOMETRIC STIFFNESS  
ELSE IF (KGDAT(2).EQ.2) THEN  
S( 2, 2)= D2  
S( 2, 8)= -D2  
S( 8, 2)= -D2  
S( 8, 8)= D2  
S( 2, 12)= E2  
S( 6, 2)= D2  
S( 6, 6)= A2  
S( 6, 8)= -D2  
S( 6, 12)= C2  
S( 8, 2)= -F2  
S( 8, 6)= -D2  
S( 8, 8)= F2  
S( 8, 12)= -E2  
S( 12, 2)= E2  
S( 12, 6)= -C2  
S( 12, 8)= -E2  
S( 12, 12)= B2  
C-----  
S( 3, 3)= F2  
S( 3, 5)= -D2  
S( 3, 9)= -F2  
S( 3, 11)= -E2  
S( 5, 3)= -D2  
S( 5, 5)= D2  
S( 5, 9)= D2  
S( 5, 11)= -C2  
S( 9, 3)= -F2  
S( 9, 5)= D2  
S( 9, 9)= F2  
S( 9, 11)= E2  
S( 11, 3)= -E2  
S( 11, 5)= -C2  
S( 11, 9)= E2  
S( 11, 11)= B2  
ENDIF  
IF (BTEST(IBUG,8)) CALL WMATRX ( S ,12,12, 'KG -LOCAL COORD ' )  
C----- CALCULATE SAT  
DO 1060 I=1,12  
DO 1060 J=1,12  
SAT(I,J)=0  
DO 1060 K=1,12  
SAT(I,J)=SAT(I,J)+S(I,K)\*A(K,J)  
1060 CONTINUE  
C----- CALCULATE ASAT  
DO 1070 I=1,12  
DO 1070 J=1,12  
ASAT(I,J)=0  
DO 1070 K=1,12  
ASAT(I,J)=ASAT(I,J)+S(I,K)\*A(K,J)  
1070 CONTINUE  
IF (BTEST(IBUG,9)) CALL WMATRX (ASAT ,12,12, 'A\*KG\*AT ' )  
C----- CONVERT TO HALF STORAGE MODE BY COLUMNS  
KGDOP=0  
DO 1090 I=1,12  
IF (LMT(I).EQ.0 .OR. ASAT(I,I).LE.0) GO TO 1090  
KGDOP=KGDOP+LMT(I)  
LWKG(KGDOP)=LMT(I)  
DO 1080 J=1,12  
IF (LMT(J).EQ.0 .OR. ASAT(J,J).LE.0) GO TO 1080  
L=L+1  
STIFF(L)=ASAT(I,J)  
1080 CONTINUE  
1090 CONTINUE  
IF ( BTEST(IBUG,7) ) THEN  
WRITE (6,\*) 'KGDOP',KGDOP, 'LWKG', (LWKG(I),I=1,KGDOP)  
CALL LWMTX(STIFF(LWKG),MD,KGDOP,(L-LWG))  
'ELEMENT GEOMETRIC STIFFNESS (UNIT LOAD)'  
ENDIF  
C-----  
180 IREL=156-L  
C-----  
PRINT COLUMN DATA  
IF (FIRST .OR. BTEST(IBUG,7) .OR. BTEST(IBUG,8) ) WRITE (6,192)  
191 WRITE(6,193) NAME,IELNO,MAT,NODEI,NODEJ,IREL,LENGTH,  
& VY(1),VT(2),VT(3),XS,-XE,PKG  
C-----  
192 FORMAT(' ELEMENT NO, 3D BEAM ELEMENT //  
& T22, # MATL START END, ' REL CD',3X, 'LENGTH',3X,  
& 11(' '), ' Y-AXIS '  
& 12(' '), ' START DIST END DIST PKG')  
193 FORMAT(1X,A15,4(16,2X,A,1P,G12.4),  
& 4(' '), ' I',SP,F9.5, ' J',F9.5, ' K',1P,SS,3G12.4)  
RETURN  
C----- DETERMINE GEOMETRIC STIFF  
300 CONTINUE  
C----- KG IS DETERMINED FOR A UNIT LOAD WITH IOPT=1,  
C----- THE ACTUAL LOAD (PGEOM) IS DETERMINED HERE.  
C----- KG IS MULTIPLIED BY PGEOM WHEN THE TOTAL STIFFNESS IS ASSEMBLED  
ELEM2660  
ELEM2670  
ELEM2680  
ELEM2690  
ELEM2700  
ELEM2710  
ELEM2720  
ELEM2730  
ELEM2740  
ELEM2750  
ELEM2760  
ELEM2770  
ELEM2780  
ELEM2790  
ELEM2800  
ELEM2810  
ELEM2820  
ELEM2830  
ELEM2840  
ELEM2850  
ELEM2860  
ELEM2870  
ELEM2880  
ELEM2890  
ELEM2900  
ELEM2910  
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ELEM2930  
ELEM2940  
ELEM2950  
ELEM2960  
ELEM2970  
ELEM2980  
ELEM2990  
ELEM3000  
ELEM3010  
ELEM3020  
ELEM3030  
ELEM3040  
ELEM3050  
ELEM3060  
ELEM3070  
ELEM3080  
ELEM3090  
ELEM3100  
ELEM3110  
ELEM3120  
ELEM3130  
ELEM3140  
ELEM3150  
ELEM3160  
ELEM3170  
ELEM3180  
ELEM3190  
ELEM3200  
ELEM3210  
ELEM3220  
ELEM3230  
ELEM3240  
ELEM3250  
ELEM3260  
ELEM3270  
ELEM3280  
ELEM3290  
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ELEM3890  
ELEM3900  
ELEM3910  
ELEM3920  
ELEM3930  
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ELEM3950  
ELEM3960  
ELEM3970  
ELEM3980  
ELEM3990  
ELEM4000  
ELEM4010  
ELEM4020  
ELEM4030  
ELEM4040  
ELEM4050  
ELEM4060  
ELEM4070  
ELEM4080  
ELEM4090  
ELEM4100  
ELEM4110  
ELEM4120  
ELEM4130  
ELEM4140  
ELEM4150  
ELEM4160  
ELEM4170  
ELEM4180  
ELEM4190  
ELEM4200  
ELEM4210  
ELEM4220  
ELEM4230  
ELEM4240  
ELEM4250  
ELEM4260  
ELEM4270  
ELEM4280  
ELEM4290  
ELEM4300  
ELEM4310  
ELEM4320  
ELEM4330  
ELEM4340  
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ELEM4370  
ELEM4380  
ELEM4390  
ELEM4400  
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ELEM4990  
ELEM5000  
ELEM5010  
ELEM5020  
ELEM5030  
ELEM5040  
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ELEM5060  
ELEM5070  
ELEM5080  
ELEM5090  
ELEM5100  
ELEM5110  
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C
DO 512 I=1,12
  IF ( ABS(FI(I)).GT.ABS(FMAX(I*12-12+1)) ) THEN
    DO 511 J=0,11
      FMAX(I*12-11+J)=FI(I)+J
    ENDDF
511 CONTINUE
ELSE IF (ICOMN.EQ.1.OR.ICOMN.EQ.2) THEN
  DO 513 I=1,12
    FI(I)=RSAP(I)
  ENDDF
513 CONTINUE
520 FORMAT (' I=',I3,' R=',I3,' P,G12.4,' R',I3,' G12.4,'
  ' F',I3,' G12.4,' FT',I3,' G12.4')
LOAD=1
IF (BTST(I,BUG,0).AND.PRINT)
  & WRITE (6,493) IELNO,LOAD,NODEI,(RT(J),J=1,6),
  & IF ( PRINT) WRITE (6,492) IELNO,LOAD,NODEJ,(FT(J),J=1,6),
  & NODEJ,(FT(J),J=7,12)
RETURN
C----- DETERMINE FIXED END FORCES. AND ADD TO FORCE=
600 IF (MAXELD.LE.0) RETURN
C----- CALL BMLQA TO DETERMINE THE FIXED END FORCES
CALL BMLQA(IELENO,FEMFLG,LENTH,REL,MAXELD,NELD,NLOAD,
  FEM,LELD,ELD)
FEMFLG=FEMFLG
IF (.NOT. FEMFLG) RETURN
C----- ADD FIXED END FORCES TO LOAD MATRIX
DO 620 J=1,3
  DO 610 I=1,3
    GFEM(I)=0.
    GFEM(I-3)=0.
    GFEM(I+6)=0.
    GFEM(I+9)=0.
    DO 610 J=1,3
      D1=GFEM(J,LOAD)
      D2=GFEM(J+3,LOAD)
      D3=GFEM(J+6,LOAD)
      D4=GFEM(J+9,LOAD)
      GFEM(I)=GFEM(I)+CTS(I,J)*D1
      GFEM(I+3)=GFEM(I+3)+BS(I,J)*D1+CTS(I,J)*D2
      GFEM(I+6)=GFEM(I+6)+CTE(I,J)*D3
      GFEM(I+9)=GFEM(I+9)+BE(I,J)*D4+CTE(I,J)*D4
610 IF (BTST(I,BUG,7)) THEN
  WRITE(6,*) 'IELNO',IELENO
  CALL WMATRX( FEM , 1 , 12, 'LOCAL FIXED END FORCES')
  CALL WMATRX(GFEM , 1 , 12, 'GLOBAL FIXED END FORCES')
ENDDF
C
DO 620 J=1,6
  J1=IDOF(JOINT1,J)
  J2=IDOF(JOINT2,J)
  FORCE(J1,LOAD)=FORCE(J1,LOAD)+GFEM(J)
  FORCE(J2,LOAD)=FORCE(J2,LOAD)+GFEM(J+6)
  IF (BTST(I,BUG,7)) THEN
    CALL WMATRX(FORCE ,NDOF,NLOAD,'GLOBAL LOAD MATRIX')
  ENDDF
RETURN
C----- WRITE MEMBER FORCES TO OUTPUT FILE -----
700 CONTINUE
WRITE (IUNIT,710) NODEI,(FT(J),J=1,6),NODEJ,(FT(J),J=7,12)
WRITE (IUNIT,710) NODEI,(RT(J),J=1,6),NODEJ,(RT(J),J=7,12)
710 FORMAT ('I6,I3,P,6G12.5')
RETURN
C----- READ AND PRINT MEMBER FORCES FROM OUTPUT FILE -----
800 CONTINUE
BACKSPACE(IUNIT)
READ (IUNIT,*) ITYPE,IELNO,IWRITE,TO,DT
WRITE (6,805)
ISTEP=IWRITE
810 READ (IUNIT,815,END=830)
  & NODEI,(FT(J),J=1,6),NODEJ,(RT(J),J=7,12)
  & NODEI,(RT(J),J=1,6),NODEJ,(RT(J),J=7,12)
815 FORMAT ('I6,6G12.5')
ISTEP=ISTEP+IWRITE
T=TO+DT*ISTEP
MODU=MOD(ISTEP,INC)
IF (MOD(ISTEP,INC).EQ.0) THEN
  WRITE (6,820) ISTEP,T,NODEI,(FT(J),J=1,6),
  & NODEJ,(RT(J),J=7,12)
  & WRITE (6,920) (RT(J),J=1,12)
ENDDF
GO TO 810
C
805 FORMAT (' / 3D BEAM FORCES... /
  & STEP TIME NODES',I3,' AXIAL',I3,' I1X',I3,' I2X',I3,'
  & I1X',I3,' TORSION',I3,' MT',I3,' M2',I3)
820 FORMAT (' / I6,I3,P,6G15.6, I6, 6G15.6)
  & I6, 6G15.6)
920 FORMAT (' 25X',DISP , I6, 6G15.6,
  & /I3X, 6G15.6)
C
830 RETURN
C----- DETERMINE THE LENGTH OF THE MATERIAL DATA
900 CONTINUE
NHTST=1
ISE=0
C
MAT = RINPUT(1)
CALL MATLIB
  & ( 0 ,LSTTYP,FALSE,IREL,FALSE,NAME,ESSE,EPSE,DUCT,EXCR,
  & FT ,F ,RT ,R ,V ,VL ,LELEM ,LMAT ,
  & MAT ,MATTP,SE,IELENO,DAMAGE)
ISE=ISE+LELEM
RETURN
C----- DETERMINE THE TOTAL STRAIN ENERGY
1000 CONTINUE
ESSE=0
EPSE=0
C
DO 1010 I=1,12
  ESSE=ESSE + (FT(I)+F(I))*(R(I)+RT(I))/2.
  ENGDAT(1) = ESSE
  ENGDAT(2) = 0
C
RETURN
C----- DUCTILITIES AND EXCURSION RATIOS
NO D & E RATIOS FOR ELASTIC ELEMENT
1100 RETURN
C----- PRINT MAXIMUM ELEMENT LOADS
1200 CONTINUE
LOAD2=0
IF (FIRST) WRITE(6,494) ' COLUMN ( LOAD) REPRESENTS THE DOF WHICH'
  & ' HAS MAXIMUM VALUE'
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IF (FIRST) WRITE (6,491) ' MAXIMUM VALUES FOR ALL STEPS '
  & ' NOTE: MAXIMUM VALUES WITH THE OTHER DOFS FORCES'
  & ' ARE PRINT OUT AT THE SAME TIME'
DO 309 I=0,11
  LOAD2=I+1
  IF (FMAX(I*12+1+I).EQ.0.) GO TO 309
  WRITE (6,492) IELNO,LOAD2,NODEI,(FMAX(I*12+J),J=1,6),
  & NODEJ,(FMAX(I*12+J),J=7,12)
309 CONTINUE
C
RETURN
C----- RESET ELEMENT FORCES
1300 CONTINUE
C----- ZERO MATRICES
DO 1340 I=1,12
  F(I)=0
  FT(I)=0
  R(I)=0
  1340 ST(I)=0
  WRITE (6,1330) IELNO
  1330 FORMAT (' 3D BEAM..... ELEMENT #',I6,
  & ' INTERNAL FORCES AND HYSTERESIS MODELS ARE RESET TO ZERO')
  & LSTTYP,FALSE,IREL,FALSE,NAME,ESSE,EPSE,DUCT,EXCR,
  & FT ,F ,RT ,R ,V ,VL ,LELEM ,LMAT ,
  & MAT ,MATTP,SE,IELENO,DAMAGE)
RETURN
C----- DAMAGE INDEX
1400 DAMAGE = 0.
RETURN
END
C----- DEBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
C----- END DEBUG UNIT(6)
SUBROUTINE ELEG2
  & IOPT ,IUNIT ,NEWK ,FIRST ,PRINT ,
  & NDOF ,NINOD ,MAXNOD ,NCCOS ,
  & ID ,IDOF ,COORD ,COSINE ,JTCS ,
  & CONST ,DISPL ,VELOC ,FORCE ,FUB ,
  & IREL ,BUG ,IBUG ,ACCEL ,KGDATA ,
  & GRUV ,POSDOM ,ELSTIC ,NAME ,STEPID ,
  & IELNG ,RINPUT ,ESSE ,EPSE ,INC ,
  & DAMAGE ,DUCTFAG ,
  & IELTYP ,IELENO ,NODEI ,NODEJ ,JOINT1 ,
  & JOINT2 ,RT ,FT ,
  & MAT ,A ,LM ,ISE ,H ,LENGTH ,
  & SE ,STIFF )
C
IMPLICIT REAL(A-H,O-S)
COMMON /RSA/ICOMN,RSAP(12)
REAL ICOMN
C
LOGICAL ERR,FIRST,PRINT,BUG,FALSE,NEWK,ELSTIC
LOGICAL BTST
CHARACTER*80 NAME
CHARACTER*(*) STEPID
CHARACTER*9 TIPS(6)
DIMENSION IDOF(MAXNOD,6),COORD(MAXNOD,6),COSINE(3,3,NCCOS)
DIMENSION CONST(MAXNOD,6),ID(MAXNOD),JTCS(MAXNOD)
DIMENSION SAT(2,12),STIFF(78)
C ERR FLAG, SF(2,3),LM(12),FMAX(2)
DIMENSION CTS( 3, 3),CTE( 3, 3),VX(3),VT(3),VZ(3)
DIMENSION BE( 3, 3),BE( 3, 3),SE(ISE)
DIMENSION DISPL(NDOF,NLOAD),FUB(NDOF)
DIMENSION VELOC(NDOF)
DIMENSION FORCE(NDOF,NLOAD)
DIMENSION RINPUT(100),WORK(6),DUCT(3),EXCR(6)
REAL LENGTH
REAL IX, IY, IZ, KI1, KJ1, KI2, KJ2, KI3, KJ3, KI4, KJ4, KI5, KJ5, KI6, KJ6, KI7, KJ7, KI8, KJ8, KI9, KJ9, KI10, KJ10, KI11, KJ11, KI12, KJ12
INTDGRN REL,RELT
DIMENSION MD(13)
DATA MD/1,2,4,7,11,16,22,29,37,46,56,67,79/
DATA TYPE/' AXIAL', ' SHEAR-T', ' SHEAR-S',
  & ' TORSION', ' BENDING-T', ' BENDING-S' /
C-----
C
VARIABLES
C----- GLOBAL VARIABLES
C IOPT = 1, INITIALIZE BEAM COLUMN ELEMENT
C = 2, CALCULATE GEOMETRIC STIFFNESS
C = 3, CALCULATE STIFFNESS
C = 4, CALCULATE INCREMENTAL FORCES
C = 5, CALCULATE TOTAL FORCES AND ENERGIES
C IUNIT = OUTPUT FILE UNIT # FOR PRINTING OUTPUT
C ERR FLAG, SF(2,3),LM(12),FMAX(2)
C FIRST = FLAG, FIRST=.TRUE., PRINT HEADERS
C PRINT = FLAG, PRINT=.TRUE., PRINT DATA
C NDOF = # OF GLOBAL DOF
C NLOAD = # OF LOAD COMBINATIONS
C MAXNOD = # ROW DIMENSION OF NODE ARRAY
C NNODE = # OF NODES
C ID = ARRAY OF EXTERNAL NODE NUMBERS
C IDOF = ARRAY OF DEGREES OF FREEDOM
C COORD = ARRAY OF COORDINATES
C COSINE = ARRAY OF DIRECTION COSINES OF NODES
C CONST = ARRAY OF CONSTRAINT TRANSFORMATIONS
C DISPL = GLOBAL DISPLACEMENT MATRIX
C Z = REAL GLOBAL STORAG VECTOR
C NZ = INTEGER GLOBAL STORAG VECTOR
C IZMAT = LOCATION OF MATERIAL DATA IN GLOBAL STORAGE VECTOR
C NAME = ELEMENT NAME
C IELTYP = ELEMENT NUMBER
C RINPUT = INPUT DATA
C STIFF = OUTPUT ELEMENT STIFFNESS (UPPER TRIANGULAR FORM)
C LM = OUTPUT LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C H,Q,A1,AJ,C,B1,BJ,S22,S26,S212,S33,S35,S311,D
C = INPUT LOAD FOR FORMING GEOMETRIC STIFFNESS MATRIX...
C R = INCREMENTAL DISPLACEMENTS
C FT = TOTAL DISPLACEMENTS
C F = INCREMENTAL FORCES
C FT = TOTAL FORCES
C LM = LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C CTS = LOCAL TO GLOBAL ROTATION MATRIX START
C CTE = LOCAL TO GLOBAL ROTATION MATRIX END
C BS = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX START
C BE = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX END
C STIFF = ELEMENT STIFFNESS
C IELTYP = ELEMENT TYPE = 1
C NODEI = EXTERNAL JOINT NUMBER, END I
C NODEJ = EXTERNAL JOINT NUMBER, END J
C JOINT1 = INTERNAL JOINT NUMBER, END I
C JOINT2 = INTERNAL JOINT NUMBER, END J
C----- TEMPORARY VARIABLES
C VX = VECTOR DEFINING THE ELEMENT X AXIS
C VT = VECTOR DEFINING THE ELEMENT Y AXIS
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C
IF (BEST(IBUG,7)) THEN
WRITE (6,*) IELDOF, IELDOF, LM '-', (LM(I), I=1, IELDOF)
CALL LMATRX(STIFF, MD, IELDOF, (L+1), 'SPRING STIFFNESS')
ENDIF
C
RETURN
C----- DETERMINE INCREMENTAL FORCES -----
400 CONTINUE
IF (PRINT.AND.FIRST) WRITE (6,491) STEPID
C
DO 430 LOAD=1, NLOAD
C----- TRANSFORM FRAME DISPL INTO LOCAL DISPL
C... LOCAL DISPL ARE AT THE END END OF THE SPRING, THUS
C... POSITIVE IS TENSION AND NEGATIVE IS COMPRESSION
R=0
V=0
DO 410 I=1, IELDOF
J=LM(I)
A11= A(I,1)
A12= A(I,2)
DISP= DISPL(J,LOAD)
VEL= VELOC(J)
R= R + (A(I,2)-A(I,1)) * DISPL(J,LOAD)
V= V + (A(I,2)+A(I,1)) * VELOC(J)
C----- INCREMENTAL FORCES
F= H*R/LENGTH
C----- CHECK STIFFNESS AND CALCULATE UNBALANCED FORCES
C THIS IS PREFORMED ONLY FOR INELASTIC ANALYSIS
P=FT+F
IF (.NOT. ELSTIC) THEN
-- SET UP TEMPORARY - TOTAL LASTS, DISPL'S AND VELOC
PL=FT
D=(RT+R)/LENGTH
DL=RT/LENGTH
VC=VT+V
VL=VT
-- TRANSFER PERMANENT HYSTERESIS DATA TO TEMPORARY STORAGE...
ISE2=ISE/2
DO 415 I=1, ISE2
SE(I)=ISE2+SE(I)
C 415 -- CALL HYST MODEL TO GET NEW STIFFNESS AND
C THE TOTAL FORCE P AT DISPL D.
MOPT=3
CALL MATLIB
& (MOPT, LSTTYP, FALSE, IREL, FALSE, NAME, ESE, EPSE, DUCT, EXCR,
& P, PL, D, DL, VC, VL, LELEM, LMAT,
& MAT, MATTYP, SE, IELNO, DAMAGE)
IF (SE(ISE2+1).NE. SE(I)) NEWK=TRUE.
ENDIF
C----- SAVE PEAK VALUES FOR MULTIPLE LOAD CASES
IF (NLOAD.GT.1 .AND. ABS(F).GT.ABS(FMAX(1))) THEN
FMAX(1)=F
FMAX(2)=R
ENDIF
C----- PRINT DATA
HH= H/LENGTH
IF (PRINT) WRITE (6,492) IELNO, LOAD, TYPE(KTYPE), NODEI, NODEJ, F, R, H, ELEM04490
C
430 CONTINUE
C----- THIS IS FOR SOLOSA USED ONLY
IF (ICOMN.EQ. 1 .OR. ICOMN.EQ. 2) THEN
RFAF(1)=F
ENDIF
C----- CALCULATE THE UNBALANCED MEMBER FORCE ON A JOINT.
UBAL= F-PT-P
DO 440 I=1, IELDOF
J=LM(I)
A11= A(I,1)
A12= A(I,2)
FUB(J)= FUB(J) + (A(I,1)-A(I,2)) * UBAL
IF (BEST(IBUG,7))
& WRITE (6,494) (I, LM(I), FUB(I), I=1, IELDOF)
C 440 CONTINUE
491 FORMAT (' SPRING FORCES...', 5X, A /
& ' ELEMENT LOAD', T17, ' TYPE', ' NODEI', ' NODEJ',
& ' FORCE ' ' DISPLACEMENT ' ' STIFFNESS ' )
492 FORMAT (' I0, I5, 2R, A9, 2I6, IP, 3G15.6 )
493 FORMAT (' F', IP, G16.8, ' FT', G16.8, ' P', G16.8 )
494 FORMAT (' I', I5, ' LM(I)', I6, ' FUB(I)', IP, G16.8 )
C----- ADJUST F FOR UNBALANCED LOAD P
F=P-FT
RETURN
C----- DETERMINE TOTAL FORCES, ENERGIES ECT -----
500 CONTINUE
IF (ICOMN.NE. 1 .AND. ICOMN.NE. 2) THEN
IF (PRINT.AND.FIRST) WRITE (6,491) STEPID
C
C----- TOTAL FORCES, DISPLACEMENTS AND VELOCITIES
FT=PT+F
RT=RT+R
VT=VT+V
IF (ABS(FT).GT.ABS(FMAX(1))) THEN
FMAX(1)=FT
FMAX(2)=RT
ENDIF
C----- PRINT DATA
HH= H/LENGTH
IF (PRINT) WRITE(6,492) IELNO, 1, TYPE(KTYPE), NODEI, NODEJ, FT, RT, H, ELEM05000
ELSE IF (ICOMN.EQ. 1 .OR. ICOMN.EQ. 2) THEN
FT=RFAF(1)
IF (PRINT.AND.FIRST) WRITE (6,497) STEPID
IF (PRINT) WRITE(6,492) IELNO, 1, TYPE(KTYPE), NODEI, NODEJ, FT
ENDIF
C 497 FORMAT (' SPRING FORCES...', 5X, A /
& ' ELEMENT LOAD', T17, ' TYPE', ' NODEI', ' NODEJ',
& ' FORCE ' )
C----- TRANSFER TEMPORARY HYSTERESIS DATA TO PERMANENT STORAGE...
ISE2=ISE/2
IF (.NOT. ELSTIC) THEN
DO 510 I=1, ISE2
SE(I)=SE(I)+ISE2
510 ENDF
C
RETURN
C----- DETERMINE FIXED END FORCES, AND ADD TO FORCE=
600 RETURN
C----- FIXED END FORCES ARE NOT AVAILABLE FOR THE SPRING
C----- WRITE MEMBER FORCES TO OUTPUT FILE -----
700 CONTINUE

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      ELEMENT, I12, , DAMAGE INDEX F MAX      ELE06690      J3=IQUICK(NODE3, ID, NNODE)      ELE01330
      C      1234567890ABCDEF1234567890ABCDE ELE06700      J4=IQUICK(NODE4, ID, NNODE)      ELE01340
      C      D MAX ESE                          ELE06710      C-----                                ELE01350
      C      PSE                                ELE06720      C GET GLOBAL TO LOCAL TRANSFORMATION MATRIX ELE01360
1420 FORMAT (110, IP, 6G15.6)                ELE06730      C-----                                ELE01370
      C      DAMAGE=DAMAGE*(ESES+EPSSE)        ELE06740      C-----                                ELE01380
      C      RETURN                            ELE06750      C-----                                ELE01390
      C      END                               ELE06760      C-----                                ELE01400
      C-----                                ELE06770      C-----                                ELE01410
      C      DEBUG UNIT(6), TRACE, SUBCHK, INIT, SUBTRACE ELE06780      C-----                                ELE01420
      C      END DEBUG                        ELE06790      C-----                                ELE01430
      C-----                                ELE06800      C-----                                ELE01440
      C      SUBROUTINE IELO3                  ELE06810      C-----                                ELE01450
      C      I (IOPT, IUNIT, NEWK, FIRST, PRINT, ELE06820      C-----                                ELE01460
      C      & NDOF, NLOAD, MAXNOD, NNODE, NCOS, ELE06830      C-----                                ELE01470
      C      & ID, IDOF, COORD, COSINE, JTCOS, ELE06840      C-----                                ELE01480
      C      & CONST, DISPL, VELOC, FORCE, FUB, ELE06850      C-----                                ELE01490
      C      & IREL, BUG, IBUG, ACCEL, KGDATA, ELE06860      C-----                                ELE01500
      C      & GRAV, PGEOM, ELSTIC, NAME, STEPID, ELE06870      C-----                                ELE01510
      C      & IELNO, RINPUT, AXIAL, ESE, PSE, ELE06880      C-----                                ELE01520
      C      & INC, DAMAGE, DUCTFAG, ELE06890      C-----                                ELE01530
      C      & IELTYP, IELOOP, ISEB, ISES, ISEA, ELE06900      C-----                                ELE01540
      C      & ISE, J1, J2, J3, J4, ELE06910      C-----                                ELE01550
      C      & MATB, MATS, MATA, BLANK, R, ELE06920      C-----                                ELE01560
      C      & RT, FT, VT, ELE06930      C-----                                ELE01570
      C      & PKG, LM, VMG, A, ELE06940      C-----                                ELE01580
      C      & SB, SS, SA, TEMP1, TEMP2, ELE06950      C-----                                ELE01590
      C      & KGDOF, LENGTH, FMAX, SE, STIFF ) ELE06960      C-----                                ELE01600
      C      IMPLICIT REAL(A-H,O-Z)           ELE06970      C-----                                ELE01610
      C      COMMON /RSA/ICOM, RSAP(12)       ELE06980      C-----                                ELE01620
      C      REAL ICOMN                        ELE06990      C-----                                ELE01630
      C-----                                ELE07000      C-----                                ELE01640
      C      DOUBLE PRECISION SX, SXX, SY, SXY, SN ELE07010      C-----                                ELE01650
      C      LOGICAL ERR, FIRST, PRINT, BUG, FALSE, NEWK, ELSTIC, AXIAL, BTEST ELE07020      C-----                                ELE01660
      C      CHARACTER*80 NAME                ELE07030      C-----                                ELE01670
      C      CHARACTER*(*) STEPID             ELE07040      C-----                                ELE01680
      C      DOUBLE PRECISION SAT, DSTIFF(600) ELE07050      C-----                                ELE01690
      C      DIMENSION IDOF(MAXNOD, 6), COORD(MAXNOD, 6), COSINE(3, J), NCOS) ELE07060      C-----                                ELE01700
      C      DIMENSION CONST(MAXNOD, 6), ID(MAXNOD), JTCOS(MAXNOD) ELE07070      C-----                                ELE01710
      C      DIMENSION SAT(3, 24), STIFF(600), KGDATA(3), SAT2(6, 24) ELE07080      C-----                                ELE01720
      C      DIMENSION AT(24, 6), A(24, 3), S(6, 6), LM(24), AC(6, 3), LMG(24) ELE07090      C-----                                ELE01730
      C      DIMENSION CT(3, 3), BS(3, 3), VX(3), VT(3), VXB(3), VXT(3), VJT(3), VSB(3) ELE07100      C-----                                ELE01740
      C      DIMENSION SE(18), A11(6, 3), A22(6, 3), A44(6, 3), A33(6, 3) ELE07110      C-----                                ELE01750
      C      DIMENSION P(3), R(3), V(3), FT(3), RT(3), VT(3), P(3) ELE07120      C-----                                ELE01760
      C      DIMENSION DISPL(NDOF, NLOAD), FUB(NDOF), EPUB(24), FMAX(6) ELE07130      C-----                                ELE01770
      C      DIMENSION VEGD(NDOF)              ELE07140      C-----                                ELE01780
      C      DIMENSION FORCE(NDOF, NLOAD)       ELE07150      C-----                                ELE01790
      C      DIMENSION RINPUT(100), WORK(1), DUCT(3, J), EXCR(6, 3) ELE07160      C-----                                ELE01800
      C      REAL LENGTH                       ELE07170      C-----                                ELE01810
      C      DIMENSION MD(25)                  ELE07180      C-----                                ELE01820
      C      DATA MD/ 1, 2, 4, 7, 11, 16, 22, 29, 37, 46, 56, 67, ELE07190      C-----                                ELE01830
      C      79, 92, 106, 121, 137, 154, 172, 191, 211, 232, 254, 277, 301/ ELE07200      C-----                                ELE01840
      C-----                                ELE07210      C-----                                ELE01850
      C      VARIABLES:                        ELE07220      C-----                                ELE01860
      C-----                                ELE07230      C-----                                ELE01870
      C      1. GLOBAL VARIABLES                ELE07240      C-----                                ELE01880
      C      IOPT = INITIALISE BEAM COLUMN ELEMENT ELE07250      C-----                                ELE01890
      C      2. CALCULATE GEOMETRIC STIFFNESS ELE07260      C-----                                ELE01900
      C      3. FORM STIFFNESS                  ELE07270      C-----                                ELE01910
      C      4. CALCULATE INCREMENTAL FORCES ELE07280      C-----                                ELE01920
      C      5. CALCULATE TOTAL FORCES AND ENERGIES ELE07290      C-----                                ELE01930
      C      6. OUTPUT FILE UNIT FOR PRINTING OUTPUT ELE07300      C-----                                ELE01940
      C      7. ERR OR FLAG IF AN ERR OR HAS OCCURED, ERR=.TRUE. ELE07310      C-----                                ELE01950
      C      8. FLAG, FIRST=.TRUE., PRINT HEADERS ELE07320      C-----                                ELE01960
      C      9. FLAG, PRINT=.TRUE., PRINT DATA ELE07330      C-----                                ELE01970
      C      10. GLOBAL DOF                     ELE07340      C-----                                ELE01980
      C      11. # OF LOAD COMBINATIONS          ELE07350      C-----                                ELE01990
      C      12. # ROW DIMENSION OF NODE ARRAY ELE07360      C-----                                ELE02000
      C      13. # OF NODES                     ELE07370      C-----                                ELE02010
      C      14. ARRAY OF EXTERNAL NODE NUMBERS ELE07380      C-----                                ELE02020
      C      15. ARRAY OF DEGREES OF FREEDOM ELE07390      C-----                                ELE02030
      C      16. ARRAY OF COORDINATES           ELE07400      C-----                                ELE02040
      C      17. ARRAY OF DIRECTION COSINES OF NODES ELE07410      C-----                                ELE02050
      C      18. ARRAY OF CONSTRAINT TRANSFORMATIONS ELE07420      C-----                                ELE02060
      C      19. GLOBAL DISPLACEMENT MATRIX ELE07430      C-----                                ELE02070
      C      20. REAL GLOBAL STORAG VECTOR ELE07440      C-----                                ELE02080
      C      21. INTEGER GLOBAL STORAG VECTOR ELE07450      C-----                                ELE02090
      C      22. LOCATION OF MATERIAL DATA IN GLOBAL STORAGE VECTOR ELE07460      C-----                                ELE02100
      C      23. ELEMENT NAME                   ELE07470      C-----                                ELE02110
      C      24. ELEMENT NUMBER                ELE07480      C-----                                ELE02120
      C      25. INPUT DATA                    ELE07490      C-----                                ELE02130
      C      26. OUTPUT ELEMENT STIFFNESS (UPPER TRIANGULAR FORM) ELE07500      C-----                                ELE02140
      C      27. OUTPUT LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX ELE07510      C-----                                ELE02150
      C-----                                ELE07520      C-----                                ELE02160
      C      ELEMENT VARIABLES                  ELE07530      C-----                                ELE02170
      C      H, Q, A1, AJ, C, BI, BJ, S22, S26, S212, S33, S35, S311, D ELE07540      C-----                                ELE02180
      C      PKG = INDIVIDUAL TERMS IN THE ELEMENT STIFFNESS (S) MATRIX ELE07550      C-----                                ELE02190
      C      R = INPUT LOCAL TO GLOBAL STIFFNESS MATRIX... ELE07560      C-----                                ELE02200
      C      I = INCREMENTAL DISPLACEMENTS ELE07570      C-----                                ELE02210
      C      RT = TOTAL DISPLACEMENTS          ELE07580      C-----                                ELE02220
      C      F = INCREMENTAL FORCES            ELE07590      C-----                                ELE02230
      C      LM = TOTAL FORCES                  ELE07600      C-----                                ELE02240
      C      LM = LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX ELE07610      C-----                                ELE02250
      C      CTS = LOCAL TO GLOBAL ROTATION MATRIX START ELE07620      C-----                                ELE02260
      C      CTE = LOCAL TO GLOBAL ROTATION MATRIX END ELE07630      C-----                                ELE02270
      C      BS = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX START ELE07640      C-----                                ELE02280
      C      BE = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX END ELE07650      C-----                                ELE02290
      C      STIFF = ELEMENT STIFFNESS         ELE07660      C-----                                ELE02300
      C      IELTYP = ELEMENT TYPE = 10        ELE07670      C-----                                ELE02310
      C      NODEI = EXTERNAL JOINT NUMBER, END I ELE07680      C-----                                ELE02320
      C      NODEJ = EXTERNAL JOINT NUMBER, END J ELE07690      C-----                                ELE02330
      C      JOINTI = INTERNAL JOINT NUMBER, END I ELE07700      C-----                                ELE02340
      C      JOINTJ = INTERNAL JOINT NUMBER, END J ELE07710      C-----                                ELE02350
      C-----                                ELE07720      C-----                                ELE02360
      C      TEMPROART VARIABLES                ELE07730      C-----                                ELE02370
      C      VX = VECTOR DEFINING THE ELEMENT X AXIS ELE07740      C-----                                ELE02380
      C      VY = VECTOR DEFINING THE ELEMENT Y AXIS ELE07750      C-----                                ELE02390
      C      COSLOC = LOCAL COSINE MATRIX      ELE07760      C-----                                ELE02400
      C      A = GLOBAL TO LOCAL FORCE TRANSFORMATION MATRIX ELE07770      C-----                                ELE02410
      C      S = ELEMENT STIFFNESS AT LOCAL COORD, AT ENDS OF FLEXIBLE PART ELE07780      C-----                                ELE02420
      C      SAT = PRODUCT OF S*F              ELE07790      C-----                                ELE02430
      C      ASAT = PRODUCT OF A*SAT           ELE07800      C-----                                ELE02440
      C-----                                ELE07810      C-----                                ELE02450
      C      CHOOSE OPTION                      ELE07820      C-----                                ELE02460
      C      IF (IOPT.LT.1 .OR. IOPT.GT.14) ELE07830      C-----                                ELE02470
      C      & WRITE (6,*) 'INVALID OPTION IN ELE-02, IOPT=', IOPT ELE07840      C-----                                ELE02480
      C      GO TO (100, 200, 300, 400, 500, 600, 700, 800, 900, 1000, 1100, 1200, ELE07850      C-----                                ELE02490
      C      1300, 1400), IOPT ELE07860      C-----                                ELE02500
      C      100 CONTINUE                       ELE07870      C-----                                ELE02510
      C-----                                ELE07880      C-----                                ELE02520
      C      INTERPERATE INPUT DATA           ELE07890      C-----                                ELE02530
      C      IELTYP = 3                          ELE07900      C-----                                ELE02540
      C      MATB = RINPUT( 1)                  ELE07910      C-----                                ELE02550
      C      MATS = RINPUT( 2)                  ELE07920      C-----                                ELE02560
      C      MATA = RINPUT( 3)                  ELE07930      C-----                                ELE02570
      C      NODE1 = RINPUT( 4)                  ELE07940      C-----                                ELE02580
      C      NODE2 = RINPUT( 5)                  ELE07950      C-----                                ELE02590
      C      NODE3 = RINPUT( 6)                  ELE07960      C-----                                ELE02600
      C      NODE4 = RINPUT( 7)                  ELE07970      C-----                                ELE02610
      C      ALPIN = RINPUT( 8)                 ELE07980      C-----                                ELE02620
      C      PKG = RINPUT( 9)                   ELE07990      C-----                                ELE02630
      C-----                                ELE08000      C-----                                ELE02640
      C      GET INTERNAL NODE # ASSOCIATED WITH EXTERNAL NODE #'S ELE08010      C-----                                ELE02650
      C      J1=IQUICK(NODE1, ID, NNODE)        ELE08020      C-----                                ELE02660
      C      J2=IQUICK(NODE2, ID, NNODE)        ELE08030      C-----                                ELE02670
      C      J3=IQUICK(NODE3, ID, NNODE)        ELE08040      C-----                                ELE02680
      C      J4=IQUICK(NODE4, ID, NNODE)        ELE08050      C-----                                ELE02690
      C      IF ( (IDOF(J1, J1).EQ.IDOF(J4, J4)) .OR. ELE08060      C-----                                ELE02700
      C      (IDOF(J1, J1).EQ.IDOF(J3, J3)) .OR. ELE08070      C-----                                ELE02710
      C      (IDOF(J2, J2).EQ.IDOF(J4, J4)) .OR. ELE08080      C-----                                ELE02720
      C      (IDOF(J2, J2).EQ.IDOF(J3, J3)) ) GO TO 50 ELE08090      C-----                                ELE02730

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C
IELDOF=IELDOF+1
IF (I.EQ.1) THEN
  LMTI=IDOF(J1,J)
  AT(IELDOF,1)=A11(J,1)
  AT(IELDOF,2)=A11(J,2)
ELSE IF (I.EQ.2) THEN
  LMTI=IDOF(J2,J)
  AT(IELDOF,3)=A22(J,2)
ELSE IF (I.EQ.3) THEN
  LMTI=IDOF(J3,J)
  AT(IELDOF,4)=A33(J,2)
ELSE IF (I.EQ.4) THEN
  LMTI=IDOF(J4,J)
  AT(IELDOF,5)=A44(J,1)
  AT(IELDOF,6)=A44(J,2)
ENDIF

C
LM(IELDOF)=LMTI
IF(AT(IELDOF,1).EQ.0.AND.(AT(IELDOF,2).EQ.0).AND.
  & (AT(IELDOF,3).EQ.0).AND.(AT(IELDOF,4).EQ.0).AND.
  & (AT(IELDOF,5).EQ.0).AND.(AT(IELDOF,6).EQ.0)) THEN
  IELDOF=IELDOF-1
ELSE IF (IELDOF.GT.1) THEN
  DO 49 I=1,IELDOF-1
    IF (LM(I),EQ.LM(IELDOF)) THEN
      DO 48 JJ=1,6
        AT(I,JJ)=AT(I,JJ)+AT(IELDOF,JJ)
        AT(IELDOF,JJ)=0
      ENDIF
    ELSE
      CONTINUE
    ENDIF
  ENDIF
CONTINUE
C
50 CONTINUE
C----- A241 FOR STIFFNESS ONLY...
AC(1,1)=0.
AC(2,1)=-1./W
AC(3,1)=1./W
AC(4,1)=-1./W
AC(5,1)=0.
AC(6,1)=1./W
AC(1,2)=1.
AC(2,2)=ALPHA/W
AC(3,2)=-ALPHA/W
AC(4,2)=BETA/W
AC(5,2)=1.
AC(6,2)=BETA/W
AC(1,3)=0.
AC(2,3)=1./2.
AC(3,3)=1./2.
AC(4,3)=-1./2.
AC(5,3)=0.
AC(6,3)=-1./2.
C
DO 55 I=1,IELDOF
DO 55 J=1,3
  A(I,J)=0.
DO 55 K=1,6
  A(I,J)=A(I,J)+AT(I,K)*AC(K,J)
C
IF (BTEST(1BUG,7)) THEN
  CALL WMTRK2(AT,IELDOF,6,AT,24)
  CALL WMTRK AC,6,3,AC,3)
  CALL WMTRK2(A,IELDOF,3,A,24)
ENDIF
C
C----- GET SPRING STIFFNESS
FALSE=.FALSE.
LSTTYP=0
CALL MATLIB
& (2,LSTTYP, FALSE, IREL, FALSE, NAME,
& ESBB, PSEB, DUCT(1,1), EXCR(1,1),
& FT, F, RT, R, 0., 0., ,LELEM, LMAT,
& MATB, MATTYP, SE(ISEB), IELNO, DAMAGE)
CALL MATLIB
& (2,LSTTYP, FALSE, IREL, FALSE, NAME,
& ESBB, PSEB, DUCT(1,2), EXCR(1,2),
& FT, F, RT, R, 0., 0., ,LELEM, LMAT,
& MATS, MATTYP, SE(ISES), IELNO, DAMAGE)
CALL MATLIB
& (2,LSTTYP, FALSE, IREL, FALSE, NAME,
& ESBA, PSEA, DUCT(1,3), EXCR(1,3),
& FT, F, RT, R, 0., 0., ,LELEM, LMAT,
& MATA, MATTYP, SE(ISEA), IELNO, DAMAGE)
C
ESE=0.
PSE=0.
C
WRITE (6,*) 'AFTER MAT', 'LIB IN ELDO2 '
111 FORMAT (9P, 'ZMATR(', '16, '1', '115, 1P, G2D, 10)
C----- SET UP THE LOCAL STIFFNESS MATRIX
SB=SE(ISEB)
SS=SE(ISES)
SA=SE(ISEA)
IF (BTEST(1BUG,7)) THEN
  WRITE (6,*) 'BENDING STIFFNESS KB=', SB
  WRITE (6,*) 'SHEAR STIFFNESS KS=', SS
  WRITE (6,*) 'AXIAL STIFFNESS KA=', SA
ENDIF
C----- CALCULATE SAT
SBL=SB/LENGTH
SSL=SS/LENGTH
SAL=SA/LENGTH
DO 60 J=1,IELDOF
  SAT(1,J)=SBL*A(J,1)
  SAT(2,J)=SSL*A(J,2)
  SAT(3,J)=SAL*A(J,3)
60 SAT(3,J)=SAL*A(J,3)
C----- CALCULATE ASAT AND CONVERT TO HALF STORAGE MODE BY COLUMNS
C
1 3 6 10 15 21 28 36 45 55 66 78 300. LOCATION INDICATES
2 5 9 14 20 27 35 44 54 65 77 . LOCATION OF DATA
4 8 13 19 26 34 43 53 64 76 . IN ARRAY STIFF
7 12 18 25 33 42 52 63 75 .
11 17 24 32 41 51 62 74 .
16 23 31 40 50 61 73 .
22 30 39 49 60 72 .
29 38 48 59 71 .
37 47 58 70 .
46 57 69 .
56 68 .
67 .
. .
. .
. .
. .
L=0
DO 70 J=1,IELDOF
DO 70 I=J,1,-1
  L=L+1
  DSTIFF(L)=0
DO 70 K=1,3
  70 DSTIFF(L)=DSTIFF(L)+A(I,K)*SAT(K,J)
DO 71 J=1,L
  71 STIFF(J)=DSTIFF(J)
C
ELE02750 IF (BTEST(1BUG,7)) THEN
ELE02760 WRITE (6,*) 'IELDOF', IELDOF, 'LM =', (LM(I), I=1, IELDOF)
ELE02770 CALL LMATRK(STIFF, MD, IELDOF, (L+1), 'WALL STIFFNESS')
ELE02780 ENDF
ELE02790
ELE02800
ELE02810 C ASSEMBLE GEOMETRIC STIFFNESS MATRIX FOR A UNIT LOAD...
ELE02820
ELE02830 LKG=1.
ELE02840 IF (PKG.EQ.0.AND.(KGDATA(1).NE.2)) GO TO 180
ELE02850 IF (KGDATA(1).EQ.0.OR.(KGDATA(2).EQ.0)) GO TO 180
ELE02860 AXIAL=.TRUE.
ELE02870
ELE02880 C----- DETERMINE THE AXIAL LOAD TO BE USED FOR KG...
ELE02890 IF (KGDATA(1).EQ.1) THEN
ELE02900 PGEOM = PKG
ELE02910 ELSE IF (KGDATA(1).EQ.2) THEN
ELE02920 PGEOM = -F(3)
ELE02930 ENDF
ELE02940
ELE02950 C----- ASSEMBLE TRANSFORMATION MATRIX A
ELE02960 C NOTE: AT IS DIMENSIONED AS A 12X6 MATRIX, BUT ONLY THE FIRST IELDOF
ELE02970 C ROWS CONTAIN INFORMATION. THE BALANCE OF THE MATRIX IS
ELE02980 C UNDEFINED. THE GLOBAL DOF CORRESPONDING TO THE ROW'S OF A
ELE02990 C ARE STORED IN VECTOR LM. AGAIN, ONLY THE FIRST IELDOF OF A
ELE03000 C OF LM ARE DEFINED.
ELE03010 DO 140 I=1,24
ELE03020 DO 140 K=1,6
ELE03030 140 AT(I,K)=0.00
ELE03040 KGDOF=0
ELE03050 DO 145 I=1,4
ELE03060 DO 145 J=1,6
ELE03070
ELE03080 KGDOF=KGDOF+1
ELE03090 IF (I.EQ.1) THEN
ELE03100 LMTI=IDOF(J1,J)
ELE03110 AT(KGDOF,1)=A11(J,1)
ELE03120 AT(KGDOF,2)=A11(J,3)
ELE03130 ELSE IF (I.EQ.2) THEN
ELE03140 LMTI=IDOF(J2,J)
ELE03150 AT(KGDOF,3)=A22(J,3)
ELE03160 ELSE IF (I.EQ.3) THEN
ELE03170 LMTI=IDOF(J3,J)
ELE03180 AT(KGDOF,4)=A33(J,3)
ELE03190 ELSE IF (I.EQ.4) THEN
ELE03200 LMTI=IDOF(J4,J)
ELE03210 AT(KGDOF,5)=A44(J,1)
ELE03220 AT(KGDOF,6)=A44(J,3)
ELE03230 ENDF
ELE03240 LMRG(KGDOF)=LMTI
ELE03250 IF (AT(KGDOF,1).EQ.0.AND.(AT(KGDOF,2).EQ.0).AND.
ELE03260 & (AT(KGDOF,3).EQ.0).AND.(AT(KGDOF,4).EQ.0).AND.
ELE03270 & (AT(KGDOF,5).EQ.0).AND.(AT(KGDOF,6).EQ.0)) THEN
ELE03280 KGDOF=KGDOF-1
ELE03290 ELSE IF (KGDOF.GT.1) THEN
ELE03300 DO 149 I=1,KGDOF-1
ELE03310 IF (LMRG(I).EQ.LMRG(KGDOF)) THEN
ELE03320 DO 148 JJ=1,6
ELE03330 AT(I,JJ)=AT(I,JJ)+AT(KGDOF,JJ)
ELE03340 AT(KGDOF,JJ)=0
ELE03350 KGDOF=KGDOF-1
ELE03360 GO TO 145
ELE03370 ENDF
ELE03380 149 CONTINUE
ELE03390 ENDF
ELE03400
ELE03410 C 145 CONTINUE
ELE03420
ELE03430 C----- LOCAL GEOMETRIC STIFFNESS MATRIX
ELE03440 DO 150 I=1,12
ELE03450 DO 150 JI=1,12
ELE03460 150 S(I,JI)=0.0
ELE03470 S(1,1)=1.0/LENGTH
ELE03480 S(2,2)=0.5/LENGTH
ELE03490 S(3,3)=0.5/LENGTH
ELE03500 S(4,4)=0.5/LENGTH
ELE03510 S(5,5)=1.0/LENGTH
ELE03520 S(6,6)=0.5/LENGTH
ELE03530 S(1,9)=-1.0/LENGTH
ELE03540 S(2,6)=-0.5/LENGTH
ELE03550 S(3,4)=-0.5/LENGTH
ELE03560 S(5,1)=-1.0/LENGTH
ELE03570 S(6,2)=-0.5/LENGTH
ELE03580 S(4,3)=-0.5/LENGTH
ELE03590
ELE03600 C----- CALCULATE SAT
ELE03610 DO 160 I=1,6
ELE03620 DO 160 J=1,KGDOF
ELE03630 SAT2(I,J)=0
ELE03640 DO 160 K=1,6
ELE03650 160 SAT2(I,J)=SAT2(I,J)+S(I,K)*AT(J,K)
ELE03660
ELE03670 C----- CALCULATE ASAT AND CONVERT TO HALF STORAGE MODE BY COLUMNS
ELE03680 DO 170 J=1,KGDOF
ELE03690 DO 170 I=J,1,-1
ELE03700 L=I
ELE03710 STIFF(L)=0
ELE03720 DO 170 K=1,6
ELE03730 STIFF(L)=STIFF(L)+AT(I,K)*SAT2(K,J)
ELE03740
ELE03750 C----- PRINT GEOMETRIC STIFF DATA FOR DEBUGGING
ELE03760 IF (BTEST(1BUG,7)) THEN
ELE03770 CALL WMTRK2(AT,KGDOF,6,AT,24)
ELE03780 CALL WMTRK(S,6,6,KGDOF,3)
ELE03790 CALL WMTRK(SAT2,6,KGDOF,SAT2)
ELE03800 WRITE (6,*) 'KGDOF', KGDOF, 'LMRG =', (LMRG(I), I=1, KGDOF)
ELE03810 CALL LMATRK(STIFF(LKG), MD, KGDOF, 300, 'WALL GEOMETRIC STIFF.')
ELE03820 ENDF
ELE03830
ELE03840 C----- RELEASE UNUSED STORAGE
ELE03850 180 IREL=600-L
ELE03860
ELE03870 C PRINT COLUMN DATA
ELE03880 IF (FIRST.OR.(BTEST(1BUG,7)).OR.(BTEST(1BUG,8))) WRITE (6,192)
ELE03890 191 WRITE(6,193) NAME, IELNO, MATB, MATS, MATA,
ELE03900 & ID(J1), ID(J2), ID(J3), ID(J4),
ELE03910 & LENGTH, W, ALPIN, PKG
ELE03920
ELE03930 C 192 FORMAT('/' ELEMENT 03, R/C SHEAR WALL ELEMENT '/' T17,
ELE03940 & ' ELEM', ' BEND', ' SHEAR', ' AXIAL', ' JOINT', ' JOINT',
ELE03950 & ' JOINT', ' JOINT', ' LENGTH', ' WIDTH',
ELE03960 & ' ALPHA', ' PKG', '/', T17,
ELE03970 & ' MATL', ' MATL', ' MATL', ' #1', ' #2',
ELE03980 & ' #3', ' #4')
ELE03990
ELE04000 C 193 FORMAT(1X, A15, B16, 2X, 1P, A12, 4)
ELE04010 IOPT=IOPT
ELE04020 RETURN
ELE04030
ELE04040 C----- DETERMINE GEOMETRIC STIFF -----
ELE04050 C- KG IS DETERMINED FOR A UNIT LOAD WITH IOPT=1.
ELE04060 C- THE ACTUAL LOAD (PGEOM) IS DETERMINED HERE.
ELE04070 C- KG IS MULTIPLIED BY PGEOM WHEN THE TOTAL STIFFNESS IS ASSEMBLED
ELE04080 C- NEGATIVE PGEOM IS COMPRESSION....
ELE04090
ELE04100 AXIAL=.TRUE.
ELE04110 C----- DETERMINE THE AXIAL LOAD TO BE USED FOR KG...
ELE04120 IF (KGDATA(1).EQ.1) THEN
ELE04130 PGEOM = PKG
ELE04140 ELSE IF (KGDATA(1).EQ.2) THEN
ELE04150 PGEOM = -F(3)
ELE04160 ENDF
ELE04170
ELE04180
ELE04190
ELE04200
ELE04210
ELE04220
ELE04230
ELE04240
ELE04250
ELE04260
ELE04270
ELE04280
ELE04290
ELE04300
ELE04310
ELE04320
ELE04330
ELE04340
ELE04350
ELE04360
ELE04370
ELE04380
ELE04390
ELE04400
ELE04410
ELE04420
ELE04430
ELE04440
ELE04450
ELE04460
ELE04470
ELE04480
ELE04490
ELE04500
ELE04510
ELE04520
ELE04530
ELE04540
ELE04550
ELE04560
ELE04570
ELE04580
ELE04590
ELE04600
ELE04610
ELE04620
ELE04630
ELE04640
ELE04650
ELE04660
ELE04670
ELE04680
ELE04690
ELE04700
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IF (BTST(IBUG,7)) WRITE (6,*) 'AXIAL ',AXIAL,' PGEOM1',PGEOM
C
RETURN
C----- DETERMINE STIFFNESS -----
300 CONTINUE
SB=SB
SS=SS
SA=SA
IF (BTST(IBUG,7)) WRITE (6,402) IELNO,SB,SS,SA
IF (BTST(IBUG,7)) WRITE (6,402) IELNO,SE(ISEB),SE(ISES),SE(ISEA)
C----- SB, SS AND SA CONTAIN THE STIFFNESSES, WHICH WERE DETERMINED IN
C THE LAST CALL TO I11B
C IF SB=SE(ISEB) AND SS=SE(ISES) AND SA=SE(ISEA)
C THEN THE STIFFNESS HAS NOT CHANGED, RETURN
C
IF (SB.EQ.SE(ISEB).AND.SS.EQ.SE(ISES).AND.SA.EQ.SE(ISEA)) RETURN
C----- SET UP THE LOCAL STIFFNESS MATRIX
SB=SE(ISEB)
SS=SE(ISES)
SA=SE(ISEA)
IF (BTST(IBUG,7)) WRITE (6,402) IELNO,SB,SS,SA
SBI=SE(ISEB)/LENGTH
SSI=SE(ISES)/LENGTH
SAL=SE(ISEA)/LENGTH
C----- CALCULATE SAT
DO 360 J=1,IELDOF
SAT(J,3)=SBI*A(J,1)
SAT(J,2)=SSI*A(J,2)
360 SAT(J,1)=SAL*A(J,3)
C----- CALCULATE ASAT, STORE IN UPPER TRIANGULAR MATRIX
L=0
DO 370 J=1,IELDOF
DO 370 I=J,1,-1
L=L+1
DO 370 K=1,3
370 DSTIFF(L)=DSTIFF(L)+A(I,K)*SAT(K,J)
371 STIFF(J)=DSTIFF(J)
IF (BTST(IBUG,7)) THEN
WRITE (6,*) 'IELDOF',IELDOF,'LM=',(LM(I),I=1,IELDOF)
CALL LMATRIX(STIFF,MD,IELDOF,(L-1),'WALL STIFFNESS')
ENDIF
RETURN
C----- DETERMINE INCREMENTAL FORCES -----
400 CONTINUE
IF (PRINT.AND.FIRST) WRITE (6,491) STEPID
IF (BTST(IBUG,7)) THEN
WRITE (6,401) IELNO
401 FORMAT (1X,30(' '),$SHEAR WALL, ELEMENT#',I6,1X,30(' '),$)
ENDIF
C----- SET UP THE COMPONENT STIFFNESSES
SB=SE(ISEB)
SS=SE(ISES)
SA=SE(ISEA)
IF (BTST(IBUG,7)) WRITE (6,402) IELNO,SB,SS,SA
402 FORMAT (1X,IELNO',11,' SB',IP,G12.5,' SS',G12.5,' SA',G12.5)
C----- LOOP FOR EACH LOAD
DO 430 LOAD=1,NLOAD
C----- TRANSFORM FRAME DISPL INTO LOCAL DISPL
DO 410 I=1,3
R(I)=0
V(I)=0
DO 410 J=1,IELDOF
K=LM(J)
R(I)=R(I)+A(J,I)*DISPL(K,LOAD)
V(I)=V(I)+A(J,I)*VELOC(K)
410
C----- INCREMENTAL FORCES
F(1)=SB*R(1)/LENGTH
F(2)=SS*R(2)/LENGTH
F(3)=SA*R(3)/LENGTH
C----- CHECK STIFFNESS AND CALCULATE UNBALANCED FORCES
C THIS IS PREPARED ONLY FOR INELASTIC ANALYSIS
P(1)=F(1)+F(1)
P(2)=F(2)+F(2)
P(3)=F(3)+F(3)
IF (.NOT. ELSTIC) THEN
-- TRANSFER PERMANENT Hysteresis DATA TO TEMPORARY STORAGE...
ISE2=ISE2
DO 415 I=1,ISE2
SE(I)=SE(I)+SE(I)
415
-- BENDING -----
-- SET UP TEMPORARY - TOTAL LOADS, DISPL'S AND VELOC
P(1)=F(1)
DL=(R(1)+R(1))/LENGTH
VC=V(1)+V(1)
V(1)=V(1)+V(1)
-- CALL HYST MODEL TO GET NEW STIFFNESS AND
-- THE TOTAL FORCE P AT DISPL D.
MOPP=3
CALL MATLIB
( MOPP,LSTTYP,FALSE ,IREL,FALSE,NAME,
& ESEB,PSEB,DUCT(1,1),EXCR(1,1),
& P(1),PL,D,DL,VC,VL,LELEM ,LMAT ,
& MATB ,MATTP,SE(ISE2+ISES),IELNO,DAMAGE)
IF (SE(ISE2+ISEB).NE.SE(ISES)) NEWK=.TRUE.
C -- SHEAR -----
-- SET UP TEMPORARY - TOTAL LOADS, DISPL'S AND VELOC
P(1)=F(1)
DL=(R(1)+R(1))/LENGTH
VC=V(1)+V(1)
V(1)=V(1)+V(1)
-- CALL HYST MODEL TO GET NEW STIFFNESS AND
-- THE TOTAL FORCE P AT DISPL D.
MOPP=3
CALL MATLIB
( MOPP,LSTTYP,FALSE ,IREL,FALSE,NAME,
& ESEA,PSEA,DUCT(1,3),EXCR(1,3),
& P(1),PL,D,DL,VC,VL,LELEM ,LMAT ,
& MATA ,MATTP,SE(ISE2+ISEA),IELNO,DAMAGE)
IF (SE(ISE2+ISEA).NE.SE(ISEA)) NEWK=.TRUE.
ENDIF

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C ESE=(E5EB+E5ES+E5EA)\*LENGTH  
PSE=(P5EB+P5ES+P5EA)\*LENGTH  
ENDIF  
IF (PRINT) WRITE (IUNIT,710) ID(J1),ID(J2),ID(J3),ID(J4)  
WRITE (IUNIT,711) (FT(I),RT(I),I=1,3),ESE,PSE  
& (DUCT(I,J),EXCR(I,J),I=1,3),J=1,3)  
710 FORMAT (4I5,1P,8E10.3/(8E10.3))  
711 FORMAT (1P,8E10.3)  
RETURN  
C----- READ AND PRINT MEMBER FORCES FROM OUTPUT FILE -----  
800 CONTINUE  
SX=0  
SXX=0  
SY=0  
SKY=0  
SN=0  
BACKSPACE(IUNIT)  
READ (IUNIT,\*) ITYPE,IELNO,IWRITE,TO,DT  
READ (IUNIT,710) NODE1,NODE2,NODE3,NODE4  
ISTEP=IWRITE  
AVEMV=0  
ICOUNT=0  
CC810 READ (IUNIT,815,END=830) NODE1,NODE2,NODE3,NODE4,(F(I),R(I),I=1,3),  
& ESE,PSE,(DUCT(I,J),EXCR(I,J),I=1,3),J=1,3)  
CC815 FORMAT (4I5, 8E10.3/(8E10.3))  
810 READ (IUNIT,815,END=830) (F(I),R(I),I=1,3),ESE,PSE  
815 FORMAT (8E10.3)  
ISTEP=IWRITE  
IT=TO-DT\*ISTEP  
IF (ISTEP.EQ.0) WRITE (6,805) NODE1,NODE2,NODE3,NODE4  
IF (MOD(ISTEP,INC),EQ.0)  
& WRITE (6,820) ISTEP,T,(F(I),R(I),I=1,3),ESE,PSE  
SX = SX + F(2)  
SXX = SXX + F(2)\*F(2)  
SY = SY + F(1)  
SKY = SKY + F(1)\*F(2)  
SN = SN + F(1)  
IF (F(2),NE.0) THEN  
RATMV= F(1)/F(2)  
IF (ABS(RATMV),LT.25) THEN  
AVEMV=AVEMV+RATMV  
ICOUNT=ICOUNT+1  
ENDIF  
ENDIF  
GO TO 810  
830 CONTINUE  
AVEMV=AVEMV/ MAX(ICOUNT,1)  
WRITE (6,831) AVEMV,ICOUNT  
IF (SY,NE.0) THEN  
RM=SY/SX  
ICOUNT=SN  
WRITE (6,832) RM,ICOUNT  
ENDIF  
831 FORMAT (7F10, 'AVERAGE MOMENT/SHEAR RATIO'-.1P,G12.4,5X,OP.16,  
& ' POINTS WERE USED',  
& ' POINTS WITH V=0 ARE EXCLUDED')  
832 FORMAT (7F10, 'REGRESSION MOMENT-SHEAR EQU:',  
& ' M =',1P,G12.4, ' \* V +',G12.4,5X,  
& OP.16, ' POINTS WERE USED')  
833 FORMAT (7F10, 'REGRESSION MOMENT-SHEAR EQU:',  
& ' M =',1P,G12.4, ' \* V',12X,5X,  
& OP.16, ' POINTS WERE USED')  
WRITE (6,835)  
C  
WRITE (6,840) 'BENDING COMPONENT', (DUCT(I,1),EXCR(I,1),I=1,3)  
C  
WRITE (6,840) 'SHEAR COMPONENT', (DUCT(I,2),EXCR(I,2),I=1,3)  
C  
WRITE (6,840) 'AXIAL COMPONENT', (DUCT(I,3),EXCR(I,3),I=1,3)  
C  
805 FORMAT (//, 'R/C SHEAR WALL FORCES...//'  
& 5X,'NODE1',16,5X,'NODE2',16,5X,'NODE3',16,5X,'NODE4',16,11  
& ' STEP TIME'  
& ' ROTATION'  
& ' SHEAR SHEAR DISPL.'  
& ' AXIAL AXIAL DISPL.'  
& ' ESE PSE //')  
820 FORMAT (16,1P,G12.4,6G14.5,2G12.4)  
835 FORMAT (/7F10, ' MAXIMUM DUCTILITIES AND EXCURSION RATIOS'//  
& ' T10, '  
840 FORMAT (/7F10, '  
& T15, 'DISPLACEMENT DEFINITIONS: U1=',1P,G12.4,5X,'E1=',G12.4//  
& T15, 'ENERGY DEFINITION #1: U2=',1P,G12.4,5X,'E2=',G12.4//  
& T15, 'ENERGY DEFINITION #2: U3=',1P,G12.4,5X,'E3=',G12.4)  
RETURN  
C----- DETERMINE THE LENGTH OF THE MATERIAL DATA  
900 CONTINUE  
ISE=0  
C----- LOOP FOR EACH MATERIAL -----  
DO 910 I=1,3  
IF (I.EQ.1) THEN  
ISE=ISE+1  
ELSE IF (I.EQ.2) THEN  
ISE=ISE+1  
ELSE IF (I.EQ.3) THEN  
ISE=ISE+1  
ENDIF  
MATI = RINP(I)  
CALL MATLIB  
& (0, LETTYP, FALSE, IREL, FALSE, NAME,  
& ESEB, PSEB, DUCT(1,2), EXCR(1,2),  
& FT, F, RT, R, .0, .0, .0, 'LELEM, LMAT',  
& MATI, MATTYP, SE(ISEB), IELNO, DAME)  
910 ISE=ISE + LELEM  
C----- DOUBLE THE STORAGE, TO ALLOW FOR TEMPORARY VALUES...  
ISE=ISE\*2  
C  
RETURN  
C----- DETERMINE THE STRAIN ENERGY  
1000 CONTINUE  
C----- DUCTILITIES AND EXCURSION RATIOS  
... NOTE ELEC3 WAS LAST CALLED TO CALC INCREMENTAL DISPL.  
THE ENERGIES ASSOCIATED WITH INCR DISPL ARE STORED IN  
THE ADDRESSES: ISEB+ISE/2  
ISES+ISE/2  
ISEA+ISE/2  
C----- BENDING ENERGY, DUCTILITY AND EXCURSION .....  
ISE2=ISE/2  
CALL MATLIB  
& (4, LETTYP, FALSE, IREL, FALSE, NAME,  
& ESEB, PSEB, DUCT(1,1), EXCR(1,1),  
& FT, F, RT, R, .0, .0, .0, 'LELEM, LMAT',  
& MATB, MATTYP, SE(ISEB+ISE2), IELNO, DAME)  
C----- SHEAR ENERGY, DUCTILITY AND EXCURSION .....  
CALL MATLIB  
& (5, LETTYP, FALSE, IREL, FALSE, NAME,  
& ESEB, PSEB, DUCT(1,1), EXCR(1,1),  
& FT, F, RT, R, .0, .0, .0, 'LELEM, LMAT',  
& MATB, MATTYP, SE(ISEB+ISE2), IELNO, DAME)  
RETURN

```

END
-----
C DEBUG UNIT(6), TRACE, SUBCHK, INIT, SUBTRACE
C END DEBUG
-----
SUBROUTINE ELE08
  ILOPT, IUNIT, NEWK, FIRST, PRINT
  NDOF, NLOAD, MAXNOD, NNODE, NCCS
  ID, IOPT, COORD, COSINE, JTCS
  CONST, DISPL, VELOC, FORCE, PUB
  IREL, BUG, IBUG, ACCEL, KGDATA
  GRAY, POGON, ELSTIC, NAME, STEPID
  IELNO, RINPUT, ESE, EPSE, INC
  DAMAGE, DUCFAG,
  IELTYP, IELDOF, NODEI, NODEJ, JOINTI
  JOINTJ, R, RT, F, H, LENGTH
  V, VT, ISE, XE, LENGTH,
  MAT, A, LM, KTYPE, FMAX
  SE, STIFF
  IMPLICIT REAL(A-H,O-S)
  COMMON /RSA/ICOMN, RSAF(12)
  COMMON /DSTEP/ISTEP
  REAL ICOMN
  LOGICAL ERR, FIRST, PRINT, BUG, FALSE, NEWK, ELSTIC
  CHARACTER*20 NAME
  CHARACTER*(*) STEPID
  CHARACTER*9 TYPE(1)
  DIMENSION IOPT(MAXNOD,6),COORD(MAXNOD,6),COSINE(3,3,NCOS)
  DIMENSION CONST(MAXNOD,6), ID(MAXNOD), JTCS(MAXNOD)
  DIMENSION SRT(1,2),STIFF(78)
  DIMENSION A(12,2),S(2,2),LM(1,2),FMAX(2)
  DIMENSION CTS( 3, 3),CTE( 3, 3),VX(3),VT(3),VE(3)
  DIMENSION BS( 3, 3),BE( 3, 3),SE(1SE)
  DIMENSION DIMENOF(NLOAD),PUB(NDOF)
  DIMENSION VELOC(NDOF)
  DIMENSION FORCE(NDOF,NLOAD)
  DIMENSION RINPUT(100),WORK(6),DUCT(3),EXCR(6)
  REAL LENGTH
  REAL IX, IY, IZ, KIIZ, KIJZ, KIJJ, KIIT, KIJT, KIYI, KTI, TTJ
  INTEGER REL, RELT
  DIMENSION MD(13)
  DATA MD/1,2,4,7,11,16,22,29,37,46,56,67,79/
  DATA TYPE/' AXIAL'/
  C-----
  C VARIABLES:
  C-----
  C----- GLOBAL VARIABLES
  C IOPT = 1, INITIALIZE BEAM COLUMN ELEMENT
  C = 2, CALCULATE GEOMETRIC STIFFNESS
  C = 3, FORM STIFFNESS
  C = 4, CALCULATE INCREMENTAL FORCES
  C = 5, CALCULATE TOTAL FORCES AND ENERGIES
  C IUNIT = OUTPUT FILE UNIT # FOR PRINTING OUTPUT
  C ERR = FLAG FLAG, IF AN ERR OR HAS OCCURRED, ERR=.TRUE.
  C FIRST = FLAG, FIRST=.TRUE., PRINT HEADERS
  C PRINT = FLAG, PRINT=.TRUE., PRINT DATA
  C NDOF = # OF GLOBAL DOF
  C NLOAD = # OF LOAD COMBINATIONS
  C MAXNOD = # ROW DIMENSION OF NODE ARRAY
  C NNODE = # OF NODES
  C ID = ARRAY OF EXTERNAL NODE NUMBERS
  C IOPT = ARRAY OF DEGREES OF FREEDOM
  C COORD = ARRAY OF COORDINATES
  C COSINE = ARRAY OF DIRECTION COSINES OF NODES
  C CONST = ARRAY OF CONSTRAINT TRANSFORMATIONS
  C DISPL = GLOBAL DISPLACEMENT MATRIX
  C N2 = REAL GLOBAL STORAGE VECTOR
  C N2 = INTEGER GLOBAL STORAGE VECTOR
  C ZMAT = LOCATION OF MATERIAL DATA IN GLOBAL STORAGE VECTOR
  C NAME = ELEMENT NAME
  C IELNO = ELEMENT NUMBER
  C RINPUT = INPUT DATA
  C STIFF = OUTPUT ELEMENT STIFFNESS (UPPER TRIANGULAR FORM)
  C LM = OUTPUT LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
  C-----
  C----- ELEMENT VARIABLES
  C H,Q,AL,AJ,C,BI,BJ,S22,S26,S211,S33,S35,S311,D
  C = INDIVIDUAL TERMS IN THE ELEMENT STIFFNESS (S) MATRIX
  C PKG = INPUT LOAD FOR FORMING GEOMETRIC STIFFNESS MATRIX...
  C R = INCREMENTAL DISPLACEMENTS
  C TR = TOTAL DISPLACEMENTS
  C F = INCREMENTAL FORCES
  C FT = TOTAL FORCES
  C LM = LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
  C CTS = LOCAL TO GLOBAL ROTATION MATRIX START
  C CTE = LOCAL TO GLOBAL ROTATION MATRIX END
  C BS = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX START
  C BE = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX END
  C STIFF = ELEMENT STIFFNESS
  C IELTYP = ELEMENT TYPE = 1-9
  C NODEI = EXTERNAL JOINT NUMBER, END I
  C NODEJ = EXTERNAL JOINT NUMBER, END J
  C JOINTI = INTERNAL JOINT NUMBER, END I
  C JOINTJ = INTERNAL JOINT NUMBER, END J
  C-----
  C----- TEMPORARY VARIABLES
  C VX = VECTOR DEFINING THE ELEMENT X AXIS
  C VY = VECTOR DEFINING THE ELEMENT Y AXIS
  C COSLOC = LOCAL COSINE MATRIX
  C A = GLOBAL TO LOCAL FORCE TRANSFORMATION MATRIX
  C S = ELEMENT STIFFNESS AT LOCAL COORD, AT ENDS OF FLEXIBLE PART
  C SAT = PRODUCT OF S*A
  C ASAT = PRODUCT OF A*SAT
  C-----
  C----- CHOOSE OPTION
  C IF (ILOPT.GT.1.OR.ILOPT.GT.14)
  C WRITE (6,*) 'INVALID IOPT IN ELE-08, IOPT=',IOPT
  C GO TO (100,200,300,400,500,600,700,800,900,1000,1100,1200,
  C 1300,1400),IOPT
  C 100 CONTINUE
  C-----
  C----- INTERPERATE INPUT DATA
  IELTYP=3
  MAT = RINPUT( 1)
  NODEI = RINPUT( 2)
  NODEJ = RINPUT( 3)
  KTYPE = RINPUT( 4)
  SLK = RINPUT( 5)
  VT(1) = RINPUT( 6)
  VT(2) = RINPUT( 7)
  VT(3) = RINPUT( 8)
  XS = RINPUT( 9)
  XE = RINPUT(10)
  C-----
  C----- GET INTERNAL NODE # ASSOCIATED WITH EXTERNAL NODE #'S
  JOINTI=IQUICK(NODEI, ID, NNODE)
  JOINTJ=IQUICK(NODEJ, ID, NNODE)
  WRITE (6,*) 'ID',ID
  C-----
  C IF (KTYPE.LT.1.OR.KTYPE.GT.1) THEN
  C WRITE(6,102) IELNO,NODEI,NODEJ,KTYPE
  C 102 FORMAT(1X,20(' ',ER, 'ROR - INVALID KTYPE' /
  C 5X, 'IELNO=',15,5X, 'JOINTI=',15,5X, 'JOINTJ=',15,
  C 5X, 'KTYPE=',15, /
  C-----
  C----- GET GLOBAL TO LOCAL TRANSFORMATION MATRIX
  C-----
  C----- DEFINE VECTOR X, LENGTH
  VX(1)=COORD(JOINTJ,1)-COORD(JOINTI,1)
  VX(2)=COORD(JOINTJ,2)-COORD(JOINTI,2)
  VX(3)=COORD(JOINTJ,3)-COORD(JOINTI,3)
  LENGTH=SQRT(VX(1)**2+VX(2)**2+VX(3)**2)-XS-XE
  SE(5)=LENGTH
  SE(2)=SLK
  IF (LENGTH.LE.0) THEN
  C WRITE(6,101) IELNO,JOINTI,JOINTJ,LENGTH
  C 101 FORMAT(1X,20(' ',ER, 'ROR - LENGTH IS LE 0' /
  C 5X, 'IELNO=',15,5X, 'JOINTI=',15,5X, 'JOINTJ=',15,
  C 5X, 'LENGTH=',1F,0.15,6 /
  C 5X, 'REVISE INPUT...')
  C LENGTH=1.0
  C ENDF
  IF (BESTEST( IBUG,7) ) THEN
  C WRITE (6,*) 'JOINTS',JOINTI,JOINTJ
  C WRITE (6,*) 'VX',VX
  C WRITE (6,*) 'VT',VT
  C WRITE (6,*) 'LENGTH',LENGTH
  C ENDF
  C-----
  C----- GET LOCAL TO GLOBAL TRANSFORMATION MATRICES CT AND B
  ICS=JTCS(JOINTI)
  ICE=JTCS(JOINTJ)
  CALL ROTYX(VX,VT,COSINE(1,1,ICS),CTS, XS, .0., .0., .BS,
  C & CONST(JOINTI,1),MAXNOD)
  CALL ROTYX(VX,VT,COSINE(1,1,ICE),CTE,-XE, .0., .0., .BE,
  C & CONST(JOINTJ,1),MAXNOD)
  IF (BESTEST( IBUG,7) ) THEN
  C CALL WMATX( CTS, 3, 3, 'CTS ' )
  C CALL WMATX( CTE, 3, 3, 'CTE ' )
  C ENDF
  C-----
  C----- GET GLOBAL CONSTRAINT MATRIX BS, BE
  BS(1,1)=CONST(JOINTI,1)*CTS(2,1)+CONST(JOINTI,2)*CTS(3,1)+BS(1,1)
  BS(1,2)=CONST(JOINTI,1)*CTS(2,2)+CONST(JOINTI,2)*CTS(3,2)+BS(1,2)
  BS(1,3)=CONST(JOINTI,1)*CTS(2,3)+CONST(JOINTI,2)*CTS(3,3)+BS(1,3)
  BS(2,1)=CONST(JOINTI,3)*CTS(1,1)+CONST(JOINTI,4)*CTS(3,1)+BS(2,1)
  BS(2,2)=CONST(JOINTI,3)*CTS(1,2)+CONST(JOINTI,4)*CTS(3,2)+BS(2,2)
  BS(2,3)=CONST(JOINTI,3)*CTS(1,3)+CONST(JOINTI,4)*CTS(3,3)+BS(2,3)
  BS(3,1)=CONST(JOINTI,5)*CTS(1,1)+CONST(JOINTI,6)*CTS(2,1)+BS(3,1)
  BS(3,2)=CONST(JOINTI,5)*CTS(1,2)+CONST(JOINTI,6)*CTS(2,2)+BS(3,2)
  BS(3,3)=CONST(JOINTI,5)*CTS(1,3)+CONST(JOINTI,6)*CTS(2,3)+BS(3,3)
  C-----
  C----- GET GLOBAL CONSTRAINT MATRIX BS, BE
  BE(1,1)=CONST(JOINTJ,1)*CTE(2,1)+CONST(JOINTJ,2)*CTE(3,1)+BE(1,1)
  BE(1,2)=CONST(JOINTJ,1)*CTE(2,2)+CONST(JOINTJ,2)*CTE(3,2)+BE(1,2)
  BE(1,3)=CONST(JOINTJ,1)*CTE(2,3)+CONST(JOINTJ,2)*CTE(3,3)+BE(1,3)
  BE(2,1)=CONST(JOINTJ,3)*CTE(1,1)+CONST(JOINTJ,4)*CTE(3,1)+BE(2,1)
  BE(2,2)=CONST(JOINTJ,3)*CTE(1,2)+CONST(JOINTJ,4)*CTE(3,2)+BE(2,2)
  BE(2,3)=CONST(JOINTJ,3)*CTE(1,3)+CONST(JOINTJ,4)*CTE(3,3)+BE(2,3)
  BE(3,1)=CONST(JOINTJ,5)*CTE(1,1)+CONST(JOINTJ,6)*CTE(2,1)+BE(3,1)
  BE(3,2)=CONST(JOINTJ,5)*CTE(1,2)+CONST(JOINTJ,6)*CTE(2,2)+BE(3,2)
  BE(3,3)=CONST(JOINTJ,5)*CTE(1,3)+CONST(JOINTJ,6)*CTE(2,3)+BE(3,3)
  C-----
  C IF (BESTEST( IBUG,7) ) THEN
  C CALL WMATX(BS, 3, 3, 'BS ' )
  C CALL WMATX(BE, 3, 3, 'BE ' )
  C ENDF
  C-----
  C----- ZERO MATRICES
  F=0
  R=0
  SRT=0
  V=0
  W=0
  FMAX(1)=0
  FMAX(2)=0
  C-----
  C----- ASSEMBLE TRANSFORMATION MATRIX A
  C NOTE: A IS DIMENSIONED AS A 12X2 MATRIX, BUT ONLY THE FIRST IELDOF
  C ROWS CONTAIN INFORMATION. THE BALANCE OF THE MATRIX IS
  C UNDEFINED. THE GLOBAL DOF CORRESPONDING TO THE ROWS OF A
  C ARE STORED IN VECTOR LM. AGAIN, ONLY THE FIRST IELDOF ROW'S
  C OF LM ARE DEFINED.
  IELDOF=0
  DO 50 I=1,12
  C REMOVE DOF'S THAT ARE CONSTRAINED TO THE SAME DOF...
  J=I
  IF (I.GT.6) J=I-6
  IF ( IDOF(JOINTI,J),EQ.IDOF(JOINTJ,J) ) GO TO 50
  IF (I.LE.6) THEN
  C LM(I)=IDOF(JOINTI,I)
  ELSE
  C LM(I)=IDOF(JOINTI,J)
  C ENDF
  C-----
  C A1=0
  C A2=0
  C IF (I.LE.3) THEN
  C IF (KTYPE.LE.3) A1=CTS(I,KTYPE)
  C ELSE IF (I.LE.6) THEN
  C IF (KTYPE.LE.3) THEN
  C A1=BS(I-3,KTYPE)
  C ELSE
  C A1=CTS(I-3,KTYPE-3)
  C ENDF
  C IF (I.LE.9) THEN
  C IF (KTYPE.LE.3) A2=CTE(I-6,KTYPE)
  C ELSE
  C IF (KTYPE.LE.3) THEN
  C A2=BE(I-9,KTYPE)
  C ELSE
  C A2=CTE(I-9,KTYPE-3)
  C ENDF
  C ENDF
  C IF (A1.EQ.0.AND.A2.EQ.0) GO TO 50
  C IELDOF=IELDOF+1
  C A( IELDOF, 1)=A1
  C A( IELDOF, 2)=A2
  C LM( IELDOF)=LMTI
  C 50 CONTINUE
  C IF (BESTEST( IBUG,7) ) CALL WMATX( A, IELDOF, 2, ' A ', 12)
  C-----
  C----- GET GOBL STRUT STIFFNESS
  FALSE=.FALSE.
  LSTYP=0
  CALL MATLIS
  & ( 2, LSTYP, FALSE, IREL, FALSE, NAME, ESE, EPSE, DUCT, EXCR,
  C & FT, 'RT', R, 'D', .0., .LELEM, LMAT,
  C & MAT, MATYP, SE, IELNO, DAMAGE)
  C WRITE (6,*) 'AFTER MA', THIS IN ELE08 '
  C 111 FORMAT(0P, ' ZMATX(1,6,')', I15, I9, G20, 10)
  C H IS THE STRUT STIFFNESS
  C WRITE(6,*) 'AFTER MATLIS-2, STIFF=',SE(1)
  C-----

```



H = SE(1)
HM= SE(1)
S(1,1)= HH
S(1,2)= HH
S(2,1)= HH
S(2,2)= HH
C----- CALCULATE SAT
DO 60 J=1,IELDOF
DO 60 K=1,2
SAT(I,J)=SAT(I,J)+S(I,K)\*A(J,K)
C----- CALCULATE ASAT AND CONVERT TO HALF STORAGE MODE BY COLUMNS
C-----
1 3 6 10 15 21 28 36 45 55 66 78 NUMBER INDICATES
2 5 9 14 20 27 35 44 54 65 77 LOCATION OF DATA
4 8 13 19 26 34 43 53 64 76 IN ARRAY STIFF
7 12 18 25 33 42 52 63 75
11 17 24 32 41 51 62 74
16 23 31 40 50 61 73
22 30 39 49 60 72
29 38 48 59 71
37 47 58 70
46 57 69
56 68
L=0
DO 70 J=1,IELDOF
DO 70 I=J,1,-1
L=L+1
STIFF(L)=0
DO 70 K=1,2
STIFF(L)=STIFF(L)+A(I,K)\*SAT(K,J)
IF ( BTEST(IBUG,7) ) THEN
WRITE (6,\*) 'IELDOF', IELDOF, 'LM =', LM(I), I=1, IELDOF
CALL LMATRX(STIFF, MD, IELDOF, (L-1), 'STRUT STIFFNESS')
ENDIF
C----- RELEASE UNUSED STORAGE
180 IREL=78-L
PRINT COLUMN DATA
IF ( FIRST .OR. BTEST(IBUG,7) .OR. BTEST(IBUG,8) ) WRITE (6,192)
191 WRITE (IELNO, MAT, NODEI, NODEJ, TYPE(KTYPE), LENGTH,
6 VT(1), VT(2), VT(3), XS, -XE
192 FORMAT ('// ELEMENT 08, ONE (LOCAL) DOP STRUT ELEMENT//
4 T2, # MATL START END, X, #-TYPE=, #, X, LENGTH, J,
6 11(2), T-AXIS ,
4 12(2), START DIST END DIST')
193 FORMAT ('X, A15, A16, 2X, A9, 3X, 1P, G12.4, OP
6 # F9.5, # 1', SP, F9.5, ' #', F9.5, ' K', 1P, SS, G12.4)
IOPT=IOPT+1
C----- DETERMINE GEOMETRIC STIFF -----
200 RETURN
C THE GEOMETRIC STIFFNESS MATRIX IS NOT AVAILABLE FOR THIS ELEMENT
C----- DETERMINE STIFFNESS -----
300 CONTINUE
C----- H CONTAINS THE STRUT STIFFNESS, WHICH WAS DETERMINED IN THE
LAST CALL TO MAT\_LIB
IF H=SE(1) THEN THE STIFFNESS HAS NOT CHANGED, RETURN
IF (H.EQ.SE(1)) RETURN
H = SE(1)
HM= SE(1)
S(1,1)= HH
S(1,2)= HH
S(2,1)= HH
S(2,2)= HH
C----- CALCULATE SAT
DO 360 J=1,IELDOF
DO 360 K=1,2
SAT(I,J)=SAT(I,J)+S(I,K)\*A(J,K)
C----- CALCULATE ASAT, STORE IN UPPER TRIANGULAR MATRIX
L=0
DO 370 J=1,IELDOF
DO 370 I=J,1,-1
L=L+1
STIFF(L)=0
DO 370 K=1,2
STIFF(L)=STIFF(L)+A(I,K)\*SAT(K,J)
IF ( BTEST(IBUG,7) ) THEN
WRITE (6,\*) 'IELDOF', IELDOF, 'LM =', LM(I), I=1, IELDOF
CALL LMATRX(STIFF, MD, IELDOF, (L+1), 'STRUT STIFFNESS')
ENDIF
IOPT=IOPT+1
RETURN
C----- DETERMINE INCREMENTAL FORCES -----
400 CONTINUE
IF (PRINT.AND.FIRST) WRITE (6,491) STEPID
DO 430 I=1, NLOAD
C----- TRANSFORM FRAME DISPL INTO LOCAL DISPL
C... LOCAL DISPL ARE AT THE END OF THE STRUT, THUS
C... POSITIVE IS TENSION AND NEGATIVE IS COMPRESSION
V=0
DO 410 I=1, IELDOF
J=LM(I)
A1= A(I,1)
A2= A(I,2)
DISP= DISPL(J,LOAD)
VEL= VELOC(J)
410 R = ( A(I,2)-A(I,1) ) \* DISPL(J,LOAD)
V = V + ( A(I,2)-A(I,1) ) \* VELOC(J)
C----- INCREMENTAL FORCES
F = H\*R
C----- CHECK STIFFNESS AND CALCULATE UNBALANCED FORCES
THIS IS PERFORMED ONLY FOR INELASTIC ANALYSIS
P = FT-F
IF (.NOT. ELSTIC) THEN
-- SET UP TEMPORARY - TOTAL LOADS, DISPL'S AND VELOC
PL=FT
D = (RT+R)
DL=RT
VC=VT+V
VL=VT
C----- TRANSFER PERMANENT HYSTERESIS DATA TO TEMPORARY STORAGE...
ISE2=ISE2/2
DO 415 I=1, ISE2
SE(I)=SE(I)
-- CALL HYST MODEL TO GET NEW STIFFNESS AND
-- THE TOTAL FORCE P AT DISPL D.
MOPT=3
CALL MATLIB
( MOPT, LSTTYP, FALSE, IREL, FALSE, NAME, ESEB, EPSE, DUCT, EXCR,
, PL, D, DL, VC, VL, VELEM, LMAT,
, MAT, MATTYP, SE, IELNO, DAMAGE)
IF ( SE( ISE2+1 ), NE, SE( I ) ) NEWK=.TRUE.
WRITE (6,\*) 'AFTER MATLIB=2, STIF=', SE( I SE2+1 )
ENDIF
C----- SAVE PEAK VALUES FOR MULTIPLE LOAD CASES(FOR STATICS CASE)
IF ( NLOAD.GT.1 .AND. ABS(F).GT.ABS(FMAX(1)) ) THEN
FMAX(1)=F
FMAX(2)=R
ENDIF
C----- PRINT DATA
HM= H
IF (PRINT) WRITE (6,492) IELNO, LOAD, TYPE(KTYPE), NODEI, NODEJ, F, R, H, H,
430 CONTINUE
C----- THIS IS FOR SOL5A USE ONLY
IF ( ICOMN.EQ.1 .OR. ICOMN.EQ.2 ) THEN
RSAFE(1)=F
ENDIF
C----- CALCULATE THE UNBALANCED MEMBER FORCE ON A JOINT.
UBAL= F-FT-P
F = FT
FT = P
DO 440 I=1, IELDOF
IF ( BTEST(IBUG,7) ) WRITE (6,493) F, FT, P
P = P
A1= A(I,1)
A2= A(I,2)
FUB(J)= FUB(J) + ( A(I,1)-A(I,2) ) \* UBAL
C WRITE (6,\*) 'UBAL, FUB(J), A(I,1), A(I,2)'
IF ( BTEST(IBUG,7) )
IF ( BTEST(IBUG,7) )
WRITE (6,494) ( I, LM(I), FUB(I), I=1, IELDOF)
440 CONTINUE
491 FORMAT ('// GOEL FORCES...', 5X, A, //
, ELEMENT LOAD', T17, ' TYPE', ' NODEJ, ' NODEJ, '
, FORCE ', ' DISPLACEMENT ', ' STIFFNESS')
492 FORMAT (' I15, 2X, A9, 2T6, 1P, G15.6)
493 FORMAT (' F1, 1P, G16.8, ' FT1, G16.8, ' P1, G16.8)
494 FORMAT (' I1, 15, ' LM(I), I6, ' FUB(I), 1P, G16.8)
C----- ADJUST P FOR UNBALANCED LOAD P
P=P-FT
IOPT=IOPT+1
RETURN
C----- DETERMINE TOTAL FORCES, ENERGIES ECT -----
500 CONTINUE
IF ( ICOMN.NE.1 .AND. ICOMN.NE.2 ) THEN
IF (PRINT.AND.FIRST) WRITE (6,491) STEPID
C----- TOTAL FORCES, DISPLACEMENTS AND VELOCITIES
FT=FT+P
RT=RT+R
VT=VT+V
IF ( ABS(FT).GT.ABS(FMAX(1)) ) THEN
FMAX(1)=FT
ENDIF
IF ( ABS(RT).GT.ABS(FMAX(2)) ) THEN
FMAX(2)=RT
ENDIF
C----- PRINT DATA
HM= H
IF (PRINT) WRITE(6,492) IELNO, I, TYPE(KTYPE), NODEI, NODEJ, FT, RT, H, H
ELSE IF ( ICOMN.EQ.1 .OR. ICOMN.EQ.2 ) THEN
IF (PRINT.AND.FIRST) WRITE (6,497) STEPID
IF (PRINT) WRITE(6,492) IELNO, I, TYPE(KTYPE), NODEI, NODEJ, FT, H, H
ENDIF
497 FORMAT ('// GOEL FORCES...', 5X, A, //
, ELEMENT LOAD', T17, ' TYPE', ' NODEJ, '
, FORCE ', ' STIFFNESS')
C----- TRANSFER TEMPORARY HYSTERESIS DATA TO PERMANENT STORAGE...
ISE2=ISE2/2
IF (.NOT. ELSTIC) THEN
DO 510 I=1, ISE2
SE(I)=SE(I+ISE2)
ENDIF
RETURN
C----- DETERMINE FIXED END FORCES, AND ADD TO FORCE=
600 RETURN
C----- FIXED END FORCES ARE NOT AVAILABLE FOR THE STRUT
C----- WRITE MEMBER FORCES TO OUTPUT FILE -----
700 CONTINUE
CALL MATLIB
( 4, LSTTYP, FALSE, IREL, FALSE, NAME, ESEB, EPSE, DUCT, EXCR,
, FT, RT, R, D, VELEM, LMAT,
, MAT, MATTYP, SE, IELNO, DAMAGE)
WRITE ( IUNIT, 710 ) NODEI, NODEJ, KTYPE, FT, RT, H, ESE, EPSE,
, IF (MOD(ISTEP, INC).EQ.0)
( DUCT(I), EXCR(I), I=1, 3)
710 FORMAT (' I6, 1P, G12.4 / G12.4 )
RETURN
C----- READ AND PRINT MEMBER FORCES FROM OUTPUT FILE -----
800 CONTINUE
BACKSPACE(IUNIT)
READ ( IUNIT, \* ) I, PSE
ISTEP=IWRITE
810 READ ( IUNIT, 815, END=830 ) NODEI, NODEJ, KTYPE, FT, RT, H, ESE, PSE,
, ( WORK(I), I=1, 6)
815 FORMAT (' I6, 1P, G12.4 / G12.4 )
ISTEP=ISTEP+IWRITE
T=TO+DT\*ISTEP
IF ( ISTEP.EQ.0 ) WRITE (6,805) NODEI, NODEJ, TYPE(KTYPE)
IF (MOD(ISTEP, INC).EQ.0)
, WRITE (6,820) ISTEP, T, FT, RT, H, ESE, PSE
GO TO 810
805 FORMAT ('// GOEL MEMBER FORCES...//
, 5X, ' NODEJ', ' I6, 5X, ' NODEJ', ' I6, 5X, ' GOEL'S TYPE', ' A, //
, STEP TIME '
, FORCE ', ' DISPLACEMENT ', ' STIFFNESS',
, ESE, PSE, ' )
820 FORMAT (' I10, 1P, G15.5, G13.5 )
830 WRITE (6,840) ( WORK(I), I=1, 6)
840 FORMAT (' I10, \*\*\*\*\* MAXIMUM DUCTILITIES AND EXCURSION RATIOS//
, I10, \*\*\*\*\*//
, T15, 'DISPLACEMENT DEFINITION: U1=', 1P, G12.4, 5X, ' E1=', G12.4 /
, T15, 'ENERGY DEFINITION #1: U2=', 1P, G12.4, 5X, ' E2=', G12.4 /
, T15, 'ENERGY DEFINITION #2: U3=', 1P, G12.4, 5X, ' E3=', G12.4 /
)
RETURN
C-----
ELE02840 CALL MATLIB ELE04260
ELE02850 ( MOPT, LSTTYP, FALSE, IREL, FALSE, NAME, ESEB, EPSE, DUCT, EXCR, ELE04270
ELE02860 , PL, D, DL, VC, VL, VELEM, LMAT, ELE04280
ELE02870 , MAT, MATTYP, SE, IELNO, DAMAGE) ELE04290
ELE02880 IF ( SE( ISE2+1 ), NE, SE( I ) ) NEWK=.TRUE. ELE04300
ELE02890 ELE04310
ELE02900 C WRITE (6,\*) 'AFTER MATLIB=2, STIF=', SE( I SE2+1 ) ELE04320
ELE02910 C ENDIF ELE04330
ELE02920 C C----- SAVE PEAK VALUES FOR MULTIPLE LOAD CASES(FOR STATICS CASE) ELE04350
ELE02930 IF ( NLOAD.GT.1 .AND. ABS(F).GT.ABS(FMAX(1)) ) THEN ELE04360
ELE02940 FMAX(1)=F ELE04370
ELE02950 FMAX(2)=R ELE04380
ELE02960 ENDIF ELE04390
ELE02970 C ELE04400
ELE02980 C----- PRINT DATA ELE04410
ELE02990 HM= H ELE04420
ELE03000 IF (PRINT) WRITE (6,492) IELNO, LOAD, TYPE(KTYPE), NODEI, NODEJ, F, R, H, H, ELE04430
ELE03010 ELE04440
ELE03020 C 430 CONTINUE ELE04450
ELE03030 C----- THIS IS FOR SOL5A USE ONLY ELE04460
ELE03040 IF ( ICOMN.EQ.1 .OR. ICOMN.EQ.2 ) THEN ELE04470
ELE03050 RSAFE(1)=F ELE04480
ELE03060 ENDIF ELE04490
ELE03070 C ELE04500
ELE03080 C----- CALCULATE THE UNBALANCED MEMBER FORCE ON A JOINT. ELE04520
ELE03090 UBAL= F-FT-P ELE04530
ELE03100 F = FT ELE04540
ELE03110 FT = P ELE04550
ELE03120 DO 440 I=1, IELDOF ELE04560
ELE03130 IF ( BTEST(IBUG,7) ) WRITE (6,493) F, FT, P ELE04570
ELE03140 P = P ELE04580
ELE03150 A1= A(I,1) ELE04590
ELE03160 A2= A(I,2) ELE04600
ELE03170 FUB(J)= FUB(J) + ( A(I,1)-A(I,2) ) \* UBAL ELE04610
ELE03180 C WRITE (6,\*) 'UBAL, FUB(J), A(I,1), A(I,2)' ELE04620
ELE03190 IF ( BTEST(IBUG,7) ) ELE04630
ELE03200 IF ( BTEST(IBUG,7) ) ELE04640
ELE03210 WRITE (6,494) ( I, LM(I), FUB(I), I=1, IELDOF) ELE04650
ELE03220 ELE04660
ELE03230 440 CONTINUE ELE04670
ELE03240 ELE04680
ELE03250 491 FORMAT ('// GOEL FORCES...', 5X, A, // ELE04690
ELE03260 , ELEMENT LOAD', T17, ' TYPE', ' NODEJ, ' NODEJ, ' ELE04700
ELE03270 , FORCE ', ' DISPLACEMENT ', ' STIFFNESS') ELE04710
ELE03280 492 FORMAT (' I15, 2X, A9, 2T6, 1P, G15.6) ELE04720
ELE03290 493 FORMAT (' F1, 1P, G16.8, ' FT1, G16.8, ' P1, G16.8) ELE04730
ELE03300 494 FORMAT (' I1, 15, ' LM(I), I6, ' FUB(I), 1P, G16.8) ELE04740
ELE03310 C----- ADJUST P FOR UNBALANCED LOAD P ELE04750
ELE03320 P=P-FT ELE04760
ELE03330 IOPT=IOPT+1 ELE04780
ELE03340 RETURN ELE04790
ELE03350 C----- DETERMINE TOTAL FORCES, ENERGIES ECT ----- ELE04800
ELE03360 500 CONTINUE ELE04815
ELE03370 IF ( ICOMN.NE.1 .AND. ICOMN.NE.2 ) THEN ELE04820
ELE03380 IF (PRINT.AND.FIRST) WRITE (6,491) STEPID ELE04830
ELE03390 C----- TOTAL FORCES, DISPLACEMENTS AND VELOCITIES ELE04840
ELE03400 FT=FT+P ELE04850
ELE03410 RT=RT+R ELE04860
ELE03420 VT=VT+V ELE04870
ELE03430 IF ( ABS(FT).GT.ABS(FMAX(1)) ) THEN ELE04880
ELE03440 FMAX(1)=FT ELE04890
ELE03450 ENDIF ELE04900
ELE03460 IF ( ABS(RT).GT.ABS(FMAX(2)) ) THEN ELE04910
ELE03470 FMAX(2)=RT ELE04920
ELE03480 ENDIF ELE04930
ELE03490 C----- PRINT DATA ELE04940
ELE03500 HM= H ELE04950
ELE03510 IF (PRINT) WRITE(6,492) IELNO, I, TYPE(KTYPE), NODEI, NODEJ, FT, RT, H, H ELE04960
ELE03520 ELSE IF ( ICOMN.EQ.1 .OR. ICOMN.EQ.2 ) THEN ELE04970
ELE03530 IF (PRINT.AND.FIRST) WRITE (6,497) STEPID ELE04980
ELE03540 IF (PRINT) WRITE(6,492) IELNO, I, TYPE(KTYPE), NODEI, NODEJ, FT, H, H ELE04990
ELE03550 ENDIF ELE05000
ELE03560 497 FORMAT ('// GOEL FORCES...', 5X, A, // ELE05010
ELE03570 , ELEMENT LOAD', T17, ' TYPE', ' NODEJ, ' ELE05020
ELE03580 , FORCE ', ' STIFFNESS') ELE05030
ELE03590 C----- TRANSFER TEMPORARY HYSTERESIS DATA TO PERMANENT STORAGE... ELE05040
ELE03600 ISE2=ISE2/2 ELE05050
ELE03610 IF (.NOT. ELSTIC) THEN ELE05060
ELE03620 DO 510 I=1, ISE2 ELE05070
ELE03630 SE(I)=SE(I+ISE2) ELE05080
ELE03640 ENDIF ELE05090
ELE03650 RETURN ELE05100
ELE03660 C----- DETERMINE FIXED END FORCES, AND ADD TO FORCE= ELE05110
ELE03670 600 RETURN ELE05120
ELE03680 C----- FIXED END FORCES ARE NOT AVAILABLE FOR THE STRUT ELE05130
ELE03690 C----- WRITE MEMBER FORCES TO OUTPUT FILE ----- ELE05140
ELE03700 700 CONTINUE ELE05150
ELE03710 CALL MATLIB ELE05160
ELE03720 ( 4, LSTTYP, FALSE, IREL, FALSE, NAME, ESEB, EPSE, DUCT, EXCR, ELE05170
ELE03730 , FT, RT, R, D, VELEM, LMAT, ELE05180
ELE03740 , MAT, MATTYP, SE, IELNO, DAMAGE) ELE05190
ELE03750 WRITE ( IUNIT, 710 ) NODEI, NODEJ, KTYPE, FT, RT, H, ESE, EPSE, ELE05200
ELE03760 , IF (MOD(ISTEP, INC).EQ.0) ELE05210
ELE03770 ( DUCT(I), EXCR(I), I=1, 3) ELE05220
ELE03780 710 FORMAT (' I6, 1P, G12.4 / G12.4 ) ELE05230
ELE03790 RETURN ELE05240
ELE03800 C----- READ AND PRINT MEMBER FORCES FROM OUTPUT FILE ----- ELE05250
ELE03810 800 CONTINUE ELE05300
ELE03820 BACKSPACE(IUNIT) ELE05300
ELE03830 READ ( IUNIT, \* ) I, PSE ELE05310
ELE03840 ISTEP=IWRITE ELE05320
ELE03850 810 READ ( IUNIT, 815, END=830 ) NODEI, NODEJ, KTYPE, FT, RT, H, ESE, PSE, ELE05330
ELE03860 , ( WORK(I), I=1, 6) ELE05340
ELE03870 815 FORMAT (' I6, 1P, G12.4 / G12.4 ) ELE05350
ELE03880 ISTEP=ISTEP+IWRITE ELE05360
ELE03890 T=TO+DT\*ISTEP ELE05370
ELE03900 IF ( ISTEP.EQ.0 ) WRITE (6,805) NODEI, NODEJ, TYPE(KTYPE) ELE05380
ELE03910 IF (MOD(ISTEP, INC).EQ.0) ELE05390
ELE03920 , WRITE (6,820) ISTEP, T, FT, RT, H, ESE, PSE ELE05400
ELE03930 GO TO 810 ELE05410
ELE03940 805 FORMAT ('// GOEL MEMBER FORCES...// ELE05420
ELE03950 , 5X, ' NODEJ', ' I6, 5X, ' NODEJ', ' I6, 5X, ' GOEL'S TYPE', ' A, // ELE05430
ELE03960 , STEP TIME ' ELE05440
ELE03970 , FORCE ', ' DISPLACEMENT ', ' STIFFNESS', ELE05450
ELE03980 , ESE, PSE, ' ) ELE05460
ELE03990 820 FORMAT (' I10, 1P, G15.5, G13.5 ) ELE05470
ELE04000 830 WRITE (6,840) ( WORK(I), I=1, 6) ELE05480
ELE04010 840 FORMAT (' I10, \*\*\*\*\* MAXIMUM DUCTILITIES AND EXCURSION RATIOS// ELE05490
ELE04020 , I10, \*\*\*\*\*// ELE05500
ELE04030 , T15, 'DISPLACEMENT DEFINITION: U1=', 1P, G12.4, 5X, ' E1=', G12.4 / ELE05510
ELE04040 , T15, 'ENERGY DEFINITION #1: U2=', 1P, G12.4, 5X, ' E2=', G12.4 / ELE05520
ELE04050 , T15, 'ENERGY DEFINITION #2: U3=', 1P, G12.4, 5X, ' E3=', G12.4 / ELE05530
ELE04060 ) ELE05540
ELE04070 RETURN ELE05550
ELE04080 C----- ELE05560
ELE04090 ELE05570
ELE04100 ELE05580
ELE04110 ELE05590
ELE04120 ELE05600
ELE04130 ELE05610
ELE04140 ELE05620
ELE04150 ELE05630
ELE04160 ELE05640
ELE04170 ELE05650
ELE04180 ELE05660
ELE04190 ELE05670
ELE04200 ELE05680
ELE04210 ELE05690
ELE04220 ELE05700
ELE04230 ELE05710
ELE04240 ELE05720
ELE04250 ELE05730
ELE04260 ELE05740
ELE04270 ELE05750
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ELE04290 ELE05770
ELE04300 ELE05780
ELE04310 ELE05790
ELE04320 ELE05800
ELE04330 ELE05810
ELE04340 ELE05820
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ELE04360 ELE05840
ELE04370 ELE05850
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ELE04400 ELE05880
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ELE04590 ELE06070
ELE04600 ELE06080
ELE04610 ELE06090
ELE04620 ELE06100
ELE04630 ELE06110
ELE04640 ELE06120
ELE04650 ELE06130
ELE04660 ELE06140
ELE04670 ELE06150
ELE04680 ELE06160
ELE04690 ELE06170
ELE04700 ELE06180
ELE04710 ELE06190
ELE04720 ELE06200
ELE04730 ELE06210
ELE04740 ELE06220
ELE04750 ELE06230
ELE04760 ELE06240
ELE04770 ELE06250
ELE04780 ELE06260
ELE04790 ELE06270
ELE04800 ELE06280
ELE04810 ELE06290
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ELE05690 ELE07170
ELE05700 ELE07180
ELE05710 ELE07190
ELE05720 ELE07200
ELE05730 ELE07210
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ELE05750 ELE07230
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ELE05800 ELE07280
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ELE07790 ELE09270
ELE07800 ELE09280
ELE07810 ELE09290
ELE07820 ELE09300
ELE07830 ELE09310
ELE07840 ELE09320
ELE07850 ELE09330
ELE07860 ELE09340
ELE07870 ELE09350
ELE07880 ELE09360
ELE07890 ELE09370
ELE07900 ELE09380
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ELE07990 ELE09470
ELE08000 ELE09480
ELE08010 ELE09490
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ELE08470 ELE09950
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ELE08490 ELE09970
ELE08500 ELE09980
ELE08510 ELE09990
ELE08520 ELE10000
ELE08530 ELE10010
ELE08540 ELE10020
ELE08550 ELE10030
ELE0

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C----- DETERMINE THE LENGTH OF THE MATERIAL DATA
900 CONTINUE
ISE=0
C
MAT = RINPUT( 1 )
CALL MATLIB
( 0 ,LSTTYP ,FALSE ,IREL ,FALSE ,NAME ,ESEE ,EPSE , DUCT ,EXCR ,
( FT ,F ,RT ,R ,0 ,0 ,0 ,LELEM ,LMAT ,
( MAT ,MATTYP ,SE ,IELNO ,DAMAGE )
ISE=ISE+LELEM
C
C----- DOUBLE THE STORAGE, TO ALLOW FOR TEMPORARY VALUES...
ISE=ISE*2
RETURN
C
C----- DETERMINE THE STRAIN ENERGY
1000 CONTINUE
C
C----- DUCTILITIES AND EXCURSION RATIOS
1100 CONTINUE
C
C.... OPTIONS 10 AND 11 HAVE THE SAME CODE FOR THIS ELEMENT...
C.... THE ENERGY WAS CALCULATED WITH THE LAST CALL FOR INCREMENTAL FORCE...
C THE TOTAL FORCES HAVE NOT BEEN CALLED YET, THUS THE ENERGIES RESIDUE...
C IN THE MATRIX SE BEGINNING AT ADDRESS ISE21.
C
ISE21=ISE/2 +1
CALL MATLIB
( 4 ,LSTTYP ,FALSE ,IREL ,FALSE ,NAME ,ESEE ,EPSE , DUCT ,EXCR ,
( FT ,F ,RT ,R ,0 ,0 ,0 ,LELEM ,LMAT ,
( MAT ,MATTYP ,SE (ISE21) ,IELNO ,DAMAGE )
C
IF (ABS(DUCT(1)) .LT. 1 ) DUCT(1)=0
IF (DUCFAG .EQ. 0 ) THEN
IF (ABS(DUCT(1)) .LT. 1 ) DUCFAG=0
IF (ABS(DUCT(1)) .GE. 1 ) DUCFAG=ABS(DUCT(1))
ENDIF
C
IF (ELSTIC) ESEE=(FT+F)*(RT*R)*0.50
IF (PRINT.AND.FIRST) WRITE (6,1191)
IF (PRINT) WRITE (6,1192) IELNO,(DUCT(I),EXCR(I),I=1,3)
1191 FORMAT (' STRUT DUCTILITIES AND EXCURSION RATIOS.....',5X,
( ' ELEMENT',T12, ' DUCTILITY DEFINITION #1 = ',
( ' DUCTILITY DEFINITION #2 = ',
( ' DUCTILITY DEFINITION #3 = ', /, T12.
( ' DUCTILITY EXCURSION ',
( ' DUCTILITY EXCURSION ',
( ' DUCTILITY EXCURSION ' )
1234567890ABCDEF1234567890ABCDE
C
IF (PRINT) WRITE (6,1192) IELNO,DUCT(1),EXCR(1)
1192 FORMAT (' STRUT DUCTILITIES AND EXCURSION RATIOS.....',5X,
( ' ELEMENT',T12, ' DUCTILITY DEFINITION #1 = ', /, T12.
( ' DUCTILITY EXCURSION ' )
1192 FORMAT (110,IP,6G15.6)
C
RETURN
C
C----- MAXIMUM FORCES
1200 CONTINUE
IF (FIRST) WRITE (6,1491) 'MAXIMUM LOAD AND DISPL AT MAXIMUM LOAD'
// NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY
WRITE(6,1492) IELNO,0,TYPE(KTYPE),NODEJ,NODEJ,FMAX(1),FMAX(2)
RETURN
C
C----- RESET INTERNAL FORCES
1300 CONTINUE
C
C----- ZERO MATRICES
F=0
PT=0
R=0
RT=0
V=0
VT=0
FMAX(1)=0
FMAX(2)=0
SLK=SE(2)
DO 1310 I=1,ISE
SE(I)=0
1310 SE(2)=SLK
SE(5)=LENGTH
C
WRITE (6,1330) IELNO
1330 FORMAT (' GOLE..... ELEMENT #',16.
( ' INTERNAL FORCES AND HYSTERESIS MODELS ARE RESET TO ZERO')
CALL MATLIB
( 2 ,LSTTYP ,FALSE ,IREL ,FALSE ,NAME ,ESEE ,EPSE , DUCT ,EXCR ,
( FT ,F ,RT ,R ,0 ,0 ,0 ,LELEM ,LMAT ,
( MAT ,MATTYP ,SE ,IELNO ,DAMAGE )
RETURN
C
C----- DAMAGE INDEX -----
1400 IF (ELSTIC) RETURN
C
FMAX1=FMAX(1)
FMAX2=FMAX(2)
CALL MATLIB
( 5 ,LSTTYP ,FALSE ,IREL ,FALSE ,NAME ,ESEE ,EPSE , DUCT ,EXCR ,
( FMAX1,0 ,FMAX2,0 ,0 ,0 ,0 ,LELEM ,LMAT ,
( MAT ,MATTYP ,SE ,IELNO ,DAMAGE )
IF (PRINT.AND.FIRST) WRITE (6,1410)
IF (PRINT) WRITE (6,1420) IELNO,DAMAGE,FMAX1,FMAX2,ESEE,EPSE
1410 FORMAT ('
( ' ELEMENT',T12, ' DAMAGE INDEX F MAX
( ' D MAX ESE
( ' P SE ' )
1420 FORMAT (110,IP,6G15.6)
DAMAGE=DAMAGE*(ESEE+EPSE)
RETURN
C
END
#PROCESS SDUMP GOSTMT XREF
C-----
DEBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
C
END DEBUG
C-----
SUBROUTINE ELE09
( IOPT ,IUNIT ,INC,NSMK ,FIRST ,PRINT
( NDOF ,NLOAD ,MKNOD ,NNODE ,NCCS
( ID,ELSTIC ,IOFP ,COORD ,COSINE ,JTCCS
( CONST ,DISPL ,FORCE ,IREL ,BUG
( IBUG ,ACCEL ,KGDATA ,GRAV ,PECOM
( MAXEED ,NLEO ,TELO ,NAME
( STEPID ,AXIAL ,IELNO ,RINPUT ,PUB
( ESEE ,EPSE ,DAMAGE ,DUCFAG ,IELTYP ,IELDOF
( MFA ,MTB ,MZA ,M2B ,MKA
( MFA ,STIF1 ,STIF2 ,STIF12 ,STIF22
( YILDG1,YILDEA,YILOD1,YILD2,YILD12,YILD22,
( ISEMTA,ISEMTB,ISEMKA,ISEMTB,ISEMKA,ISEMKA,PKG
( R,RT ,F,FT ,LM,CTS,CTE ,BS ,BE
( NODEJ ,NODEJ ,JOINTI ,JOINTJ
( REL ,LENGTH ,KGDOP ,LKG ,LMKG
( ISE ,ENGDAT ,FMAX ,
( ISE ,ENGDAT ,FMAX ,LAFAG ,STBFAG
( MATMTA ,MATMTB ,MATMTA ,MATMTB
( MATMFA ,H,22,S22,S21,533,5311,Q,BT,AY,AZ,CZ,DTT,AGZ

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C----- PRINT DATA FOR PLOTTING
IF (BTST(1,BUG,7)) THEN
  WRITE(7,10) NODEJ, NODEI,
  & (COORD(JOINTJ,1),I=1,3), (COORD(JOINTI,1),J=1,3)
  10 FORMAT (15,1X,15,1P,6G12.4)
ENDIF

C----- GET GLOBAL TO LOCAL TRANSFORMATION MATRIX
C----- DEFINE VECTOR X, LENGTH
VK(1)=COORD(JOINTJ,1)-COORD(JOINTI,1)
VK(2)=COORD(JOINTJ,2)-COORD(JOINTI,2)
VK(3)=COORD(JOINTJ,3)-COORD(JOINTI,3)
LENGTH=SQRT(VK(1)**2+VK(2)**2+VK(3)**2)-XS-XB
IF (LENGTH.LE.0) THEN
  WRITE(6,101) IELNO,JOINTI,JOINTJ,LENGTH
  101 FORMAT (1X,20(' '),ER',ROR = LENGTH IS LE 0./
  & 5X,' IELNO=' ,15,5X,' JOINTI=' ,15,5X,' JOINTJ=' ,15,
  & 5X,' LENGTH=' ,1P,615.6/
  & 5X,' REVISE INPUT.....'/)
  LENGTH=1.0
ENDIF
IF (BTST(1,BUG,7)) THEN
  WRITE (6,*) 'JOINTS',JOINTI,JOINTJ
  WRITE (6,*) 'VK',VK
  WRITE (6,*) 'VT',VT
  WRITE (6,*) 'LENGTH',LENGTH
ENDIF
C----- GET LOCAL TO GLOBAL TRANSFORMATION MATRICES CT AND B
ICB=JTCOS(JOINTI)
ICB=JTCOS(JOINTJ)
CALL ROTX2(VX,VT,COSINE(1,1),ICS,CTS,XS,0,0,0,BS,
  & CONST(JOINTI,1),MAXMOD)
CALL ROTX2(VX,VT,COSINE(1,1),ICE,CXE,XB,0,0,0,BE,
  & CONST(JOINTJ,1),MAXMOD)
IF (BTST(1,BUG,7)) THEN
  CALL WMATRX(CTS,3,3,'CTS')
  CALL WMATRX(CXE,3,3,'CXE')
ENDIF
C----- GET GLOBAL CONSTRAINT MATRIX SS, BE
BS(1,1)=CONST(JOINTI,1)*CTS(2,1)+CONST(JOINTI,2)*CTS(3,1)+BS(1,1)
BS(1,2)=CONST(JOINTI,1)*CTS(2,2)+CONST(JOINTI,2)*CTS(3,2)+BS(1,2)
BS(1,3)=CONST(JOINTI,1)*CTS(2,3)+CONST(JOINTI,2)*CTS(3,3)+BS(1,3)
BS(2,1)=CONST(JOINTI,3)*CTS(1,1)+CONST(JOINTI,4)*CTS(2,1)+BS(2,1)
BS(2,2)=CONST(JOINTI,3)*CTS(1,2)+CONST(JOINTI,4)*CTS(2,2)+BS(2,2)
BS(2,3)=CONST(JOINTI,3)*CTS(1,3)+CONST(JOINTI,4)*CTS(2,3)+BS(2,3)
BS(3,1)=CONST(JOINTI,5)*CTS(1,1)+CONST(JOINTI,6)*CTS(2,1)+BS(3,1)
BS(3,2)=CONST(JOINTI,5)*CTS(1,2)+CONST(JOINTI,6)*CTS(2,2)+BS(3,2)
BS(3,3)=CONST(JOINTI,5)*CTS(1,3)+CONST(JOINTI,6)*CTS(2,3)+BS(3,3)
BE(1,1)=CONST(JOINTJ,1)*CXB(2,1)+CONST(JOINTJ,2)*CXB(3,1)+BE(1,1)
BE(1,2)=CONST(JOINTJ,1)*CXB(2,2)+CONST(JOINTJ,2)*CXB(3,2)+BE(1,2)
BE(1,3)=CONST(JOINTJ,1)*CXB(2,3)+CONST(JOINTJ,2)*CXB(3,3)+BE(1,3)
BE(2,1)=CONST(JOINTJ,3)*CXB(1,1)+CONST(JOINTJ,4)*CXB(2,1)+BE(2,1)
BE(2,2)=CONST(JOINTJ,3)*CXB(1,2)+CONST(JOINTJ,4)*CXB(2,2)+BE(2,2)
BE(2,3)=CONST(JOINTJ,3)*CXB(1,3)+CONST(JOINTJ,4)*CXB(2,3)+BE(2,3)
BE(3,1)=CONST(JOINTJ,5)*CXB(1,1)+CONST(JOINTJ,6)*CXB(2,1)+BE(3,1)
BE(3,2)=CONST(JOINTJ,5)*CXB(1,2)+CONST(JOINTJ,6)*CXB(2,2)+BE(3,2)
BE(3,3)=CONST(JOINTJ,5)*CXB(1,3)+CONST(JOINTJ,6)*CXB(2,3)+BE(3,3)
IF (BTST(1,BUG,7)) THEN
  CALL WMATRX(BS,3,3,'BS')
  CALL WMATRX(BE,3,3,'BE')
ENDIF
C----- ZERO MATRICES
DO 40 I=1,12
  FMX(I)=0
  F(I)=0
  FT(I)=0
  R(I)=0
  RT(I)=0
  V(I)=0
  VL(I)=0
  DO 40 J=1,12
    S(I,J)=0
    A(I,J)=0
  40
C----- ASSEMBLE TRANSFORMATION MATRIX A
DO 50 I=1,3
  DO 50 J=1,3
    A(I,J)=CTS(I,J)
  A(I+3,J+3)=CXB(I,J)
  A(I+6,J+6)=CXB(I,J)
  A(I+9,J+9)=CXB(I,J)
  A(I+3,J)=BS(I,J)
  A(I+9,J+6)=BE(I,J)
  50
IF (BTST(1,BUG,7)) CALL WMATRX(A,12,12,'A')
C----- GET MATERIAL PROPERTIES
FALSE=.FALSE.
LSTTT=0
CALL MATLIB
&(2,LSTTT,FALSE,IREL,FALSE,NAME,ESE,EPSE,DUCT(1,1),EXCR(1,1),
  & FT,F,RT,R,V,VL,LELEM,LMAT
  & MATM2A,MATTP,SE(ISEM2A),IELNO,DAMAGE)
M2A=MATTP
IF (MATTP.NE.9.AND.MATTP.NE.10.AND.MATTP.NE.11)
  & WRITE(6,190)IELNO,MATTP,NODEI,NODEJ
& CALL MATLIB
&(2,LSTTT,FALSE,IREL,FALSE,NAME,ESE,EPSE,DUCT(1,2),EXCR(1,2),
  & FT,F,RT,R,V,VL,LELEM,LMAT
  & MATM2A,MATTP,SE(ISEM2A),IELNO,DAMAGE)
M2A=MATTP
IF (MATTP.NE.9.AND.MATTP.NE.10.AND.MATTP.NE.11)
  & WRITE(6,190)IELNO,MATTP,NODEI,NODEJ
& CALL MATLIB
&(2,LSTTT,FALSE,IREL,FALSE,NAME,ESE,EPSE,DUCT(1,3),EXCR(1,3),
  & FT,F,RT,R,V,VL,LELEM,LMAT
  & MATM2A,MATTP,SE(ISEM2A),IELNO,DAMAGE)
M2A=MATTP
IF (MATTP.NE.9.AND.MATTP.NE.10.AND.MATTP.NE.11)
  & WRITE(6,190)IELNO,MATTP,NODEI,NODEJ
& CALL MATLIB
&(2,LSTTT,FALSE,IREL,FALSE,NAME,ESE,EPSE,DUCT(1,5),EXCR(1,5),
  & FT,F,RT,R,V,VL,LELEM,LMAT
  & MATM2A,MATTP,SE(ISEM2A),IELNO,DAMAGE)
M2A=MATTP
IF (MATTP.NE.10) WRITE(6,190)IELNO,MATTP,NODEI,NODEJ
SE(ISEPKA+4)=LENGTH
CALL MATLIB
&(2,LSTTT,FALSE,IREL,FALSE,NAME,ESE,EPSE,DUCT(1,6),EXCR(1,6),
  & FT,F,RT,R,V,VL,LELEM,LMAT
  & MATM2A,MATTP,SE(ISEPKA),IELNO,DAMAGE)
M2A=MATTP
IF (MATTP.NE.10.AND.MATTP.NE.13)
  & WRITE(6,190)IELNO,MATTP,NODEI,NODEJ
```

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ELE01690
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ELE02990
ELE03000
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ELE03090
C 190 FORMAT (' ELEMENT: ',16,' INCOMPATIBLE MATERIAL PROP.#',15,
  & ' START JOINT:',15,' END JOINT:',15,
  & ' THE ELEMENT IS REJECTED AND ER',ROR CHECKING CONTINUES')
C ...CHECK INTERACTIVE OPTION
IF (M2A.EQ.10.AND.M2B.EQ.10.AND.
  & M2A.EQ.10.AND.M2B.EQ.10.AND.M2A.EQ.10) THEN
  IF (SE(ISEM2A+4).EQ.2.AND.SE(ISEM2B+4).EQ.2) THEN
    IAFAG=1
  ENDF
  IF (SE(ISEM2A+4).EQ.3.AND.SE(ISEM2B+4).EQ.3.AND.
    & SE(ISEM2A+4).EQ.3.AND.SE(ISEM2B+4).EQ.3) THEN
    IAFAG=5
  ENDF
  ENDF
C ESE=0
  EPSE=0
C----- NO END RELEASE FOR THIS ELEMENT
REL=0
C----- SET STIFFNESS COEFFICIENTS...
C
C FOR BENDING IN Y AXIS:
IF (M2A.EQ.9.AND.M2B.EQ.9).OR.
  & (M2A.EQ.11.AND.M2B.EQ.11).OR.
  & (M2A.EQ.6.AND.M2B.EQ.6) THEN
  STIF1=SE(ISEM2A)
  STIF2=SE(ISEM2B)
  IF (M2A.EQ.9) THEN
    SA1=SE(ISEM2A+60)
    SA2=SE(ISEM2B+60)
  ELSE IF (M2A.EQ.11) THEN
    SA1=SE(ISEM2A+30)
    SA2=SE(ISEM2B+30)
  ELSE IF (M2A.EQ.6) THEN
    SA1=SE(ISEM2A+47)/LENGTH
    SA2=SE(ISEM2B+47)/LENGTH
  STIF1=STIF1/LENGTH
  STIF2=STIF2/LENGTH
  EI=SE(ISEM2A+48)
  TERM1=(LENGTH**2)/(12*EI*EI)
  TERM2=LENGTH/(3*EI)
  ENDF
  IF (SAL.EQ.0) THEN
    FLX1=0
  ELSE IF (SAL.NE.0) THEN
    FLX1=(1/STIF1)-(1/SA1)
  ENDF
  IF (SA2.EQ.0) THEN
    FLX2=0
  ELSE IF (SA2.NE.0) THEN
    FLX2=(1/STIF2)-(1/SA2)
  ENDF
  IF (M2A.EQ.6) THEN
    DCOF=TERM1+TERM2*(FLX1+FLX2)+FLX1*FLX2
  ELSE IF (M2A.NE.6) THEN
    DCOF=(3/(SA1**2))+2/SA1*(FLX1+FLX2)+FLX1*FLX2
  ENDF
  DCOF=1/DCOF
C
  IF (M2A.EQ.6) THEN
    BT=DCOF*(TERM2+FLX2)
    BT=DCOF*(TERM1+FLX1)
    DT=DCOF*(0.5*TERM2)
  ELSE IF (M2A.NE.6) THEN
    BT=DCOF*((2/SA1)+FLX2)
    BT=DCOF*((2/SA2)+FLX1)
    DT=DCOF*(1/SA1)
  ENDF
  ELSE IF (M2A.EQ.10.AND.M2B.EQ.10) THEN
    ELAS=SE(ISEM2A+4)
    SP=SE(ISEM2A+5)
    EI=SE(ISEM2A)
    YILD1=SE(ISEM2A+3)
    YILD2=SE(ISEM2B+3)
    IF (YILD1.EQ.1).OR.(YILD2.EQ.1) STBFA=1
  SE(ISEM2A+12)=SE(ISEM2A+1)*LENGTH/(6*EI)
  SE(ISEM2B+12)=SE(ISEM2B+1)*LENGTH/(6*EI)
C
  IF (ELAS.EQ.0) THEN
    BT=4*EI/LENGTH
    BT=4*EI/LENGTH
    DT=2*EI/LENGTH
  ELSE IF (ELAS.NE.0) THEN
    IF (YILD1.EQ.0.AND.YILD2.EQ.0) THEN
      BY=4*EI/LENGTH
      BY=4*EI/LENGTH
      DY=2*EI/LENGTH
    ELSE IF (YILD1.EQ.1.AND.YILD2.EQ.0) THEN
      BY=SP*(4*EI/LENGTH)+(1-SP)*(3*EI/LENGTH)
      BY=SP*(4*EI/LENGTH)+(1-SP)*(3*EI/LENGTH)
    ELSE IF (YILD1.EQ.0.AND.YILD2.EQ.1) THEN
      BY=SP*(4*EI/LENGTH)+(1-SP)*(3*EI/LENGTH)
      BY=SP*(4*EI/LENGTH)+(1-SP)*(3*EI/LENGTH)
    ELSE IF (YILD1.EQ.1.AND.YILD2.EQ.1) THEN
      BY=SP*(4*EI/LENGTH)
      BY=SP*(4*EI/LENGTH)
      DY=SP*(2*EI/LENGTH)
    ENDF
  ENDF
  ELSE
    WRITE(6,69)M2A,M2B,IELNO
    69 FORMAT(1X,'SE INVALID MATERIAL TYPE M2A=',12,' M2B=',12,
      & ' FOR ELEMENT NO. ',15,' **')
  ENDF
C
C FOR BENDING IN Z AXIS:
IF (M2A.EQ.9.AND.M2B.EQ.9).OR.
  & (M2A.EQ.11.AND.M2B.EQ.11).OR.
  & (M2A.EQ.6.AND.M2B.EQ.6) THEN
  STIF1=SE(ISEM2A)
  STIF2=SE(ISEM2B)
  IF (M2A.EQ.9) THEN
    SA1=SE(ISEM2A+60)
    SA2=SE(ISEM2B+60)
  ELSE IF (M2A.EQ.11) THEN
    SA1=SE(ISEM2A+30)
    SA2=SE(ISEM2B+30)
  ELSE IF (M2A.EQ.6) THEN
    SA1=SE(ISEM2A+47)/LENGTH
    SA2=SE(ISEM2B+47)/LENGTH
  STIF1=STIF1/LENGTH
  STIF2=STIF2/LENGTH
  EI=SE(ISEM2A+48)
  TERM1=(LENGTH**2)/(12*EI*EI)
  TERM2=LENGTH/(3*EI)
  ENDF
  IF (SAL.EQ.0) THEN
    FLX1=0
  ELSE IF (SAL.NE.0) THEN
    FLX1=(1/STIF1)-(1/SA1)
  ENDF
  IF (SA2.EQ.0) THEN
```

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ELE03100
ELE03110
ELE03120
ELE03130
ELE03140
ELE03150
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ELE04330
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ELE04350
ELE04360
ELE04370
ELE04380
ELE04390
ELE04400
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ELE04460
ELE04470
ELE04480
ELE04490
ELE04500
ELE04510
```

```
FLX2=0
ELSE IF ( SA2 .NE. 0 ) THEN
  FLX2=(1/STIFF2)-(1/SA2)
ENDIF
IF ( M2A .EQ. 6 ) THEN
  DCOF=TERM1 + TERM2*(FLX1-FLX2) + FLX1*FLX2
ELSE IF ( M2A .NE. 6 ) THEN
  DCOF=(3/(SA1**2)) + (2/SA1)*(FLX1+FLX2) + FLX1*FLX2
ENDIF
DCOF=1/DCOF
IF ( M2A .EQ. 6 ) THEN
  A5 =DCOF*(TERM2 + FLX2)
  A22=DCOF*(TERM2 + FLX1)
  C2=DCOF*(0.5*TERM2)
ELSE IF ( M2A .NE. 6 ) THEN
  A2 =DCOF*((2/SA1)*FLX2)
  A22=DCOF*((2/SA2)*FLX1)
  C2=DCOF*(1/SA1)
ENDIF
ELSE IF( M2A .EQ. 10 .AND. M2B .EQ. 10 ) THEN
  ELAS=SE(ISEM2A+4)
  SP =SE(ISEM2A+5)
  E12 =SE(ISEM2A )
  YILD12=SE(ISEM2A+3)
  YILD22=SE(ISEM2B+3)
  IF( YILD12 .EQ. 1 .OR. YILD22 .EQ. 1 ) STBFAG=1
  SE(ISEM2A+12)=SE(ISEM2A+1)*LENGTH/(6*E12)
  SE(ISEM2B+12)=SE(ISEM2B+1)*LENGTH/(6*E12)
  IF( ELAS .EQ. 0 ) THEN
    A2 =4*E12/LENGTH
    A22=4*E12/LENGTH
    C2 =E12/LENGTH
  ELSE IF( ELAS .NE. 0 ) THEN
    IF( YILD12 .EQ. 0 .AND. YILD22 .EQ. 0 ) THEN
      A2 =4*E12/LENGTH
      A22=4*E12/LENGTH
      C2 =2*E12/LENGTH
    ELSE IF( YILD12 .EQ. 1 .AND. YILD22 .EQ. 0 ) THEN
      A2 =SP*(4*E12/LENGTH)+(1-SP)*(3*E12/LENGTH)
      A22=SP*(4*E12/LENGTH)+(1-SP)*(3*E12/LENGTH)
      C2 =SP*(2*E12/LENGTH)
    ELSE IF( YILD12 .EQ. 1 .AND. YILD22 .EQ. 1 ) THEN
      A2 =SP*(4*E12/LENGTH)
      A22=SP*(4*E12/LENGTH)
      C2 =SP*(2*E12/LENGTH)
    ENDIF
  ENDIF
  ELSE
    WRITE(6,79)M2A,M2B,IELNO
    79 FORMAT(1X,'** INVALID MATERIAL TYPE M2A=',12,' M2B=',12,
    & ' FOR ELEMENT NO. ',15,' **')
  ENDIF
  C FOR AXIAL IN X AXIS:
  IF( MFXA .EQ. 10 ) THEN
    ELAS=SE(ISEM3A+4)
    SP =SE(ISEM3A+5)
    SA =SE(ISEM3A )
    YILDEA=SE(ISEM3A+3)
    IF( YILDEA .EQ. 1 ) STBFAG=1
    SE(ISEM3A+12)=SE(ISEM3A+1)*LENGTH/EA
    IF( ELAS .EQ. 0 ) THEN
      H=EA/LENGTH
    ELSE IF( ELAS .NE. 0 ) THEN
      IF( YILDEA .EQ. 0 ) THEN
        H=EA/LENGTH
      ELSE IF( YILDEA .EQ. 1 ) THEN
        H=SP*(EA/LENGTH)
      ENDIF
    ENDIF
    ELSE IF( MFXA .EQ. 13 ) THEN
      H=SE(ISEM3A)
    ELSE IF( MFXA .NE. 10 .OR. MFXA .NE. 13 ) THEN
      WRITE(6,89)MFXA,IELNO
    89 FORMAT(1X,'** INVALID MATERIAL TYPE MFXA=',12,
    & ' FOR ELEMENT NO. ',15,' **')
    ENDIF
    C FOR TORSION IN X AXIS:
    IF( MKA .EQ. 10 ) THEN
      ELAS=SE(ISEM4A+4)
      SP =SE(ISEM4A+5)
      GJ =SE(ISEM4A )
      YILDOJ=SE(ISEM4A+3)
      IF( YILDOJ .EQ. 1 ) STBFAG=1
      SE(ISEM4A+12)=SE(ISEM4A+1)*LENGTH/GJ
      IF( ELAS .EQ. 0 ) THEN
        Q=GJ/LENGTH
      ELSE IF( ELAS .NE. 0 ) THEN
        IF( YILDOJ .EQ. 0 ) THEN
          Q=GJ/LENGTH
        ELSE IF( YILDOJ .EQ. 1 ) THEN
          Q=SP*(GJ/LENGTH)
        ENDIF
      ENDIF
      ELSE
        WRITE(6,99)MKA,IELNO
    99 FORMAT(1X,'** INVALID MATERIAL TYPE MKA=',12,
    & ' FOR ELEMENT NO. ',15,' **')
    ENDIF
    S22=(A2+2*C2+A22)/(LENGTH**2)
    S26=(A2-C2)/LENGTH
    S12=(A12+C2)/LENGTH
    S33=(B1+2*DT+BYT)/(LENGTH**2)
    S35=(B1-DT)/LENGTH
    S311=(BT+DT)/LENGTH
    C----- SET CONSISTENT MASS GEOMETRIC STIFFNESS COEFFICIENTS...
    GA2= 2 * LENGTH / 15.
    GB2= GA2
    GC2= LENGTH / 30.
    GD2= 0.10
    GE2= 0.10
    GF2= 1.2 / LENGTH
    GAT= 2 * LENGTH / 15.
    GBT= GAT
    GCT= LENGTH / 30.
    GDT= 0.10
    GET= 0.10
    GFT= 1.2 / LENGTH
    C-----
    C ASSEMBLE LOCAL STIFFNESS MATRIX
    S( 1, 1 ) = H
    S( 1, 7 ) = -H
    ELEM4520 S( 2, 2 ) = S22
    ELEM4530 S( 2, 6 ) = S26
    ELEM4540 S( 2, 8 ) = -S22
    ELEM4550 S( 2,12 ) = S212
    ELEM4560 S( 3, 3 ) = S33
    ELEM4570 S( 3, 5 ) = -S35
    ELEM4580 S( 3, 9 ) = -S33
    ELEM4590 S( 3,11 ) = -S311
    ELEM4600 S( 4, 4 ) = Q
    ELEM4610 S( 4,10 ) = -Q
    ELEM4620 S( 5, 5 ) = BT
    ELEM4630 S( 5, 9 ) = S35
    ELEM4640 S( 5,11 ) = DT
    ELEM4650 S( 6, 6 ) = A2
    ELEM4660 S( 6, 8 ) = -S26
    ELEM4670 S( 6,12 ) = C2
    ELEM4680 S( 7, 7 ) = H
    ELEM4690 S( 8, 8 ) = S22
    ELEM4700 S( 8,12 ) = -S212
    ELEM4710 S( 9, 9 ) = S33
    ELEM4720 S( 9,11 ) = S311
    ELEM4730 S(10,10) = Q
    ELEM4740 S(11,11) = BT
    ELEM4750 S(12,12) = A22
    ELEM4760 S( 5, 3 ) = S( 3, 5 )
    ELEM4770 S( 6, 2 ) = S( 2, 6 )
    ELEM4780 S( 7, 1 ) = S( 1, 7 )
    ELEM4790 S( 8, 2 ) = S( 2, 8 )
    ELEM4800 S( 8, 6 ) = S( 6, 8 )
    ELEM4810 S( 9, 3 ) = S( 3, 9 )
    ELEM4820 S( 9, 5 ) = S( 5, 9 )
    ELEM4830 S(10, 4 ) = S( 4,10 )
    ELEM4840 S(11, 3 ) = S( 3,11 )
    ELEM4850 S(11, 5 ) = S( 5,11 )
    ELEM4860 S(11, 9 ) = S( 9,11 )
    ELEM4870 S(12, 2 ) = S( 2,12 )
    ELEM4880 S(12, 6 ) = S( 6,12 )
    ELEM4890 S(12, 8 ) = S( 8,12 )
    C IF (FIRST) CALL WMATRIX( S ,12,12,' S ' )
    C-----
    C----- CALCULATE SAT
    DO 60 I=1,12
    ELEM4930 DO 60 J=1,12
    ELEM4940 SAT(I,J)=0
    ELEM4950 DO 60 K=1,12
    ELEM4960 DO 60 L=1,12
    ELEM4970 SAT(I,J)=SAT(I,J)+S(I,K)*A(J,K)
    ELEM4980
    ELEM4990
    ELEM5000
    ELEM5010
    ELEM5020
    ELEM5030
    ELEM5040
    ELEM5050
    ELEM5060
    ELEM5070
    ELEM5080
    ELEM5090
    ELEM5100
    ELEM5110
    ELEM5120
    ELEM5130
    ELEM5140
    ELEM5150
    ELEM5160
    ELEM5170
    ELEM5180
    ELEM5190
    ELEM5200
    ELEM5210
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    ELEM5660
    ELEM5670
    ELEM5680
    ELEM5690
    ELEM5700
    ELEM5710
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    ELEM5800
    ELEM5810
    ELEM5820
    ELEM5830
    ELEM5840
    ELEM5850
    ELEM5860
    ELEM5870
    ELEM5880
    ELEM5890
    ELEM5900
    ELEM5910
    ELEM5920
    ELEM5930
    S( I, J ) = 0
    999
    C----- DETERMINE IF AXIAL DEFORMATION IS CONSIDERED
    BIT #6 OF REL IS TRUE IF AXIAL DEFORMATION IS CONSIDERED
    DO 999 I=1,3
    CS=CTE(I,1)
    CE=CTE(I,1)
    IF (ABS(CE).LE. .0001 .AND. ABS(CE).LE. .0001 ) GO TO 999
    I5=IDOF(JOINT,I)
    I6=IDOF(JOINT,I)
    IF (I5.EQ.I6) GO TO 999
    REL=IBEST(REL,6)
    999 CONTINUE
    AXIAL=BTBST(REL,6)
    IF ( BTBST(18,6) ) WRITE (6,*) 'AXIAL',AXIAL
    IF (KGDATA(1).NE.0 .AND. KGDATA(2).NE.0 .AND. BTBST(REL,6) ) THEN
    C----- DETERMINE THE AXIAL LOAD TO BE USED FOR KG...
    IF (KGDATA(1).EQ.1) THEN
      PGDOM = PKG
    ELSE IF (KGDATA(1).EQ.2) THEN
      PGDOM = 0.50*(F11)-F(7)
    ENDIF
    C----- ASSEMBLE LOCAL GEOMETRIC STIFFNESS MATRIX FOR A UNIT LOAD...
    C DO LOCAL KG MATRIX
    DO 998 I=1,12
    DO 998 J=1,12
    998 S(I,J)=0
    ELEM5940
    ELEM5950
    ELEM5960
    ELEM5970
    ELEM5980
    ELEM5990
    ELEM6000
    ELEM6010
    ELEM6020
    ELEM6030
    ELEM6040
    ELEM6050
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C----- LUMPED MASS FORM OF GEOMETRIC STIFFNESS
IF (KGDATA(2),EQ.1) THEN
  F1=1/LENGTH
  S( 2, 2)= F1
  S( 2, 8)= -F1
  S( 8, 2)= -F1
  S( 8, 8)= F1
  S( 3, 3)= F1
  S( 3, 9)= -F1
  S( 9, 3)= -F1
  S( 9, 9)= F1
C----- CONSISTENT MASS FORM OF GEOMETRIC STIFFNESS
ELSE IF (KGDATA(2),EQ.2) THEN
  S( 2, 2)= GDZ
  S( 2, 8)= -GFZ
  S( 2,12)= -GFZ
  S( 8, 2)= GDZ
  S( 8, 6)= GAZ
  S( 8, 8)= -GDZ
  S( 8,12)= -GDZ
  S( 8, 3)= -GFZ
  S( 8, 6)= -GFZ
  S( 8, 8)= -GFZ
  S( 8,12)= GBZ
C-----
  S( 3, 3)= GFT
  S( 3, 5)= -GDT
  S( 3, 9)= -GFT
  S( 3,11)= -GDT
  S( 5, 5)= GAT
  S( 5, 9)= GDT
  S( 5,11)= -GDT
  S( 9, 3)= -GFT
  S( 9, 5)= GDT
  S( 9, 9)= GFT
  S( 9,11)= GDT
  S(11, 3)= -GDT
  S(11, 5)= -GDT
  S(11, 9)= GDT
  S(11,11)= GDT
  ENDF
  IF (BTST(1BUG,0)) CALL WMATRX( S ,12,12,' KG -LOCAL COORD ')
C----- CALCULATE SAT
DO 1060 J=1,12
  DO 1060 I=1,12
    SAT(I,J)=0
  DO 1060 K=1,12
    SAT(I,J)=SAT(I,J)+S(I,K)*A(J,K)
1060
C----- CALCULATE ASAT
DO 1070 J=1,12
  DO 1070 I=1,12
    ASAT(I,J)=0
  DO 1070 K=1,12
    ASAT(I,J)=ASAT(I,J)+A(I,K)*SAT(K,J)
1070
  IF (BTST(1BUG,0)) CALL WMATRX(ASAT ,12,12,' A*KG*AT ')
C----- CONVERT TO HALF STORAGE MODE BY COLUMNS
  KGDOF=0
  DO 1090 I=1,12
    IF (LMT(I),EQ.0 .OR. ASAT(I,I),LE.0) GO TO 1090
    IF (LMT(I),EQ.0) GO TO 1090
    KGDOF=KGDOF+1
    LMKG(KGDOF)=LMT(I)
    DO 1080 J=1,12
      IF (LMT(J),EQ.0 .OR. ASAT(J,J),LE.0) GO TO 1080
      IF (LMT(J),EQ.0) GO TO 1080
      L=L+1
      STIFF(L)=ASAT(I,J)
1080
  CONTINUE
1090
  CONTINUE
  IF ( BTST(1BUG,7) ) THEN
    WRITE (6,*) 'KGDOF',KGDOF,' LMKG =',(LMKG(I),I=1,KGDOF)
    CALL LMATRX(STIFF(LG=1),MD,KGDOF,(L-LKG))
    ELEMENT GEOMETRIC STIFFNESS (UNIT LOAD)
  ENDF
  ENDF
C----- RELEASE UNUSED STORAGE
180 IBSL=156-L
C-----
C----- PRINT COLUMN DATA
  IF (FIRST .OR. BTST(1BUG,7) .OR. BTST(1BUG,8) ) WRITE (6,192)
191 WRITE(6,193) NAME,I,END,MODEI,MODEJ,LENGTH,
  & VT(1),VT(2),VT(3),XS,-XE,PKG
  WRITE(6,194) MATFXA,MATMOA,MATMA,MATMB,MATMSA,MATMSB
192 FORMAT(' ELEMENT 09, I2D3EAM ELEMENT///
  & I22,4 START END,3X,LENGTH',3X,
  & 11(' '), Y-AXIS ',
  & 12(' '), START DIST END DIST PKG')
193 FORMAT(1X,A15,216.1X,16.1X,1P,G10.4,9P,
  & 5,1,1,5P,9.5,3,1P,5,1,1,12.1X)
194 FORMAT(16X,' MATERIAL # 1 (FX )',12.1X, '(MY )',12.1X,
  & '(MZ-A)',12.1X, '(MY-B)',12.1X,
  & '(MZ-B)',12.1X, '(MZ-B)',12.1X)
  RETURN
C----- DETERMINE GEOMETRIC STIFF
200 CONTINUE
C----- KG IS DETERMINED FOR A UNIT LOAD WITH IOPT=1,
C----- THE ACTUAL LOAD (PGEOM) IS DETERMINED HERE
C----- KG IS MULTIPLIED BY PGEOM WHEN THE TOTAL STIFFNESS IS ASSEMBLED
C----- NEGATIVE PGEOM IS COMPRESSION....
  AXIAL=BTST(RSL,6)
C----- DETERMINE THE AXIAL LOAD TO BE USED FOR KG...
  IF (AXIAL) THEN
    IF (KGDATA(1),EQ.1) THEN
      PGEOM = PKG
    ELSE IF (KGDATA(1),EQ.2) THEN
      PGEOM = 0.50*(F(1)-F(7))
    ENDF
  ENDF
  IF (BTST(1BUG,7)) WRITE (6,*) 'AXIAL ',AXIAL,' PGEOM',PGEOM
  RETURN
C----- DETERMINE STIFFNESS
300 CONTINUE
  STIFF1,STIFF2,STIFF12,STIFF22,YILD0J,YILDEA,YILD1Y,YILD2Y,
  YILD1Z, AND YILD2Z CONTAIN STIFFNESSES, WHICH WERE DETERMINED IN
  THE LAST CALL TO MA_TLIB
  IF STIFF1 = SE(ISEMZA) FOR MAT09,MAT11,MAT06
  STIFF2 = SE(ISEMZA) FOR MAT09,MAT11,MAT06
  STIFF12 = SE(ISEMZA) FOR MAT09,MAT11,MAT06
  STIFF22 = SE(ISEMZA) FOR MAT09,MAT11,MAT06
  YILD0J = SE(ISEMZA+3) FOR MAT10
  YILDEA = SE(ISEMZA+3) FOR MAT10
  OR
  YILD1Y = SE(ISEMZA+3) FOR MAT10
  YILD2Y = SE(ISEMZA+3) FOR MAT10
  YILD1Z = SE(ISEMZA+3) FOR MAT10
  YILD2Z = SE(ISEMZA+3) FOR MAT10

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  YILD0J = SE(ISEMZA+3) FOR MAT10
  YILDEA = SE(ISEMZA+3) FOR MAT10
  IF (MZA .EQ. 9 .AND. MZB .EQ. 9) THEN
    IF (STIFF1 .NE. SE(ISEMZA) .OR. STIFF2 .NE. SE(ISEMZA)) GO TO 3
  ELSE IF (MZA .EQ. 11 .AND. MZB .EQ. 11) THEN
    IF (STIFF1 .NE. SE(ISEMZA) .OR. STIFF2 .NE. SE(ISEMZA)) GO TO 3
  ELSE IF (MZA .EQ. 6 .AND. MZB .EQ. 6) THEN
    IF (STIFF1 .NE. (SE(ISEMZA)/LENGTH) .OR.
    STIFF2 .NE. (SE(ISEMZA)/LENGTH) ) GO TO 3
  ELSE IF (MZA .EQ. 10 .AND. MZB .EQ. 10) THEN
    IF (YILD1Y .NE. SE(ISEMZA+3) .OR. YILD2Y .NE. SE(ISEMZA+3)) GO TO 3
  ENDF
  IF (MZA .EQ. 9 .AND. MZB .EQ. 9) THEN
    IF (STIFF1 .NE. SE(ISEMZA) .OR. STIFF2 .NE. SE(ISEMZA)) GO TO 3
  ELSE IF (MZA .EQ. 11 .AND. MZB .EQ. 11) THEN
    IF (STIFF1 .NE. SE(ISEMZA) .OR. STIFF2 .NE. SE(ISEMZA)) GO TO 3
  ELSE IF (MZA .EQ. 6 .AND. MZB .EQ. 6) THEN
    IF (STIFF1 .NE. (SE(ISEMZA)/LENGTH) .OR.
    STIFF2 .NE. (SE(ISEMZA)/LENGTH) ) GO TO 3
  ELSE IF (MZA .EQ. 10 .AND. MZB .EQ. 10) THEN
    IF (YILD1Y .NE. SE(ISEMZA+3) .OR. YILD2Y .NE. SE(ISEMZA+3)) GO TO 3
  ENDF
  IF (YILD0J .NE. SE(ISEMZA+3) ) GO TO 3
  IF (MZA .EQ. 10) THEN
    IF (YILDEA .NE. SE(ISEMZA+3) ) GO TO 3
  ELSE IF (MZA .EQ. 13) THEN
    IF (H .NE. SE(ISEMZA) ) GO TO 3
  ENDF
  RETURN
3 CONTINUE
C----- SET STIFFNESS COEFFICIENTS...
C----- FOR BENDING IN Y AXIS
  IF ( (MZA .EQ. 9 .AND. MZB .EQ. 9) .OR.
  & (MZA .EQ. 11 .AND. MZB .EQ. 11) .OR.
  & (MZA .EQ. 6 .AND. MZB .EQ. 6) ) THEN
    STIFF1=SE(ISEMZA)
    STIFF2=SE(ISEMZA)
    IF (MZA .EQ. 9) THEN
      SA1=SE(ISEMZA+60)
      SA2=SE(ISEMZA+60)
    ELSE IF (MZA .EQ. 11) THEN
      SA1=SE(ISEMZA+30)
      SA2=SE(ISEMZA+30)
    ELSE IF (MZA .EQ. 6) THEN
      SA1=SE(ISEMZA+47)/LENGTH
      SA2=SE(ISEMZA+47)/LENGTH
    STIFF1=STIFF1/LENGTH
    STIFF2=STIFF2/LENGTH
    EI=SE(ISEMZA+48)
    TERM1=(LENGTH**2)/(12*EI*EI)
    TERM2=LENGTH/(3*EI)
  ENDF
  IF ( SA1 .EQ. 0 ) THEN
    FLX1=0
  ELSE IF ( SA1 .NE. 0 ) THEN
    FLX1=(1/STIFF1)-(1/SA1)
  ENDF
  IF ( SA2 .EQ. 0 ) THEN
    FLX2=0
  ELSE IF ( SA2 .NE. 0 ) THEN
    FLX2=(1/STIFF2)-(1/SA2)
  ENDF
  IF (MZA .EQ. 6) THEN
    DCOF=TERM1 + TERM2*(FLX1+FLX2) + FLX1*FLX2
  ELSE IF (MZA .NE. 6) THEN
    DCOF=(3/(SA1**2)) * (2/SA1)*(FLX1+FLX2) + FLX1*FLX2
  ENDF
  DCOF=1/DCOF
C-----
  IF (MZA .EQ. 6) THEN
    BT = DCOF*(TERM2 + FLX2)
    STIFF=DCOF*(TERM2 + FLX1)
    DT = DCOF*(0.5*TERM2)
  ELSE IF (MZA .NE. 6) THEN
    BT = DCOF*(2/SA1+FLX2)
    STIFF=DCOF*(2/SA2+FLX1)
    DT = DCOF*(1/SA1)
  ENDF
  ELSE IF (MZA .EQ. 10 .AND. MZB .EQ. 10) THEN
    ELAS=SE(ISEMZA+4)
    SP = SE(ISEMZA+5)
    EIT = SE(ISEMZA)
    YILD1Y=SE(ISEMZA+3)
    YILD2Y=SE(ISEMZA+3)
    IF (MZA .EQ. 1) .OR. YILD2Y .EQ. 1) ST0FAS=1
    IF ( ELAS .EQ. 0 ) THEN
      BT = 4*EIT/LENGTH
      STIFF=4*EIT/LENGTH
      DT = 2*EIT/LENGTH
    ELSE IF ( ELAS .NE. 0 ) THEN
      IF ( YILD1Y .EQ. 0 .AND. YILD2Y .EQ. 0 ) THEN
        BT = 4*EIT/LENGTH
        STIFF=4*EIT/LENGTH
        DT = 2*EIT/LENGTH
      ELSE IF ( YILD1Y .EQ. 1 .AND. YILD2Y .EQ. 0 ) THEN
        BT = SP*(4*EIT/LENGTH)
        STIFF=SP*(4*EIT/LENGTH)+(1-SP)*(3*EIT/LENGTH)
        DT = SP*(2*EIT/LENGTH)
      ELSE IF ( YILD1Y .EQ. 0 .AND. YILD2Y .EQ. 1 ) THEN
        BT = SP*(4*EIT/LENGTH)+(1-SP)*(3*EIT/LENGTH)
        STIFF=SP*(4*EIT/LENGTH)
        DT = SP*(2*EIT/LENGTH)
      ELSE IF ( YILD1Y .EQ. 1 .AND. YILD2Y .EQ. 1 ) THEN
        BT = SP*(4*EIT/LENGTH)
        STIFF=SP*(4*EIT/LENGTH)
        DT = SP*(2*EIT/LENGTH)
      ENDF
    ENDF
  ENDF
  WRITE(6,69)MZA,MZB,IELNO
  ENDF
C----- FOR BENDING IN Z AXIS
  IF ( (MZA .EQ. 9 .AND. MZB .EQ. 9) .OR.
  & (MZA .EQ. 11 .AND. MZB .EQ. 11) .OR.
  & (MZA .EQ. 6 .AND. MZB .EQ. 6) ) THEN
    STIFF12=SE(ISEMZA)
    STIFF22=SE(ISEMZA)
    IF (MZA .EQ. 9) THEN
      SA1=SE(ISEMZA+60)
      SA2=SE(ISEMZA+60)
    ELSE IF (MZA .EQ. 11) THEN
      SA1=SE(ISEMZA+30)
      SA2=SE(ISEMZA+30)
    ELSE IF (MZA .EQ. 6) THEN
      SA1=SE(ISEMZA+47)/LENGTH
      SA2=SE(ISEMZA+47)/LENGTH
    STIFF1=STIFF12/LENGTH
    STIFF2=STIFF22/LENGTH
    EI=SE(ISEMZA+48)
    TERM1=(LENGTH**2)/(12*EI*EI)
    TERM2=LENGTH/(3*EI)
  ENDF
  IF ( SA1 .EQ. 0 ) THEN
    FLX1=0

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ELSE IF ( SA1 .NE. 0 ) THEN
  FLX1=(1/STIF13)-(1/SA1)
ENDIF
IF ( SA2 .EQ. 0 ) THEN
  FLX2=0
ELSE IF ( SA2 .NE. 0 ) THEN
  FLX2=(1/STIF23)-(1/SA2)
ENDIF
WRITE(6,*)1.,STIF11, STIF23
WRITE(6,*)2.,SA1,SA2
WRITE(6,*)3.,FLX1,FLX2
IF ( M2A .EQ. 6 ) THEN
  DCOF=TERM1 + TERM2*(FLX1+FLX2) + FLX1*FLX2
ELSE IF ( M2A .NE. 6 ) THEN
  DCOF=(3/(SA1**2)) + (2/SA1)*(FLX1+FLX2) + FLX1*FLX2
ENDIF
DCOF=1/DCOF
IF ( M2A .EQ. 6 ) THEN
  A3 =DCOF*(TERM2 + FLX2)
  A22=DCOF*(TERM2 + FLX1)
  C2=DCOF*(0.5*TERM2)
ELSE IF ( M2A .NE. 6 ) THEN
  A3 =DCOF*(1/SA1)+FLX2
  A22=DCOF*(1/SA1)+FLX1
  C2=DCOF*(1/SA1)
ENDIF
ELSE IF ( M2A .EQ. 10 .AND. M2B .EQ. 10 ) THEN
  ELAS=SE(ISEM2A+4)
  SP =SE(ISEM2A+5)
  E12 =SE(ISEM2A)
  YILD12=SE(ISEM2A+3)
  YILD23=SE(ISEM2B+3)
  IF ( YILD12 .EQ. 1 .OR. YILD23 .EQ. 1 ) STBFAG=1
  IF ( ELAS .EQ. 0 ) THEN
    A2 =4*E12/LENGTH
    A23=4*E12/LENGTH
    C2 =2*E12/LENGTH
  ELSE IF ( ELAS .NE. 0 ) THEN
    IF ( YILD12 .EQ. 0 .AND. YILD23 .EQ. 0 ) THEN
      A2 =4*E12/LENGTH
      A23=4*E12/LENGTH
      C2 =2*E12/LENGTH
    ELSE IF ( YILD12 .EQ. 1 .AND. YILD23 .EQ. 0 ) THEN
      A2 =SP*(4*E12/LENGTH)+(1-SP)*(3*E12/LENGTH)
      C2 =SP*(2*E12/LENGTH)
    ELSE IF ( YILD12 .EQ. 0 .AND. YILD23 .EQ. 1 ) THEN
      A2 =SP*(4*E12/LENGTH)+(1-SP)*(3*E12/LENGTH)
      C2 =SP*(2*E12/LENGTH)
    ELSE IF ( YILD12 .EQ. 1 .AND. YILD23 .EQ. 1 ) THEN
      A2 =SP*(4*E12/LENGTH)
      A23=SP*(4*E12/LENGTH)
      C2 =SP*(2*E12/LENGTH)
    ENDIF
  ELSE
    WRITE(6,79)M2A,M2B,IELNO
  ENDIF
FOR AXIAL IN X AXIS
  IF ( M2A .EQ. 10 ) THEN
    ELAS=SE(ISEFXA+4)
    SP =SE(ISEFXA+5)
    EA =SE(ISEFXA)
    YILDEA=SE(ISEFXA+3)
    IF ( YILDEA .EQ. 1 ) STBFAG=1
    IF ( ELAS .EQ. 0 ) THEN
      H=EA/LENGTH
    ELSE IF ( ELAS .NE. 0 ) THEN
      IF ( YILDEA .EQ. 0 ) THEN
        H=EA/LENGTH
      ELSE IF ( YILDEA .EQ. 1 ) THEN
        H=SP*(EA/LENGTH)
      ENDIF
    ELSE IF ( MFXA .EQ. 13 ) THEN
      H=SE(ISEFXA)
    ELSE IF ( MFXA .NE. 10 .OR. MFXA .NE. 13 ) THEN
      WRITE(6,89)MFXA,IELNO
    ENDIF
FOR TORSION IN X AXIS
  IF ( M2A .EQ. 10 ) THEN
    ELAS=SE(ISEM2A+4)
    SP =SE(ISEM2A+5)
    GJ =SE(ISEM2A)
    YILDGJ=SE(ISEM2A+3)
    IF ( YILDGJ .EQ. 1 ) STBFAG=1
    IF ( ELAS .EQ. 0 ) THEN
      Q=GJ/LENGTH
    ELSE IF ( ELAS .NE. 0 ) THEN
      IF ( YILDGJ .EQ. 0 ) THEN
        Q=GJ/LENGTH
      ELSE IF ( YILDGJ .EQ. 1 ) THEN
        Q=SP*(GJ/LENGTH)
      ENDIF
    ELSE
      WRITE(6,99)M2A,IELNO
    ENDIF
S22=(A3+2*C2+A23)/(LENGTH**2)
S26=(A2+C2)/LENGTH
S12=(A22+C2)/LENGTH
S33=(BT+2*DT+BTY)/(LENGTH**2)
S35=(BT+DT)/LENGTH
S31=(BT+DT)/LENGTH
ASSEMBLE LOCAL STIFFNESS MATRIX
DO 894 J=1,12
DO 894 J=1,12
894 S(I,J)=0
S(1,1)=H
S(1,7)=-H
S(2,2)=S22
S(2,6)=S26
S(2,8)=-S22
S(2,12)=S212
S(3,3)=S33
S(3,5)=-S35
S(3,9)=-S31
S(3,11)=-S311
S(4,4)=Q
S(4,10)=-Q
S(5,5)=BT
S(5,9)=S35
S(5,11)=DT
S(6,6)=A2
S(6,8)=-S26
S(6,12)=C2
S(7,7)=H
S(8,8)=S22
S(8,12)=-S212

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S(9,9)=S33
S(9,11)=S311
S(10,10)=Q
S(11,11)=BTY
S(12,12)=A23
S(5,3)=S(3,5)
S(6,2)=S(2,6)
S(7,1)=S(1,7)
S(8,2)=S(2,8)
S(8,6)=S(6,8)
S(9,3)=S(3,9)
S(9,5)=S(5,9)
S(10,4)=S(4,10)
S(11,3)=S(3,11)
S(11,5)=S(5,11)
S(11,9)=S(9,11)
S(12,2)=S(2,12)
S(12,6)=S(6,12)
S(12,8)=S(8,12)
IF (FIRST) CALL WMATRX( 5 ,12,12,' S ')
C----- CALCULATE SAT
DO 52 J=1,12
DO 62 K=1,12
SAT(I,J)=0
52 SAT(I,J)=SAT(I,J)+S(I,K)*A(J,K)
C----- CALCULATE ASAT
DO 72 J=1,12
DO 72 K=1,12
ASAT(I,J)=0
72 ASAT(I,J)=ASAT(I,J)+A(I,K)*SAT(K,J)
IF ( BTEST(1BUG,8) ) CALL WMATRX(ASAT ,12,12,' ASAT ')
C----- DEGREE OF FREEDOM
C REMOVE DOF'S THAT ARE CONSTRAINED TO THE SAME DOF...
DO 77 I=1,6
IF (IDOF(JOINTI,I) .NE. IDOF(JOINTJ,I) ) THEN
  LMT(I)=IDOF(JOINTI,I)
  LMT(J)=IDOF(JOINTJ,I)
ELSE
  LMT(I)=0
  LMT(J)=0
77 ENDIF
CONTINUE
C----- CONVERT TO HALF STORAGE MODE BY COLUMNS
L=0
IELDOF=0
DO 92 I=1,12
IF (LMT(I) .EQ. 0 .OR. ASAT(I,I) .LE. 0) GO TO 92
IF (LMT(I) .EQ. 0) GO TO 92
IELDOF=IELDOF+1
LW(IELDOF)=LMT(I)
DO 82 J=1,12
IF (LMT(J) .EQ. 0 .OR. ASAT(J,J) .LE. 0) GO TO 82
IF (LMT(J) .EQ. 0) GO TO 82
L=L+1
STIFF(L)=ASAT(I,J)
82 CONTINUE
92 CONTINUE
IF ( BTEST(1BUG,7) ) THEN
  WRITE(6,*) 'LMT',LMT
  WRITE(6,*) 'IELDOF',IELDOF,'LM =',(LM(1),I=1,IELDOF)
  CALL LMATRX(STIFF,ND,IELDOF,(L+1),'ELEMENT STIFFNESS')
  WRITE(6,*) 'KGDATA',KGDATA,' PKG',PKG
ENDIF
RETURN
C----- DETERMINE INCREMENTAL FORCES -----
400 CONTINUE
IF (PRINT.AND.FIRST) WRITE (6,491) STEPID
C----- CALL BMLCA TO DETERMINE THE FIXED END FORCES
IF (MAYELD.GT.0) THEN
  CALL BMLCA(IELNO,FEMFLG,LENGTH,REL,MAXELD,NELD,NLOAD,
    & FEM,IELD,ELD)
ELSE
  FEMFLG=.FALSE.
ENDIF
C----- LOOP FOR EACH LOAD
DO 430 LOAD=1,NLOAD
C----- TRANSFORM FRAME DISPL INTO LOCAL DISPL
DO 410 I=1,3
R(I)=0.
R(I+3)=0.
R(I+6)=0.
R(I+9)=0.
DO 410 J=1,3
D1=DISPL(IDOF(JOINTI,J),LOAD)
D2=DISPL(IDOF(JOINTI,J+3),LOAD)
D3=DISPL(IDOF(JOINTI,J),LOAD)
D4=DISPL(IDOF(JOINTI,J+3),LOAD)
R(1)=R(1)+CTE(J,1)*D1 +SE(J,1)*D2
R(I+3)=R(I+3)+CTE(J,1)*D3 +SE(J,1)*D4
R(I+6)=R(I+6)+CTE(J,1)*D1 +SE(J,1)*D4
410 R(I+9)=R(I+9)+CTE(J,1)*D4
C----- INCREMENTAL FORCES
F(1)=H*(R(1)-R(7))
F(2)=S22*(R(2)-R(8))+S26*(R(6)+S212*(R(12)))
F(3)=S33*(R(3)-R(9))-S35*(R(5))-S311*(R(11))
F(4)=Q*(R(4)-R(10))
F(5)=S35*(R(3)-R(9))+BT*(R(5))+DT*(R(11))
F(6)=S26*(R(2)-R(8))+A2*(R(6))+C2*(R(12))
F(7)=-H*(R(1)-R(7))
F(8)=-S22*(R(2)-R(8))-S26*(R(6))-S212*(R(12))
F(9)=-S33*(R(3)-R(9))+S35*(R(5))+S311*(R(11))
F(10)=-Q*(R(4)-R(10))
F(11)=-S311*(R(3)-R(9))+DT*(R(5))+BT*(R(11))
F(12)=S212*(R(2)-R(8))+C2*(R(6))+A22*(R(12))
C----- RELATIVE ROTATION FOR BENDINGS
R(5)=R(5)-(R(3)-R(9))/LENGTH
R(6)=R(6)-(R(8)-R(2))/LENGTH
R(11)=R(11)-(R(9)-R(3))/LENGTH
R(12)=R(12)-(R(8)-R(2))/LENGTH
C IF (M2A .EQ. 9 .OR. M2A .EQ. 11) .AND.
& (M2B .EQ. 9 .OR. M2B .EQ. 11) ) THEN
  F(5)=STIF1*(R(5))
  F(11)=STIF2*(R(11))
  F(9)=(F(5)+F(11))/LENGTH
  F(3)=-F(9)
ENDIF
IF (M2A .EQ. 9 .OR. M2A .EQ. 11) .AND.
& (M2B .EQ. 9 .OR. M2B .EQ. 11) ) THEN
  F(12)=STIF2*(R(12))
  F(2)=(F(5)+F(11))/LENGTH
  F(8)=-F(2)
ENDIF
C----- SUBTRACT OF FIXED END FORCES
IF (FEMFLG) THEN

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DO 420 I=1,12
  IF ( I.EQ. 5 .OR. I.EQ. 11 .OR.
    & I.EQ. 3 .OR. I.EQ. 9 ) THEN
    F(I)=FEM(I,LOAD)
    IF ( MTA.NE. 10 .OR. MYB.NE. 10 ) GO TO 420
  ENDF
  IF ( I.EQ. 6 .OR. I.EQ. 12 .OR.
    & I.EQ. 2 .OR. I.EQ. 8 ) THEN
    IF ( M2A.NE. 10 .OR. M2B.NE. 10 ) GO TO 420
  F(I)=FEM(I,LOAD)
  ENDF
420 CONTINUE
  ENDF
C----- DETERMINE MAXIMUM VALUES FOR MULTIPLE LOAD CASES
  IF (NLOAD.GT.1) THEN
    DO 425 I=1,12
      IF ( ABS(F(I)).GT.ABS(FMAX(I*12-12-I)) ) THEN
        DO 426 J=0,11
          FMAX(I*12-11+J)=F(J+1)
        ENDF
      ENDF
425 CONTINUE
  ENDF
C----- PRINT FORCES AND DEFORMATIONS
  IF (STEST(18,0).AND.PRINT)
    IF ( PRINT)
      WRITE (6,492) IELNO,LOAD,NODEJ,(R(J),J=1,6),
        & NODEJ,(R(J),J=7,12)
      IF ( PRINT) WRITE (6,492) IELNO,LOAD,NODEJ,(F(J),J=1,6),
        & NODEJ,(F(J),J=7,12),
        & STBFAJ
C
430 CONTINUE
C-----THIS IS FOR SOLOSA USE ONLY
  IF (ICOMN.EQ.1 .OR. ICOMN.EQ.2) THEN
    DO 431 I=1,12
      R5AF(I)=F(I)
    ENDF
431
C-----CHECK STIFFNESS
  THIS IS PERFORMED ONLY FOR INELASTIC ANALYSIS
  P(1)=F(5) + F(5)
  P(2)=F(11) + F(11)
  P(3)=F(6) + F(6)
  P(4)=F(12) + F(12)
  P(5)=F(10) + F(10)
  P(6)=F(7) + F(7)
  IF (.NOT. ELSTIC) THEN
C.....CHECK INTERACTIVE STRENGTH CRITERIA
    PT = SE(ISEFXA+1)
    TMPY = SE(ISEMTA+1)
    TMPZ = SE(ISEMZA+1)
    TMPFA = SE(ISEMTA+1)
    TMPFB = SE(ISEMTB+1)
    TMPFA = SE(ISEMZA+1)
    TMPFB = SE(ISEMZA+1)
    IF ( IAFAG.EQ. 1 ) THEN
      CALL ITERS(P(6),P(1),P(2),TMPY,PT,TMPFA,TMPFB,
        & P(3),P(4),TMPZ, TMPZA,TMPZB,IAFAG)
    IF ( IAFAG.EQ. 2 ) THEN
      SE(ISEMZA+1)=ABS(P(1))*SE(ISEMZA+8)
      SE(ISEMZA+1)=ABS(P(3))*SE(ISEMZA+8)
      SE(ISEMZA+2)=-SE(ISEMZA+1)
      SE(ISEMZA+2)=-SE(ISEMZA+1)
      ELSE IF ( IAFAG.EQ. 3 ) THEN
      SE(ISEMZA+1)=ABS(P(2))*SE(ISEMZA+8)
      SE(ISEMZA+1)=ABS(P(4))*SE(ISEMZA+8)
      SE(ISEMZA+2)=-SE(ISEMZA+1)
      SE(ISEMZA+2)=-SE(ISEMZA+1)
      ELSE IF ( IAFAG.EQ. 4 ) THEN
      SE(ISEMZA+1)=TMPFA*SE(ISEMZA+8)
      SE(ISEMZA+1)=TMPFA*SE(ISEMZA+8)
      SE(ISEMZA+2)=-TMPFB*SE(ISEMZA+8)
      SE(ISEMZA+2)=-TMPFB*SE(ISEMZA+8)
      SE(ISEMZA+3)=TMPFA*SE(ISEMZA+8)
      SE(ISEMZA+3)=TMPFA*SE(ISEMZA+8)
      SE(ISEMZA+4)=-SE(ISEMZA+1)
      SE(ISEMZA+4)=-SE(ISEMZA+1)
      SE(ISEMZA+5)=-SE(ISEMZA+1)
      SE(ISEMZA+5)=-SE(ISEMZA+1)
      SE(ISEMZA+6)=-SE(ISEMZA+1)
      SE(ISEMZA+6)=-SE(ISEMZA+1)
      ENDF
C..... ASSIGN AXIAL RESIDUAL STRENGTH
    IF ( SE(ISEFXA+1).EQ.-SE(ISEFXA+2) .AND. IAFAG.NE.1 ) THEN
      SE(ISEFXA+1)=SE(ISEFXA+2)
      SE(ISEFXA+2)=-ABS(P(6))*SE(ISEFXA+8)
      WRITE(6,705) IELNO,JSTEP,IAFAG
        & FORMAT('X',' ELEMENT ',15,' LOCAL BUCKLING OCCURRED ',
        & AT STEP ',15,' IAFAG=',15,' ')
      ENDF
    ELSE IF ( IAFAG.EQ. 2 ) THEN
      CALL ITERS(P(6),P(1),P(2),TMPY,PT,TMPFA,TMPFB,
        & P(3),P(4),TMPZ, TMPZA,TMPZB,IAFAG)
    IF ( IAFAG.EQ. 4 ) THEN
      SE(ISEMZA+1)=ABS(P(2))*SE(ISEMZA+8)
      SE(ISEMZA+1)=ABS(P(4))*SE(ISEMZA+8)
      SE(ISEMZA+2)=-SE(ISEMZA+1)
      SE(ISEMZA+2)=-SE(ISEMZA+1)
      WRITE(6,705) IELNO,JSTEP,IAFAG
      ENDF
    ELSE IF ( IAFAG.EQ. 3 ) THEN
      CALL ITERS(P(6),P(1),P(2),TMPY,PT,TMPFA,TMPFB,
        & P(3),P(4),TMPZ, TMPZA,TMPZB,IAFAG)
    IF ( IAFAG.EQ. 4 ) THEN
      SE(ISEMZA+1)=ABS(P(1))*SE(ISEMZA+8)
      SE(ISEMZA+1)=ABS(P(3))*SE(ISEMZA+8)
      SE(ISEMZA+2)=-SE(ISEMZA+1)
      SE(ISEMZA+2)=-SE(ISEMZA+1)
      WRITE(6,705) IELNO,JSTEP,IAFAG
      ENDF
    ELSE IF ( IAFAG.EQ. 5 ) THEN
      CALL ITERS(P(6),P(1),P(2),TMPY,PT,TMPFA,TMPFB,
        & P(3),P(4),TMPZ, TMPZA,TMPZB,IAFAG)
    IF ( IAFAG.GT.5 ) THEN
      WRITE(6,706) IELNO,JSTEP,IAFAG
        & FORMAT('X',' ELEMENT ',15,' MODEL OVERSTRENGTH ',
        & AT STEP ',15,' IAFAG=',15,' ')
      ENDF
      ENDF
C----- TRANSFER PERMANENT HYSTERESIS DATA TO TEMPORARY STORAGE
377 ISE2=ISE2
  DO 522 I=1,ISE2
    SE(I+ISE2)=SE(I)
  ENDF
C----- CALCULATE INELASTIC ROTATION
  IF ( MTA.EQ. 9 .AND. MYB.EQ. 9 ) .OR.
    & ( MTA.EQ. 11 .AND. MYB.EQ. 11 ) .OR.
    & ( MTA.EQ. 6 .AND. MYB.EQ. 6 ) ) THEN
    STIF1=SE(ISEMZA)
    STIF2=SE(ISEMZA)
    IF ( MTA.EQ. 9 ) THEN
      SA1=SE(ISEMZA+60)
      SA2=SE(ISEMZA+60)
    ELSE IF ( MTA.EQ. 11 ) THEN
      SA1=SE(ISEMZA+30)
      SA2=SE(ISEMZA+30)
    ELSE IF ( MTA.EQ. 6 ) THEN
      SA1=SE(ISEMZA+47)/LENGTH
      SA2=SE(ISEMZA+47)/LENGTH
  ENDF

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ELEM3040 SA2=SE(ISEMZA+47)/LENGTH
ELEM3050 STIF1=STIF1/LENGTH
ELEM3060 STIF2=STIF2/LENGTH
ENDF
IF ( SA1.EQ. 0 ) THEN
  FLX1=0
ELSE IF ( SA1.NE. 0 ) THEN
  FLX1=(1/STIF1)-(1/SA1)
ENDF
IF ( SA2.EQ. 0 ) THEN
  FLX2=0
ELSE IF ( SA2.NE. 0 ) THEN
  FLX2=(1/STIF2)-(1/SA2)
ENDF
FLX1=(1/STIF1)-(1/SA1)
FLX2=(1/STIF2)-(1/SA2)
R1E2A=ROIE2A + FLX1*F(5)
R1E2B=ROIE2B + FLX2*F(11)
ENDF
IF ( M2A.EQ. 11 .AND. M2B.EQ. 11 ) .OR.
  & ( M2A.EQ. 6 .AND. M2B.EQ. 6 ) ) THEN
  STIF12=SE(ISEMZA)
  STIF22=SE(ISEMZA)
  IF ( M2A.EQ. 9 ) THEN
    SA1=SE(ISEMZA+60)
    SA2=SE(ISEMZA+60)
  ELSE IF ( M2A.EQ. 11 ) THEN
    SA1=SE(ISEMZA+30)
    SA2=SE(ISEMZA+30)
  ELSE IF ( M2A.EQ. 6 ) THEN
    SA1=SE(ISEMZA+47)/LENGTH
    SA2=SE(ISEMZA+47)/LENGTH
  STIF12=STIF1/LENGTH
  STIF22=STIF2/LENGTH
  ENDF
  IF ( SA1.EQ. 0 ) THEN
    FLX1=0
  ELSE IF ( SA1.NE. 0 ) THEN
    FLX1=(1/STIF1)-(1/SA1)
  ENDF
  IF ( SA2.EQ. 0 ) THEN
    FLX2=0
  ELSE IF ( SA2.NE. 0 ) THEN
    FLX2=(1/STIF2)-(1/SA2)
  ENDF
  FLX1=(1/STIF1)-(1/SA1)
  FLX2=(1/STIF2)-(1/SA2)
  R1E2A=ROIE2A + FLX1*F(6)
  R1E2B=ROIE2B + FLX2*F(12)
  ENDF
C----- BENDING IN A END T DIRECTION
  SET UP TEMPERART - TOTAL LOADS,DISP'S
  PL=PT(5)
  D=RT(5)+R(5)
  DL=RT(5)
  IF ( MTA.EQ. 9 ) THEN
    DL=ROIE2A + P(1)/SE(ISEMZA+60)
    DL=ROIE2A + PL/SE(ISEMZA+60)
  ELSE IF ( MTA.EQ. 11 ) THEN
    DL=ROIE2A + P(1)/SE(ISEMZA+30)
    DL=ROIE2A + PL/SE(ISEMZA+30)
  ELSE IF ( MTA.EQ. 6 ) THEN
    IF ( SE(ISEMZA+47).NE. 0 ) THEN
      DL=(ROIE2A/LENGTH) + P(1)/SE(ISEMZA+47)
      DL=(ROIE2A/LENGTH) + PL/SE(ISEMZA+47)
    ENDF
    MPTP=3
    CALL MATLIB
    (MPTP,LETTYP,FALSE,IREL,FALSE,NAME,SESE,EPSE,DUCT(1,1),EXCR(1,1),
    & P(1),PL,D,DL,V,VL,LELEM,LMAT ,
    & MATMYB,MATTP,SE(ISE2+ISEMTA),IELNO,DAMAGE)
    IF ( MATTP.EQ. 9 ) THEN
      ROIE2A=D-P(1)/SE(ISEMZA+60)
      ROIE2A=D-P(1)/SE(ISEMZA+30)
    ELSE IF ( MATTP.EQ. 6 ) THEN
      IF ( SE(ISE2+ISEMTA).NE. SE(ISEMZA) ) NEWK=.TRUE.
      ELSE IF ( MATTP.EQ. 11 ) THEN
      IF ( SE(ISE2+ISEMTA).NE. SE(ISEMZA) ) NEWK=.TRUE.
      ELSE IF ( MATTP.EQ. 9 ) THEN
      IF ( SE(ISE2+ISEMTA).NE. SE(ISEMZA) ) NEWK=.TRUE.
      IF ( SE(ISEMZA+47).NE. 0 ) THEN
        ROIE2A=(D-P(1)/SE(ISEMZA+47))*LENGTH
      ENDF
      ELSE IF ( MATTP.EQ. 10 ) THEN
      IF ( SE(ISE2+ISEMTA+3).NE. SE(ISEMZA+3) ) NEWK=.TRUE.
      ENDF
C----- BENDING IN B END T DIRECTION
  SET UP TEMPERART - TOTAL LOADS,DISP'S
  PL=PT(11)
  D=RT(11)+R(11)
  DL=RT(11)
  IF ( MYB.EQ. 9 ) THEN
    DL=ROIE2B + P(2)/SE(ISEMZA+60)
    DL=ROIE2B + PL/SE(ISEMZA+60)
  ELSE IF ( MYB.EQ. 11 ) THEN
    DL=ROIE2B + P(2)/SE(ISEMZA+30)
    DL=ROIE2B + PL/SE(ISEMZA+30)
  ELSE IF ( MYB.EQ. 6 ) THEN
    IF ( SE(ISEMZA+47).NE. 0 ) THEN
      DL=(ROIE2B/LENGTH) + P(2)/SE(ISEMZA+47)
      DL=(ROIE2B/LENGTH) + PL/SE(ISEMZA+47)
    ENDF
    CALL MATLIB
    (MPTP,LETTYP,FALSE,IREL,FALSE,NAME,SESE,EPSE,DUCT(1,2),EXCR(1,2),
    & P(2),PL,D,DL,V,VL,LELEM,LMAT ,
    & MATMYB,MATTP,SE(ISE2+ISEMTB),IELNO,DAMAGE)
    IF ( MATTP.EQ. 9 ) THEN
      ROIE2B=D-P(2)/SE(ISEMZA+60)
      ROIE2B=D-P(2)/SE(ISEMZA+30)
    ELSE IF ( MATTP.EQ. 6 ) THEN
      IF ( SE(ISE2+ISEMTB).NE. SE(ISEMZA) ) NEWK=.TRUE.
      ELSE IF ( MATTP.EQ. 11 ) THEN
      IF ( SE(ISE2+ISEMTB).NE. SE(ISEMZA) ) NEWK=.TRUE.
      IF ( SE(ISEMZA+47).NE. 0 ) THEN
        ROIE2B=(D-P(2)/SE(ISEMZA+47))*LENGTH
      ENDF
      ELSE IF ( MATTP.EQ. 10 ) THEN
      IF ( SE(ISE2+ISEMTA+3).NE. SE(ISEMZA+3) ) NEWK=.TRUE.
      ENDF
C----- BENDING IN A END 2 DIRECTION
  SET UP TEMPERART - TOTAL LOADS,DISP'S
  PL=PT(6)
  D=RT(6)+R(6)
  DL=RT(6)
  IF ( M2A.EQ. 9 ) THEN
    DL=R1E2A + P(3)/SE(ISEMZA+60)
    DL=R1E2A + PL/SE(ISEMZA+60)
  ELSE IF ( M2A.EQ. 11 ) THEN
    DL=R1E2A + P(3)/SE(ISEMZA+30)
    DL=R1E2A + PL/SE(ISEMZA+30)
  ELSE IF ( M2A.EQ. 6 ) THEN
    IF ( SE(ISEMZA+47).NE. 0 ) THEN
      DL=(R1E2A/LENGTH) + P(3)/SE(ISEMZA+47)
      DL=(R1E2A/LENGTH) + PL/SE(ISEMZA+47)
    ENDF
    ENDF

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ELEM4460
ELEM4470
ELEM4480
ELEM4490
ELEM4500
ELEM4510
ELEM4520
ELEM4530
ELEM4540
ELEM4550
ELEM4560
ELEM4570
ELEM4580
ELEM4590
ELEM4600
ELEM4610
ELEM4620
ELEM4630
ELEM4640
ELEM4650
ELEM4660
ELEM4670
ELEM4680
ELEM4690
ELEM4700
ELEM4710
ELEM4720
ELEM4730
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ELEM4960
ELEM4970
ELEM4980
ELEM4990
ELEM5000
ELEM5010
ELEM5020
ELEM5030
ELEM5040
ELEM5050
ELEM5060
ELEM5070
ELEM5080
ELEM5090
ELEM5100
ELEM5110
ELEM5120
ELEM5130
ELEM5140
ELEM5150
ELEM5160
ELEM5170
ELEM5180
ELEM5190
ELEM5200
ELEM5210
ELEM5220
ELEM5230
ELEM5240
ELEM5250
ELEM5260
ELEM5270
ELEM5280
ELEM5290
ELEM5300
ELEM5310
ELEM5320
ELEM5330
ELEM5340
ELEM5350
ELEM5360
ELEM5370
ELEM5380
ELEM5390
ELEM5400
ELEM5410
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ELEM5600
ELEM5610
ELEM5620
ELEM5630
ELEM5640
ELEM5650
ELEM5660
ELEM5670
ELEM5680
ELEM5690
ELEM5700
ELEM5710
ELEM5720
ELEM5730
ELEM5740
ELEM5750
ELEM5760
ELEM5770
ELEM5780
ELEM5790
ELEM5800
ELEM5810
ELEM5820
ELEM5830
ELEM5840
ELEM5850
ELEM5860
ELEM5870

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CALL MATLIB
*(MOPT,LSSTYP,FALSE,IREL,FALSE,NAME,EESSE,EPSE,DUCT(1,3),EXCR(1,3),
* P(3),PL,D,VL,LELEM,LMAT,
* MATM3A,MATTP,SE(ISE2+ISEM3A),IELNO,DAMAGE)
IF(MATTP.EQ.9) THEN
  IF(SE(ISE2+ISEM3A).NE.SE(ISEM3A)) NEWK=.TRUE.
  ROIE2A=D-P(3)/SE(ISEM3A+60)
ELSE IF(MATTP.EQ.11) THEN
  IF(SE(ISE2+ISEM3A).NE.SE(ISEM3A)) NEWK=.TRUE.
  ROIE2A=D-P(3)/SE(ISEM3A+30)
ELSE IF(MATTP.EQ.6) THEN
  IF(SE(ISE2+ISEM3A).NE.SE(ISEM3A)) NEWK=.TRUE.
  IF(SE(ISE2+ISEM3A+7).NE.0) THEN
    ROIE2A=(D-P(3)/SE(ISEM3A+7))*LENGTH
  ENDIF
ELSE IF(MATTP.EQ.10) THEN
  IF(SE(ISE2+ISEM3A+3).NE.SE(ISEM3A+3)) NEWK=.TRUE.
ENDIF
C----- BENDING IN B END Z DIRECTION
C SET UP TEMPERRARY - TOTAL LOADS, DISP'S
PL=FT(12)
D=RT(12)+R(12)
DL=RT(12)
IF(M3B.EQ.9) THEN
  D=RIE2B+P(4)/SE(ISEM2B+60)
  DL=ROIE2B+PL/SE(ISEM2B+60)
ELSE IF(M3B.EQ.11) THEN
  D=RIE2B+P(4)/SE(ISEM2B+30)
  DL=ROIE2B+PL/SE(ISEM2B+30)
ELSE IF(M3B.EQ.6) THEN
  IF(SE(ISE2+ISEM2B+7).NE.0) THEN
    D=(RIE2B/LENGTH)+P(4)/SE(ISEM2B+47)
    DL=(ROIE2B/LENGTH)+PL/SE(ISEM2B+47)
  ENDIF
ENDIF
CALL MATLIB
*(MOPT,LSSTYP,FALSE,IREL,FALSE,NAME,EESSE,EPSE,DUCT(1,4),EXCR(1,4),
* P(4),PL,D,VL,LELEM,LMAT,
* MATM2B,MATTP,SE(ISE2+ISEM2B),IELNO,DAMAGE)
IF(MATTP.EQ.9) THEN
  IF(SE(ISE2+ISEM2B).NE.SE(ISEM2B)) NEWK=.TRUE.
  ROIE2B=D-P(4)/SE(ISEM2B+60)
ELSE IF(MATTP.EQ.11) THEN
  IF(SE(ISE2+ISEM2B).NE.SE(ISEM2B)) NEWK=.TRUE.
  ROIE2B=D-P(4)/SE(ISEM2B+30)
ELSE IF(MATTP.EQ.6) THEN
  IF(SE(ISE2+ISEM2B).NE.SE(ISEM2B)) NEWK=.TRUE.
  IF(SE(ISE2+ISEM2B+7).NE.0) THEN
    ROIE2B=(D-P(4)/SE(ISEM2B+7))*LENGTH
  ENDIF
ELSE IF(MATTP.EQ.10) THEN
  IF(SE(ISE2+ISEM2B+3).NE.SE(ISEM2B+3)) NEWK=.TRUE.
ENDIF
C----- TORSION IN A END X DIRECTION
C SET UP TEMPERRARY - TOTAL LOADS, DISP'S
PL=FT(10)
D=RT(10)+R(10)-RT(4)-R(4)
DL=RT(10)-RT(4)
CALL MATLIB
*(MOPT,LSSTYP,FALSE,IREL,FALSE,NAME,EESSE,EPSE,DUCT(1,5),EXCR(1,5),
* P(5),PL,D,VL,LELEM,LMAT,
* MATM1A,MATTP,SE(ISE2+ISEM1A),IELNO,DAMAGE)
IF(SE(ISE2+ISEM1A+3).NE.SE(ISEM1A+3)) NEWK=.TRUE.
C----- AXIAL IN A END X DIRECTION
C SET UP TEMPERRARY - TOTAL LOADS, DISP'S
PL=FT(10)
D=RT(7)+R(7)-RT(1)-R(1)
DL=RT(7)-RT(1)
CALL MATLIB
*(MOPT,LSSTYP,FALSE,IREL,FALSE,NAME,EESSE,EPSE,DUCT(1,6),EXCR(1,6),
* P(6),PL,D,VL,LELEM,LMAT,
* MATFXA,MATTP,SE(ISE2+ISEFXA),IELNO,DAMAGE)
IF(MFXA.EQ.10) THEN
  IF(SE(ISE2+ISEFXA+3).NE.SE(ISEFXA+3)) NEWK=.TRUE.
  IF(MFXA.EQ.13) THEN
    IF(SE(ISE2+ISEFXA).NE.SE(ISEFXA)) NEWK=.TRUE.
  ENDIF
ENDIF
C
C----- CALCULATE THE UNBALANCED MEMBER FORCES ON THE JOINT
UF(5)=- (FT(5)+F(5)-P(1))
UF(11)=- (FT(11)+F(11)-P(2))
UF(6)=- (FT(6)+F(6)-P(3))
UF(12)=- (FT(12)+F(12)-P(4))
UF(10)=- (FT(10)+F(10)-P(5))
UF(7)=- (FT(7)+F(7)-P(6))
UF(9)=(UF(5)+UF(11))/LENGTH
UF(3)=-UF(9)
UF(2)=(UF(6)+UF(12))/LENGTH
UF(8)=-UF(2)
UF(1)=-UF(7)
UF(4)=-UF(10)
C
DO 440 I=1,IELDOP
  R=LM(I)
  EFUB(I)=0
  DO 460 J=1,12
    FUB(J)=FUB(I)+A(I,J)*UF(J)
  460 CONTINUE
  440 CONTINUE
IF (BTST('IBUG,7)) THEN
  WRITE(6,97) UNBALANCE FORCES FOR ELEMENT ',IELNO,' '
  WRITE(6,97) (I,P(I),I=1,6)
  WRITE(6,97) (I,F(I),FT(I),UF(I),I=1,12)
  WRITE(6,97) (I,LM(I),EFUB(I),I=1,IELDOP)
ENDIF
C----- ADJUST F FOR UNBALANCED LOAD P
DO 445 I=1,12
  P(I)=F(I)+UF(I)
C
491 FORMAT (// 'E3D BEAM FORCES...5X/7X,A',
* ' ELEMENT LOAD NODE',7X,'AXIAL',11X,'FY',13X,'FZ',
* ' 11X','TORSION',10X,'MT',13X,'M2 STBFAG')
492 FORMAT (//,110,15.2X,'FORCE',16,1P,6G15.6,
* /15X,'2X','FORCE',16, 6G15.6,3X,11)
493 FORMAT (//,110,15.2X,'DISPL',16,1P,6G15.6,
* /15X,'2X','DISPL',16, 6G15.6)
494 FORMAT (7X,A)
495 FORMAT (' I',12,' F',1P,1P,G12.4,' FT',G12.4,' UF',G12.4)
496 FORMAT (' I',12,' LM(I)',1P,G12.4,' FUB(I)',G12.4)
497 FORMAT (' I',12,' P(1)',1P,G12.4)
RETURN
C----- DETERMINE TOTAL FORCES -----
500 CONTINUE
IF (PRINT.AND.FIRST) WRITE (6,491) STEPID
IF(ICOMN.NE.1.AND.ICOMN.NE.2) THEN
C----- TOTAL FORCES AND DISPLACEMENTS
DO 510 I=1,12
  IF (BTST('IBUG,7)) WRITE(6,520) I,R(I),RT(I),F(I),FT(I)
  FT(I)=FT(I)+F(I)
  RT(I)=RT(I)+R(I)
510 CONTINUE
DO 512 I=1,12
  IF (ABS(FT(I)).GT.ABS(FMAX(I*12-12+I))) THEN
    511 FMAX(I*12-11+J)=FT(J+1)
  ENDIF
ENDIF

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C CTE = LOCAL TO GLOBAL ROTATION MATRIX END ELE00890
C BS = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX START ELE00900
C BE = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX END ELE00910
C STIFF = ELEMENT STIFFNESS ELE00920
C IELTYP = ELEMENT TYPE = 12 ELE00930
C NODEI = EXTERNAL JOINT NUMBER, END I ELE00940
C NODEJ = EXTERNAL JOINT NUMBER, END J ELE00950
C JOINTI = INTERNAL JOINT NUMBER, END I ELE00960
C JOINTJ = INTERNAL JOINT NUMBER, END J ELE00970
C----- TEMPORARY VARIABLES
C VX = VECTOR DEFINING THE ELEMENT X AXIS ELE00980
C VY = VECTOR DEFINING THE ELEMENT Y AXIS ELE00990
C COSLOC = LOCAL COSINE MATRIX ELE01000
C A = GLOBAL TO LOCAL FORCE TRANSFORMATION MATRIX ELE01010
C S = ELEMENT STIFFNESS AT LOCAL COORD. AT ENDS OF FLEXIBLE PART ELE01020
C SAT = PRODUCT OF S*A ELE01030
C ASAT = PRODUCT OF A*SAT ELE01040
C-----
C----- CHOOSE OPTION ELE01050
C IF (IOPT.LT.1 .OR. IOPT.GT.14) ELE01060
  & WRITE (6,*) 'INVALID OPTION IN ELE-09, IOPT=', IOPT ELE01070
  GO TO (100,200,300,400,500,600,700,800,900,1000,1100,1200, ELE01080
    1300,1400), IOPT ELE01090
C 100 CONTINUE ELE01100
C----- ZERO STRAIN ENERGY ELE01110
  ENGDAT(2) = 0 ELE01120
C IF (BTEST(1BUG,7)) THEN ELE01130
  CALL WMATRX(RINPUB,8,1,'RINPUB') ELE01140
  ENDF ELE01150
C----- INTERPERTE INPUT DATA ELE01160
  IELTYP=12 ELE01170
  MAT = RINPUB(1) ELE01180
  NODEI = RINPUB(2) ELE01190
  NODEJ = RINPUB(3) ELE01200
  VI(1) = RINPUB(4) ELE01210
  VI(2) = RINPUB(5) ELE01220
  VI(3) = RINPUB(6) ELE01230
  XS = RINPUB(7) ELE01240
  XE = RINPUB(8) ELE01250
C----- GET INTERNAL NODE # ASSOCIATED WITH EXTERNAL NODE #'S ELE01260
  JOINTI=IDUCK(NODEI,1D,NNODE) ELE01270
  JOINTJ=IDUCK(NODEJ,1D,NNODE) ELE01280
  WRITE (6,*) 'ID',ID ELE01290
C----- PRINT DATA FOR PLOTTING ELE01300
  IF (BTEST(1BUG,2)) THEN ELE01310
    WRITE(7,10) NODEJ, NODEI, ELE01320
    & (COORD(JOINTJ),I=1,3), (COORD(JOINTI),J=1,3) ELE01330
    10 FORMAT (15,1X,15,1P,6G12.4) ELE01340
    ENDF ELE01350
C----- GET GLOBAL TO LOCAL TRANSFORMATION MATRIX ELE01360
C----- DEFINE VECTOR X, LENGTH ELE01370
  VX(1)=COORD(JOINTJ,1)-COORD(JOINTI,1) ELE01380
  VX(2)=COORD(JOINTJ,2)-COORD(JOINTI,2) ELE01390
  VX(3)=COORD(JOINTJ,3)-COORD(JOINTI,3) ELE01400
  LENGTH=SQRT(VX(1)**2+VX(2)**2+VX(3)**2)-XS-XE ELE01410
C SE(79)=LENGTH ELE01420
C IF (LENGTH.LE.0) THEN ELE01430
  WRITE(6,10) IELNG,JOINTI,JOINTJ,LENGTH ELE01440
  101 FORMAT(1X,20(' '), 'ER', 'ROR - LENGTH IS LE 0',// ELE01450
    & 5X, 'IELNG=',15,5X, 'JOINTI=',15,5X, 'JOINTJ=',15, ELE01460
    & 5X, 'LENGTH=',15,615.67 ELE01470
    & 5X, 'REVISE INPUT.....'//) ELE01480
  LENGTH=1.0 ELE01490
  ENDF ELE01500
  IF (BTEST(1BUG,3)) THEN ELE01510
    WRITE (6,*) 'JOINTS', JOINTI,JOINTJ ELE01520
    WRITE (6,*) 'VX', VX ELE01530
    WRITE (6,*) 'VI', VI ELE01540
    WRITE (6,*) 'LENGTH', LENGTH ELE01550
  ENDF ELE01560
C----- GET LOCAL TO GLOBAL TRANSFORMATION MATRICES CT AND B ELE01570
  ICS=JUCOS(JOINTI) ELE01580
  ICE=JUCOS(JOINTJ) ELE01590
C CALL ROTX2(VX,VI,COSINE(1,1,ICS),CTS, XE,0.,0.,BS, ELE01600
  & CONST(JOINTI,1),MAXNO) ELE01610
  CALL ROTX2(VX,VI,COSINE(1,1,ICE),CTE, XE,0.,0.,BE, ELE01620
  & CONST(JOINTJ,1),MAXNO) ELE01630
  IF (BTEST(1BUG,4)) THEN ELE01640
    CALL WMATRX(CTS,3,3,'CTS') ELE01650
    CALL WMATRX(CTE,3,3,'CTE') ELE01660
  ENDF ELE01670
C----- GET GLOBAL CONSTRAINT MATRIX BS, BE ELE01680
  BS(1,1)=CONST(JOINTI,1)*CTS(2,1)+CONST(JOINTI,2)*CTS(3,1)+BS(1,1) ELE01690
  BS(1,2)=CONST(JOINTI,1)*CTS(2,2)+CONST(JOINTI,2)*CTS(3,2)+BS(1,2) ELE01700
  BS(1,3)=CONST(JOINTI,1)*CTS(2,3)+CONST(JOINTI,2)*CTS(3,3)+BS(1,3) ELE01710
  BS(2,1)=CONST(JOINTI,3)*CTS(1,1)+CONST(JOINTI,4)*CTS(3,1)+BS(2,1) ELE01720
  BS(2,2)=CONST(JOINTI,3)*CTS(1,2)+CONST(JOINTI,4)*CTS(3,2)+BS(2,2) ELE01730
  BS(2,3)=CONST(JOINTI,3)*CTS(1,3)+CONST(JOINTI,4)*CTS(3,3)+BS(2,3) ELE01740
  BS(3,1)=CONST(JOINTI,5)*CTS(1,1)+CONST(JOINTI,6)*CTS(3,1)+BS(3,1) ELE01750
  BS(3,2)=CONST(JOINTI,5)*CTS(1,2)+CONST(JOINTI,6)*CTS(3,2)+BS(3,2) ELE01760
  BS(3,3)=CONST(JOINTI,5)*CTS(1,3)+CONST(JOINTI,6)*CTS(3,3)+BS(3,3) ELE01770
C BS(1,1)=CONST(JOINTJ,1)*CTE(2,1)+CONST(JOINTJ,2)*CTE(3,1)+BS(1,1) ELE01780
  BS(1,2)=CONST(JOINTJ,1)*CTE(2,2)+CONST(JOINTJ,2)*CTE(3,2)+BS(1,2) ELE01790
  BS(1,3)=CONST(JOINTJ,1)*CTE(2,3)+CONST(JOINTJ,2)*CTE(3,3)+BS(1,3) ELE01800
  BS(2,1)=CONST(JOINTJ,3)*CTE(1,1)+CONST(JOINTJ,4)*CTE(3,1)+BS(2,1) ELE01810
  BS(2,2)=CONST(JOINTJ,3)*CTE(1,2)+CONST(JOINTJ,4)*CTE(3,2)+BS(2,2) ELE01820
  BS(2,3)=CONST(JOINTJ,3)*CTE(1,3)+CONST(JOINTJ,4)*CTE(3,3)+BS(2,3) ELE01830
  BS(3,1)=CONST(JOINTJ,5)*CTE(1,1)+CONST(JOINTJ,6)*CTE(3,1)+BS(3,1) ELE01840
  BS(3,2)=CONST(JOINTJ,5)*CTE(1,2)+CONST(JOINTJ,6)*CTE(3,2)+BS(3,2) ELE01850
  BS(3,3)=CONST(JOINTJ,5)*CTE(1,3)+CONST(JOINTJ,6)*CTE(3,3)+BS(3,3) ELE01860
C IF (BTEST(1BUG,7)) THEN ELE01870
  CALL WMATRX(BS,3,3,'BS') ELE01880
  CALL WMATRX(BE,3,3,'BE') ELE01890
  ENDF ELE01900
C----- ZERO MATRICES ELE01910
  DO 40 I=1,12 ELE01920
    FMAX(I)=0 ELE01930
    F(I)=0 ELE01940
    FT(I)=0 ELE01950
    R(I)=0 ELE01960
    RT(I)=0 ELE01970
    DO 40 J=1,12 ELE01980
      S(I,J)=0 ELE01990
      A(I,J)=0 ELE02000
  40 CONTINUE ELE02010
C----- ASSEMBLE TRANSFORMATION MATRIX A ELE02020
  DO 50 I=1,3 ELE02030
    DO 50 J=1,3 ELE02040
      A(I,J)=CTS(I,J) ELE02050
      A(I+3,J+3)=CTS(I,J)+BE(I,J) ELE02060
      A(I+6,J+6)=CTE(I,J) ELE02070
  50 CONTINUE ELE02080
  K=0 ELE02090
  DO 87 I=1,12 ELE02100
    DO 87 J=1,12 ELE02110
      K=K+1 ELE02120
      IF (ABS(SE(K)).LT.1.E-6) SE(K)=0 ELE02130
      S(I,J)=SE(K) ELE02140
      S(J,I)=S(I,J) ELE02150
  87 CONTINUE ELE02160
  K=0 ELE02170
  DO 77 I=1,12 ELE02180
    DO 77 J=1,12 ELE02190
      K=K+1 ELE02200
      IF (ABS(SE(K)).LT.1.E-6) SE(K)=0 ELE02210
      S(I,J)=SE(K) ELE02220
      S(J,I)=S(I,J) ELE02230
  77 CONTINUE ELE02240
  IF (FIRST) CALL WMATRX(S,12,12,'S') ELE02250
C----- CALCULATE SAT ELE02260
  DO 60 I=1,12 ELE02270
    DO 60 J=1,12 ELE02280
      SAT(I,J)=0 ELE02290
      DO 60 K=1,12 ELE02300
        SAT(I,J)=SAT(I,J)+S(I,K)*A(J,K) ELE02310
  60 CONTINUE ELE02320
C----- CALCULATE ASAT ELE02330
  DO 70 I=1,12 ELE02340
    DO 70 J=1,12 ELE02350
      ASAT(I,J)=0 ELE02360
      DO 70 K=1,12 ELE02370
        ASAT(I,J)=ASAT(I,J)+A(I,K)*SAT(K,J) ELE02380
  70 CONTINUE ELE02390
  IF (BTEST(1BUG,8)) CALL WMATRX(ASAT,12,12,'ASAT') ELE02400
C----- DEGREES OF FREEDOM ELE02410
  REMOVE DOF'S THAT ARE CONSTRAINED TO THE SAME DOF... ELE02420
  DO 75 I=1,6 ELE02430
    IF (IDOF(JOINTI,1).NE.IDOF(JOINTJ,1)) THEN ELE02440
      LMT(I)=IDOF(JOINTI,1) ELE02450
      LMT(I+6)=IDOF(JOINTJ,1) ELE02460
    ELSE ELE02470
      LMT(I)=0 ELE02480
      LMT(I+6)=0 ELE02490
  75 CONTINUE ELE02500
C----- CONVERT TO HALF STORAGE MODE BY COLUMNS ELE02510
  DO 90 I=1,12 ELE02520
    DO 90 J=1,12 ELE02530
      IF (LMT(I).EQ.0 .OR. ASAT(I,I).LE.0) GO TO 90 ELE02540
      IF (LMT(J).EQ.0 .OR. ASAT(J,J).LE.0) GO TO 90 ELE02550
      IF (LMT(I).EQ.0 .OR. ASAT(I,I).LE.0) GO TO 90 ELE02560
      IF (LMT(J).EQ.0 .OR. ASAT(J,J).LE.0) GO TO 90 ELE02570
      L=1 ELE02580
      STIFF(L)=ASAT(I,J) ELE02590
      CONTINUE ELE02600
      IF (BTEST(1BUG,7)) THEN ELE02610
        WRITE (6,*) 'LMT=',LMT ELE02620
        WRITE (6,*) 'IELDOF',IELDOF, 'LM=',(LMT(I),I=1,IELDOF) ELE02630
        CALL WMATRX(STIFF,MD,IELDOF,(L+1),'ELEMENT STIFFNESS') ELE02640
      ENDF ELE02650
C----- RELEASE UNUSED STORAGE ELE02660
  180 IREL=78-L ELE02670
  PRINT COLUMN DATA ELE02680
  IF (FIRST .OR. BTEST(1BUG,7) .OR. BTEST(1BUG,8)) WRITE (6,192) ELE02690
  191 WRITE(6,193) NAME,IELNG,MAT,NODEI,NODEJ,LENGTH, ELE02700
  & VI(1),VI(2),VI(3),XS,XE ELE02710
C 192 FORMAT(' ELEMENT 12, STABILITY ELEMENT'// ELE02720
  & ' DO 40 J=1,12, START END, JX, LENGTH, 3X, ELE02730
  & ' I1(-)', I-Axis ELE02740
  & ' I2(-)', I-Start DIST END DIST ELE02750
  193 FORMAT(1X,A15,3I6,1X,16,2X,1P,G10,4,0P, ELE02760
  & ' F9.5', ' I',6P,F9.5', ' J',F9.5', ' K',1P,SS,2G12.4) ELE02770
  RETURN ELE02780
C----- DETERMINE GEOMETRIC STIFF ELE02790
  200 CONTINUE ELE02800
  200 GEOMETRIC STIFFNESS IS INCLUDED IN THE ELEMENT STIFFNESS. ELE02810
  C NO NEED TO CALCULATED ELE02820
  RETURN ELE02830
C----- DETERMINE STIFFNESS ELE02840
  300 CONTINUE ELE02850
  RETURN ELE02860
C----- STIFFNESS CHANGES AT EACH STEP FOR STABILITY ELEMENT ELE02870
C ASSEMBLE LOCAL STIFFNESS MATRIX ELE02880
  K=0 ELE02890
  DO 87 I=1,12 ELE02900
    DO 87 J=1,12 ELE02910
      K=K+1 ELE02920
      IF (ABS(SE(K)).LT.1.E-6) SE(K)=0 ELE02930
      S(I,J)=SE(K) ELE02940
      S(J,I)=S(I,J) ELE02950
  87 CONTINUE ELE02960

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87 CONTINUE
IF (FIRST) CALL WMATRX ( S ,12,12, S )
C
C----- CALCULATE SAT
DO 62 I=1,12
DO 62 J=1,12
SAT(I,J)=0
DO 62 K=1,12
SAT(I,J)=SAT(I,J)+S(I,K)*A(J,K)
C
62 CONTINUE
C----- CALCULATE ASAT
DO 72 I=1,12
DO 72 J=1,12
ASAT(I,J)=0
DO 72 K=1,12
ASAT(I,J)=ASAT(I,J)+A(I,K)*SAT(K,J)
IF ( BTEST( IBUG,8 ) ) CALL WMATRX(ASAT ,12,12, ASAT )
C
C----- DEGREES OF FREEDOM
REMOVE DOF'S THAT ARE CONSTRAINED TO THE SAME DOF...
DO 12 I=1,6
IF ( IDOF(JOINTI,1),NE, IDOF(JOINTJ,1) ) THEN
LMT(I)=IDOF(JOINTI,1)
ELSE
LMT(I)=IDOF(JOINTJ,1)
ENDIF
CONTINUE
12 CONTINUE
C
C----- CONVERT TO HALF STORAGE MODE BY COLUMNS
L=0
IELDOF=0
DO 92 I=1,12
IF (LMT(I).EQ.0 .OR. ASAT(I,I).LE.0) GO TO 92
IF (LMT(I).EQ.0) GO TO 92
IELDOF=IELDOF+1
LW( IELDOF )=LMT(I)
DO 92 J=1,12
IF (LMT(J).EQ.0 .OR. ASAT(J,J).LE.0) GO TO 82
IF (LMT(J).EQ.0) GO TO 82
L=L+1
STIFF(I)=ASAT(I,J)
CONTINUE
92 CONTINUE
IF ( BTEST( IBUG,7 ) ) THEN
WRITE (6,*) 'LMT=',LMT
WRITE (6,*) 'IELDOF',IELDOF, 'LW=',(LW(I),I=1,IELDOF)
CALL LMATRX(STIFF,MD,IELDOF,(L-1), 'ELEMENT STIFFNESS')
WRITE (6,*) 'KGDATA',KGDATA, 'PKGI',PKGI
ENDIF
RETURN
C----- DETERMINE INCREMENTAL FORCES -----
400 CONTINUE
IF (PRINT.AND.FIRST) WRITE (6,491) STEPID
C
C----- LOOP FOR EACH LOAD
DO 430 LOAD=1,NLOAD
C----- TRANSFORM FRAME DISPL INTO LOCAL DISPL
DO 410 I=1,3
R(I)=0.
R(I+3)=0.
R(I+6)=0.
R(I+9)=0.
DO 410 J=1,3
D1=DISPL(IDOF(JOINTI,J),LOAD)
D2=DISPL(IDOF(JOINTI,J+3),LOAD)
D3=DISPL(IDOF(JOINTI,J+6),LOAD)
D4=DISPL(IDOF(JOINTI,J+9),LOAD)
R(I)=R(I)+D1*CTSI(J,I)*D1 + D2*CTSI(J,I)*D2
R(I+3)=R(I+3)+D3*CTSI(J,I)*D3
R(I+6)=R(I+6)+D4*CTSI(J,I)*D4
R(I+9)=R(I+9)+D4*CTSI(J,I)*D4
410 CONTINUE
C----- INCREMENTAL FORCES
DO 435 I=1,12
DO 435 J=1,12
F(I)=0
DO 435 K=1,12
F(I)=F(I)+S(I,K)*R(K)
435 CONTINUE
C----- DETERMINE MAXIMUM VALUES FOR MULTIPLE LOAD CASES
IF (NLOAD.GT.1) THEN
DO 425 I=1,12
IF ( ABS(F(I)).GT.ABS(FMAX(I*12-12+I)) ) THEN
DO 426 J=0,11
FMAX(I*12-11+J)=F(I)
ENDIF
CONTINUE
ENDIF
425 CONTINUE
C----- PRINT FORCES AND DEFORMATIONS
IF (BTEST( IBUG,0 ).AND. PRINT)
SP=SE(84)
FLP=SE(98)
IF ( PRINT )
WRITE (6,493) IELNO,LOAD,NODEI,(RT(J),J=1,6),
NODEJ,(RT(J),J=7,12),
FLP
IF ( PRINT ) WRITE (6,492) IELNO,LOAD,NODEI,(F(J),J=1,6),
NODEJ,(F(J),J=7,12),
SP
430 CONTINUE
C----- THIS IS FOR SOLOSA USE ONLY
IF ( ICOMN.EQ.1 .OR. ICOMN.EQ.2 ) THEN
DO 431 I=1,12
431 ASAT(I)=F(I)
ENDIF
C----- CALL MATLIB TO GET STIFFNESS
THIS IS PERFORMED ONLY FOR INELASTIC ANALYSIS
IF (.NOT. ELSTIC) THEN
DO 93 I=1,12
P(I)=FT(I)-F(I)
D(I)=RT(I)+R(I)
PL(I)=FT(I)
DL(I)=RT(I)
93 CONTINUE
C----- TRANSFER PERMANENT HYSTERESIS DATA TO TEMPORARY STORAGE
ISE2=ISE/2
DO 522 I=1,ISE2
SE(I-ISE2)=SE(I)
522 CONTINUE
MOPP=3
CALL MATLIB
( MOPP, LSTTYP, FALSE, IREL, FALSE, NAME, EISE, EPSE, DUCT, EXCR,
P, PL, D, DL, V, VL, LELEM, LMAT,
MAT, MATTP, SE( ISE2+1 ), IELNO, DAMAGE )
NEWK=.TRUE.
ENDIF
C----- CALCULATE THE UNBALANCED MEMBER FORCES ON THE JOINT
ELEM3730 C THE EXACT MEMBER FORCES ARE DETERMINED FROM HYSTERESIS MODEL
ELEM3740 C BASED ON DISPLACEMENT D. BECAUSE NO HYSTERESIS MODEL FOR MAT12.
ELEM3750 C THE MEMBER FUB ARE NOT CALCULATED IN THIS ELEMENT
DO 440 I=1,3
I1=IDOF(JOINTI,1)
I2=IDOF(JOINTI,1+3)
I3=IDOF(JOINTI,1+6)
I4=IDOF(JOINTI,1+9)
DO 440 J=1,3
FUB(I1)=FUB(I1)+CTS(I,J)*D
FUB(I2)=FUB(I2)+BS(I,J)*D
FUB(I3)=FUB(I3)+CTS(I,J)*D
FUB(I4)=FUB(I4)+CTS(I,J)*D
440 CONTINUE
491 FORMAT (// 'STABILITY ELEMENT FORCES... 5X/7X,A/,
' ELEMENT LOAD NODE',7X, 'AXIAL',11X, 'FY',13X, 'FZ',
' 11X, 'TORSION',10X, 'MY',13X, 'M2' FLP, SP )
492 FORMAT (//,110,15,2X, 'FORCE',16,1P,6G15.6,
' /15X, 2X, 'FORCE',16, 6G15.6,1X,G10.3)
493 FORMAT (//,110,15,2X, 'DISPL',16,1P,6G15.6,
' /15X, 2X, 'DISPL',16, 6G15.6,1X,G10.3)
494 FORMAT (7X,A)
RETURN
C----- DETERMINE TOTAL FORCES -----
500 CONTINUE
IF (PRINT.AND.FIRST) WRITE (6,491) STEPID
IF ( ICOMN.NE.1 .AND. ICOMN.NE.2 ) THEN
C----- TOTAL FORCES AND DISPLACEMENTS
DO 510 I=1,12
IF (BTEST( IBUG,7 )) WRITE (6,520) I,R(I),RT(I),F(I),FT(I)
FT(I)=FT(I)+F(I)
RT(I)=RT(I)+R(I)
510 CONTINUE
DO 512 I=1,12
IF ( ABS(FT(I)).GT.ABS(FMAX(I*12-12+I)) ) THEN
511 IF (.NOT. ELSTIC) THEN
FMAX(I*12-11+J)=FT(I)
ENDIF
512 CONTINUE
520 FORMAT (// 'I',13, ' R',1P,G12.4, ' RT',G12.4,
' F',1P,G12.4, ' FT',G12.4)
ELSE IF ( ICOMN.EQ.1 .OR. ICOMN.EQ.2 ) THEN
513 FT(I)=RSAT(I)
ENDIF
LOAD=1
ISE2=ISE/2
SP=SE( ISE2+84 )
FLP=SE( ISE2+98 )
IF (BTEST( IBUG,0 ).AND. PRINT)
IF ( PRINT )
WRITE (6,493) IELNO,LOAD,NODEI,(RT(J),J=1,6),
NODEJ,(RT(J),J=7,12),
FLP
IF ( PRINT ) WRITE (6,492) IELNO,LOAD,NODEI,(F(J),J=1,6),
NODEJ,(F(J),J=7,12),
SP
C----- TRANSFER TEMPORARY HYSTERESIS DATA TO PERMANENT STORAGE
ISE2=ISE/2
IF (.NOT. ELSTIC) THEN
DO 415 I=1,ISE2
SE(I)=SE(I+ISE2)
ENDIF
RETURN
C----- DETERMINE FIXED END FORCES, AND ADD TO FORCE-----
600 CONTINUE
IF (BTEST( IBUG,0 ).AND. PRINT)
C----- FIXED END FORCES ARE NOT AVAILABLE FOR STABILITY ELEMENT
RETURN
C----- WRITE MEMBER FORCES TO OUTPUT FILE -----
700 CONTINUE
SP=SE(84)
FLP=SE(98)
WRITE ( IUNIT,710 ) NODEI,(FT(J),J=1,6),NODEJ,(FT(J),J=7,12),SP
WRITE ( IUNIT,710 ) NODEI,(RT(J),J=1,6),NODEJ,(RT(J),J=7,12),FLP
710 FORMAT (16,1P,6G12.5/,
' 16,1P,6G12.5,1X,G10.3)
RETURN
C----- READ AND PRINT MEMBER FORCES FROM OUTPUT FILE ----
800 CONTINUE
BACKSPACE( IUNIT )
READ ( IUNIT,* ) ITYPE,IELNO,IWRITE,TO,DT
WRITE (6,805)
ISTEP=IWRITE
810 READ ( IUNIT,815,END=830 )
NODEI,(FT(J),J=1,6),NODEJ,(FT(J),J=7,12),SP
READ ( IUNIT,815,END=830 )
NODEI,(RT(J),J=1,6),NODEJ,(RT(J),J=7,12),FLP
815 FORMAT (16,6G12.5/,
' 16,6G12.5,1X,G10.3)
ISTEP=ISTEP+IWRITE
T=TO-DT*ISTEP
MOD=MOD+ISTEP*INC
IF (MOD.GT.ISTEP*INC.EQ.0) THEN
WRITE (6,820) ISTEP,T,NODEI,(FT(J),J=1,6),
NODEJ,(FT(J),J=7,12),SP
820 CONTINUE
ENDIF
GO TO 810
805 FORMAT (// 'STABILITY ELEMENT FORCES...//
' STEP TIME NODE',7X, 'AXIAL',11X, 'FY',13X, 'FZ',
' 11X, 'TORSION',10X, 'MY',13X, 'M2' FLP, SP )
820 FORMAT (//,110,1P,G15.5,OP,15,1P,6G15.6,
' /15X, 2X, 'FORCE',16, 6G15.6,1X,G10.3)
920 FORMAT (//,110,15,2X, 'DISP',16, 6G15.6,1X,G10.3)
' /13X, 2X, 'DISP',16, 6G15.6,1X,G10.3)
830 RETURN
C----- DETERMINE THE LENGTH OF THE MATERIAL DATA
900 CONTINUE
ISE=0
MAT = RINPUT(1)
CALL MATLIB
( I, LSTTYP, FALSE, IREL, FALSE, NAME, EISE, EPSE, DUCT, EXCR,
P, PL, D, DL, V, VL, LELEM, LMAT,
MAT, MATTP, SE( IELNO, DAMAGE ) )
ISE=ISE+LELEM
C----- DOUBLE THE STORAGE, TO ALLOW FOR TEMPORARY VALUES
ISE=ISE*2
RETURN
C----- DETERMINE THE TOTAL STRAIN ENERGY
1000 CONTINUE
EISE=0
EPSE=0
ELEM5150 C
ELEM5160 C
ELEM5170 C
ELEM5180 C
ELEM5190 C
ELEM5200 C
ELEM5210 C
ELEM5220 C
ELEM5230 C
ELEM5240 C
ELEM5250 C
ELEM5260 C
ELEM5270 C
ELEM5280 C
ELEM5290 C
ELEM5300 C
ELEM5310 C
ELEM5320 C
ELEM5330 C
ELEM5340 C
ELEM5350 C
ELEM5360 C
ELEM5370 C
ELEM5380 C
ELEM5390 C
ELEM5400 C
ELEM5410 C
ELEM5420 C
ELEM5430 C
ELEM5440 C
ELEM5450 C
ELEM5460 C
ELEM5470 C
ELEM5480 C
ELEM5490 C
ELEM5500 C
ELEM5510 C
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ELEM5600 C
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ELEM5630 C
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ELEM5650 C
ELEM5660 C
ELEM5670 C
ELEM5680 C
ELEM5690 C
ELEM5700 C
ELEM5710 C
ELEM5720 C
ELEM5730 C
ELEM5740 C
ELEM5750 C
ELEM5760 C
ELEM5770 C
ELEM5780 C
ELEM5790 C
ELEM5800 C
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ELEM5820 C
ELEM5830 C
ELEM5840 C
ELEM5850 C
ELEM5860 C
ELEM5870 C
ELEM5880 C
ELEM5890 C
ELEM5900 C
ELEM5910 C
ELEM5920 C
ELEM5930 C
ELEM5940 C
ELEM5950 C
ELEM5960 C
ELEM5970 C
ELEM5980 C
ELEM5990 C
ELEM6000 C
ELEM6010 C
ELEM6020 C
ELEM6030 C
ELEM6040 C
ELEM6050 C
ELEM6060 C
ELEM6070 C
ELEM6080 C
ELEM6090 C
ELEM6100 C
ELEM6110 C
ELEM6120 C
ELEM6130 C
ELEM6140 C
ELEM6150 C
ELEM6160 C
ELEM6170 C
ELEM6180 C
ELEM6190 C
ELEM6200 C
ELEM6210 C
ELEM6220 C
ELEM6230 C
ELEM6240 C
ELEM6250 C
ELEM6260 C
ELEM6270 C
ELEM6280 C
ELEM6290 C
ELEM6300 C
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ELEM6400 C
ELEM6410 C
ELEM6420 C
ELEM6430 C
ELEM6440 C
ELEM6450 C
ELEM6460 C
ELEM6470 C
ELEM6480 C
ELEM6490 C
ELEM6500 C
ELEM6510 C
ELEM6520 C
ELEM6530 C
ELEM6540 C
ELEM6550 C
ELEM6560 C

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C /* ENERGY FORMULATION FOR ELEM2 ELEMENT IS NOT AVAILABLE */
C IF (ELSTIC) THEN
C DO 1010 I=1,12
C     BESS=BESS + (FT(I)+F(I))*R(I)-RT(I))/2.
C     ENDOAT(1) = BESS
C     ENDOAT(2) = 0
C     RETURN
C ----- DUCTILITIES AND EXCURSION RATIOS
C NOT AVAILABLE CURRENTLY
C 1100 RETURN
C ----- PRINT MAXIMUM ELEMENT LOADS
C 1200 CONTINUE
C LOAD=0
C IF (FIRST) WRITE(6,494) ' COLUMN ( LOAD ) REPRESENTS THE DOF WHICH '
C     ' // HAS MAXIMUM VALUE '
C IF (FIRST) WRITE(6,491) ' MAXIMUM VALUES FOR ALL STEPS '
C     ' // NOTE! MAXIMUM VALUES WITH THE OTHER DOFS FORCES '
C     ' // ARE PRINT OUT AT THE SAME TIME '
C DO 309 I=0,11
C     LOAD2=I+1
C     IF (FMAX(I+12)+I) .EQ. 0.) GO TO 309
C     WRITE(6,492) IELNO,LOAD2,NODEI,(FMAX(I+12+J),J=1,6),
C     ' NODEJ,(FMAX(I+12+J),J=7,12)
C 309 CONTINUE
C RETURN
C ----- RESET ELEMENT FORCES
C 1300 CONTINUE
C ----- ZERO MATRICES
C DO 1340 I=1,12
C     FMAX(I+12-11)=0
C     F(I)=0
C     FT(I)=0
C     R(I)=0
C     RT(I)=0
C 1340 DO 1310 I=1,ISE
C 1310 SE(I)=0
C     SE(79)=LENGTH
C WRITE(6,1330) IELNO
C 1330 FORMAT ' STABILITY ELEMENT ..... ELEMENT #',I6,
C ' * INTERNAL FORCES AND HYSTERESIS MODELS ARE RESET TO ZERO * )
C     FALSE=.FALSE.
C     LSTTYP=0
C     CALL MATS1
C     ' ( LSTTYP, FALSE, IREL, FNAME, BESS, EFUS, DUCT, EXCR,
C     ' FT, F, RT, R, V, VL, LBELEM, LMAT,
C     ' MAT, MATTYP, SE, IELNO, DAMAGE)
C RETURN
C ----- DAMAGE INDEX
C 1400 DAMAGE = 0.
C RETURN
C END
@PROCESS OPT(0) XREF MAP GOSTMT
C -----
C DEBUG UNIT(8), TRACE, SUBCHR, INIT, SUBTRACE
C END DEBUG
C -----
C SUBROUTINE ELEB14
C ( IOPT, IUNIT, NEWK, FIRST, PRINT,
C NDOF, NLOAD, MAXNOD, NNODE, NCOS,
C ID, IDOP, COSINE, JTCS,
C CONST, DISPL, VELOC, FORCE, FUB,
C IREL, BUG, IBUG, ACCEL, KGDATA,
C GRAV, PGEOM, ELSTIC, NAME, STEPID,
C IELNO, RINPUT, AXIAL, SE, PSE,
C INC, DAMAGE, DUCFANG,
C IELTYP, ISELDOP, ISESS, ISEAL, ISEAR,
C ISE, J1, J2, J3, J4,
C MATS, MATA1, MATA2, BLANK, R,
C RT, P, PT, V, VT,
C PKG, LKG, LAM, LMKG, A,
C SE, SA1, SA2, TEMP1, TEMP2,
C KGDOP, LENGTH, FMAX, SE, STIFF )
C COMMON /RSV/ICOMN,RSAP(12)
C REAL ICOMN
C REAL LWP,MQD
C -----
C DOUBLE PRECISION SX,SY,ST,STX,SN
C LOGICAL ERR,FIRST,PRINT,BUG,FALSE,NEWK,ELSTIC,AXIAL,BTEST
C CHARACTER*80 NAME
C CHARACTER*(*) STEPID
C DOUBLE PRECISION SAT,DTSTFF(600)
C DIMENSION IDOF(MAXNOD,6),COORD(MAXNOD,6),COSINE(3,3),NCOS,
C DIMENSION CONST(MAXNOD,6), ID(MAXNOD), JTCS(MAXNOD),
C DIMENSION SAT(3,24),STIFF(600),KGDATA(3),SAT2(6,24)
C DIMENSION AT(24,6),A(24,3),S(6,6),CM(24),AC(6,3),LMKG(24),
C DIMENSION CT(3,3),BE(3,3),VX(3),VT(3),VX2(3),VT2(3),VZ(3),
C DIMENSION SE(ISE),A11(6,3),A22(6,3),A44(6,3),A33(6,3),
C DIMENSION F(3),V(3),PT(3),RT(3),VT(3),P(3),AD(10),AV(50)
C DIMENSION DISPL(NDOF,NLOAD),FUB(NDOF,EFUB(24),FMAX(6))
C DIMENSION VELOC(NDOF)
C DIMENSION FORCE(NDOF,NLOAD)
C DIMENSION RINPUT(100),WORK(1),DUCT(3,3),EXCR(6,3)
C REAL LENGTH
C DIMENSION MD(25)
C DATA MD/ 1, 2, 4, 7, 11, 16, 22, 29, 37, 46, 56, 67,
C 79, 92, 106, 121, 137, 154, 172, 191, 211, 232, 254, 277, 301/
C -----
C VARIABLES:
C -----
C GLOBAL VARIABLES
C IOPT = 1, INITIALIZE BEAM COLUMN ELEMENT
C = 2, CALCULATE GEOMETRIC STIFFNESS
C = 3, FORM STIFFNESS
C = 4, CALCULATE INCREMENTAL FORCES
C = 5, CALCULATE TOTAL FORCES AND ENERGIES
C IUNIT = OUTPUT FILE UNIT # FOR PRINTING OUTPUT
C ERR = ERR OR FLAG, IF AN ERR OR HAS OCCURED, ERR=.TRUE.
C FIRST = FLAG, FIRST=.TRUE., PRINT HEADERS
C PRINT = FLAG, PRINT=.TRUE., PRINT DATA
C NDOF = # OF GLOBAL DOF
C NLOAD = # OF LOAD COMBINATIONS
C MAXNOD = # ROW DIMENSION OF NODE ARRAY
C NNODE = # OF NODES
C ID = ARRAY OF EXTERNAL NODE NUMBERS
C IDOF = ARRAY OF DEGREES OF FREEDOM
C COORD = ARRAY OF COORDINATES
C COSINE = ARRAY OF DIRECTION COSINES OF NODES
C CONST = ARRAY OF CONSTRAINT TRANSFORMATIONS
C DISPL = GLOBAL DISPLACEMENT MATRIX
C Z = REAL GLOBAL STORAG VECTOR
C NZ = INTEGER GLOBAL STORAG VECTOR
C IZMAT = LOCATION OF MATERIAL DATA IN GLOBAL STORAGE VECTOR
C ELEMN = ELEMENT NAME
C IELNO = ELEMENT NUMBER

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ELEM0570
ELEM0580
ELEM0590
ELEM0600
ELEM0610
ELEM0620
ELEM0630
ELEM0640
ELEM0650
ELEM0660
ELEM0670
ELEM0680
ELEM0690
ELEM0700
ELEM0710
ELEM0720
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ELEM0970
ELEM0980
ELEM0990
ELEM1000
ELEM1010
ELEM1020
ELEM1030
ELEM1040
ELEM1050
ELEM1060
ELEM1070
ELEM1080
ELEM1090
ELEM1100
ELEM1110
ELEM1120
ELEM1130
ELEM1140
ELEM1150
ELEM1160
ELEM1170
ELEM1180
ELEM1190
ELEM1200
ELEM1210
ELEM1220
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ELEM1600
ELEM1610
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ELEM1800
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ELEM1820
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ELEM1880
ELEM1890
ELEM1900
ELEM1910
ELEM1920
ELEM1930
ELEM1940
ELEM1950
ELEM1960
ELEM1970
ELEM1980
ELEM1990
ELEM2000
ELEM2010
ELEM2020
ELEM2030
ELEM2040
ELEM2050
ELEM2060
ELEM2070
ELEM2080
ELEM2090
ELEM2100
ELEM2110
ELEM2120
ELEM2130
ELEM2140
ELEM2150
ELEM2160
C STIFF = INPUT DATA
C RINPUT = OUTPUT ELEMENT STIFFNESS (UPPER TRIANGULAR FORM)
C LM = OUTPUT LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C ----- ELEMENT VARIABLES
C H,C,Q,AI,AJ,C,BI,BJ,S22,S26,S212,S33,S35,S311,D
C = INDIVIDUAL TERMS IN THE ELEMENT STIFFNESS (S) MATRIX
C PKG = INPUT LOAD FOR FORMING GEOMETRIC STIFFNESS MATRIX...
C = INCREMENTAL DISPLACEMENTS
C RT = TOTAL DISPLACEMENTS
C F = INCREMENTAL FORCES
C FT = TOTAL FORCES
C VT = VECTOR DEFINING THE ELEMENT X AXIS
C CTS = LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C CTE = LOCAL TO GLOBAL ROTATION MATRIX START
C BS = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX START
C BE = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX END
C STIFF = ELEMENT STIFFNESS
C IELTYP = ELEMENT TYPE = 10
C NODEI = EXTERNAL JOINT NUMBER, END I
C NODEJ = EXTERNAL JOINT NUMBER, END J
C JOINTI = INTERNAL JOINT NUMBER, END I
C JOINTJ = INTERNAL JOINT NUMBER, END J
C ----- TEMPORARY VARIABLES
C VX = VECTOR DEFINING THE ELEMENT X AXIS
C VY = VECTOR DEFINING THE ELEMENT Y AXIS
C COSLOC = LOCAL COSINE MATRIX
C A = GLOBAL TO LOCAL FORCE TRANSFORMATION MATRIX
C S = ELEMENT STIFFNESS AT LOCAL COORD, AT ENDS OF FLEXIBLE PART
C SAT = PRODUCT OF A,SAT
C ASAT = PRODUCT OF A,SAT
C -----
C ----- CHOOSE OPTION
C IF (IOPT.LT.1 .OR. IOPT.GT.14)
C WRITE(6,*) 'INVALID OPTION IN ELE-14, IOPT=',IOPT
C GO TO (100,200,300,400,500,600,700,800,900,1000,1100,1200,
C 1300,1400),IOPT
C 100 CONTINUE
C ----- INTERPATE INPUT DATA
C IELTYP = 14
C MATS = RINPUT( 1)
C MATA = RINPUT( 2)
C MATB = RINPUT( 3)
C NODEI = RINPUT( 4)
C NODEJ = RINPUT( 5)
C NODE3 = RINPUT( 6)
C NODE4 = RINPUT( 7)
C ALP1 = RINPUT( 8)
C PKG = RINPUT( 9)
C ----- GET INTERNAL NODE # ASSOCIATED WITH EXTERNAL NODE #'S
C J1=IQUICK(NODEI,ID,NNODE)
C J2=IQUICK(NODEJ,ID,NNODE)
C J3=IQUICK(NODE3,ID,NNODE)
C J4=IQUICK(NODE4,ID,NNODE)
C -----
C GET GLOBAL TO LOCAL TRANSFORMATION MATRIX
C ----- DEFINE VECTOR X, AT BOTTOM OF THE WALL
C VXB(1)=COORD(J4,1)-COORD(J3,1)
C VXB(2)=COORD(J4,2)-COORD(J3,2)
C VXB(3)=COORD(J4,3)-COORD(J3,3)
C ----- DEFINE VECTOR X, AT TOP OF THE WALL
C VXT(1)=COORD(J1,1)-COORD(J2,1)
C VXT(2)=COORD(J1,2)-COORD(J2,2)
C VXT(3)=COORD(J1,3)-COORD(J2,3)
C ----- CALCULATE Y VECTOR ...
C VT(1)=(COORD(J1,1)+COORD(J2,1)-COORD(J3,1)-COORD(J4,1))/2.
C VT(2)=(COORD(J1,2)+COORD(J2,2)-COORD(J3,2)-COORD(J4,2))/2.
C VT(3)=(COORD(J1,3)+COORD(J2,3)-COORD(J3,3)-COORD(J4,3))/2.
C ----- CALCULATE AVERAGE LENGTH, ALPHA AND BETA...
C LENGTH=SQRT(VT(1)**2+VT(2)**2+VT(3)**2)
C SE(1:SE-7)=LENGTH
C IF (LENGTH.LE.0) THEN
C WRITE(6,101) IELNO,LENGTH
C 101 FORMAT(1X,20,'**ERR**',ROM = LENGTH IS LE 0.0 /
C 5X,' IELNO=',I5,5X,' LENGTH=',1P,515.6/
C 5X,' REVERSE INPUT.....')
C LENGTH=1.0
C ENDIF
C ALPHA=ALP1*LENGTH
C BETA = LENGTH-ALPHA
C ----- NORMALIZE Y VECTOR ...
C VT(1)=VT(1)/LENGTH
C VT(2)=VT(2)/LENGTH
C VT(3)=VT(3)/LENGTH
C ----- CALCULATE THE WIDTH ...
C CALL VCROSS(VZ,VT,VT)
C CALL VCROSS(VZ,VXB,VT)
C WT = SQRT(VT(1)**2 + VZ(2)**2 + VZ(3)**2)
C WB = SQRT(VXB(1)**2 + VXB(2)**2 + VXB(3)**2)
C W = (WT-WB)/2.0
C ----- NORMALIZE Z VECTOR ...
C VZB(1)=VZB(1)/W
C VZB(2)=VZB(2)/W
C VZB(3)=VZB(3)/W
C SE(1:SE-7)=W
C ----- CHECK FOR TWIST
C TWIST=ACOS(VCOS(VZT,VZB))*57.2958
C IF (ABS(TWIST).GT.5.0) WRITE(6,102) TWIST
C 102 FORMAT(5X,10,'**',WARNING THE WALL IS TWISTED',1P,612.4,
C 6 ' DEGREES, REVERSE INPUT AND RERUN')
C ----- CALCULATE VX, AND NORMALIZE
C CALL VCROSS(VX,VT,VZB)
C WX=SQRT(VX(1)**2+VX(2)**2+VX(3)**2)
C VX(1)=VX(1)/WX
C VX(2)=VX(2)/WX
C VX(3)=VX(3)/WX
C ----- CHECK FOR TWIST
C IF ( BTEST(1:BUG,7) ) THEN
C WRITE(6,*) 'JOINTS',J1,J2,J3,J4
C WRITE(6,*) ' VX, VY '
C WRITE(6,*) ' VT, VZ '
C WRITE(6,*) ' VZ, VZB '
C WRITE(6,*) ' LENGTH, ALPHA, BETA',LENGTH,ALPHA,BETA
C ENDIF
C ----- GET LOCAL TO GLOBAL TRANSFORMATION MATRICES
C ----- JOINT #1
C CALL ROTXZ(VX,VT,COSINE(1,1),JTCS(J1),CT,
C DO 21 I=1,3
C DO 21 J=1,3
C A1(I, J)=CT(I,J)
C 21 A1(I+3,J)=BS(I,J)
C ----- JOINT #2
C CALL ROTXZ(VX,VT,COSINE(1,1),JTCS(J2),CT,

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4      DO 22 I=1,3      0.,0.,0.,BS, CONST(J2,1),MAXNOD)
      DO 22 J=1,3
      A22(I,J)=CT(I,J)
C--- JOINT #3
      CALL ROTXYZ(VX,VT,COSINE(1.1,JCOS(J3)),CT,
      0.,0.,0.,BS, CONST(J3,1),MAXNOD)
      DO 23 I=1,3
      DO 23 J=1,3
      A33(I,J)=CT(I,J)
      A33(I+3,J)=BS(I,J)
C--- JOINT #4
      CALL ROTXYZ(VX,VT,COSINE(1.1,JCOS(J4)),CT,
      0.,0.,0.,BS, CONST(J4,1),MAXNOD)
      DO 24 I=1,3
      DO 24 J=1,3
      A44(I,J)=CT(I,J)
      A44(I+3,J)=BS(I,J)
C
      IF ( BTEST(1BUG,7) ) THEN
      WRITE (6,*) 'JOINT #1, J1',J1
      CALL WMATRX(A11, 6, 3, 'A11 ')
      WRITE (6,*) 'JOINT #2, J2',J2
      CALL WMATRX(A22, 6, 3, 'A22 ')
      WRITE (6,*) 'JOINT #3, J3',J3
      CALL WMATRX(A33, 6, 3, 'A33 ')
      WRITE (6,*) 'JOINT #4, J4',J4
      CALL WMATRX(A44, 6, 3, 'A44 ')
      ENDIF
C----- ZERO MATRICIES
      DO 25 I=1,3
      FMAX(I) = 0
      FMAX(I-3) = 0
      F(I)=0
      FT(I)=0
      R(I)=0
      RT(I)=0
      V(I)=0
      WT(I)=0
      DO 27 I=1,24
      LM(I)=0
      LMG(I)=0
      DO 26 I=1,3
      DO 27 K=1,6
      AT(I,K)=0.00
      DO 27 I=1,6
      AT(I,K)=0.00
C----- ASSEMBLE TRANSFORMATION MATRIX A = A5 X A4 X A3
NOTE: AT IS DIMENSIONED AS A 24X6 MATRIX, BUT ONLY THE FIRST IELODF
      ROW'S CONTAIN INFORMATION. THE BALANCE OF THE MATRIX IS
      UNDEFINED. THE GLOBAL DOF COORDS CORRESPONDING TO THE ROW'S OF A
      ARE STORED IN VECTOR LM. AGAIN, ONLY THE FIRST IELODF ROW'S
      OF LM ARE DEFINED.
      IELODF=0
      DO 50 I=1,4
      DO 50 J=1,6
      REMOVE DOF'S THAT ARE CONSTRAINED TO THE SAME DOF...
      IF ( (IDOF(J1,J),EQ,IDOF(J4,J)) .OR.
      (IDOF(J1,J),EQ,IDOF(J3,J)) .OR.
      (IDOF(J2,J),EQ,IDOF(J4,J)) .OR.
      (IDOF(J2,J),EQ,IDOF(J3,J)) ) GO TO 50
      IELODF=IELODF+1
      IF (I.EQ.1) THEN
      LMTI=IDOF(J1,J)
      AT(IELODF,1)=A11(J,1)
      AT(IELODF,2)=A11(J,2)
      ELSE IF (I.EQ.2) THEN
      LMTI=IDOF(J2,J)
      AT(IELODF,3)=A22(J,2)
      ELSE IF (I.EQ.3) THEN
      LMTI=IDOF(J3,J)
      AT(IELODF,4)=A33(J,2)
      ELSE IF (I.EQ.4) THEN
      LMTI=IDOF(J4,J)
      AT(IELODF,5)=A44(J,1)
      AT(IELODF,6)=A44(J,2)
      ENDIF
      LM(IELODF)=LMTI
      IF ((AT(IELODF,1),EQ,0).AND.(AT(IELODF,2),EQ,0).AND.
      (AT(IELODF,3),EQ,0).AND.(AT(IELODF,4),EQ,0).AND.
      (AT(IELODF,5),EQ,0).AND.(AT(IELODF,6),EQ,0)) THEN
      IELODF=IELODF-1
      ELSE IF (IELODF.GT.1) THEN
      DO 49 I=1,IELODF-1
      IF (LM(I),EQ,LM(IELODF)) THEN
      DO 48 JJ=1,6
      AT(II,JJ)=AT(II,JJ)+AT(IELODF,JJ)
      AT(IELODF,JJ)=0
      IELODF=IELODF-1
      GO TO 50
      ENDIF
      CONTINUE
      ENDIF
      CONTINUE
C
      DO 50 CONTINUE
C----- A21 FOR STIFFNESS ONLY...
      AC(1,1)= 1.
      AC(2,1)= ALPHA/W
      AC(3,1)= -ALPHA/W
      AC(4,1)= -BETA /W
      AC(5,1)= -1.
      AC(6,1)= BETA /W
      AC(1,2)= 0.
      AC(2,2)= 1.
      AC(3,2)= 0.
      AC(4,2)= 0.
      AC(5,2)= 0.
      AC(6,2)= 0.
      AC(1,3)= 0.
      AC(2,3)= 1.
      AC(3,3)= 0.
      AC(4,3)= 0.
      AC(5,3)= 0.
      AC(6,3)= 0.
C
      DO 55 I=1,IELODF
      DO 55 J=1,3
      A(I,J)=0.
      DO 55 K=1,6
      A(I,J)=A(I,J)+AT(I,K)*AC(K,J)
C
      IF ( BTEST(1BUG,7) ) THEN
      CALL WMATRX( A , IELODF,6 , ' A ',24)
      CALL WMATRX( AC , 6,3 , ' AC ', )
      CALL WMATRX( A , IELODF,3 , ' A ',24)
      ENDIF
C
C----- GET SPRING STIFFNESS
      FALSE=.FALSE.
      LSTIIP=0
      CALL MATLIB
      ( 2 ) GETTYP ,FALSE ,IREL,FALSE,NAME,
      ( 2 ) ESSE,PSE,DUCT(1.1),EXCR(1.1),
      ( 1 ) F ,RT ,R ,0.,0.,0.,LELEM ,LMAT ,
      ( 1 ) MATS ,MATTP,SE(1SES),IELNO,DAMAGE)
      CALL MATLIB
      ( 2 ) GETTYP ,FALSE ,IREL,FALSE,NAME,
      ( 2 ) ESSE2,PSE2,DUCT(1.3),EXCR(1.3),
      ( 1 ) F ,RT ,R ,0.,0.,0.,LELEM ,LMAT ,
      ( 1 ) MAT2 ,MATTP,SE(1SE2),IELNO,DAMAGE)
      C TO CHECK KKL,CYCLE,C01,C02,C05
      C
      BSE=0.
      PSE=0.
C
      C----- SET UP THE LOCAL STIFFNESS MATRIX
      SS=SE(1SES)
      SA1=SE(1SE1)
      SA2=SE(1SE2)
      IF ( BTEST(1BUG,7) ) THEN
      WRITE (6,*) 'SHEAR STIFFNESS KS=',SS
      WRITE (6,*) 'AXIAL STIFFNESS KA=',SA1
      ENDIF
C----- CALCULATE SAT
      SAL=SA1/(LENGTH+2)
      SAL2=SA2/(LENGTH+2)
      SSL=SS
      DO 60 J=1,IELODF
      SAT(1,J)=SSL*A(J,1)
      SAT(2,J)=SAL*A(J,2)
      DO 60 J=1,6
      SAT(3,J)=SAL2*A(J,3)
C----- CALCULATE ASAT AND CONVERT TO HALF STORAGE MODE BY COLUMNS
      DO 1 3 6 10 15 21 28 36 45 55 66 78 300. NUMBER INDICATES
      2 5 9 14 20 27 35 44 54 65 77 . LOCATION OF DATA
      4 8 13 19 26 34 43 53 64 76 . IN ARRAYS STIFF
      7 12 18 25 33 42 52 63 75 .
      11 17 24 32 41 51 62 74 .
      16 23 31 40 50 61 73 .
      22 30 39 49 60 72 .
      29 38 48 59 71 .
      37 47 58 70 .
      46 57 69 .
      56 68 .
      67 .
      L=0
      DO 70 J=1,IELODF
      DO 70 I=J,1,-1
      L=L+1
      DSTIFF(L)=0
      DO 70 K=1,3
      70 DSTIFF(L)=DSTIFF(L)+A(I,K)*SAT(K,J)
      DO 71 J=1,L
      71 STIFF(J)=DSTIFF(J)
C
      IF ( BTEST(1BUG,7) ) THEN
      WRITE (6,*) 'IELODF',IELODF,'LM=',LM(1),I=1,IELODF)
      CALL LMATRX(STIFF,MD,IELODF,(L+1),'WALL STIFFNESS')
      ENDIF
C----- ASSEMBLE GEOMETRIC STIFFNESS MATRIX FOR A UNIT LOAD...
      LMG=L-1
      IF (KGDATA(1),EQ,0).AND.(KGDATA(2),EQ,0) GO TO 180
      IF (KGDATA(1),EQ,0).OR.(KGDATA(2),EQ,0) GO TO 180
      AXIAL=.TRUE.
C----- DETERMINE THE AXIAL LOAD TO BE USED FOR KG...
      IF (KGDATA(1),EQ,1) THEN
      PGBDM = PKG
      ELSE IF (KGDATA(1),EQ,2) THEN
      PGBDM = -P(2)
      ENDIF
C----- ASSEMBLE TRANSFORMATION MATRIX A
NOTE: AT IS DIMENSIONED AS A 12X6 MATRIX, BUT ONLY THE FIRST IELODF
      ROW'S CONTAIN INFORMATION. THE BALANCE OF THE MATRIX IS
      UNDEFINED. THE GLOBAL DOF COORDS CORRESPONDING TO THE ROW'S OF A
      ARE STORED IN VECTOR LM. AGAIN, ONLY THE FIRST IELODF ROW'S
      OF LM ARE DEFINED.
      DO 140 I=1,24
      DO 140 K=1,6
      AT(I,K)=0.00
      KGDOF=0
      DO 145 I=1,4
      DO 145 J=1,6
      KGDOF = KGDOF +1
      IF (I.EQ.1) THEN
      LMTI=IDOF(J1,J)
      AT(KGDOF,1)=A11(J,1)
      AT(KGDOF,2)=A11(J,3)
      ELSE IF (I.EQ.2) THEN
      LMTI=IDOF(J2,J)
      AT(KGDOF,3)=A22(J,3)
      ELSE IF (I.EQ.3) THEN
      LMTI=IDOF(J3,J)
      AT(KGDOF,4)=A33(J,3)
      ELSE IF (I.EQ.4) THEN
      LMTI=IDOF(J4,J)
      AT(KGDOF,5)=A44(J,1)
      AT(KGDOF,6)=A44(J,3)
      ENDIF
      LMG(KGDOF)=LMTI
      IF ((AT(KGDOF,1),EQ,0).AND.(AT(KGDOF,2),EQ,0).AND.
      (AT(KGDOF,3),EQ,0).AND.(AT(KGDOF,4),EQ,0).AND.
      (AT(KGDOF,5),EQ,0).AND.(AT(KGDOF,6),EQ,0)) THEN
      KGDOF=KGDOF-1
      ELSE IF (KGDOF.GT.1) THEN
      DO 149 II=1,KGDOF-1
      IF (LMG(II),EQ,LMG(KGDOF)) THEN
      DO 148 JJ=1,6
      AT(II,JJ)=AT(II,JJ)+AT(KGDOF,JJ)
      AT(KGDOF,JJ)=0
      KGDOF=KGDOF-1
      GO TO 145
      ENDIF
      CONTINUE
      ENDIF
      CONTINUE
C----- LOCAL GEOMETRIC STIFFNESS MATRIX
      DO 150 I=1,12
      DO 150 J=1,12
      S(I2,J1)=0.0
      S(1,1)= 1./LENGTH
      S(2,2)= 0.5/LENGTH
      S(3,3)= 0.5/LENGTH
      S(4,4)= 0.5/LENGTH
      S(5,5)= 1./LENGTH
      S(6,6)= 0.5/LENGTH
      S(1,5)= -1./LENGTH
      S(2,6)= -0.5/LENGTH
      S(3,4)= -0.5/LENGTH

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S(5,1) = -1.0/LENGTH
S(6,2) = -0.5/LENGTH
S(4,3) = -0.5/LENGTH
C----- CALCULATE SAT
DO 160 J=1, IELDOF
  SAT2(1,J) = 0
  DO 160 K=1, 6
    SAT2(1,J) = SAT2(1,J) + S(K,1)*AT(I,K)*AT(J,K)
C----- CALCULATE ASAT AND CONVERT TO HALF STORAGE MODE BY COLUMNS
DO 170 J=1, IELDOF
  DO 170 I=J, 1, -1
    L=L-1
    STIFF(L)=0
    DO 170 K=1, 6
      STIFF(L) = STIFF(L) + AT(I,K)*SAT2(K,J)
C----- PRINT GEOMETRIC STIFF DATA FOR DEBUGGING
IF ( BTEST(1BUG,7) ) THEN
  CALL WMTRX2( AT , KGDOF, 6, ' AT ' , 24)
  CALL WMTRX( S , 6, 6, ' KG-S ' )
  CALL WMTRX( SAT2 , 6, KGDOF, ' SAT2 ' )
  WRITE ( 6, * ) ' KGDOF , KGDOF , LMRG ' , ( LMRG(I), I=1, KGDOF )
  CALL WMTRX( STIFF(LMG), MD, KGDOF, 300, ' WALL GEOMETRIC STIFF. ' )
  ENDDIF
C----- RELEASE UNUSED STORAGE
190 IREL=600-L
C----- PRINT COLUMN DATA
IF ( FIRST .OR. BTEST(1BUG,7) .OR. BTEST(1BUG,8) )
  4 WRITE ( 6, 192 ) SE(ISES+47)
  191 WRITE(6, 193) NAME, IELNO, MATS, MAT1, MAT2,
    & ID(J1), ID(J2), ID(J3), ID(J4),
    & LENGTH, W, ALPIN, PKG
  192 FORMAT(// ELEMENT 14, R/C SHEAR WALL ELEMENT(//
    & ' SHEAR SPAN LENGTH RATIO= ', G12.4, ' )// T17,
    & ' ELEM ' , SHEAR , ' AXIAL ' , AXIAL2 , ' JOINT ' , JOINT ,
    & ' JOINT ' , JOINT , ' LENGTH ' , WIDTH ,
    & ' ALPIN ' , PKG , // T17)
    & ' ' , ' MATL ' , MATL , ' MATL ' , MATL , ' #1 ' , #2 ,
    & ' #3 ' , #4 //)
  193 FORMAT(1X, A10, B16, 2X, I, A12, 4)
  IOPT=IOPT
  RETURN
C----- DETERMINE GEOMETRIC STIFF -----
200 CONTINUE
C- KG IS DETERMINED FOR A UNIT LOAD WITH IOPT=1.
C- THE ACTUAL LOAD (PGDOM) IS DETERMINED HERE.
C- KG IS MULTIPLIED BY PGDOM WHEN THE TOTAL STIFFNESS IS ASSEMBLED
C- NEGATIVE PGDOM IS COMPRESSION....
AXIAL=.TRUE.
C----- DETERMINE THE AXIAL LOAD TO BE USED FOR KG...
IF ( KGDATA(1).EQ.1 ) THEN
  PGDOM = PKG
ELSE IF ( KGDATA(1).EQ.2 ) THEN
  PGDOM = -P(2)
ENDDIF
IF ( BTEST(1BUG,7) ) WRITE ( 6, * ) ' AXIAL ' , AXIAL , ' PGDOM ' , PGDOM
RETURN
C----- DETERMINE STIFFNESS -----
300 CONTINUE
SS=SS
SA1=SA1
SA2=SA2
IF ( BTEST(1BUG,7) ) WRITE ( 6, 402 ) IELNO, SS, SA1, SA2
IF ( BTEST(1BUG,7) ) WRITE ( 6, 402 ) IELNO, SE(ISES), SE(ISEA1)
& SE(ISEA2)
C----- SS, SA1, SA2 CONTAIN THE STIFFNESSES, WHICH WERE DETERMINED IN
C THE LAST CALL TO MATLIB
C IF SS=SE(ISES) AND SA1=SE(ISEA1) AND SA2=SE(ISEA2)
C THEN THE STIFFNESS HAS NOT CHANGED, RETURN
IF ( SS.EQ.SE(ISES) .AND. SA1.EQ.SE(ISEA1)
  & .AND. SA2.EQ.SE(ISEA2) ) RETURN
C----- SET UP THE LOCAL STIFFNESS MATRIX
SS= SE(ISES)
SA1= SE(ISEA1)
SA2= SE(ISEA2)
IF ( BTEST(1BUG,7) ) WRITE ( 6, 402 ) IELNO, SS, SA1, SA2
SA1L= SE(ISEA1)/LENGTH*2
SA2L= SE(ISEA2)/LENGTH*2
SSL= SE(ISES)
C----- CALCULATE SAT
DO 360 J=1, IELDOF
  SAT(1,J) = SSL*A(J,1)
  SAT(2,J) = SA1L*A(J,2)
  SAT(3,J) = SA2L*A(J,3)
C----- CALCULATE ASAT, STORE IN UPPER TRIANGULAR MATRIX
L=0
DO 370 J=1, IELDOF
  DO 370 I=J, 1, -1
    L=L-1
    DSTIFF(L)=0
    DO 370 K=1, 3
      DSTIFF(L) = DSTIFF(L) + A(I,K)*SAT(K,J)
    371 STIFF(J) = DSTIFF(J)
C IF ( BTEST(1BUG,7) ) THEN
  WRITE ( 6, * ) ' IELDOF ' , IELDOF , ' LM ' , ( LM(I), I=1, IELDOF )
  CALL WMTRX( STIFF, MD, IELDOF, ( L+1 ), ' WALL STIFFNESS ' )
ENDDIF
RETURN
C----- DETERMINE INCREMENTAL FORCES -----
400 CONTINUE
IF ( PRINT.AND.FIRST ) WRITE ( 6, 491 ) STBPID
IF ( BTEST(1BUG,7) ) THEN
  WRITE ( 6, 401 ) IELNO
  401 FORMAT (1X, 30( ' ' ), ' SHEAR WALL, ELEMENT# ' , 15, I, 30( ' ' ) )
  ENDDIF
C----- SET UP THE COMPONENT STIFFNESSES
SS= SE(ISES)
SA1= SE(ISEA1)
SA2= SE(ISEA2)
IF ( BTEST(1BUG,7) ) WRITE ( 6, 402 ) IELNO, SS, SA1, SA2
402 FORMAT( ' IELNO ' , 11, ' SS ' , G12.5, ' SA1 ' , G12.5, ' SA2 ' , G12.5)
C----- LOOP FOR EACH LOAD
DO 430 LOAD=1, NLOAD
C----- TRANSFORM FRAME DISPL INTO LOCAL DISPL
DO 410 I=1, 3
  R(I)=0
  V(I)=0
  DO 410 J=1, IELDOF
    K=LM(J)
    410 R(I)=R(I)+A(J,I)*DISPL(K,LOAD)
C----- INCREMENTAL FORCES
ELE05010 C----- INCREMENTAL FORCES
ELE05020 F(2)=SA1*R(2)/(LENGTH*2)
ELE05030 F(3)=SA2*R(3)/(LENGTH*2)
ELE05040 F(1)=SS*R(1)
ELE05050
ELE05060
ELE05070 C----- CHECK STIFFNESS AND CALCULATE UNBALANCED FORCES
ELE05080 THIS IS PREFORMED ONLY FOR INELASTIC ANALYSIS
ELE05090 P(1) = FT(1)+F(1)
ELE05100 P(2) = FT(2)+F(2)
ELE05110 P(3) = FT(3)+F(3)
ELE05120 IF (.NOT. ELSTIC) THEN
ELE05130 -- TRANSFER PERMANENT HYSTERESIS DATA TO TEMPORARY STORAGE..
ELE05140 DO 415 I=1, ISE2
ELE05150 SE(I+ISE2)=SE(I)
ELE05160 -- SHEAR -----
ELE05170 VC=VT(1)+V(1)
ELE05180 PL=FT(1)
ELE05190 DL=RT(1)
ELE05200 D=(RT(1)+R(1))
ELE05210 VC=VT(1)+V(1)
ELE05220 VL=VT(1)
ELE05230 -- SET INITIAL KKL BASED ON INITIAL DISP. INCREMENT(DSINC)
ELE05240 -- IF INITIAL DISPL. INCREMENT IS POSITIVE, POSTFG=.TRUE.
ELE05250 IF (SE(ISE2+44).EQ.0) THEN
ELE05260 IF (SE(ISE2+41).GT.0) THEN
ELE05270 SE(ISE2+31)=1
ELE05280 SE(ISE2+44)=1
ELE05290 END IF
ELE05300 IF (SE(ISE2+41).LT.0) THEN
ELE05310 SE(ISE2+3)=3
ELE05320 SE(ISE2+44)=1
ELE05330 SE(ISE2+37)=0
ELE05340 ENDF
ELE05350 END IF
ELE05360 -- CALL HYST MODEL TO GET NEW STIFFNESS AND
ELE05370 THE TOTAL FORCE P AT DISPL D.
ELE05380 MOP=3
ELE05390 CALL MATLIB
ELE05400 (MOP, LSTTTP, FALSE, IREL, FALSE, NAME,
ELE05410 ESES, PSES, DUCT(1,1), EXCR(1,1),
ELE05420 P(1), PL, D, DL, VC, VL, LELEM, LMAT ,
ELE05430 MATS, MAT1, SE(ISE2+ISES), IELNO, DAMAGE)
ELE05440 IF (SE(ISE2+ISES).NE.SE(ISES)) NEWK=.TRUE.
ELE05450 -- AXIAL -----
ELE05460 -- SET UP TEMPORARY - TOTAL LOADS, DISPL'S AND VELOC
ELE05470 P(2)=P(2)+2
ELE05480 D=(RT(2)+R(2))/LENGTH
ELE05490 DL=RT(2)/LENGTH
ELE05500 VC=VT(2)+V(2)
ELE05510 VL=VT(2)
ELE05520 -- CALL HYST MODEL TO GET NEW STIFFNESS AND
ELE05530 THE TOTAL FORCE P AT DISPL D.
ELE05540 MOP=3
ELE05550 CALL MATLIB
ELE05560 (MOP, LSTTTP, FALSE, IREL, FALSE, NAME,
ELE05570 ESEAL, PSEAL, DUCT(1,2), EXCR(1,2),
ELE05580 P(2), PL, D, DL, VC, VL, LELEM, LMAT ,
ELE05590 MAT2, MAT1, SE(ISE2+ISEA1), IELNO, DAMAGE)
ELE05600 P(2)=P(2)+2
ELE05610 D=(RT(2)+R(2))/LENGTH
ELE05620 DL=RT(2)/LENGTH
ELE05630 VC=VT(2)+V(2)
ELE05640 PL=PT(3)+2
ELE05650 P(3)=P(3)+2
ELE05660 D=(RT(3)+R(3))/LENGTH
ELE05670 DL=RT(3)/LENGTH
ELE05680 VC=VT(3)+V(3)
ELE05690 VL=VT(3)
ELE05700 -- CALL HYST MODEL TO GET NEW STIFFNESS AND
ELE05710 THE TOTAL FORCE P AT DISPL D.
ELE05720 MOP=3
ELE05730 CALL MATLIB
ELE05740 (MOP, LSTTTP, FALSE, IREL, FALSE, NAME,
ELE05750 ESEA2, PSEA2, DUCT(1,3), EXCR(1,3),
ELE05760 P(3), PL, D, DL, VC, VL, LELEM, LMAT ,
ELE05770 MAT2, MAT1, SE(ISE2+ISEA2), IELNO, DAMAGE)
ELE05780 P(3)=P(3)+2
ELE05790 IF (SE(ISE2+ISEA2).NE.SE(ISEA2)) NEWK=.TRUE.
ELE05800 ENDDIF
ELE05810 IF (ELSTIC) THEN
ELE05820 ESES=0.50*(FT(1)+F(1))*(RT(1)+R(1))
ELE05830 ESEAL=0.50*(FT(2)+F(2))*(RT(2)+R(2))
ELE05840 ESEA2=0.50*(FT(3)+F(3))*(RT(3)+R(3))
ELE05850 ESE=ESES+ESEAL+ESEA2
ELE05860 PSE=0.0
ELE05870 ELSE
ELE05880 PSE=PSES+PSEAL+PSEA2
ELE05890 ESE=ESES+ESEAL+ESEA2
ELE05900 ENDDIF
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ELE09990
ELE10000

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493 FORMAT (' I=1,1, F=1,1P,G12.4, FT=1,G12.4, P=1,G12.4)
494 FORMAT (' I=1,15, DM(I),ID(J3),ID(J4) SUB(1),1P,G12.4)
C
      IOPT=IOPT
      RETURN
C----- DETERMINE TOTAL MEMBER FORCES, ENERGIES ECT -----
500 CONTINUE
      IF (ICOMN.NE.1 .AND. ICOMN.NE.2) THEN
      IF (PRINT.AND.FIRST) WRITE (6,491) STEPID
C----- TOTAL FORCES, DISPLACEMENTS AND VELOCITIES
      DO 510 I=1,3
      FT(I) = FT(I) + F(I)
      RT(I) = RT(I) + R(I)
      VT(I) = VT(I) + V(I)
      IF ( ABS(FT(1)),GT.ABS(FMAX(I)) ) THEN
      FMAX(I) =FT(1)
      FMAX(I-3)=RT(1)
      ENDF
510 CONTINUE
C
      IF (PRINT) DATA
      IF (PRINT) WRITE (6,492) IELNO,LOAD,
      ID(J1),ID(J2),ID(J3),ID(J4),
      (FT(I),RT(I),I=1,3)
      ELSE IF (ICOMN.EQ.1 .OR. ICOMN.EQ.2) THEN
      IF (PRINT .AND. FIRST) WRITE(6,497) STEPID
      DO 513 I=1,3
      FT(I)=RFA(I)
      IF (PRINT) WRITE (6,492) IELNO,LOAD,
      ID(J1),ID(J2),ID(J3),ID(J4),
      (FT(I),I=1,3)
      ENDF
497 FORMAT (' / R/C SHEAR WALL FORCES.../SK,A/
      / ELEMENT LOAD,T17,
      / JNT-1, JNT-2, JNT-3, JNT-4,
      / SHEAR / AXIAL AXIAL2 ')
C----- TRANSFER TEMPORARY HYSTERESIS DATA TO PERMANENT STORAGE...
      ISE2=ISE2
      IF (.NOT.ELSTIC) THEN
      DO 520 I=1,ISE2
      SE(I)=SE(I+ISE2)
      ENDF
      RETURN
C----- DETERMINE FIXED END FORCES, AND ADD TO FORCE-----
600 RETURN
C----- FIXED END FORCES ARE NOT AVAILABLE FOR R/C SHEAR WALL
C----- WRITE MEMBER FORCES TO OUTPUT FILE -----
700 CONTINUE
C----- SHEAR ENERGY, DUCTILITY AND EXCURSION -----
      IF (ELSTIC) THEN
      ESSE=0.50*(FT(1)+F(1))*(RT(1)+R(1))
      ESSE1=0.50*(FT(2)+F(2))*(RT(2)+R(2))
      ESSE2=0.50*(FT(3)+F(3))*(RT(3)+R(3))
C CHANGE FOR DAMAGE
      ESE= ESSE/LENGTH+ESEA1+ESEA2
      PSE=0.0
      ELSE
      CALL MATLIB
      / ( 4 ,LSTTYP ,FALSE ,IREL,FALSE,NAME,
      / ESSE,PSE,DUCT(1,1),EXCR(1,1),
      / FT ,F ,RT ,R ,0 ,0 ,LELEM ,LMAT ,
      / MATS ,MATTYP,SE(ESSE),IELNO,DAMAGE)
C----- AXIAL ENERGY, DUCTILITY AND EXCURSION (1)..
      CALL MATLIB
      / ( 4 ,LSTTYP ,FALSE ,IREL,FALSE,NAME,
      / ESSE1,PSEAL,DUCT(1,2),EXCR(1,2),
      / FT ,F ,RT ,R ,0 ,0 ,LELEM ,LMAT ,
      / MATAL ,MATTYP,SE(ESSE1),IELNO,DAMAGE)
C----- AXIAL ENERGY, DUCTILITY AND EXCURSION (2)..
      CALL MATLIB
      / ( 4 ,LSTTYP ,FALSE ,IREL,FALSE,NAME,
      / ESSE2,PSEA2,DUCT(1,3),EXCR(1,3),
      / FT ,F ,RT ,R ,0 ,0 ,LELEM ,LMAT ,
      / MATA2 ,MATTYP,SE(ESSE2),IELNO,DAMAGE)
      ESE=ESSE+ESEA1+ESEA2
      PSE=PSE+PSEAL+PSEA2
      ENDF
      IF (PRINT) WRITE (IUNIT,710) ID(J1),ID(J2),ID(J3),ID(J4)
      WRITE (IUNIT,711) (FT(I),RT(I),I=1,3),ESE,PSE
      / (DUCT(I,J),EXCR(I,J),I=1,3),J=1,3)
710 FORMAT (4I5,1P,6E10.3)
711 FORMAT (1P,6E10.3)
      RETURN
C----- READ AND PRINT MEMBER FORCES FROM OUTPUT FILE -----
800 CONTINUE
      BACKSPACE(IUNIT)
      READ (IUNIT,' ) ITYPE,IELNO,IWRITE,TO,DT
      READ (IUNIT,710) NODE1,NODE2,NODE3,NODE4
      ISTEP=IWRITE
C
      810 READ (IUNIT,815,END=830) (F(I),R(I),I=1,3),ESE,PSE
      815 FORMAT (8E10.3)
      ISTEP=ISTEP+IWRITE
      T=TO-DT*ISTEP
      IF (ISTEP.EQ.0) WRITE (6,805) NODE1,NODE2,NODE3,NODE4
      IF (MOD(ISTEP,INC).EQ.0)
      / WRITE (6,820) ISTEP,T,(F(I),R(I),I=1,3),ESE,PSE
      GO TO 810
C
      830 CONTINUE
      WRITE (6,835)
C
      WRITE (6,840) 'BENDING COMPONENT',(DUCT(1,1),EXCR(1,1),I=1,3)
      WRITE (6,840) 'SHEAR COMPONENT' ,(DUCT(1,2),EXCR(1,2),I=1,3)
      WRITE (6,840) 'AXIAL COMPONENT' ,(DUCT(1,3),EXCR(1,3),I=1,3)
C
      805 FORMAT (' / R/C SHEAR WALL FORCES.../
      / 5X, NODE1=1,16,5X, NODE2=1,16,5X, NODE3=1,16,5X,
      / STEP TIME /
      / SHEAR / SHEAR DISPL. /
      / AXIAL / AXIAL DISPL. /
      / AXIAL2 / AXIAL DISPL. 2 /
      / ESE / PSE ')
      820 FORMAT (16,1P,6I14.5,2G12.4)
      835 FORMAT (/T10, / MAXIMUM DUCTILITIES AND EXCURSION RATIOS/
      / T10, /)
      840 FORMAT (/T10,A/
      / T15, / DISPLACEMENT DEFINITION: U1=1,P,G12.4,5X, 'E1',G12.4 /
      / T15, / ENERGY DEFINITION #1: U2=1,P,G12.4,5X, 'E2',G12.4 /
      / T15, / ENRGY DEFINITION #2: U3=1,P,G12.4,5X, 'E3',G12.4 /
      /)
      RETURN
C----- DETERMINE THE LENGTH OF THE MATERIAL DATA
900 CONTINUE
      ISE=0
C----- LOOP FOR EACH MATERIAL -----
      DO 910 I=1,3
      IF (I.EQ.1) THEN
      ISE=ISE+1
      ELSE IF (I.EQ.2) THEN
      ISEAL=ISE+1
      ELSE IF (I.EQ.3) THEN
      ISE2=ISE+1
      ENDF
      MATI = RINPUT(I)
      CALL MATLIB
      / ( 4 ,LSTTYP ,FALSE ,IREL,FALSE,NAME,
      / ESSE,PSE,DUCT(1,1),EXCR(1,1),
      / FT ,F ,RT ,R ,0 ,0 ,LELEM ,LMAT ,
      / MATI ,MATTYP,SE(ESSE),IELNO,DAMAGE)
      ISE=ISE + LELEM
C----- DOUBLE THE STORAGE, TO ALLOW FOR TEMPORARY VALUES...
      ISE=ISE*2
      RETURN
C----- DETERMINE THE STRAIN ENERGY
1000 CONTINUE
C----- DUCTILITIES AND EXCURSION RATIOS
1100 CONTINUE
      / NOTE ELEM3 WAS LAST CALLED TO CALC INCREMENTAL DISPL.
      / THE ADDRESSES: ISE=ISE/2
      ISEAL=ISE/2
      ISE2=ISE/2
C----- SHEAR ENERGY, DUCTILITY AND EXCURSION ....
      CALL MATLIB
      / ( 4 ,LSTTYP ,FALSE ,IREL,FALSE,NAME,
      / ESSE,PSE,DUCT(1,1),EXCR(1,1),
      / FT ,F ,RT ,R ,0 ,0 ,LELEM ,LMAT ,
      / MATS ,MATTYP,SE(ESSE),IELNO,DAMAGE)
C----- AXIAL ENERGY, DUCTILITY AND EXCURSION (1)..
      CALL MATLIB
      / ( 4 ,LSTTYP ,FALSE ,IREL,FALSE,NAME,
      / ESSE1,PSEAL,DUCT(1,2),EXCR(1,2),
      / FT ,F ,RT ,R ,0 ,0 ,LELEM ,LMAT ,
      / MATAL ,MATTYP,SE(ESSE1),IELNO,DAMAGE)
C----- AXIAL ENERGY, DUCTILITY AND EXCURSION (2)..
      CALL MATLIB
      / ( 4 ,LSTTYP ,FALSE ,IREL,FALSE,NAME,
      / ESSE2,PSEA2,DUCT(1,3),EXCR(1,3),
      / FT ,F ,RT ,R ,0 ,0 ,LELEM ,LMAT ,
      / MATA2 ,MATTYP,SE(ESSE2),IELNO,DAMAGE)
      IF (DUCFAG.EQ.0) THEN
      IF (ABS(DUCT(1,1)).GE.1 .OR. ABS(DUCT(1,2)).GE.1
      / .OR. ABS(DUCT(1,3)).GE.1) THEN
      DUCFAG=ABS(DUCT(1,1))
      IF (DUCFAG.LT. ABS(DUCT(1,2))) DUCFAG=ABS(DUCT(1,2))
      IF (DUCFAG.LT. ABS(DUCT(1,3))) DUCFAG=ABS(DUCT(1,3))
      ENDF
      ENDF
      IF (ELSTIC) THEN
      ESSE=0.50*(FT(1)+F(1))*(RT(1)+R(1))
      ESSE1=0.50*(FT(2)+F(2))*(RT(2)+R(2))
      ESSE2=0.50*(FT(3)+F(3))*(RT(3)+R(3))
      ESE= ESSE+ESEA1+ESEA2
      PSE=0.0
      ELSE
      ESE=ESSE+ESEA1+ESEA2
      PSE=PSE+PSEAL+PSEA2
      ENDF
      IF (BEST(IBUG,7)) THEN
      WRITE (6,*) 'ESE ESSE ESEA2 ',ESE,ESSE,ESEA2
      WRITE (6,*) 'PSE PSEAL PSEA2 ',PSE,PSEAL,PSEA2
      WRITE (6,*) 'ESE PSE LENGTH',ESE,PSE,LENGTH
      ENDF
      IF (PRINT.AND.FIRST) WRITE (6,1191)
      IF (PRINT) WRITE (6,1192) IELNO,(DUCT(I,J),EXCR(I,J),I=1,3),J=1,3)
1192 FORMAT (I5,15F8.2)
1191 FORMAT (' / R/C SHEAR WALL DUCTILITY AND EXCURSION RATIOS /%X,
      / I----- SHEAR COMPONENT -----/
      / I----- AXIAL COMPONENT 1-----/
      / I----- AXIAL COMPONENT 2-----/
      / 123456789012345678901234567890123456789012
      / I--- DEFN 1--- --- DEFN 2--- --- DEFN 3---/
      / I--- DEFN 1--- --- DEFN 2--- --- DEFN 3---/6X,
      / DUCT EXCR. DUCT. EXCR. DUCT. EXCR. /
      / DUCT. EXCR. DUCT. EXCR. DUCT. EXCR. /
      / 123456789012345678901234567890123456789012
      RETURN
C----- WRITE MAXIMUM AND MINIMUM VALUES
1200 CONTINUE
      IF (FIRST) WRITE(6,491) MAXIMUM LOADS AND DISPL AT MAXIMUM LOADS
      / FMAX(I) / NOT AT MAXIMUM VALUES MAT NOT OCCUR SIMULTANEOUSLY
      / WRITE (6,492) IELNO,LOAD,
      / ID(J1),ID(J2),ID(J3),ID(J4),
      / (FMAX(I),FMAX(3+I),I=1,3)
      RETURN
C----- RESET INTERNAL FORCES
1300 CONTINUE
C----- ZERO MATRICES
      W = SE(ISE + 78)
      COI=SE(ISE + 37)
      COJ=SE(ISE + 44)
      COI2=SE(ISE + 45)
      COJ2=SE(ISE + 46)
      DO 1310 I=1,ISE
      1310 SE(I)=0
      SE(ISE+77)=LENGTH
      SE(ISE+78)=W
      SE(ISE + 37)=COI
      DO 1325 I=1,2
      FMAX(I)=0
      F(I)=0
      RT(I)=0
      R(I)=0
      VT(I)=0
      V(I)=0
      1325 VT(I)=0
      WRITE (6,1330) IELNO
      1330 FORMAT ( / SHEAR WALL.... ELEMENT #, I6,
      / INTERNAL FORCES AND HYSTERESIS MODELS ARE RESET TO ZERO')
      CALL MATLIB
      / ( 2 ,LSTTYP ,FALSE ,IREL,FALSE,NAME,
      / ESSE,PSE,DUCT(1,1),EXCR(1,1),
      / FT ,F ,RT ,R ,0 ,0 ,LELEM ,LMAT ,
      / MATS ,MATTYP,SE(ESSE),IELNO,DAMAGE)
      / ( 2 ,LSTTYP ,FALSE ,IREL,FALSE,NAME,
      / ESSE1,PSEAL,DUCT(1,2),EXCR(1,2),
      / FT ,F ,RT ,R ,0 ,0 ,LELEM ,LMAT ,
      / MATAL ,MATTYP,SE(ESSE1),IELNO,DAMAGE)
      / ( 2 ,LSTTYP ,FALSE ,IREL,FALSE,NAME,
      / ESSE2,PSEA2,DUCT(1,3),EXCR(1,3),

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4 FT , F , RT , R , .0 , .0 , .LELEM , LMAT ,
4 MATA2 , MATTYP , SE(1:SEA2) , IELNO , DAMAGE)
C
RETURN
C----- DAMAGE INDEX
1400 IF (ELSTIC) RETURN
FMAX1=FMAX(1)
FMAX2=FMAX(2)
FMAX3=FMAX(3)
FMAX4=FMAX(4)
FMAX5=FMAX(5)
FMAX6=FMAX(6)
C----- SHEAR ENERGY, AND DAMAGE INDEX
CALL MATLIB
4 ( 5 LSTTYP , FALSE , IREL , FALSE , NAME ,
4 ESEA1 , PSEA1 , DUCT(1,2) , EXCR(1,1) ,
4 ESES , PSES , DUCT(1,1) , EXCR(1,1) ,
4 FMAX1 , .0 , FMAX4 , .0 , .0 , .LELEM , LMAT ,
4 MATS , MATTYP , SE(1:SEA2) , IELNO , DAMS)
C----- AXIAL ENERGY, AND DAMAGE INDEX (1)
CALL MATLIB
4 ( 5 LSTTYP , FALSE , IREL , FALSE , NAME ,
4 ESEA1 , PSEA1 , DUCT(1,2) , EXCR(1,2) ,
4 FMAX2 , .0 , FMAX5 , .0 , .0 , .LELEM , LMAT ,
4 MATA1 , MATTYP , SE(1:SEA1) , IELNO , DAMA1)
C----- AXIAL ENERGY, AND DAMAGE INDEX (2)
CALL MATLIB
4 ( 5 LSTTYP , FALSE , IREL , FALSE , NAME ,
4 ESEA2 , PSEA2 , DUCT(1,3) , EXCR(1,3) ,
4 FMAX3 , .0 , FMAX6 , .0 , .0 , .LELEM , LMAT ,
4 MATA2 , MATTYP , SE(1:SEA2) , IELNO , DAMA2)
C
ESE=ESES-ESEA1-ESEA2
PSE=PSES-PSEA1-PSEA2
DAMAGE=DAMS*(ESE+PSE)+DAMA1*(ESEA1+PSEA1)
+DAMA2*(ESEA2+PSEA2)
IF (FIRST) WRITE (6,1420)
WRITE (6,1410) IELNO , DAMS , ESES , PSES , DAMA1 , ESEA1 , PSEA1 , DAMA2 ,
ESEA2 , PSEA2
1410 FORMAT (1X,15,1P,3(F14.5,2G14.5))
1420 FORMAT (/6X,
4 /----- SHEAR COMPONENT -----/,
4 /----- AXIAL COMPONENT -----/, 'ELEM#',
4 /----- AXIAL ENERGY -----/,
4 /----- DAMAGE ----- ESE ----- PSE -----/,
4 /----- DAMAGE ----- ESE ----- PSE -----/)
RETURN
END
PROCESS SOUNP GOSTMT XREF
C-----
C DEBUG UNIT(6), SUBCHR, SUBTRACE, INIT
C END DEBUG
C-----
C ENERGY BALANCE
C-----
SUBROUTINE ENERGY(L1,L2,L3,L4,L5,L6,NDOF,MAXNOD,NCOS,PT2,COND,
4 IMD,IMDMS,IMDKG,MD,MDMS,MDKG,DYN,WRITE,ABORT,
4 STIFF,KG,MASS,
4 EIS,ESE,PSE,EKE,EDD,
4 ALPHA,BETA,KGFORM,NELMT,NNODE,DT,SUMUB,
4 SUMRC,DSUMRC,SUMFD,DSUMFD,FDAMP,DPDAMP,PZ,
4 GA,GV,GD,GAT,GVT,GDT,WORK,WORK2,FG,DFG,
4 DACC,DVEL,DDISP,ACC,VEL,DISP,LOAD,DLOAD,
4 IDOF, IDOF , CNST , JTFLG , COORD
4 COSINE , JTCOS )
DOUBLE PRECISION EIS,ESE,DEIE,PSE,DEDD,EDD,EKE
COMMON /ISPEC/ ISPL,SGD(3000),REAC(6)
DIMENSION COORD(MAXNOD,3),ID(MAXNOD),
4 IDOF(MAXNOD,6),COSINE(3,3,NCOS),CNST(MAXNOD,6),
4 JTFLG(MAXNOD),JTCOS(MAXNOD)
C
REAL MASS,KG,LOAD
C
CHARACTER*80 NAME
DIMENSION STIFF(IMD),MD(L6-1)
DIMENSION MASS(IMDMS),MDMS(L6-1)
DIMENSION KG(IMDKG),MDKG(L6-1)
DIMENSION RINPUT(100),FDAMP(L6),DFDAMP(L6),FG(6),DFG(6)
DIMENSION DLOAD(6),DEUMRC(6),SUMFD(6),DEUMFD(6),SUMUB(6)
LOGICAL BTTEST,PRINT,HEAD,PT2,AXIAL,COND,F,7,DYN,WRITE,ABORT
C
DIMENSION GD(3),GV(3),GA(3)
DIMENSION DT(3),DVT(3),GAT(3)
DIMENSION DDISP(L6),DVEL(L6),DACC(L6)
DIMENSION DISP(L6),VEL(L6),ACC(L6)
DIMENSION WORK(L6),WORK2(L6),LOAD(L6),DLOAD(L6)
DATA F/,FALSE,/,T/,TRUE,/
C----- NOMENCLATURE
C DACC INCREMENTAL RELATIVE ACCELERATION
C DVEL INCREMENTAL RELATIVE VELOCITY
C DDISP INCREMENTAL RELATIVE DISPLACEMENT
C ACC PREVIOUS STEP'S RELATIVE ACCELERATION
C VEL PREVIOUS STEP'S RELATIVE VELOCITY
C DISP PREVIOUS STEP'S RELATIVE DISPLACEMENT
C DLOAD INCREMENTAL APPLIED LOAD, NOT W*AG
C LOAD PREVIOUS STEP'S APPLIED LOAD, NOT W*AG
C DFDAMP INCREMENTAL DAMPING FORCE
C FDAMP PREVIOUS STEP'S DAMPING FORCE
C
C GD INCREMENTAL GROUND DISPLACEMENT
C GV INCREMENTAL GROUND VELOCITY
C GA INCREMENTAL GROUND ACCELERATION
C GVT TOTAL GROUND VELOCITY
C GDT TOTAL GROUND DISPLACEMENT
C GAT TOTAL GROUND ACCELERATION
C WORK TEMP FORCE VECTOR
C WORK2 TEMP DISPL OR VELOCITY VECTOR
C-----
L4=L4+1
C-----
C== INTEGRATE GROUND ACCELERATION ==
C-----
IF (DTN) THEN
DO I=1,3
GA(I)=GAT(I)
GV(I)=(2*GAT(I)+GAT(I))*DT/2
GD(I)=GVT(I)*DT+(3*GAT(I)+GAT(I))*DT*DT/6
ENDDO
ELSE
WRITE (6,22) (I,ACC(I),VEL(I),DISP(I),LOAD(I),I=1,L6)
WRITE (6,23) (I,DACC(I),DVEL(I),DDISP(I),DLOAD(I),I=1,L6)
WRITE (6,24) (I,FG(I),DFG(I),FDAMP(I),DFDAMP(I),I=1,L6)
ELSE
WRITE (6,22) (I,ACC(I),VEL(I),DISP(I),LOAD(I),I=1,L6)
WRITE (6,23) (I,DACC(I),DVEL(I),DDISP(I),DLOAD(I),I=1,L6)
WRITE (6,24) (I,FG(I),DFG(I),FDAMP(I),DFDAMP(I),I=1,L6)
ENDIF
ENDIF
IF (PT2) THEN
IF (DTN) THEN
WRITE (6,22) (I,ACC(I),VEL(I),DISP(I),LOAD(I),I=1,L4)
WRITE (6,23) (I,DACC(I),DVEL(I),DDISP(I),DLOAD(I),I=1,L4)
WRITE (6,24) (I,FG(I),DFG(I),FDAMP(I),DFDAMP(I),I=1,L6)
ELSE
WRITE (6,22) (I,ACC(I),VEL(I),DISP(I),LOAD(I),I=1,L6)
WRITE (6,23) (I,DACC(I),DVEL(I),DDISP(I),DLOAD(I),I=1,L6)
WRITE (6,24) (I,FG(I),DFG(I),FDAMP(I),DFDAMP(I),I=1,L6)
ENDIF
ENDIF
22 FORMAT(/10,'TOTAL RELATIVE RESPONSE FOR PREVIOUS TIME STEP'/
4 (/10,'DOF',I6,' ACC',I1P,G13.5,' VEL',I1P,G13.5,' DSPI',G13.5,
4 'LOAD',I1P,G13.5))
23 FORMAT(/10,'INCREMENTAL RELATIVE RESPONSE'/
4 (/10,'DOF',I6,' ACC',I1P,G13.5,' VEL',I1P,G13.5,' DSPI',G13.5,
4 'LOAD',I1P,G13.5))
24 FORMAT(/10,'SPECIAL FORCES '/
4 (/10,'DOF',I6,' FDI',I1P,G13.5,' FDI',I1P,G13.5,' FDAMP',I1P,G13.5,
4 'FDAMP',I1P,G13.5))
ENDIF
ENDIF
C-----
C== CALCULATE TOTAL STRAIN ENERGY ==
C-----
C----- CALCULATE THE TOTAL STRAIN ENERGY
C-----
DESE=ESE
DPSE=PSE
ESE=0
PSE=0
C----- GET ELASTIC AND PLASTIC STRAIN ENERGY FOR EACH ELEMENT
LSTTYP=0
PRINT=FALSE
HEAD=FALSE
DO 60 IELNO=1,NELMT
CALL ELEMIB
4 (10,LSTTYP, .FALSE, IREL, .FALSE, NAME, EISE, EPSE, DAMAGE,
4 IELNO, IELDOF, KGDOF, PGEOM, RINPUT, AXIAL, IZDISP, IZLOAD, MSTRCR)
IF (PT2) WRITE (6,55) IELNO, EISE, EPSE
55 FORMAT(' ELEMENT#',I5,' ELASTIC SE=',I1P,G13.4,
4 ' PLASTIC SE=',I1P,G13.4)
ESE = ESE + EISE
PSE = PSE + EPSE
C----- CALCULATE THE INCREMENTAL STRAIN ENERGY
DESE=ESE-DESE
DPSE=PSE-DPSE
C-----
C== CALCULATE KINETIC ENERGY ==
C-----
IF (DTN) THEN
C----- GET THE ABSOLUTE VELOCITY AND STORE IN VECTOR WORK2....
SAVE RELATIVE RESPONSE IN VECTOR WORK2
DO 15 I=1,L4
WORK2(I)=VEL(I)+DVEL(I)
ENDDO

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BUG57= BTEST(IBUG,7) .AND. BUG5
LSTTTP=0
PRINT=.FALSE.
HEAD=.FALSE.
C-----
C IOPT=1 FORM STIFFNESS MATRIX
C IOPT=2 FORM GEOMETRIC STIFFNESS MATRIX
C-----
C IF (BUG5) WRITE (6,*) ' IN ROUTINE FORM IOPT1',IOPT
C 49 FORMAT (1X,A/(' I=',I3,' MDMASS(1)',I11))
C GO TO (100,200),IOPT
C-----
C ASSEMBLE STIFFNESS -----
C-----
100 CONTINUE
  IST = IZSTIF
  IEND=IZSTIF + NZ(IZMD+NDOF) - 1
C-----
  ZERO STIFFNESS
  DO 110 I=IST,IEND
110   Z(I)=0
C-----
  LOOP FOR EACH ELEMENT
  DO 120 IELNO=1,NELMT
C-----
  GET STIFFNESS OF EACH ELEMENT
  FALSE=.FALSE.
  CALL ELEM1B
  & (3,LSTTTP, FALSE, IREL, FALSE, NAME, ESSE, EPSE, DAMAGE,
  & DUCFAC,
  & IELNO, IELDOF, KGDOF, PGEOM, Z, AXIAL, IZDISP, IZLOAD, MSTOR)
C IF (BUG57) THEN
  WRITE (ELNO,15) IELNO, (N3(I+IZLM-1), I=1, MIN(IZLDOF, 10))
15  FORMAT (' ELEMENT #', I6, ' LM', I8I6)
  CALL LMATRIX(2(IZKE), MD, IELDOF, MD(IZLDOF+1), ELNO)
  ENDF
C CALL FORML(2(IZSTIF), N3(IZMD), NDOF, N3(IZMD+NDOF),
  & (IZKE), MD, IELDOF, MD(IZLDOF+1), N3(IZLM), 1.)
C-----
  SUBTRACT GEOMETRIC STIFFNESS
  NZ(IZKGD+3) = NZ(IZKGD+3)
C IF (NZ(IZKGD+3).EQ.1) THEN
  FALSE=.FALSE.
  CALL ELEM1B
  & (2,LSTTTP, FALSE, IREL, FALSE, NAME, ESSE, EPSE, DAMAGE,
  & DUCFAC,
  & IELNO, IELDOF, KGDOF, PGEOM, Z, AXIAL, IZDISP, IZLOAD, MSTOR)
  KGDOF-KGDOF
C IF (BUG57) THEN
  WRITE (ELNO,15) IELNO, (N3(I+IZLMKG-1), I=1, MIN(KGDOF, 10))
  CALL LMATRIX(2(IZKEKG), MD, KGDOF, MD(KGDOF+1), ELNO)
  ENDF
C IF (.NOT. AXIAL) GO TO 120
  DETERMINE THE FACTOR FOR KG
  IF (NZ(IZKGD+1).EQ.1) THEN
    ACCEL= Z(IZKGD)
    PGEOM=PGEOM+(1+ACCEL)
  ENDF
C SWAP THE SIGN ON PGEOM FOR SUBTRACTION.....
  PGEOM=-PGEOM
C CALL FORML(2(IZSTIF), N3(IZMD), NDOF, N3(IZMD+NDOF),
  & (IZKEKG), MD, KGDOF, MD(KGDOF+1), N3(IZLMKG), PGEOM)
  ENDF
120 CONTINUE
C RETURN
C-----
  ASSEMBLE GEOMETRIC STIFFNESS
200 CONTINUE
  IF (NZ(IZKGD+3).NE.2) RETURN
  IST = IZK
  IEND=IZK + NZ(IZMDKG+NDOF) - 1
C-----
  ZERO STIFFNESS
  DO 210 I=IST,IEND
210   Z(I)=0
C-----
  LOOP FOR EACH ELEMENT
  DO 220 IELNO=1,NELMT
C-----
  GET STIFFNESS OF EACH ELEMENT
  FALSE=.FALSE.
  CALL ELEM1B
  & (2,LSTTTP, FALSE, IREL, FALSE, NAME, ESSE, EPSE, DAMAGE,
  & DUCFAC,
  & IELNO, IELDOF, KGDOF, PGEOM, Z, AXIAL, IZDISP, IZLOAD, MSTOR)
C IF (.NOT. AXIAL) GO TO 220
  IF (BUG57) THEN
  WRITE (6,*) 'PGEOM', PGEOM
  WRITE (ELNO,15) IELNO, (N3(I+IZLMKG-1), I=1, MIN(KGDOF, 10))
  CALL LMATRIX(2(IZKEKG), MD, KGDOF, MD(KGDOF+1), ELNO)
  ENDF
C CALL FORML(2(IZK), N3(IZMDKG), NDOF, N3(IZMDKG+NDOF),
  & (IZKEKG), MD, KGDOF, MD(KGDOF+1), N3(IZLMKG), PGEOM)
  ENDF
220 CONTINUE
C RETURN
  END
C-----
  DEBUG UNIT(6), SUBCHK, SUBTRACE
  END DEBUG
C-----
  SUBROUTINE FORML(K, MD, NDOF, IMD, KE, MDKE, IELDOF, IMOKE, LM, FACTOR)
  DOUBLE PRECISION K
  LOGICAL BTEST
  DIMENSION MD(NDOF+1), MDKE(IZLDOF+1), LM(IZLDOF)
  REAL K(IMD), KE(IMOKE)
C-----
  MATRIX STORAGE SCHEME:
  THE STIFFNESS MATRIX (K) IS BANDED AND SYMMETRIC.
  THIS ONLY THE SKYLINE ABOVE THE MAIN DIAGONAL NEEDS TO BE STORED.
  C THE MATRIX K IS STORED IN A LINEAR ARRAY BY COLUMNS.
  C THE VALUE ON THE MAIN DIAGONAL IS STORED IN THE LINEAR ARRAY FIRST.
  C RELATIVE ADDRESSES OF THE LINEAR ARRAY ELEMENTS CONTAINING THE
  MAIN DIAGONAL TERMS ARE STORED IN MD.
  C THUS: FOR (I, J)
  IF I LE J (UPPER TRIANGULAR MATRIX)
  IF MD(J)+J-I < MD(J+1)
  I=MD(J)+J-I
  K(I, J)=K(I, J)
  ELSE
  K(I, J)=0
  I (I, J) IS ABOVE THE SKYLINE
C-----
  EXAMPLE: LET B BE THE 5X5 FULL SYMMETRIC MATRIX BELOW...

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INTEG=DEFULT
HIT=.TRUE.
IF (LWORD) THEN
  ISTART=INDEX(VERB,WORD)
  IF (ISTART.LE.0) RETURN
  I=ISTART+LEN(WORD)
ELSE
  I=1
  ISTART=1
ENDIF
C
ISIGN = 1
L=LEN(VERB)
5 IF (I.GT.L) RETURN
IF (VERB(I:1).EQ.'-') THEN
  ISIGN=-1
  I=I+1
GO TO 5
ELSE IF (VERB(I:1).EQ.'+' .OR. VERB(I:1).EQ.' ') THEN
  I=I+1
GO TO 5
ENDIF
10 IF (LGE(VERB(I:1),'0') .AND. LLE(VERB(I:1),'9')) THEN
  IF (HIT) THEN
    HIT=.FALSE.
    INTEG=0
  ENDIF
  DO 20 J=1,10
    IF(VERB(I:1).EQ. NUMB(J:J)) THEN
      INTEG=10*INTEG+(J-1)
      I=I+1
      GO TO 10
    ENDIF
  CONTINUE
20
INTEG=INTEG*ISIGN
IF (MODIFY) THEN
  IF (VERB(I:1).EQ.'-') I=I+1
  VERB(ISTART:)=VERB(I:)
ENDIF
RETURN
END
-----
C ***** HISTO2 *****
C DEBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
C END DEBUG
-----
C SU_ROUTINE AXLMOD - AXIAL HYSTERESIS MODEL
C REFERENCE: ANALYSIS OF THE FULL SCALE SEVEN
C STORY R/C TEST STRUCTURE.
C
C P = CURRENT LOAD, POSITIVE IS TENSION
C D = CURRENT DISPLACEMENT, POSITIVE IS ELONGATION
C S = STIFFNESS
C KC = VIRGIN COMPRESSION STIFFNESS, EC
C KT1 = PRE-YIELD VIRGIN TENSILE STIFFNESS, 0.9*KC
C KT2 = POST-YIELD VIRGIN TENSILE STIFFNESS, 0.001*KC
C PY = TENSILE YIELD POINT, PHO*PY
C FMAX = MAXIMUM TENSILE FORCE BEFORE UNLOADING
C DMAX = MAXIMUM TENSILE DISPLACEMENT BEFORE UNLOADING
C FX = FORCE AT STIFFNESS CHANGE POINT ON THE UNLOADING CURVE
C DX = DISPL. AT STIFFNESS CHANGE POINT ON THE UNLOADING CURVE
C DT,DI = INTERSECTION OF TWO LINES
C DIT = TENSILE YIELD DISPLACEMENT = PY/KT1
C DTC = COMPRESSION YIELD DISPLACEMENT = PT/KT1
C DPC = FORCE AT COMPRESSION STIFFNESS CHANGE POINT
C DP = COMPRESSION AT COMPRESSION STIFFNESS CHANGE POINT
C KR = UNLOADING STIFFNESS = KC/(DMAX/DIT)**(20)
C KS = LOADING STIFFNESS, LINE BETWEEN (FX,DX) AND (FP,DP)
C KT = LOADING STIFFNESS, LINE BETWEEN (FP,DP) AND (-2PT,-DPT)
C
C SUBROUTINE HISTO2(P,D,PL,DL,V,VL,S,HRLN,KC,KT1,KT2,PT,ALPHA,BETA,
C & DTT,DTC,CSE,FMAX,DMAX,FX,DX,FP,DP,KR,KS,KT,NRL,A,PRINT,
C & SEQ,ESE,PSE,PSEOLD,CSE,UPOS,UNEG,EXCR,IDR)
C
REAL*4 KC,KT1,KT2,KR,KS,KT,NRL
DIMENSION UPOS(3),UNEG(3),EXCR(6),ESE(3)
INTEGER RULE1
LOGICAL LT,PRINT,PGTPL,PLTPL
8000 IF (PRINT) WRITE (6,8010) P,D,PL,DL,V,VL,S,HRLN,KC,KT1,KT2,PT,
C & ALPHA,BETA,DTT,DTC,FMAX,DMAX,FX,DX,FP,DP,KR,KS,KT,NRL,A,PRINT,
C & SEQ,ESE,PSE,PSEOLD,CSE,UPOS,UNEG,EXCR
8010 FORMAT(
C & AXLMOD=,G13.5, / DL=,G13.5, / PL=,G13.5, / DL=,G13.5, /
C & AX=,V=,G13.5, / VL=,G13.5, / S=,G13.5, / HRLN=,G13.5, /
C & AX=,KC=,G13.5, / KT1=,G13.5, / KT2=,G13.5, / PT=,G13.5, /
C & AX=ALPHA=,G13.5, / BETA=,G13.5, / DTT=,G13.5, / DTC=,G13.5, /
C & AX= FMAX=,G13.5, / DMAX=,G13.5, / FX=,G13.5, / DX=,G13.5, /
C & AX= FP=,G13.5, / DP=,G13.5, / KR=,G13.5, / KS=,G13.5, /
C & AX= KT=,G13.5, / NRL=,G13.5, / A=,G13.5, / SEQ=,G13.5, /
C & AX= ESE=,G13.5, / PSE=,G13.5, / PSEOLD=,G13.5, /
C & AX= UPOS=,G13.5, / UNEG=,G13.5, / UP=,G13.5, /
C & AX= UN1=,G13.5, / UN2=,G13.5, / UN3=,G13.5, /
C & AX= EX1=,G13.5, / EX2=,G13.5, / EX3=,G13.5, /
C & AX= EX4=,G13.5, / EX5=,G13.5, / EX6=,G13.5)
C..... SET INITIAL STIFFNESS AND RETURN
IF (NRL.EQ.0) THEN
  IF (P.LE.0) THEN
    NRL=1
    HRLN=1.1
    S=KC
    SEQ=KC
    A=P
  ELSE
    NRL=2
    HRLN=2.1
    S=KT1
    SEQ=KC
    A=P
  ENDIF
  RETURN
ENDIF
C..... RETURN IF NO STEP HAS BEEN TAKEN
IF (P.EQ.PL .OR. D.EQ.DL) RETURN
C..... SET LOADING FLAGS
PGTPL=P.GT.PL
PLTPL=P.LT.PL
C..... CALC EXCURSION RATIO AT EVERY ZERO CROSSING...
IF (P*PL.LE.0 .AND. PL.GT.0)
& CALL DNE(0,UPOS,EXCR,DTT,D,ESE,PSE,PSEOLD,CSE)
IF (P*PL.LE.0 .AND. PL.LE.0) THEN
  EXCR(4)=0
  EXCR(5)=0
  EXCR(6)=0
ENDIF
C..... CALC DUCTILITY IF LOAD GT PAST LOAD
C
DUCTILITY FOR POSITIVE LOADS ONLY
IF (P.GT.0 .AND. PGTPL)
& CALL DNE(1,UPOS,EXCR,DTT,D,ESE(2),PSE,PSEOLD,CSE)
999 CALL IDRECT(IDR,IDRV,IDRVO,P,PL,V,VL)

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710 IF (P.GE.PMAX) GO TO 600
S=KR
NRL=7
HRLN=7.1
A=FXMAX
RETURN
730 IF (P.LE.FX) GO TO 800
S=KR
NRL=7
HRLN=7.2
A=FX
RETURN
C
C RULE B - ON BRANCH BETWEEN DX & DP
800 IF (PGTPL) THEN
IF (KR.EQ.KS) GO TO 1200
GO TO 1300
ENDIF
IF (P.LE.FP) GO TO 900
S=KE
NRL=8
HRLN=8.1
A=FP
RETURN
C
C RULE 9 - ON BRANCH FP-I
900 IF (PGTPL) GO TO 1300
IF (P.LE.(-2*PY)) GO TO 1000
S=KT
NRL=9
HRLN=9.1
A=-2*PY
RETURN
C
C RULE 10 - ON THE COMPRESSION CURVE AFTER YELDING
1000 SEQ= 1. / ( 1./KC + (4./KR - 4./KC)*(PY/P)**2 )
IF (IDR) 1030,1030,1010
1010 S=K
NRL=10
HRLN=10.1
A=10*P
RETURN
1030 IF (ABS(P).LE.PT) GO TO 1100
S=K
NRL=10
HRLN=10.2
A=-PT
RETURN
C
C RULE 11 - UNLOADING BETWEEN I' & Q
1100 SEQ=KR
IF (ABS(P).GT.PT) GO TO 1000
IF ( P.GE.0.) GO TO 1200
S=KR
NRL=11
HRLN=11.1
A=0
IF (PLTPL) A=-PT
RETURN
C
C RULE 12 - LOADING FORM Q TO M
1200 IF (PLTPL) THEN
IF (KS.EQ.KR.AND. P.GT.FP) GO TO 800
GO TO 1300
ENDIF
IF ( P.GE.PMAX) GO TO 600
S=KS
NRL=12
HRLN=12.1
A=FXMAX
RETURN
C
C RULE 13 - LOADING AND UNLOADING INSIDE Q-M-D-P-I'-T'
1300 IF (KR.NE.KS) THEN
CALL INTSCT(DITENS,PITENS,D,P,KR,CMAX,FXMAX,KS)
CALL INTSCT(DICOMP,PICOMP,D,P,KR,DX ,FX,KS )
ELSE
IF (PGTPL) GO TO 1200
PITENS=D
DITENS=D
PICOMP=P
DICOMP=D
ENDIF
-----IS POINT WITHIN PY'-FP-PT' ?
IF (PICOMP.LE.FP) THEN
IF (KR.NE.KT) THEN
CALL INTSCT(DICOMP,PICOMP,D,P,KR,DP,FP,KT )
ELSE
IF (PLTPL) GO TO 900
PICOMP=P
DICOMP=D
ENDIF
IF (KR.EQ.KS .AND. PLTPL) GO TO 800
S2=KR
ENDIF
IF (PGTPL) THEN
IF (P.GT.PITENS) GO TO 1200
HRLN=13.1
ENDIF
IF (PLTPL) THEN
IF (P.LT.PICOMP) GO TO 800
A=PICOMP
HRLN=13.2
ENDIF
S=MAX(KR,S2)
NRL=13
RETURN
C
-----
C
C -----
C DEBUT UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
C END DEBUT
C -----
C -----
C SENDING HYSTERESIS MODEL.....
C -----
C -----
C INPUT VARIABLES
P = CURRENT LOAD
D = CURRENT DISPLACEMENT
PL = LAST LOAD
VEL = PRODUCT OF LAST AND CURRENT VELOCITY
NB = NUMBER OF BACKBONE CURVE POINTS
NI = MAXIMUM NUMBER OF SECONDARY LOOPS
PC = CRACKING LOAD
DC = CRACKING DISPLACEMENT
PB(J) = BACKBONE CURVE LOADING POINT J, J=1,NB
DB(J) = BACKBONE CURVE DISPLACEMENT POINT J, J=1,NB
SI = INITIAL STIFFNESS, PC/DC
C -----
C -----
C OUTPUT VARIABLES
.....AN INITIAL VALUE OF ZERO , OR FALSE

```

```

C
C..... SET MAXIMUM AND MINIMUM VALUES...
IF (DIR.EQ.1 .OR. DIR.EQ.2) THEN
  PM=PMG
  DM=DMG
ELSE IF (DIR.EQ.3 .OR. DIR.EQ.4) THEN
  PM=PMH
  DM=DMH
ENDIF

C..... CALCULATE EQUIVALENT UNLOADING STIFFNESS FOR ENERGY...
IF (BACKB .OR. RULE.EQ.1.0) THEN
  S1=FS1(DMAX)
  S2=FS2(DMAX)
  S3=FS3(DMAX)
  DO=DM - PM*(DMH-DMG)/(PMH-PMG)
  Q1=PM/4
  Q3=Q1
  DQ3=DM - 0.25*PM/S1
  S02=KMIN((DQ3-DO),Q3,S1)
  DQ1=DQ3-0.50*PM/MAX(S2,S02)
  S03=KMIN((DQ1-DO),Q1,S1)
  S0=Q3 + PM*2 / (
    (PM+Q3)*0.5*(DM-DQ3)
    + (Q3-Q1)*0.5*(DQ3-DQ1)
    + (Q1)*0.5*( Q1)/MAX(S3,S03) )
ENDIF

C..... CHECK TO SEE IF LAST RULE IS STILL IN EFFECT
IF (DIR.EQ.DIRL .AND.
  4 ((DIR.EQ.1 .AND. P.LT.A) .OR. (DIR.EQ.2 .AND. P.GT.A) .OR.
  6 ((DIR.EQ.3 .AND. P.GT.A) .OR. (DIR.EQ.4 .AND. P.LT.A))) THEN
  RETURN
ENDIF

C..... CALC EXCURSION RATIO AT EVERY ZERO CROSSING...
IF (P*PL.LE.C) CALL DNE(D,U,POS,EXCR,DT,DLI,ESS,PSE,PSOLD,CSE)

C..... ERASE SMALL LOOP FLAGS
IF (IR.GT.0) THEN
  DO 10 I=1,IT
  IF ((FR(I).EQ.'L' .AND. (P.LE.PR(I) .OR. D.LE.DR(I) )) .OR.
  6 ((FR(I).EQ.'7' .AND. (P.GE.PR(I) .OR. D.GE.DR(I) ))) THEN
    I=I-1
  GO TO 15
ENDIF
10 CONTINUE
ENDIF
15 I=IR+1

C
IF (DIR.EQ.1 .OR. DIR.EQ.3) THEN
  C..... LOADING RULES ..... RULES 1.6-11
C..... LOADING ON BACKBONE
  1 IF (BACKB .OR. RULE.EQ.0.0) THEN
    DO 18 J=2,NB
    DB=DB(J)-ABS(D)
    DP=PB(J)-AP
    IF (AP.LT.-PB(J) .AND. (DO*DP).GT.0.) THEN
      S=DP/DD
      IF (S.GT.K) THEN
        K=S
        A=SIGN(PB(J),P)
      ENDIF
    ENDIF
    18 CONTINUE
    RULE=1
    BACKB=.TRUE.
    IR=0
    IF (DIR.EQ.1) THEN
      ALPHA1=ALPHAA
    ELSE
      ALPHA2=ALPHAA
    ENDIF
  ENDIF

C..... LOADING AFTER UNLOADING
ELSE
  C..... RELOADING INSIDE SMALL LOOPS
  20 I=I-1
  IF (I.GT.0) THEN
    IF (.NOT.((FR(I).EQ.'7' .AND. DIR.EQ.1) .OR.
    6 ((FR(I).EQ.'L' .AND. DIR.EQ.3) ) GO TO 20HITS02480
    K=KMIN((D-DR(I)),(-P-PR(I)),S1)
    A=PR(I)
    RULE=I+1/1000.
  ENDIF

C..... INITIAL RELOADING BELOW PC/3
  C..... WITH REVERSAL
  ELSE IF (AP.LT.(PC/3)) THEN
    BACKB=.FALSE.
    6 IF ((DIR.EQ.1 .AND. DIRL.EQ.4) .OR.
    (DIR.EQ.3 .AND. DIRL.EQ.2)) THEN
      P2=.95*PM
      D2=FD2(DMAX)
      K=KMIN((D-D2),(-P-P2),S1)
      KB=K
      A=P2
      RULE=8
    ENDIF
    PC3=SIGN(PC/3,PM)
    D01=DM-PM*(1./FS1(DMAX)+2./FS2(DMAX)+1./FS3(DMAX))/4.
    D02=DM - PM*(DMH-DMG)/(PMH-PMG)
    IF (DIR.EQ.1) DO=MAX(D01,D02)
    IF (DIR.EQ.3) DO=MIN(D01,D02)
    DC3=DO+PC3/FS1(DMAX)
    S0=KMIN((D-DC3),(-P-PC3),S1)
    IF (S.GT.K) THEN
      K=S
      A=PC3
      RULE=8.1
    ENDIF
  ENDIF

C..... WITH REVERSAL, FAR SIDE LOADING
  6 IF ((DIR.EQ.1 .AND. D.LT.0) .OR.
  (DIR.EQ.3 .AND. D.GT.0)) THEN
    K=K8
    DO 40 J=2,NB
    IF (PB(J) .LT. ABS(PM)) GO TO 40
    DM=SIGN(DB(J),D)-D
    DP=-SIGN(PB(J),D)-P
    IF ((DP*DP).LE.D) GO TO 40
    S=DP/DD
    IF (S.GT.K .AND. S.LE.S1) THEN
      K=S
      A=SIGN(PB(J),PM)
      RULE=9
      BACKB=.TRUE.
    ENDIF
    40 CONTINUE
  ENDIF

C..... WITHOUT REVERSAL
  ELSE
    K=FS1(DMAX)
    A=SIGN(PC/3,PM)
    RULE=6
  ENDIF
ENDIF

C..... MIDRANGE RELOADING CURVE
  ELSE IF (AP.LT.ABS(.95*PM)) THEN
    BACKB=.FALSE.
    P2=.95*PM
    D2=FD2(DMAX)
    K=KMIN((D-D2),(-P-P2),S1)
    A=P2
    RULE=7
  ENDIF

C..... LOADING TOWARDS THE BACKBONE CURVE
  RULE 10HITS03140
  ELSE
    BACKB=.TRUE.
    IF (AP.GE.ABS(PM) .OR. (ABS(DM)).LE.DC) GO TO 1
    IR=0
    IF (DIR.EQ.1) THEN
      IF (ALPHA1.NE.ALPHAA .AND. ALPHA1.NE.ALPHAB)
        ALPHA1=ALPHAA
      DO=ALPHA1*DM-D
    ELSE
      IF (ALPHA2.NE.ALPHAA .AND. ALPHA2.NE.ALPHAB)
        ALPHA2=ALPHAA
      DO=ALPHA2*DM-D
    ENDIF
    DP=PM-P
    DD=DB(J)-DB(J0)
    IF (DP*DD).GT.0) THEN
      K=MIN((DP/DD),S1)
    ELSE
      K=S1
    ENDIF
    DO 50 J=2,NB
    IF (PB(J).LE.ABS(PM)) GO TO 50
    J0=J-1
    DD=DB(J)-DB(J0)
    DP=PB(J)-PB(J0)
    IF ((DP*DD).LE.0.) GO TO 50
    CALL INTSCT(DX,PK,D,P,K,SIGN(DB(J),P),
    SIGN(PB(J),P),(DP/DD))
    PK=PK
    DX=DX
    IF (ABS(DX).GE.DB(J0) .AND. ABS(DX).LE.DB(J) .AND.
    6 ABS(PK).GT.ABS(PM) .AND. PM*PK.GT.0) THEN
      IF (AP.GT.ABS(PK)) GO TO 1
      A=PK
      RULE=10
      IF (DIR.EQ.1) THEN
        ALPHA1=ALPHAB
      ELSE
        ALPHA2=ALPHAB
      ENDIF
      GO TO 100
    ENDIF
    50 CONTINUE
    GO TO 1
  ENDIF
ENDIF

C
C..... UNLOADING RULES ..... RULES 2-4
C..... INITIALIZE VALUES
  BACKB=.FALSE.
  S1=FS1(DMAX)
  S2=FS2(DMAX)
  S3=FS3(DMAX)
  DO=DM - PM*(DMH-DMG)/(PMH-PMG)
  Q1=PM/4
  Q3=Q1
  DQ3=DM - 0.25*PM/S1
  S02=KMIN((DQ3-DO),Q3,S1)
  DQ1=DQ3-0.50*PM/MAX(S2,S02)
  S03=KMIN((DQ1-DO),Q1,S1)
  S0=Q3 + PM*2 / (
    (PM+Q3)*0.5*(DM-DQ3)
    + (Q3-Q1)*0.5*(DQ3-DQ1)
    + (Q1)*0.5*( Q1)/MAX(S3,S03) )
ENDIF

C..... UNLOADING INSIDE SMALL POSITIVE LOOPS
  70 I=I-1
  IF (I.GT.0) THEN
    IF (.NOT.((DIR.EQ.2 .AND. FR(I).EQ.'L') .OR.
    6 ((DIR.EQ.4 .AND. FR(I).EQ.'7')) GO TO 70
    IF ((DIR.EQ.2 .AND. (P.GT.Q3 .AND. PR(I).GT.Q3) .OR.
    6 (Q3.GE.P .AND. P.GT.Q1 .AND. PR(I).GT.Q1) .OR.
    6 (Q1.GE.P .AND. PR(I).GT.0) ) .OR.
    6 ((DIR.EQ.4 .AND. (P.LT.Q3 .AND. PR(I).LT.Q3) .OR.
    6 (Q3.LE.P .AND. P.LT.Q1 .AND. PR(I).LT.Q1) .OR.
    6 (Q1.LE.P .AND. PR(I).LT.0) )) THEN
      K=(PR(I)-P)/(DR(I)-D)
      IF (K.LE.0 .OR. K.GT.S1) GO TO 70
      A=PR(I)
      RULE=5+I/1000.
    ELSE
      SU=KMIN((DR(I)-D),(PR(I)-P),S1)
    ENDIF
  ENDIF

C..... CALC DUCTILITY AND EXCURSION RATIO...
  IF (RULE.EQ.1 .OR. RULE.GT.5) THEN
    IF (DIR.EQ.2) THEN
      DLI=MAX(D,DL)
    CALL DNE(1,U,POS,EXCR,DT,DLI,ESS(2),PSE,PSOLD,CSE)
    ELSE IF (DIR.EQ.4) THEN
      DLI=MIN(D,DL)
    CALL DNE(1,UNEG,EXCR,-DT,DLI,ESS(3),PSE,PSOLD,CSE)
    ENDIF
  ENDIF

C..... UNLOADING ON THE TOP SEGMENT
  RULE 2HITS04130
  IF (AP.GT.ABS(Q3)) THEN
    K=MAX(S1,SU)
    A=Q3
    RULE=2
  ENDIF

C..... UNLOADING ON THE MIDDLE SEGMENT
  RULE 3HITS04180
  ELSE IF (AP.GT.ABS(Q1)) THEN
    K=MAX(S2,S02,SU)
    A=Q1
    RULE=3
  ENDIF

C..... UNLOADING ON THE BOTTOM SEGMENT
  RULE 4HITS04240
  ELSE
    K=MAX(S3,S03,SU)
    A=0.
    RULE=4
  ENDIF
ENDIF

C..... SAVE REVERSAL POINTS FOR SMALL LOOPS.....
  100 I=I-1
  REVERL=RULE.EQ.1 .OR. RULE.EQ.10 .OR. RULE.EQ.1 .OR. RULE.EQ.10
  IF ((DIRL.EQ.1 .AND. DIR.EQ.2 .AND. .NOT. REVERL)
  6 .OR. (DIRL.EQ.4 .AND. DIR.EQ.3) ) THEN
    IF (IR.LT.N1) THEN
      I=IR-1
      PR(IR)=P
      DR(IR)=D
      FR(IR)=7
    ENDIF
    ELSE IF ((DIRL.EQ.3 .AND. DIR.EQ.4 .AND. .NOT. REVERL)
    6 .OR. (DIRL.EQ.2 .AND. DIR.EQ.1) ) THEN
      IF (IR.LT.N1) THEN
        I=IR-1
        PR(IR)=P
        DR(IR)=D
        FR(IR)=7
      ENDIF
    ENDIF
  ENDIF
ENDIF

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ENDIF
IF (PRINT) WRITE (6,1040) S1,S2,S3
1040 FORMAT(' S1=',G13.5,' S2=',G13.5,' S3=',G13.5,' ',G13.5)
IF ((1.05*S1).GT.K.AND.K.GT.0) RETURN
K=0
RULE=RULE
RETURN
END
-----
C SHEAR HYSTERESIS MODEL HYST04
-----
C DEBUG UNIT(6),TRACE,SUBCHN,INIT,SUBTRACE
C END DEBUG
-----
C INPUT VARIABLES
C P = CURRENT LOAD
C D = CURRENT DISPLACEMENT
C PL = LAST LOAD
C VEL = PRODUCT OF LAST AND CURRENT VELOCITY
C NS = NUMBER OF BACKBONE CURVE POINTS
C NI = MAXIMUM NUMBER OF SECONDARY LOOPS
C PC = CRACKING LOAD
C DC = CRACKING DISPLACEMENT
C SI = INITIAL STIFFNESS PC/DC
C PS = BACKBONE CURVE LOADING POINTS
C DS = BACKBONE CURVE DISPLACEMENT POINTS
C OUTPUT VARIABLES
C AN INITIAL VALUE OF ZERO IS REQUIRED FOR ALL OUTPUT VARIABLES...
C K = STIFFNESS
C RULE = RULE NUMBER, STORED IN THE FOR_MAT RN,DIR UPON EXIT OF THIS SUBROUTINE
C A = LOAD LIMIT BEFORE NEXT RULE CHANGE
C PMG = MAXIMUM PAST LOAD IN THE POSITIVE DIRECTION
C PMH = MAXIMUM PAST LOAD IN THE NEGATIVE DIRECTION
C DMG = MAXIMUM PAST DISPLACEMENT IN THE POSITIVE DIRECTION
C DMH = MAXIMUM PAST DISPLACEMENT IN THE NEGATIVE DIRECTION
C PR = SECONDARY LOOP LOADING POINT
C DR = SECONDARY LOOP DISPLACEMENT POINT
C FR = SECONDARY LOOP FLAG
C IR = SECONDARY LOOP NUMBER
C BACKB = BACKBONE LOADING FLAG
C SR1 = RELOADING STIFFNESS FROM RULE 8 - USED BY RULE 11
C SR2 = RELOADING STIFFNESS FROM RULE 9 - USED BY RULE 11
C SR3 = RELOADING STIFFNESS FROM RULE 7 - USED BY RULE 11
C SRM = RELOADING STIFFNESS FROM RULES 8 & 9 - USED BY RULE 11
C INTERNAL VARIABLES
C J = BACKBONE CURVE POINT # IN THE CURRENT DIRECTION
C DIR = CURRENT DIRECTION
C DIR=1, POSITIVE LOADING
C DIR=2, POSITIVE UNLOADING
C DIR=3, NEGATIVE LOADING
C DIR=4, NEGATIVE UNLOADING
C DIRL = LAST DIRECTION
C IRULE = INTEGER VALUE OF RULE
C LRULE = INTEGER VALUE OF LAST RULE #
C CRACKD = CRACKED WALL FLAG
C REVRSL = LOAD REVERSAL FLAG
C PRINT = PRINT FLAG USED FOR DEBUGGING
C PC2 = PC/2 USED BY LOADING CURVES
C DC2 = DISPLACEMENT CORRESPONDING TO PC2 ON THE LOADING BRANCH
C SR = INTERMEDIATE STIFFNESS
C SRP = MINIMUM RELOADING STIFFNESS
C DC = DISPLACEMENT INTERSEPT OF PEAK DISPLACEMENTS
C DOP = DISPLACEMENT INTERSEPT OF THE UNLOADING BRANCH
C AP = ABSOLUTE VALUE OF CURRENT LOAD P
C PM = MAXIMUM PAST LOAD IN THE CURRENT DIRECTION
C DM = MAXIMUM PAST DISPLACEMENT IN THE CURRENT DIRECTION
C DMAX = MAXIMUM PAST DISPLACEMENT IN EITHER DIRECTION
C PMAX = MAXIMUM PAST LOAD IN EITHER DIRECTION
C PA = PC, USED BY UNLOADING CURVES
C PB = PC/2, USED BY UNLOADING CURVES
C DA = DISPLACEMENT AT PA, USED BY UNLOADING CURVES
C DB = DISPLACEMENT AT PB, USED BY UNLOADING CURVES
C DP = DIFFERENCE IN LOAD, USED FOR CALCULATING K
C DD = DIFFERENCE IN DISPLACEMENT, USED FOR CALCULATING K
C PX = LOAD INTERCEPT OF LOADING AND BACKBONE CURVE
C DX = DISPLACEMENT INTERCEPT OF LOADING AND BACKBONE CURVE
C PD2,D = POINT ON THE LOADING CURVE BETWEEN RULES 7 AND 10
C X = MAXIMUM DISPLACEMENT
C X1 = DISPLACEMENT INTERCEPT FOR RULE 11
C X2 = 75% PC DISPLACEMENT FOR RULE 11
C X3 = 25% PC DISPLACEMENT FOR RULE 11
C I,IT = TEMPORARY LABEL FOR SMALL LOOP LOADING AND UNLOADING POINTS
C J = J-1
C ALPHA = DEGRADING STIFFNESS FACTOR
C ALPHAP = DEGRADING STIFFNESS FACTOR USED BY RULE 10
C S1 = STIFFNESS FROM FUNCTION FS1
C S2 = STIFFNESS FROM FUNCTION FS2
C S3 = STIFFNESS FROM FUNCTION FS3
C SU = STIFFNESS BETWEEN CURRENT AND RELOADING POINT
C SO2 = STIFFNESS BETWEEN PA AND DO
C SO3 = STIFFNESS BETWEEN PB AND DO
C INTERNAL FUNCTIONS
C FD2 = DISPLACEMENT INTERCEPT OF LOADING AND UNLOADING CURVE
C FS1 = UNLOADING STIFFNESS WHEN PA>P*(PC/2)=PA
C FS2 = UNLOADING STIFFNESS WHEN PA>P*(PC/2)=PB
C FS3 = UNLOADING STIFFNESS WHEN P>PB
C FSR = LOADING BELOW PC/2, WITH PINCHING
C SUBROUTINES CALLED
C DIRECT = DETERMINES THE VALUES OF DIR AND DIRL
C INTSECT = DETERMINES THE INTERSECTION OF TWO LINES IN POINT-SLOPE FORM
C KMIN = DETERMINES THE STIFFNESS BASED ON DP,DD AND K_DEFAULT
C SUBROUTINE HYST04(P,D,PL,DL,VEL,NS,NI,PRINT,
& DC,PC,DS,SI,CSE,DT
& K,RULE,DIR,A,PMG,PMH
& DMG,DMH,BACKB,SR1,SR2,SR3,
& SRM,IR,SE,UPOS,UNEG,EXCR
& SSE,PSE,PSOOLD,PR,DR,FR)
C IMPLICIT REAL(A-H,K,O-Z)
C IMPLICIT INTEGER(I,J,L,N)
C INTEGER DIR,DIRL
C LOGICAL BACKB,CRACKD,REVRSL,PRINT,BTEST
C CHARACTER*4 FR(NI)
C DIMENSION PS(NS),DS(NS),PR(NI),DR(NI)
C DIMENSION UPOS(3),UNEG(3),EXCR(6),ESE(3)
C DATA ALPHA/1.04/
C SYMMETRIC COEFFICIENTS
CC FS1(X) = S1 * MIN(1.03 * (DC/X)**.388 ,1.)
CC FS2(X) = S1 * MIN(1.036 * (DC/X)**.458 ,1.)
CC FS3(X) = S1 * MIN(1.0789 * (DC/X)**.692 ,1.)
C UNSYMMETRIC COEFFICIENTS
C FS1(X) = S1 * MIN(1.4675 * (DC/X)**.343 ,1.)
C FS2(X) = S1 * MIN(0.714 * (DC/X)**.529 ,1.)
C FS3(X) = S1 * MIN(0.0707 * (DC/X)**1.369 ,1.)
C FSR(X) = S1 * MIN((ABS(DC/X))**1.02 ,1.)
C FD2(X) = SIGN(DMAX,PM) - .05 * SIGN(PMAX,PM) / (FS1(K))

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1000 IF (PRINT) WRITE (6,1010) P,O,PL,DL,NS,NI,PC,DC,SI,K,RULE,A,
& PMG,PMH,DMG,DMH,SR1,SR2,SR3,SRM,BACKB,IR,PSOOLD,VEL,
& SE,DT,ESE(1),PSE,UPOS,CSE,UNEG,EXCR
1010 FORMAT(' P=',G13.5,' O=',G13.5,' PL=',G13.5,' DL=',G13.5,'
& SI=',G13.5,' NI=',G13.5,' PC=',G13.5,' DC=',G13.5,'
& S1=',G13.5,' K=',G13.5,' RULE=',G13.5,' A=',G13.5,'
& SI=',G13.5,' PMH=',G13.5,' DMG=',G13.5,' DMH=',G13.5,'
& SR1=',G13.5,' SR2=',G13.5,' SR3=',G13.5,' SRM=',G13.5,'
& SI=BACKB=',G13.5,' IR=',G13.5,' PSECO=',G13.5,' VEL=',G13.5,'
& SI=SE=',G13.5,' DT=',G13.5,' ESE=',G13.5,' PSE=',G13.5,'
& SI=UP1=',G13.5,' UP2=',G13.5,' UP3=',G13.5,' CSE=',G13.5,'
& SI=UN1=',G13.5,' UN2=',G13.5,' UN3=',G13.5,'
& SI=EX1=',G13.5,' EX2=',G13.5,' EX3=',G13.5,'
& SI=EX4=',G13.5,' EX5=',G13.5,' EX6=',G13.5')
IF (IR.GT.0.AND.PRINT) WRITE (6,1020) (I,PR(I),OR(I),FR(I),I=1,IR)
1020 FORMAT(' I=',G13.5,' PR=',G13.5,' DR=',G13.5,' FR=',A)
AP=ABS(P)
C ..... DETERMINE IF ELASTIC
IF (RULE.EQ.0 .AND. AP .LT. PC) THEN
K=S1
SE=K
GO TO 9999
ENDIF
C ..... DETERMINE IF WALL IS INACTIVE, IF SO RETURN
IF (RULE.LT.0.0) GO TO 9999
C ..... DETERMINE CURRENT DIRECTION
LRULE=INT(RULE)
DIR =RDIR
CALL DIRECT(DIR,DIRL,P,PL,VEL)
RDIR =DIR
IF (PRINT) WRITE (6,1030) DIR,DIRL
1030 FORMAT(' DIR=',I8,' DIRL=',I8)
C ..... DETERMINE MAXIMUM AND MINIMUM VALUES
PMG=MAX(PMG,PC,P)
DMG=MAX(DMG,DC,D)
PMH=MIN(PMH,-PC,P)
DMH=MIN(DMH,-DC,D)
DMAX=MAX(DMG,-DMH)
PMAH=MAX(PMG,-PMH)
C ..... SET MAXIMUM AND MINIMUM VALUES...
IF (DIR.EQ.1 .OR. DIR.EQ.2) THEN
PM=PMG
DM=DMG
ELSE IF (DIR.EQ.3 .OR. DIR.EQ.4) THEN
PM=PMH
DM=DMH
ENDIF
C ..... CALCULATE EQUIVALENT UNLOADING STIFFNESS FOR ENERGY...
IF (BACKB .OR. RULE.EQ.0.0) THEN
SI=FS1(DMAX)
S2=FS2(DMAX)
S3=FS3(DMAX)
DO=DM-PM*(DMG-DMH)/(PMG-PMH)
PA=PM-SIGN(PC,PM)
DA=DM-SIGN(PC,PM)/SI
PB=SIGN((MIN(ABS(PA),(PC/2))),PM)
SO2=DMH*(DA-DO)/PA,SI)
DB=DA-(PA-PB)/MAX(S2,SO2)
SO3=MIN((DB-DO),PB,SI)
SE= 0.5 * PM**2 /
& ((DM+PA)*PM*.5*(DM+DA)
& + (PA+PB)*0.5*(DA+DB)
& + (PB)*0.5*(PB)/MAX(S3,SO3) )
ENDIF
C ..... CHECK TO SEE IF LAST RULE IS STILL IN EFFECT
IF (DIR.EQ.DIRL .AND.
& ((DIR.EQ.1 .AND. P.LT.A) .OR. (DIR.EQ.2 .AND. P.GT.A) .OR.
& (DIR.EQ.3 .AND. P.GT.A) .OR. (DIR.EQ.4 .AND. P.LT.A))) THEN
RETURN
ENDIF
C ..... CALL EXCURSION RATIO AT EVERY ZERO CROSSING...
IF (P*PL.LE.0) CALL DNE(O,UPOS,EXCR,DT,DI,ESE,PSE,PSOOLD,CSE)
C ..... ERASE SMALL LOOP FLAGS
IF (IR.GT.0) THEN
IT=IR
DO 10 I=1,IT
IF ((FR(I).EQ.'L' .AND. (P.LE.PR(I) .OR. D.LE.DR(I))) .OR.
& (FR(I).EQ.'U' .AND. (P.GE.PR(I) .OR. D.GE.DR(I)))) THEN
FR(I)=I
GO TO 15
ENDIF
10 CONTINUE
ENDIF
15 I=IR+1
C IF (DIR.EQ.1 .OR. DIR.EQ.3) THEN
C ..... LOADING RULES ..... RULES 1,5-11
PC2=SIGN((PC/2),PM)
C ..... LOADING ON BACKBONE
RULE 1
30 IF (BACKB .OR. RULE.EQ.0.0) THEN
DO 40 J=2,NS
DM=DS(J)-ABS(D)
DP=PS(J)-AP
IF (AP.LT.PS(J) .AND. (DD*DP).GT.0.0) THEN
S=DP/DD
IF (S.GT.K .AND. S.LE.SI) THEN
K=S
A=SIGN(PS(J),P)
ENDIF
ENDIF
40 CONTINUE
RULE=1
BACKB=TRUE.
IWO=0
C ..... LOADING AFTER UNLOADING
RULES 6-11
ELSE
CRACKD=(ABS(DM).GT.DC)
REVRSL=((DIR.EQ.1 .AND. DIRL.EQ.4) .OR. LRULE.EQ.8
& .OR. (DIR.EQ.3 .AND. DIRL.EQ.2) .OR. LRULE.EQ.9
& .OR. LRULE.EQ.5 .OR. LRULE.EQ.11)
IF (CRACKD) THEN
P2=.95*SIGN(PMAX,PM)
D2=PD2(DMAX)
ELSE
P2=PM
D2=DM
ENDIF
S=KMIN((D2-D),(P2-P),SI)
I=I-1
C ..... RELOADING INSIDE SMALL LOOPS
RULE 11
IF (I.GT.0) THEN
IF (.NOT. ((FR(I).EQ.'U' .AND. DIR.EQ.1) .OR.
& (FR(I).EQ.'L' .AND. DIR.EQ.3))) GO TO 20
IF (SR1.LE.0 .OR. SR2.LE.0 .OR. SR3.LE.0 .OR.
& SRM.LE.0) THEN
I=1

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GO TO 20
ENDIF
IF (AP.LT.ABS(PC2) .AND. S.GT.SRM) THEN
  REVRSL=.FALSE.
  I=1
  GO TO 20
ENDIF
IF (ABS(PR(I)).LE. PC/4) THEN
  K=KMIN((DR(I)-D),(P-PR(I)),SI)
  A=PR(I)
  RULE=1.1+I/1000.
ELSE IF (ABS(PR(I)).LE.3*PC/4) THEN
  X2=D2-(P2-1.5*PC2)/SR3
  X1=X2-PC2/SR2
  I=D*(PC2/2-P)/SR2
  IF (AP.LT.PC/4 .AND. ((DIR.EQ.1 .AND. X.LT.X1).OR.
    (DIR.EQ.3 .AND. X.GT.X1))) THEN
    K=KMIN((X1-D),(PC2/2-P),SI)
    A=SIGN((PC/4),PM)
    RULE=1.2+I/1000.
  ELSE
    K=KMIN((DR(I)-D),(PR(I)-P),SI)
    A=PR(I)
    RULE=1.3+I/1000.
  ENDIF
ELSE
  K2=DR(I)-(PR(I)-1.5*PC2)/SRM
  X1=X2-PC2/SR2
  X=D*(PC2/2-P)/SR2
  IF (AP.LT.PC/4 .AND. ((DIR.EQ.1 .AND. X.LT.X1).OR.
    (DIR.EQ.3 .AND. X.GT.X1))) THEN
    K=KMIN((X1-D),(PC2/2-P),SI)
    A=SIGN((PC/4),PM)
    RULE=1.4+I/1000.
  ELSE
    X=D*(1.5*PC2-P)/SR3
    IF (AP.LT.3*PC/4 .AND.
      ((DIR.EQ.1 .AND. X.LT.X2)
      .OR. (DIR.EQ.3 .AND. X.GT.X2))) THEN
      K=KMIN((X2-D),(1.5*PC2-P),SI)
      A=SIGN((3*PC/4),PM)
      RULE=1.5+I/1000.
    ELSE
      SR=SRM
      K=KMIN((DR(I)-D),(PR(I)-P),SI)
      A=PR(I)
      RULE=1.6+I/1000.
    ENDIF
  ENDIF
ENDIF
INITIAL RELOADING BELOW PC/2 W/O REVERSAL
ELSE IF (AP.LT.ABS(PC2) .AND. .NOT.REVRSL) THEN
  K=FS1(DMAX)
  A=PC2
  RULE=6
ENDIF
RELOADING TO P2
ELSE IF (AP.LT.ABS(P2) .AND.
  (.NOT.REVRSL .OR. AP.GE.PC*0.75)) THEN
  K=S
  A=P2
  RULE=7
ENDIF
RELOADING BELOW PC WITH REVERSAL
ELSE IF (AP.LT.PC*0.75) THEN
  SR=FSR(DMAX)
  S1=FS1(DMAX)
  S2=FS2(DMAX)
  S3=FS3(DMAX)
  PA=SIGN(PMAX,PM)-SIGN(PC,PM)
  DA=SIGN(DMAX,PM)-SIGN(PC,PM)/S1
  PB=SIGN((MIN(ABS(PA),(PC/2)),PM)
  DB=DA-(PA-PB)/S2
  DDP=DB-(PB)/S3
  DDM=DM*(DMG-DMH)/(PMG-PMH)
  IF (DIR.EQ.1) DDP=MAX(DDP,DD)
  IF (DIR.EQ.3) DDP=MIN(DDP,DD)
  DC2=DDP+PC2/S1
  SRM=KMIN((DC2-D2),(PC2-P2),SI)
  IF (S.GT.SRM) THEN
    REVRSL=.FALSE.
    GO TO 20
  ENDIF
  SR=KMIN((DC2-D),(PC2-P),SI)
  S1=MAX(SR,MIN(SR,S1))
  DC2P=D*(PC2-P)/SR1
  SR3=KMIN((D2-DC2P),(P2-PC2),SI)
  SR2=DD/(1./SR1+1./SR3)
  IF (AP.LT.PC/4) THEN
    K=SR1
    A=SIGN((PC/4),PM)
    RULE=9
  ELSE
    K=SR2
    A=SIGN(PC,PM)*0.75
    RULE=9
  ENDIF
LOADING TOWARDS THE BACKBONE CURVE
BACKB=.TRUE.
IF (.NOT.CRACKD) GO TO 30
IR=0
ALPHAP=1.0
IF (ABS(DM).EQ.ABS(DMAX)) ALPHAP=ALPHA
K=KMIN((ALPHAP*SIGN(DMAX,D2)-D2),(SIGN(PMAX,P2)-P2)
  (SI)
DO 50 J=2,NS
  IF (.PS(J).LE.ABS(PMAX)) GO TO 50
  J=J-1
  DD=DS(J)-DS(J0)
  DP=PS(J)-PS(J0)
  IF ((DP+DD).LE.0.) GO TO 50
  CALL INTCT(DL,PK,D,P,K,SIGN(DS(J),P),
    SIGN(PS(J),P),(DP/DD))
  IF (ABS(DX).GE.DS(J0) .AND. ABS(DX).LE.DS(J).AND.
    ABS(PX).GT.ABS(PMAX).AND. PM*PX.GT.0) THEN
    IF (AP.GE.ABS(PX)) GO TO 30
    K=KMIN((DX-D),(PR-P),SI)
    A=PX
    RULE=10
    GO TO 100
  ENDIF
CONTINUE
GO TO 30
ENDIF
ENDIF
ELSE
UNLOADING RULES
INITIALIZE VALUES
BACKB=.FALSE.
S1=FS1(DMAX)
S2=FS2(DMAX)
S3=FS3(DMAX)
DO=DM-PM*(DMG-DMH)/(PMG-PMH)
PA=PM-SIGN(PC,PM)
DA=DM-SIGN(PC,PM)/S1
PB=SIGN((MIN(ABS(PA),(PC/2)),PM)

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C U3 HTS00510 NRL=5 HTS01930
C QT HTS00520 HRLN=2.2 HTS01940
C MAXFLG FLAG, IF MAXFLG=0 STIFFNESS=0 BEYOND FAILURE POINT (DU,PU) HTS00530 GO TO 2000 HTS01950
C ACHNG LOAD WHERE RULE CHNGS IF LOADING CONTINUES IN CURR DIRECTION HTS00540 C ***** TAKEDA RULE 3.0 ***** LOADING ON PRIMARY CURVE AFTER YIELD HTS01960
C PRINT PRINT DATA POST TRACING HTS00550 300 BOT*MAX(ABS(DI),ABS(UM1),ABS(UM3)) HTS01970
C SE ELASTIC UNLOADING CURVE HTS00560 S1=CPT*SQRT(DY/BOTT) HTS01980
C SSE ELASTIC STRAIN ENERGY HTS00570 SE=S1 HTS01990
C PSE PLASTIC STRAIN ENERGY HTS00580 IF (IDR) J30,J30,J10 HTS02000
C PSEOLD PLASTIC STRAIN ENERGY AT LAST ZERO CROSSING HTS00590 310 IF (ABS(PI),GE,PU) GO TO 320 HTS02010
C UPOS POSITIVE DUCTILITY INFO. HTS00600 A=PU*ISGN HTS02020
C UNEG NEGATIVE DUCTILITY INFO. HTS00610 B=PI HTS02030
C EXCR EXCURSION RATIO INFO. HTS00620 S=FU HTS02040
C HTS00630 NRL=3 HTS02050
C HTS00640 HRLN=3.11 HTS02060
C # FOR CURRENT DIRECTION FOR LOADING AND UNLOADING HTS00650 GO TO 2000 HTS02070
C # FOR NEW DIRECTION FOR REVERSAL HTS00660 320 A=PU*ISGN HTS02080
C # = 2 FOR POSITIVE LOAD, # = 1 FOR NEGATIVE LOAD HTS00670 B=PI HTS02090
C IREVSIG SIGN OF LOAD IN NEW DIRECTION HTS00680 S=0 HTS02100
C ISGN SIGN OF THE CURRENT LOAD FOR LOADING AND UNLOADING HTS00690 IF (MAXFLG,NE,0) S=TU HTS02110
C SIGN OF THE NEW LOAD FOR REVERSAL HTS00700 NRL=3 HTS02120
C HTS00710 HRLN=3.12 HTS02130
C HTS00720 GO TO 2000 HTS02140
C SUBROUTINE HTS006(PI,DI,PS,DS,VI,VS,S,HRLN,A,B,IDRO,RP, HTS00730 330 B=0 HTS02150
C # PC,DC,PT,DT,DU,DC,CT,PU,DU,OC,CT,PU,CPT,CSE,RNRL,UM1,UM2,UM3,UM4, HTS00740 A=PI HTS02160
C # S1,X0,XOUM,XOT,UOD,UO,XIUM,U1,U1D,X2UO,U2,X3U1,U3,QT,MAXFLG, HTS00750 BOT*MAX(ABS(DI),ABS(UM1),ABS(UM3)) HTS02170
C # ACHNG,PRINT,SE,SEB,PSE,PSEOLD,UPOS,UNEG,EXCR) HTS00760 S1=CPT*SQRT(DY/BOTT) HTS02180
C HTS00770 S=S1 HTS02190
C REAL UM(2,2),IDRO,MAXFLG,UPOS(3),UNEG(3),EXCR(6),ESE(3) HTS00780 NRL=4 HTS02200
C LOGICAL PRINT,BTEST HTS00790 HRLN=3.2 HTS02210
C 9000 IF (PRINT) WRITE (6,8010) PI,DI,PS,DS,VI,VS,S,HRLN,A,B,RP, HTS00800 GO TO 2000 HTS02220
C # PC,DC,PT,DT,DU,DC,CT,PU,DU,OC,CT,PU,CPT,CSE,RNRL,UM1,UM2,UM3,UM4, HTS00810 C ***** TAKEDA RULE 4.0 ***** UNLOADING FROM POINT UM ON THE PRIMARY HTS02240
C # S1,X0,XOUM,XOT,UOD,UO,XIUM,U1,U1D,X2UO,U2,X3U1,U3,QT, HTS00820 CURVE AFTER YIELDING HTS02250
C # ACHNG,IDRO,MAXFLG,CSE, HTS00830 400 IF (IDR) 430,440,410 HTS02260
C # SE,ESE(1),PSE,PSEOLD,UPOS,UNEG,EXCR HTS00840 410 IF (ABS(PI),GE,ABS(UM(JI,2))) GO TO 420 HTS02270
C 8010 FORMAT( HTS00850 A=UM(JI,2) HTS02280
C # TAKEDA-PI=,G12.5, DI=,G12.5, PS=,G12.5, DS=,G12.5/HTS00860 B=PI HTS02290
C # TR- VI=,G12.5, VS=,G12.5, S=,G12.5, HRLN=,G12.5/HTS00870 S=S1 HTS02300
C # TR- A=,G12.5, B=,G12.5, RP=,G12.5, PC=,G12.5/HTS00880 NRL=4 HTS02310
C # TR- DC=,G12.5, DT=,G12.5, DU=,G12.5, PU=,G12.5/HTS00890 GO TO 2000 HTS02320
C # TR- DU=,G12.5, DC=,G12.5, CT=,G12.5, TU=,G12.5/HTS00900 420 A=PU*ISGN HTS02330
C # TR- CPT=,G12.5, RNRL=,G12.5, UM1=,G12.5, UM2=,G12.5/HTS00910 B=PI HTS02340
C # TR- UM3=,G12.5, UMA=,G12.5, X0=,G12.5, X1=,G12.5/HTS00920 S=FU HTS02350
C # TR- XOUM=,G12.5, XOT=,G12.5, UOD=,G12.5, UO=,G12.5/HTS00930 HTS02360
C # TR- XIUM=,G12.5, U1=,G12.5, U1D=,G12.5, X2UO=,G12.5/HTS00940 NRL=3 HTS02370
C # TR- U2=,G12.5, X3U1=,G12.5, U3=,G12.5, QT=,G12.5/HTS00950 HRLN=4.12 HTS02380
C # TR- ACHNG=,G12.5, IDRO=,G12.5, MAXFLG=,G12.5, CSE=,G12.5/HTS00960 GO TO 2000 HTS02390
C # TR- SE=,G12.3, ESE=,G12.5, PSE=,G12.5, PSEOLD=,G12.5/ HTS00970 A=PI HTS02400
C # TR- UPI=,G12.3, UPI=,G12.5, UPI=,G12.5/ HTS00980 B=0 HTS02410
C # TR- UM1=,G12.3, UM2=,G12.5, UM3=,G12.5/ HTS00990 S=S1 HTS02420
C # TR- EX1=,G12.3, EX2=,G12.5, EX3=,G12.5/ HTS01000 NRL=4 HTS02430
C # TR- EX4=,G12.3, EX5=,G12.5, EX6=,G12.5/ HTS01010 HRLN=4.2 HTS02440
C C..... RPN IF CURRENT STATE = PREVIOUS STATE HTS01020 GO TO 2000 HTS02450
C IF (PI.EQ.PS .OR. DI.EQ.DS) .AND. RNRL.NE.0 ) RETURN HTS01030 IF (DI.LE.DS) IDRVO=1 HTS02460
C HTS01040 IF (DI.GT.DS) IDRVO=2 HTS02470
C C..... GET DIRECTION AND RPN IF CURRENT STATE IS WITHIN LIMITS... HTS01050 IF (ABS(UM(IDRVO,1)),GT,DC) GO TO 450 HTS02480
C CALL IDRECT (IDR,IDRV,DRVO,PI,PS,VI,VS) HTS01060 A=PC*IREVSL HTS02490
C IF (A.GT.B .AND. PI.LE.A .AND. PI.GT.B .AND. IDR.EQ.IDRO) RETURN HTS01070 B=0 HTS02500
C IF (A.LT.B .AND. PI.GT.A .AND. PI.LT.B .AND. IDR.EQ.IDRO) RETURN HTS01080 S=S1 HTS02510
C C..... CALC EXCURSION RATIO AT EVERY ZERO CROSSING... HTS01090 NRL=15 HTS02520
C IF (PI*PS.LE.0)CALL DNE(G,UPOS,EXCR, DY,DI,ESE,PSE,PSEOLD,CSE) HTS01100 GO TO 2000 HTS02530
C C..... INITIALIZE DATA FOR FIRST CALL TO SUBROUTINE HTS01110 X0=DI HTS02540
C 1 IF (RNRL.EQ.0) THEN HTS01120 XOUM=(0.-UM(IDRVO,2))/(XO-UM(IDRVO,1)) HTS02550
C UM1=DC HTS01130 XOT=(0.-PT*ISGN)/(XO-DT*ISGN) HTS02560
C UM2=PC HTS01140 XOT=(0.-PT*ISGN)/(XO-DT*ISGN) HTS02570
C UM3=DC HTS01150 IF (DY.GT.ABS(UM(IDRVO,2)) .AND. XOT.GT.XOUM) GO TO 460 HTS02580
C UM4=PC HTS01160 A=UM(IDRVO,2) HTS02590
C ENDF HTS01170 B=0 HTS02600
C HTS01180 S=XOUM HTS02610
C HTS01190 S=SE HTS02620
C HTS01200 NRL=6 HTS02630
C HTS01210 HRLN=4.32 HTS02640
C C..... INITIALIZE DATA HTS01220 GO TO 2000 HTS02650
C NRL=RNRL HTS01230 UM(JI,1)=DT*ISGN HTS02660
C UM(1,1)=UM1 HTS01240 UM(JI,2)=PT*ISGN HTS02670
C UM(1,2)=UM2 HTS01250 A=UM(JI,2) HTS02680
C UM(2,1)=UM3 HTS01260 B=0 HTS02690
C UM(2,2)=UM4 HTS01270 S=XOT HTS02700
C HTS01280 S=SE HTS02710
C HTS01290 NRL=6 HTS02720
C 901 IF (PI.GT.0) THEN HTS01300 HRLN=4.33 HTS02730
C ISGN=1 HTS01310 GO TO 2000 HTS02740
C HTS01320 C ***** TAKEDA RULE 5.0 ***** UNLOADING FROM POINT UM ON THE PRIMARY HTS02750
C HTS01330 CURVE BEFORE YIELDING HTS02760
C HTS01340 500 IF (IDR) 530,540,510 HTS02770
C HTS01350 510 IF (ABS(PI),GE,ABS(UM(JI,2))) GO TO 700 HTS02780
C HTS01360 A=UM(JI,2) HTS02790
C HTS01370 B=PI HTS02800
C HTS01380 S=S1 HTS02810
C HTS01390 NRL=5 HTS02820
C HTS01400 HRLN=5.11 HTS02830
C HTS01410 GO TO 2000 HTS02840
C HTS01420 530 A=PI HTS02850
C HTS01430 B=0 HTS02860
C HTS01440 S=PI HTS02870
C HTS01450 NRL=5 HTS02880
C HTS01460 HRLN=5.2 HTS02890
C HTS01470 GO TO 2000 HTS02900
C HTS01480 540 IF (DI.LE.DS) IDRV=2 HTS02910
C HTS01490 IF (DI.GT.DS) IDRV=1 HTS02920
C HTS01500 IF (ABS(UM(DRV,1)),GT,DC) GO TO 450 HTS02930
C HTS01510 A=PC*IREVSL HTS02940
C HTS01520 B=0 HTS02950
C HTS01530 S=XOT HTS02960
C HTS01540 NRL=14 HTS02970
C HTS01550 HRLN=5.31 HTS02980
C C..... GO TO 2000 HTS02990
C 1 (100,200,300,400,500,600,700,800,900,1000, HTS01560 C ***** TAKEDA RULE 6.0 ***** LOADING TOWARDS POINT UM ON THE PRIMARY HTS03000
C 1100,1200,1300,1400,1500,1600) HTS01570 CURVE. HTS03010
C C ***** TAKEDA RULE 1.0 ***** ELASTIC STAGE HTS01580 HTS03020
C 100 IF (IDR) 130,130,110 HTS01590 600 IF (IDR) 630,630,610 HTS03030
C 110 IF (ABS(PI),GE,PC) GO TO 200 HTS01600 610 IF (ABS(PI),GE,ABS(UM(JI,2))) GO TO 210 HTS03040
C B=PI HTS01610 A=UM(JI,2) HTS03050
C A=PC*ISGN HTS01620 B=PI HTS03060
C S=OC HTS01630 XOUM=(PI-UM(JI,2))/(DI-UM(JI,1)) HTS03070
C SE=S HTS01640 S=XOUM HTS03080
C NRL=1 HRLN=1.11 HTS01650 NRL=6 HTS03090
C GO TO 2000 HTS01660 HRLN=6.11 HTS03100
C 130 B=0 HTS01670 GO TO 2000 HTS03110
C A=PC*IREVSL HTS01680 630 UOD=DI HTS03120
C IF (PI.NE.0) A=PC*SIGN(1.0,PI) HTS01690 UO=PI HTS03130
C S=OC HTS01700 A=PI HTS03140
C SE=S HTS01710 B=0 HTS03150
C NRL=1 HRLN=1.2 HTS01720 NRL=7 HTS03160
C GO TO 2000 HTS01730 GO TO 2000 HTS03170
C C ***** TAKEDA RULE 2.0 ***** LOADING ON PRIMARY CURVE UP TO YIELD HTS01740 HRLN=6.20 HTS03180
C 200 IF (IDR) 230,230,210 HTS01750 GO TO 2000 HTS03190
C 210 IF (ABS(PI),GE,PT) GO TO 300 HTS01760 C ***** TAKEDA RULE 7.0 ***** UNLOADING FROM POINT UM AFTER RULE 6 HTS03200
C B=PI HTS01770 700 IF (IDR) 730,740,710 HTS03210
C A=PT*ISGN HTS01780 710 IF (ABS(PI),GE,ABS(UM)) GO TO 720 HTS03220
C S=CT HTS01790 A=PI HTS03230
C S1=(PI+(PC*ISGN))/(DI+DC*ISGN) HTS01800 S=S1 HTS03240
C SE=S1 HTS01810 NRL=7 HRLN=7.11 HTS03250
C NRL=2 HTS01820 GO TO 2000 HTS03260
C HRLN=2.11 HTS01830 720 A=UM(JI,2) HTS03270
C GO TO 2000 HTS01840 B=PI HTS03280
C 230 B=0 HTS01850 XOUM=(0.-UM(JI,2))/(XO-UM(JI,1)) HTS03290
C A=PI HTS01860 S=XOUM HTS03300
C S1=(PI+(PC*ISGN))/(DI+DC*ISGN) HTS01870 NRL=6 HTS03310
C S=S1 HTS01880 HRLN=7.12 HTS03320
C SE=S1 HTS01890 GO TO 2000 HTS03330
C HTS01900 730 A=PI HTS03340
C HTS01920 HTS01920

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B=0.
S=S1
NRL=7
      HRLN=7.2
      GO TO 2000
740 A=UM(JI,2)
      B=0.
      XIUM=(PI-UM(JI,2))/(DI-UM(JI,1))
      S=XIUM
      NRL=8
      HRLN=7.3
      GO TO 2000
C ----- TAKEDA RULE 8.0 ----- LOADING TOWARDS POINT UM ON THE PRIMARY
C CURVE
800 IF (IDR) 830,830,810
810 IF (ABS(PI).GE.ABS(UM(JI,2))) GO TO 210
      A=UM(JI,2)
      B=PI
      XIUM=(PI-UM(JI,2))/(DI-UM(JI,1))
      S=XIUM
      NRL=8
      HRLN=8.11
      GO TO 2000
830 U1=PI
      U1D=DI
      A=PI
      B=0.
      S=S1
      NRL=9
      HRLN=8.2
      GO TO 2000
C ----- TAKEDA RULE 9.0 ----- UNLOADING FROM POINT U1 AFTER RULE 8
900 IF (IDR) 930,940,910
910 IF (ABS(PI).GE.ABS(U1)) GO TO 810
      A=U1
      B=PI
      S=S1
      NRL=9
      HRLN=9.11
      GO TO 2000
930 A=PI
      B=0.
      S=S1
      NRL=9
      HRLN=9.2
      GO TO 2000
940 A=U0
      B=0.
      X2U0=(PI-U0)/(DI-U0D)
      S=X2U0
      NRL=10
      HRLN=9.3
      GO TO 2000
C ----- TAKEDA RULE 10.0 ----- LOADING TOWARDS POINT U0
1000 IF (IDR) 1030,1030,1010
1010 IF (ABS(PI).GE.ABS(U0)) GO TO 610
      A=U0
      B=PI
      X2U0=(PI-U0)/(DI-U0D)
      S=X2U0
      NRL=10
      HRLN=10.11
      GO TO 2000
1030 U2=PI
      A=PI
      B=0.
      S=S1
      NRL=11
      HRLN=10.2
      GO TO 2000
C ----- TAKEDA RULE 11.0 ----- UNLOADING FROM POINT U2 AFTER RULE 10
1100 IF (IDR) 1130,1140,1110
1110 IF (ABS(PI).GE.ABS(U2)) GO TO 1010
      A=U2
      B=PI
      S=S1
      NRL=11
      HRLN=11.11
      GO TO 2000
1130 A=PI
      B=0.
      S=S1
      NRL=11
      HRLN=11.2
      GO TO 2000
1140 A=U1
      B=0.
      X3U1=(PI-U1)/(DI-U1D)
      S=X3U1
      NRL=12
      HRLN=11.3
      GO TO 2000
C ----- TAKEDA RULE 12.0 ----- LOADING TOWARDS POINT U1
1200 IF (IDR) 1230,1230,1210
1210 IF (ABS(PI).GE.ABS(U1)) GO TO 810
      A=U1
      B=PI
      X3U1=(PI-U1)/(DI-U1D)
      S=X3U1
      NRL=12
      HRLN=12.11
      GO TO 2000
1230 U3=PI
      A=PI
      B=0.
      S=S1
      NRL=13
      HRLN=12.2
      GO TO 2000
C ----- TAKEDA RULE 13.0 ----- UNLOADING FROM POINT U3 AFTER RULE 12
1300 IF (IDR) 1330, 940,1310
1310 IF (ABS(PI).GE.ABS(U3)) GO TO 1210
      A=U3
      B=PI
      S=S1
      NRL=13
      HRLN=13.11
      GO TO 2000
1330 A=PI
      B=0.
      S=S1
      NRL=13
      HRLN=13.2
      GO TO 2000
C ----- TAKEDA RULE 14.0 ----- LOADING IN THE UNCRACKED DIRECTION AFTER
C CRACKING IN THE OTHER DIRECTION.
1400 IF (IDR) 1430,1440,1410
1410 IF (ABS(PI).GE. PC ) GO TO 210
      A=PC*ISGN
      B=PI
      S=S1
      NRL=14
      HRLN=14.11
      GO TO 2000
1430 A=PI
      B=0.
      S=S1
      NRL=14
      HRLN=14.2
      GO TO 2000
1440 A=UM(JI,2)
      B=0.
      HYS03350 S=S1
      HYS03360 NRL=5
      HYS03370 HRLN=14.3
      HYS03380 GO TO 2000
      HYS03390 C ----- TAKEDA RULE 15.0 ----- LOADING IN THE UNCRACKED DIRECTION AFTER
      HYS03400 C YIELDING IN THE OTHER DIRECTION.
      HYS03410 1500 IF (IDR) 1530,1540,1510
      HYS03420 1510 IF (ABS(PI).GE. PC ) GO TO 1520
      HYS03430 A=PC*ISGN
      HYS03440 B=PI
      HYS03450 S=S1
      HYS03460 NRL=15
      HYS03470 HRLN=15.11
      HYS03480 GO TO 2000
      HYS03490 1520 PQ=PC*ISGN
      HYS03500 DQ=(PQ-PI)/S1+DI
      HYS03510 QT=(PT*ISGN-PQ)/(DT*ISGN-DQ)
      HYS03520 A=PT*ISGN
      HYS03530 B=PQ
      HYS03540 S=QT
      HYS03550 NRL=16
      HYS03560 HRLN=15.12
      HYS03570 GO TO 2000
      HYS03580 1530 A=PI
      HYS03590 B=0.
      HYS03600 S=S1
      HYS03610 NRL=15
      HYS03620 HRLN=15.2
      HYS03630 GO TO 2000
      HYS03640 1540 IF (ABS(PI).GE. ABS(UM(JI,2))) GO TO 1550
      HYS03650 A=UM(JI,2)
      HYS03660 B=0.
      HYS03670 S=S1
      HYS03680 NRL=4
      HYS03690 HRLN=15.31
      HYS03700 GO TO 2000
      HYS03710 1550 A=PU+IAEVEL
      HYS03720 B=0.
      HYS03730 S=PU
      HYS03740 NRL=3
      HYS03750 HRLN=15.32
      HYS03760 GO TO 2000
      HYS03770 C ----- TAKEDA RULE 16.0 ----- LOADING TOWARDS THE YIELD POINT AFTER
      HYS03780 C RULE 15.
      HYS03790 1600 IF (IDR) 1630,1630,1610
      HYS03800 1610 IF (ABS(PI).GE. PY ) GO TO 300
      HYS03810 A=PT*ISGN
      HYS03820 B=PI
      HYS03830 S=QT
      HYS03840 NRL=16
      HYS03850 HRLN=16.11
      HYS03860 GO TO 2000
      HYS03870 1630 UM(JI,1)=DI
      HYS03880 UM(JI,2)=PT
      HYS03890 UO=PI
      HYS03900 UOD=DI
      HYS03910 A=PI
      HYS03920 B=0.
      HYS03930 S=S1
      HYS03940 S2=QT
      HYS03950 X0=DI-PI/S2
      HYS03960 NRL=7
      HYS03970 HRLN=16.2
      HYS03980 GO TO 2000
      HYS03990 C ----- CALCULATE MAXIMUM PREVIOUS DEFORMATION
      HYS04000 C 2000 UM=MIN(UM(1,1),DI)
      HYS04010 UM2=MIN(UM(1,2),PI)
      HYS04020 UM3=MAX(UM(2,1),DI)
      HYS04030 UM4=MAX(UM(2,2),PI)
      HYS04040 RNR=HRLN
      HYS04050 IDR=IDR
      HYS04060 RF=0.
      HYS04070 IF (IDR.GE.0) ACHNG=A
      HYS04080 IF (IDR.LT.0) ACHNG=B
      HYS04090 RETURN
      HYS04100 END
      HYS04110 C#PROCESS SDUMP OPT(0) GOSTMT XREF
      HYS04120 C***** HST07 *****
      HYS04130 C ***** DEBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
      HYS04140 C ***** END DEBUG
      HYS04150 C *****
      HYS04160 C *****
      HYS04170 C *****
      HYS04180 C *****
      HYS04190 C *****
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      HYS04220 C *****
      HYS04230 C *****
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      HYS06650 C *****

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PTEQ=PT*(1-KIE/S)
DVEL=VI*VL
IF (PRINT) WRITE (6,*) 'PEQ,PTEQ,DVEL,VI,VL'
IF (PRINT) WRITE (6,*) 'PEQ,PTEQ,DVEL,VI,VL'
C
C----- LOAD AT OR ABOVE POSITIVE YIELD
IF (RULE.EQ.1) THEN
  IF (DIR.GT.0) THEN
    ... POSITIVE YIELD ...
    CALL STRENG (ESE,PSE,PSEOLD,PT,FL,DT,DL,S,S)
    PL=PT
    DL=DT
    GO TO 210
  ELSE
    ... NEGATIVE YIELD ...
    CALL STRENG (ESE,PSE,PSEOLD,-PT,FL,-DT,DL,S,S)
    PL=-PT
    DL=-DT
    GO TO 220
  ENDIF
ELSE
  IF (RULE.EQ.2.10) GO TO 210
  IF (RULE.EQ.2.20) GO TO 220
  IF (RULE.EQ.2.30) GO TO 230
ENDIF
WRITE (6,*) 'INVALID RULE IN HYST07-BILINEAR HYSTERESIS MODEL'
CALL SDUMP
C
C----- LOAD AT OR ABOVE POSITIVE YIELD, ON BACKBONE CURVE
210 IF (DIR.LE.0) GO TO 230
K=KIE
RULE=2.10
FC=PT*EQ*DC*KIE
B=FC
A=FC*10
CALL STRENG (ESE,PSE,PSEOLD,FC,FL,DC,DL,K,S)
KEY=1
RKEY=KEY
IF (DVEL.LT.0) THEN
  K=S
  B=FC
  A=FC
ENDIF
GO TO 1000
C
C----- LOAD AT OR BELOW NEGATIVE YIELD, ON BACKBONE CURVE
220 IF (DIR.GE.0) GO TO 230
K=KIE
RULE=2.20
FC=-PT*EQ*DC*KIE
B=FC
A=-FC*10
CALL STRENG (ESE,PSE,PSEOLD,FC,FL,DC,DL,K,S)
KEY=2
RKEY=KEY
IF (DVEL.LT.0) THEN
  K=S
  B=FC
  A=FC
ENDIF
GO TO 1000
C
C----- LOAD IN LINEAR RANGE BETWEEN BACKBONE CURVES
230 K=S
RULE=2.3
FC=S*(DC-DL)+FL
A=FC+PT*EQ-PBQ
B=FC+PT*EQ-PBQ
IF (FC.GT.A) THEN
  DA=DC*(A-FC)/S
  CALL STRENG (ESE,PSE,PSEOLD,A,FL,DA,DL,K,S)
  DL=DA
  FL=A
  GO TO 210
ELSE IF (FC.LT.B) THEN
  DB=DC*(B-FC)/S
  CALL STRENG (ESE,PSE,PSEOLD,B,FL,DB,DL,K,S)
  DL=DB
  FL=B
  GO TO 220
ENDIF
C
C----- CALC D & E'S BASED ON PEAK STRAIN ENERGY
IF (KEY.EQ.1) CALL DNE(1,UPOS,EXCR,DT,DL,ESE(2),PSE,PSEOLD,CSE)
IF (KEY.EQ.2) CALL DNE(1,UNEG,EXCR,-DT,DL,ESE(3),PSE,PSEOLD,CSE)
C
C----- CALC STRAIN ENERGY FOR THIS STEP..
PSEL=PSE
CALL STRENG (ESE,PSE,PSEOLD,FC,FL,DC,DL,K,S)
C
C----- CALC PLASTIC STRAIN ENERGY AT ZERO CROSSING ...
IF (FC=FL.LE.0) THEN
  DF=FL-FC
  IF (DF.NE.0) PSEOLD=PSE+(PSEL-PSE)*FC/(FL-FC)
  IF (DF.EQ.0) PSEOLD=PSE
ENDIF
KEY=3
RKEY=KEY
1000 IF (PRINT) WRITE (6,1011) FC,K,A,B
1011 FORMAT
1 (' E=END-FC=',F8.3,' K=',F8.3,' A=',F8.3,' B=',F8.3)
IF (PRINT) WRITE (6,1010) PC,DC,FL,DL,K,RULE,VI,VL,S,PT,KIE,
& A,B,KEY,ESE,PSE,PSEOLD,UPOS,UNEG,EXCR
RETURN
END
#PROCESS SDUMP GORTMT XREF
C***** HYST08 *****
DEBUG UNIT(6),TRACE,SUBCH,INIT,SUBTRACE
END DEBUG
C
C G0EL TRUSS HYSTERESIS MODEL FOR ANGLE AND BOX SECTION
SUBROUTINE HYST08(P,PLA,DB,DELA,V,VL,A,EL,SLK,
& DM,AREA,ADG,TS,PT,SHAPE,
& CC,CYCLE,RULE,STIP,IRV,IDUC,
& HA,VA,HC,VC,RSN,DEMAX,REGIN,PREGIN,
& PBOT,DBOT,PTOP,DTOP,PRINT,
& EXI,EXCR,DELT,ESE,PSE,PSEOLD,CSE)
COMMON /IDSTEP/ ISTEP
DIMENSION DUC(3), EXCR(6), ESE(3)
LOGICAL REVRSC,PRINT
REAL*4 IRV, IDUC
IF (IRV.EQ.0) REVRSC=.FALSE.
IF (IRV.EQ.1) REVRSC=.TRUE.
PL=2.1415926
ES=AREA*DM/EL
DELT=PT/ES
DEL=DE/DELT
PL=P/PT
ESR=SLK*EL/ADG
SLOW=STIP/ES
DA=ABS(DB)
DP=P-PLA
DDE=DE-DELA
DDEL=DE/DELT
C----- CALCULATE EXCURSION RATE AT EVERY ZERO CROSSING
IF (P*PLA.LD.0) THEN

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C
C (4) POINT E
      HE=1.+(RSM*EL/DELY)
      VE=1.
C
C (5) POINT D1
      SLOPE=(VB-VC)/(HE-HC)
      HD1=(SLOPE*HC-VC)/(SLOPE+0.267949)
      VDI=-0.267949*HD1
C
C (6) POINT D
      IF( ESR .LE. 40 ) THEN
        HD=HD1*31./ESR
        VD=VD1*31./ESR
      ELSE IF( ESR .GT. 40 ) THEN
        /* FOR BOX AND ANGLE SECTION */
        IF( SHAPE .EQ. 1 .OR. SHAPE .EQ. 3 ) THEN
          HD=HD1*60./ESR
          VD=VD1*60./ESR
        /* FOR I SHAPE SECTION */
        ELSE IF( SHAPE .EQ. 2 ) THEN
          HD=HD1*20./ESR
          VD=VD1*20./ESR
        ENDDIF
      ENDDIF
C
C DEFINE ENVELOPE SLOPE
      SLOP1=1.
      SLOP2=(VB-VA)/(HB-HA)
      SLOP3=(VC-VB)/(HC-HB)
      SLOP4=(VD-VC)/(HD-HC)
      SLOP5=(VE-VD)/(HE-HD)
      SLOP6=0.0001
C
C DEFINE ENVELOPE INTERMEDIATE POINTS
C
C (1) POINT H
      VH=(VB+SLOP2*(-VD+HD-HB))/(1.-SLOP2)
      HH=VH-VD*HD
C
C (2) POINT C1
      VC1=0.
      HC1=HC-(VC/SLOP4)
C
C (3) POINT (PREP,DRPF) : CALCULATE IN RULE NO. 4
C
C (4) POINT (HREF,VREF) : CALCULATE IN RULE NO. 5
C
C --- DEFINE CURRENT REGIN ZONE NUMBER
      PREGIN=REGIN
      IF( DEL .GE. HA ) REGIN=1
      IF( DEL .GE. HH .AND. DEL .LT. HA ) REGIN=2
      IF( DEL .GE. HB .AND. DEL .LT. HH ) REGIN=3
      IF( DEL .GE. HC .AND. DEL .LT. HB ) REGIN=4
C
      IF (PRINT) THEN
        WRITE(6,567)CYCLE,RULE,HA,VA,HB,VB,HC,VC,HD,VE,VC1,
          HD,VD,HD1,VD1,HH,VH,HE,VE
        567 FORMAT(5X,'GOEL HYSTORESIS ENVELOPE CONTROL POINTS....//
          5X,'-----'//
          5X,'CYCLE -----',F10.4, //
          5X,'RULE -----',F10.4, //
          5X,'(HA,VA) -----',1X,(' ',F10.4, ', ',F10.4, ', ') //
          5X,'(HB,VB) -----',1X,(' ',F10.4, ', ',F10.4, ', ') //
          5X,'(HC,VC) -----',1X,(' ',F10.4, ', ',F10.4, ', ') //
          5X,'(HD,VD) -----',1X,(' ',F10.4, ', ',F10.4, ', ') //
          5X,'(HE,VE) -----',1X,(' ',F10.4, ', ',F10.4, ', ') //
          5X,'(HCL,VC1) -----',1X,(' ',F10.4, ', ',F10.4, ', ') //
          5X,'(HD,VD) -----',1X,(' ',F10.4, ', ',F10.4, ', ') //
          5X,'(HD,VD) -----',1X,(' ',F10.4, ', ',F10.4, ', ') //
          5X,'(HH,VH) -----',1X,(' ',F10.4, ', ',F10.4, ', ') //
          5X,'(HE,VE) -----',1X,(' ',F10.4, ', ',F10.4, ', ') //
        ENDDIF
C
      GOEL'S HYSTORESIS LOOP RULES
C
C (1) RULE 1
      IF( RULE .EQ. 1 ) THEN
        IF( P .GE. PMAX .AND. P .LT. PT ) THEN
          STIF=SLOP1*ES
          EKI=SLOP1*ES
          IF( DEMAX .LT. 0 .AND. DE .LE. DEMAX ) THEN
            RSN=((0.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC
            ENDDIF
          ELSE IF( P .GE. PT ) THEN
            STIF=SLOP6
            RULE=6
            RSN=0
            EKI=SLOP1*ES
          ELSE IF( P .LT. PMAX ) THEN
            HA=DEL
            VA=PL
            SLOP2=(VB-VA)/(HB-HA)
            STIF=SLOP2*ES
            D1=DE-HA*DELY
            SLOP2=(VB-VA)/(HB-HA)
            STIF=SLOP2*ES
            P=PMAX*(STIF*D1)
            PL=P*PT
          ELSE IF( DEMAX .LT. 0 .AND. DE .LE. DEMAX ) THEN
            RSN=((0.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC
            ENDDIF
            EKI=SLOP1*ES
          ENDDIF
          CALL STRNG(ESSE,PSE,PSOLD,P,PLA,DE,DELA,STIF,EKI)
          CALL DNE(1, DUC, EXCR, DELT, DA, ESSE, PSE, PSOLD, CSE )
        RETURN
      ENDDIF
C
C (2) RULE 2
      IF( RULE .EQ. 2 ) THEN
        IF( DEL .GE. HH ) THEN
          IF( DDE .LE. 0 ) THEN
            IF(PREGIN .NE. REGIN ) REVRSC= .FALSE.
            STIF=SLOP2*ES
            IF( DEMAX .LT. 0 .AND. DE .LE. DEMAX ) THEN
              RSN=((0.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC
            ELSE IF( DEMAX .GT. 0 ) THEN
              RSN=((0.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC
            ENDDIF
            IF(.NOT. REVRSC )THEN
              EKI=SLOP1*ES
            ELSE IF( REVRSC )THEN
              EKI=ES*(PTOP-PL)/(DTOP-DEL)
            ENDDIF
            IF( DDE .GT. 0 ) THEN
              IF(.NOT. REVRSC )THEN
                RULE=7
                STIF=SLOP1*ES
              ELSE IF( DEL .LE. -12 ) THEN
                REVRSC= .FALSE.
              ELSE IF( DEL .LE. -12 ) THEN
                REVRSC= .FALSE.
              ELSE IF( DEL .LE. -12 ) THEN
                REVRSC= .FALSE.
              ELSE IF( DEL .LE. -12 ) THEN
                REVRSC= .FALSE.
            ENDIF
          ELSE IF( DEL .LE. 0 ) THEN
            RULE=3
            HC=DEL
            VC=PL
            IDUC=1.
            STIF=SLOP6*ES
            EKI=ES*(VD-VC)/(HD-HC)
            IF( DEMAX .LT. 0 .AND. DE .LE. DEMAX ) THEN

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      HTS01770
      HTS01780
      HTS01790
      HTS01800
      HTS01810
      HTS01820
      HTS01830
      HTS01840
      HTS01850
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      HTS01870
      HTS01880
      HTS01890
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      HTS01980
      HTS01990
      HTS02000
      HTS02010
      HTS02020
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      HTS04600

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      RSN=(0.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC  HYS04610      EKI=ES*SLOP1      HYS06030
    ELSE IF( DMAX.GT. 0 ) THEN  HYS04620      ENDIF  HYS06040
      RSN=(0.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC  HYS04630      ENDIF  HYS06050
    ENDIF  HYS04640      ELSE IF( DEL.GT. HD .AND. DDE.GT. 0 ) THEN  HYS06060
    ELSE IF( DDE.GT. 0 ) THEN  HYS04650      STIF=SLOP5*ES  HYS06070
      RULE=4  HYS04660      EKI=ES*SLOP1  HYS06080
      STIF=SLOP4*ES  HYS04670      EKI=ES*SLOP1  HYS06090
      EKI=ES*SLOP4  HYS04680      ELSE IF( DEL.LT. HC1 .AND. DEL.GE. HC ) THEN  HYS06100
      ENDIF  HYS04690      REVRSC=.FALSE.  HYS06110
    ENDIF  HYS04700      RULE=4  HYS06120
    IF( REVRSC ) IRV=1.  HYS04710      STIF=SLOP4*ES  HYS06130
    IF ( .NOT. REVRSC ) IRV=0.  HYS04720      EKI=ES*SLOP4  HYS06140
    CALL STRENG(ESE,PSE,PSOLD,P,PLA,DE,DELA,STIF,EKI)  HYS04730      ELSE IF( DEL.LT. HC ) THEN  HYS06150
    IF( ABS(DEMAX) .LT. ABS(DE) ) THEN  HYS04740      REVRSC=.FALSE.  HYS06160
      CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSOLD, CSE )  HYS04750      RULE=3  HYS06170
      DMAX=DE  HYS04760      HC=DEL  HYS06180
    ENDIF  HYS04770      VC=PL  HYS06190
    RETURN  HYS04780      IDUC=1.  HYS06200
    ENDIF  HYS04790      STIF=SLOP6*ES  HYS06210
    ENDIF  HYS04800      EKI=ES*((VD-VC)/(HD-HC))  HYS06220
    (4) RULE 7  HYS04810      ENDIF  HYS06230
    IF( REVRSC ) IRV=1.  HYS04820      IF( REVRSC ) IRV=1.  HYS06240
    IF ( .NOT. REVRSC ) IRV=0.  HYS04830      IF ( .NOT. REVRSC ) IRV=0.  HYS06250
    CALL STRENG(ESE,PSE,PSOLD,P,PLA,DE,DELA,STIF,EKI)  HYS04840      CALL STRENG(ESE,PSE,PSOLD,P,PLA,DE,DELA,STIF,EKI)  HYS06260
    STIF=SLOP1*ES  HYS04850      IF( ABS(DEMAX) .LT. ABS(DE) ) THEN  HYS06270
    EKI=ES*SLOP1  HYS04860      CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSOLD, CSE )  HYS06280
    ELSE IF( DEL.LT. DBOT .OR. P.LE. PMAX ) THEN  HYS04870      DMAX=DE  HYS06290
    IF( P.LE. PMAX ) THEN  HYS04880      ENDIF  HYS06300
      REVRSC=.FALSE.  HYS04890      RETURN  HYS06310
      PTOP=0  HYS04900      ENDIF  HYS06320
      DTOP=0  HYS04910      ENDIF  HYS06330
      PBOT=PL  HYS04920      (8) RULE 5  HYS06340
      DBOT=DEL  HYS04930      C  HYS06350
      S1=PLA/PT  HYS04940      C  HYS06360
      S2=PMAX/PT  HYS04950      IF( RULE.EQ. 5.) THEN  HYS06370
      HA=(DELA/DELT) + ((S2-S1)/(SLOP1))  HYS04960      IF( DEL.LT. HE ) THEN  HYS06380
      VA=S2  HYS04970      STIF=SLOP5*ES  HYS06390
      SLOP2=(VB-VA)/(HB-HA)  HYS04980      EKI=ES*SLOP1  HYS06400
    ENDIF  HYS04990      ELSE IF( DDE.LT. 0 ) THEN  HYS06410
    STIF=SLOP2*ES  HYS05000      HREF=(VD-SLOP5*HD-BREF)/(SLOP1-SLOP5)  HYS06420
    RULE=2  HYS05010      IF( DEL.GE. HREF ) CYCLE=CYCLE+1  HYS06430
    PL=VA+SLOP2*(DBOT-HA)  HYS05020      PTOP=P/PT  HYS06440
    P=PL*PT  HYS05030      DTOP=DEL  HYS06450
    EKI=ES*((PTOP-PL)/(DTOP-DEL))  HYS05040      DBOT=(SLOP2*HA-VA+PTOP-SLOP1*DEL)/(SLOP2-SLOP1)  HYS06460
    ELSE IF( DEL.GT. DTOP .AND. PL.LT. VE ) THEN  HYS05050      PBOT=VA+SLOP2*(DBOT-HA)  HYS06470
      STIF=SLOP5*ES  HYS05060      STIF=SLOP1*ES  HYS06480
      RULE=5  HYS05070      EKI=ES*SLOP1  HYS06490
    EKI=ES*SLOP1  HYS05080      ELSE IF( DEL.GT. DTOP .AND. PL.GE. VE ) THEN  HYS06500
    ELSE IF( DEL.GT. DTOP .AND. PL.GE. VE ) THEN  HYS05090      STIF=SLOP6*ES  HYS06510
      STIF=SLOP6*ES  HYS05100      RULE=6  HYS06520
      RULE=6  HYS05110      EKI=ES*SLOP1  HYS06530
      EKI=ES*SLOP1  HYS05120      ENDIF  HYS06540
      HYS05130      ELSE IF( DEL.GE. HE .AND. DDE.GT. 0 ) THEN  HYS06550
      HYS05140      STIF=SLOP6  HYS06560
      HYS05150      EKI=ES*SLOP1  HYS06570
      HYS05160      ENDIF  HYS06580
      HYS05170      IF( REVRSC ) IRV=1.  HYS06590
      HYS05180      IF ( .NOT. REVRSC ) IRV=0.  HYS06600
      HYS05190      CALL STRENG(ESE,PSE,PSOLD,P,PLA,DE,DELA,STIF,EKI)  HYS06610
      HYS05200      IF( ABS(DEMAX) .LT. ABS(DE) ) THEN  HYS06620
      HYS05210      CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSOLD, CSE )  HYS06630
      HYS05220      DMAX=DE  HYS06640
      HYS05230      ENDIF  HYS06650
      HYS05240      RETURN  HYS06660
      HYS05250      ENDIF  HYS06670
      HYS05260      C  HYS06680
      HYS05270      C  HYS06690
      HYS05280      C  HYS06700
      HYS05290      (9) RULE 6  HYS06710
      HYS05300      IF( RULE.EQ. 6.) THEN  HYS06720
      HYS05310      IF( DDE.GE. 0 ) THEN  HYS06730
      HYS05320      STIF=SLOP6  HYS06740
      HYS05330      EKI=ES*SLOP1  HYS06750
      HYS05340      ELSE IF( DEL.LT. 0 ) THEN  HYS06760
      HYS05350      STIF=SLOP1*ES  HYS06770
      HYS05360      RULE=1  HYS06780
      HYS05370      CYCLE=CYCLE+1.  HYS06790
      HYS05380      REVRSC=.FALSE.  HYS06800
      HYS05390      EKI=ES*SLOP1  HYS06810
      HYS05400      ENDIF  HYS06820
      HYS05410      IF( REVRSC ) IRV=1.  HYS06830
      HYS05420      IF ( .NOT. REVRSC ) IRV=0.  HYS06840
      HYS05430      CALL STRENG(ESE,PSE,PSOLD,P,PLA,DE,DELA,STIF,EKI)  HYS06850
      HYS05440      IF( ABS(DEMAX) .LT. ABS(DE) ) THEN  HYS06860
      HYS05450      CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSOLD, CSE )  HYS06870
      HYS05460      DMAX=DE  HYS06880
      HYS05470      RETURN  HYS06890
      HYS05480      ENDIF  HYS06900
      HYS05490      C  HYS06910
      HYS05500      C  HYS06920
      HYS05510      C  HYS06930
      HYS05520      (10) RULE 10  HYS06940
      HYS05530      IF( RULE.EQ. 10.) THEN  HYS06950
      HYS05540      IF( DEL.GT. DBOT .AND. DEL.LT. DTOP ) THEN  HYS06960
      HYS05550      STIF=SLOP7*ES  HYS06970
      HYS05560      EKI=ES*SLOP7  HYS06980
      HYS05570      ELSE IF( DEL.LT. DBOT ) THEN  HYS06990
      HYS05580      STIF=SLOP3*ES  HYS07000
      HYS05590      RULE=3  HYS07010
      HYS05600      REVRSC=.FALSE.  HYS07020
      HYS05610      PTOP=0  HYS07030
      HYS05620      DTOP=0  HYS07040
      HYS05630      PBOT=0  HYS07050
      HYS05640      DBOT=0  HYS07060
      HYS05650      EKI=ES*((PTOP-PL)/(DTOP-DEL))  HYS07070
      HYS05660      ELSE IF( DEL.GT. DTOP ) THEN  HYS07080
      HYS05670      STIF=SLOP4*ES  HYS07090
      HYS05680      RULE=4  HYS07100
      HYS05690      EKI=ES*SLOP1  HYS07110
      HYS05700      ENDIF  HYS07120
      HYS05710      IF( REVRSC ) IRV=1.  HYS07130
      HYS05720      IF ( .NOT. REVRSC ) IRV=0.  HYS07140
      HYS05730      CALL STRENG(ESE,PSE,PSOLD,P,PLA,DE,DELA,STIF,EKI)  HYS07150
      HYS05740      IF( ABS(DEMAX) .LT. ABS(DE) ) THEN  HYS07160
      HYS05750      CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSOLD, CSE )  HYS07170
      HYS05760      DMAX=DE  HYS07180
      HYS05770      RETURN  HYS07190
      HYS05780      ENDIF  HYS07200
      HYS05790      C  HYS07210
      HYS05800      C  HYS07220
      HYS05810      (11) RULE 11  HYS07230
      HYS05820      IF( RULE.EQ. 11.) THEN  HYS07240
      HYS05830      IF( DEL.GT. DBOT .AND. DEL.LT. DTOP ) THEN  HYS07250
      HYS05840      STIF=SLOP7*ES  HYS07260
      HYS05850      EKI=ES*SLOP7  HYS07270
      HYS05860      ELSE IF( DEL.LT. DBOT ) THEN  HYS07280
      HYS05870      STIF=SLOP2*ES  HYS07290
      HYS05880      RULE=2  HYS07300
      HYS05890      REVRSC=.FALSE.  HYS07310
      HYS05900      PTOP=0  HYS07320
      HYS05910      DTOP=0  HYS07330
      HYS05920      PBOT=0  HYS07340
      HYS05930      DBOT=0  HYS07350
      HYS05940      EKI=ES*((VD-PL)/(HD-DEL))  HYS07360
      HYS05950      ELSE IF( DEL.GT. DTOP ) THEN  HYS07370
      HYS05960      STIF=SLOP5*ES  HYS07380
      HYS05970      RULE=5  HYS07390
      HYS05980      EKI=ES*SLOP1  HYS07400
      HYS05990      ENDIF  HYS07410
      HYS06000      IF( REVRSC ) IRV=1.  HYS07420
      HYS06010      IF ( .NOT. REVRSC ) IRV=0.  HYS07430
      HYS06020      CALL STRENG(ESE,PSE,PSOLD,P,PLA,DE,DELA,STIF,EKI)  HYS07440
      CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSOLD, CSE )  HYS07450
      RETURN  HYS07460
      ENDIF  HYS07470

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STIP=(-52030208*0.0126 + 856026)*FAC
ENDIF
RULE=8
VA=2700*FACV
HA=DBOT*(2700*FACV-PBOT)/STIP
VB=VA-465*FACV
HB=HA+0.0023*FACH
*** FOR UNBALANCE FORCE ***
UD=X-XLA
UP=STIF*UD
P=PLA+UP
IF( P .GT. VA) P=VA
ELSE IF( PTOP .NE. 999999 ) THEN
CALL LOOP(VA1,HA1,PBOT,DBOT,PTOP,DTOP,HD,VD,HE,VE,
SM1,SM2,FACH,FACV,FAC)
STIF=(PBOT-VD)/(DBOT-HD)
RULE=4
*** FOR UNBALANCE FORCE ***
UD=X-XLA
UP=STIF*UD
P=PLA+UP
IF( P .GT. PTOP) P=PTOP
ENDIF
IF( DEMAX .LT. ABS(X) ) THEN
DEMAX=ABS(X)
DUC(1)=DEMAX/(HA1*FACH)
ENDIF
RETURN
ENDIF
C---RULE 3)
IF( RULE .EQ. 3 ) THEN
IF( HIGH .AND. DX .GT. 0 ) THEN
RULE=3
STIF=SLOP3
IF( P .LT. 0 ) P=0.1*VA
IF( P .LE. 0.1*VA) STIF=SLOP3*0.01
ELSE IF( NOT HIGH .AND. DX .LT. 0 ) THEN
RULE=3
STIF=SLOP3
IF( P .GT. 0 ) P=0.1*VAP
IF( P .GE. 0.1*VAP) STIF=SLOP3*0.001
ELSE IF( HIGH .AND. DX .LE. 0 ) THEN
PTOP=PLA
DTOP=XLA
IF( PBOT .EQ. 999999 ) THEN
IF( (ABS(DTOP/FACH)-ABS(HA1)+0.0042) .LE. 0.0126) THEN
STIF=(-52030208*(ABS(DTOP/FACH)-ABS(HA1)+0.0042)
+856026)*FAC
ELSE
STIF=(-52030208*0.0126 + 856026)*FAC
ENDIF
RULE=8
VAP=-2700*FACV
HAP=DTOP-(2700*FACV-PTOP)/STIP
VBP=VAP+465*FACV
HBP=HAP+0.0023*FACH
*** FOR UNBALANCE FORCE ***
UD=X-XLA
UP=STIF*UD
P=PLA+UP
IF( P .LT. VAP) P=VAP
ELSE IF( PBOT .NE. 999999 ) THEN
CALL LOOP(VA1,HA1,PTOP,DTOP,PBOT,DBOT,HD,VD,HE,VE,
SM1,SM2,FACH,FACV,FAC)
STIF=(PTOP-VD)/(DTOP-HD)
RULE=4
*** FOR UNBALANCE FORCE ***
UD=X-XLA
UP=STIF*UD
P=PLA+UP
IF( P .LT. PBOT) P=PBOT
ENDIF
ELSE IF( NOT HIGH .AND. DX .GE. 0 ) THEN
PBOT=PLA
DBOT=XLA
IF( PTOP .EQ. 999999 ) THEN
STIF=-52030208*(ABS(DBOT)-ABS(HAP)+0.0042)+856026
IF( (ABS(DBOT/FACH)-ABS(HAP1)+0.0042) .LE. 0.0126) THEN
STIF=(-52030208*(ABS(DBOT/FACH)-ABS(HAP1)+0.0042)
+856026)*FAC
ELSE
STIF=(-52030208*0.0126 + 856026)*FAC
ENDIF
RULE=8
VA=2700*FACV
HA=DBOT*(2700*FACV-PBOT)/STIP
VB=VA-465*FACV
HB=HA+0.0023*FACH
*** FOR UNBALANCE FORCE ***
UD=X-XLA
UP=STIF*UD
P=PLA+UP
IF( P .GT. VA) P=VA
ELSE IF( PTOP .NE. 999999 ) THEN
CALL LOOP(VA1,HA1,PBOT,DBOT,PTOP,DTOP,HD,VD,HE,VE,
SM1,SM2,FACH,FACV,FAC)
STIF=(PBOT-VD)/(DBOT-HD)
RULE=4
*** FOR UNBALANCE FORCE ***
UD=X-XLA
UP=STIF*UD
P=PLA+UP
IF( P .GT. PTOP) P=PTOP
ENDIF
IF( DEMAX .LT. ABS(X) ) THEN
DEMAX=ABS(X)
DUC(1)=DEMAX/(HA1*FACH)
ENDIF
RETURN
ENDIF
C---RULE 4, 7, 8, 9, 10)
IF( RULE .GT. 3 ) THEN
CALL HST98(MOPT,STIF,P,X,PLA,XLA,DP,DX,
RV,RULE,PTOP,DTOP,PBOT,DBOT,
SPTOP,SDTOP,SPBOT,SDBOT,
SSPTOP,SSDTP,SSSPBT,SSSDBT,
HA,VA,HB,VB,HD,VD,HE,VE,HAP,VAP,HBP,VBP,
SSHD,SSVD,SSHE,SSVE,
SSSPTP,SSSDTP,SSSPBT,SSSDBT,
SHD,SVD,SHE,SVE,PRINT,
SKI,DCR,EXCR,DELT,ESS,PSE,PSEOLD,CSE,
DEMAX,FACH,FACV,FAC,HIGH,SLOP1,SLOP2,SLOP3,
HA1,VA1,HB1,VB1,HAP1,VAP1,HBP1,VBP1 )
ENDIF
RETURN
END
C
SUBROUTINE HST98(MOPT,STIF,P,X,PLA,XLA,DP,DX,
RV,RULE,PTOP,DTOP,PBOT,DBOT,
SPTOP,SDTOP,SPBOT,SDBOT,
SSPTOP,SSDTP,SSSPBT,SSSDBT,
HA,VA,HB,VB,HD,VD,HE,VE,HAP,VAP,HBP,VBP,
SSHD,SSVD,SSHE,SSVE,

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C      STIF=(SVE-SPBOT)/(SHE-SDBOT)
      *** FOR UNBALANCED FORCE ***
      UD=X-EDBOT
      UP=STIF*UD
      P=SPBOT+UP
      ENDIF
      RULE=7
      ELSE IF(SHD .GT. SHE .AND. X .LE. SDBOT .AND. DX .LE. 0 ) THEN
      IF( X .GT. HBP) THEN
      RULE=2
      CALL SRV(P,RV)
      STIF=SLOP2
      *** FOR UNBALANCE FORCE ***
      UD=X-EDBOT
      UP=STIF*UD
      P=SPBOT+UP
      ELSE IF( X .LE. HBP) THEN
      RULE=1
      CALL SRV(P,RV)
      STIF=SLOP3
      IF( P .GT. 0.1*VAP .OR. SPBOT .GT. 0.1*VAP)STIF=SLOP3*0.01
      *** FOR UNBALANCE FORCE ***
      UD=X-EDBOT
      UP=STIF*UD
      P=SPBOT+UP
      ENDIF
      ELSE IF(SHD .GT. SHE .AND. DX .GT. 0 ) THEN
      RULE=9
      SSPTOP=SPTOP
      SSDTOP=SDTOP
      SSPBOT=PLA
      SDBOT=XLA
      CALL SLOOP(SSPTOP,SSDBOT,SSPTOP,SSDTOP,SSHD,SSVD,SSHE,SSVE,
      SMI,SM2,FACH,FACV,FAC)
      STIF=(SSPTOP-SSVD)/(SSDBOT-SSHD)
      *** FOR UNBALANCED FORCE ***
      UD=X-XLA
      UP=STIF*UD
      P=PLA+UP
      ELSE IF( SHD .LT. SHE .AND. X .LT. SDTOP .AND. DX .GE. 0 ) THEN
      IF( X .LT. SHD) THEN
      STIF=(SPBOT-SSVD)/(SDBOT-SHD)
      *** FOR UNBALANCED FORCE ***
      UD=X-SHD
      UP=STIF*UD
      P=SSVD+UP
      ELSE IF( X .GE. SHD .AND. X .LT. SHE) THEN
      STIF=(SSVD-SVE)/(SHD-SHE)
      *** FOR UNBALANCED FORCE ***
      UD=X-SHE
      UP=STIF*UD
      P=SVE+UP
      ELSE IF( X .GE. SHE .AND. X .LT. SDTOP) THEN
      STIF=(SVE-SPTOP)/(SHE-SDTOP)
      *** FOR UNBALANCED FORCE ***
      UD=X-SDTOP
      UP=STIF*UD
      P=SPTOP+UP
      ENDIF
      RULE=7
      ELSE IF(SHD .LT. SHE .AND. X .GE. SDTOP .AND. DX .GE. 0 ) THEN
      IF( X .LT. HB) THEN
      RULE=2
      CALL SRV(P,RV)
      STIF=SLOP3
      *** FOR UNBALANCE FORCE ***
      UD=X-SDTOP
      UP=STIF*UD
      P=SPTOP+UP
      ELSE IF( X .GE. HB) THEN
      RULE=3
      CALL SRV(P,RV)
      STIF=SLOP3
      IF( P .LT. 0.1*VA .OR. SPTOP .LT. 0.1*VA)STIF=SLOP3*0.01
      *** FOR UNBALANCE FORCE ***
      UD=X-SDTOP
      UP=STIF*UD
      P=SPTOP+UP
      ENDIF
      ELSE IF(SHD .LT. SHE .AND. DX .LT. 0 ) THEN
      RULE=9
      SSPTOP=PLA
      SSOTOP=XLA
      SSPBOT=SPBOT
      SDBOT=XDBOT
      CALL SLOOP(SSPTOP,SSDBOT,SSPTOP,SSDTOP,SSHD,SSVD,SSHE,SSVE,
      SMI,SM2,FACH,FACV,FAC)
      STIF=(SSPTOP-SSVD)/(SSDTOP-SSHD)
      *** FOR UNBALANCE FORCE ***
      UD=X-XLA
      UP=STIF*UD
      P=PLA+UP
      ENDIF
      IF( DEMAX .LT. ABS(X) ) THEN
      DEMAX=ABS(X)
      DUC(1)=DEMAX/(HA1*FACH)
      ENDIF
      RETURN
      ENDIF
C---RULE 8
IF( RULE .EQ. 8 ) THEN
IF( PTOP .EQ. 99999 ) THEN
S1=VA
ELSE IF( PTOP .NE. 99999 ) THEN
S1=PTOP
ENDIF
IF( PBOT .EQ. 99999 ) THEN
S2=VAP
ELSE IF( PBOT .NE. 99999 ) THEN
S2=PBOT
ENDIF
IF( P .LT. S1 .AND. P .GT. S2 ) THEN
RULE=8
STIF=STIF
ELSE IF( P .LE. S2 ) THEN
IF( PBOT .EQ. 99999 ) THEN
*** FOR UNBALANCE FORCE ***
UP1=P-S2
UD=UP1*STIF
UP2=UD*SLOP2
P=P-(UP1-UP2)
HAP=X-UD
VAP=S2
RULE=2
CALL SRV(P,RV)
STIF=SLOP2
HBP=HAP-0.0023*FACH
VBP=VAP+465*FACV
ELSE IF( PBOT .NE. 99999 ) THEN
IF( X .GT. HBP) THEN
RULE=2
CALL SRV(P,RV)
STIF=SLOP2
*** FOR UNBALANCED FORCE ***
UD=X-SDBOT
UP=STIF*UD
P=PBOT+UP

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RETURN
ENDIF
C
C ---RULE 10:
IF( RULE.EQ. 10 ) THEN
  IF( SSMC.GT. SSMR ) THEN
    IF( X.LT. SSSDTP.AND. X.GT. SSSDBT ) THEN
      RULE=10
      STIF=(SSSPBT-SSSPTP)/(SSSDBT-SSSDTP)
    ELSE IF( X.LT. SSSDBT ) THEN
      RULE=9
      GO TO 900
    ELSE IF( X.GT. SSSDTP ) THEN
      RULE=7
      GO TO 700
    ENDIF
  ELSE IF( SSMC.LT. SSMR ) THEN
    IF( X.LT. SSSDTP.AND. X.GT. SSSDBT ) THEN
      RULE=10
      STIF=(SSSPBT-SSSPTP)/(SSSDBT-SSSDTP)
    ELSE IF( X.GT. SSSDTP ) THEN
      RULE=9
      GO TO 900
    ELSE IF( X.LT. SSSDBT ) THEN
      RULE=7
      GO TO 700
    ENDIF
  ENDIF
  IF( DEMAX.LT. ABS(X) ) THEN
    DEMAX=ABS(X)
    DUC(1)=DEMAX/(HA1*FACH)
  ENDIF
RETURN
ENDIF
C
C PA
SUBROUTINE LOOP(VA1,HA1,VC,HC,VB,HB,HD,VD,HE,VE,
& SM1,SM2,FACH,FACV,FAC)
C
C
VA1=VA/FACH
HA1=HA/FACH
VC1=VC/FACH
HC1=HC/FACH
VB1=VB/FACH
HB1=HB/FACH
C
SM1=(VC1-VB1)/(HC1-HB1)
IF( (ABS(HC1)-ABS(HA1)+.0042) .LE. 0.0126 ) THEN
  SM2=-52030208*(ABS(HC1)-ABS(HA1)+.0042)+956026
ELSE IF( (ABS(HC1)-ABS(HA1)+.0042) .GT. 0.0126 ) THEN
  SM2=-52030208*0.0126 + 956026
ENDIF
FACT1=.5
FACT2=.1
HL=HC1-HB1
VL=VC1-VB1
H1=HB1+HL*FACT1
V1=VB1+VL*FACT1
H2=HB1+HL*FACT2
V2=VB1+VL*FACT2
HD1=(V1+(H1/SM1)-VC1+SM2*HC1)/(SM2+1/SM1)
VD1=SM2*HD1+VC1-SM2*HC1
HE1=HD1-HL*(FACT1-FACT2)
VE1=VD1-VL*(FACT1-FACT2)
HD=HD1*FACH
VD=VD1*FACH
HE=HE1*FACH
VE=VE1*FACH
C
RETURN
END
C PA
SUBROUTINE SLOOP(VC,HC,VB,HB,HD,VD,HE,VE,
& SM1,SM2,FACH,FACV,FAC)
C
C
VC1=VC/FACH
HC1=HC/FACH
VB1=VB/FACH
HB1=HB/FACH
C
SM1=(VC1-VB1)/(HC1-HB1)
SM2=447.160
FACT1=.4
FACT2=.1
HL=HC1-HB1
VL=VC1-VB1
H1=HB1+HL*FACT1
V1=VB1+VL*FACT1
H2=HB1+HL*FACT2
V2=VB1+VL*FACT2
HD1=(V1+(H1/SM1)-VC1+SM2*HC1)/(SM2+1/SM1)
VD1=SM2*HD1+VC1-SM2*HC1
HE1=HD1-HL*(FACT1-FACT2)
VE1=VD1-VL*(FACT1-FACT2)
HD=HD1*FACH
VD=VD1*FACH
HE=HE1*FACH
VE=VE1*FACH
C
RETURN
END
C
SUBROUTINE SRV(P,RV)
IF( P.GT. 0 ) THEN
  RV=C
ELSE IF( P.LT. 0 ) THEN
  RV=1
ENDIF
RETURN
END
C
***** HYST11 *****
DEBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
END DEBUG
C
C
PINO-SUAREZ TOWER SHORT DIRECTION GIRDER MOMENT-ROTATION
HYSTERESIS MODEL BASED ON HYSTERESIS LOOP OF T2A GIRDER
SUBROUTINE HYST11(MOPT,STIF,P,X,PLA,XLA,DP,DX,
& HA,VA,RATIO,PTOP,
& EXC, DUC, EXCR, DELT, ESE, PSE, PSEOLD, CSE, DEMAX, DUCMAX)
& DIMENSION DUC(3), EXCR(6),ESE(3)
LOGICAL HIGH,PRINT
C
C
C DUC ---- ELEMENT DUCTILITY RATE
C EXCR ---- ELEMENT EXCURSION RATE
C ESE ---- ELEMENT ELASTIC STRAIN RATE
C PSE ---- ELEMENT PLASTIC STRAIN RATE
C PSEOLD ---- ELEMENT PLASTIC STRAIN RATE AT LAST ZERO CROSSING
C CSE ---- ELEMENT CONSTANT STRAIN ENERGY
C RATIO ---- STRAIN HARDEN RATIO TO ELASTIC STIFFNESS
C STIF ---- ELEMENT STIFFNESS
C P ---- CURRENT ELEMENT INTERNAL FORCE
C X ---- CURRENT ELEMENT INTERNAL DISPLACEMENT
C PLA ---- PREVIOUS ELEMENT INTERNAL FORCE
C XLA ---- PREVIOUS ELEMENT INTERNAL DISPLACEMENT
C DP ---- ELEMENT INTERNAL FORCE INCREMENT
C DX ---- ELEMENT INTERNAL DISPLACEMENT INCREMENT
C HIGH ---- LOGICAL INDEX, IF P.GT. 0 THEN HIGH=.TRUE.
C RULE ---- RULE NUMBER
C PTOP ---- REVERSED LOADING POINT AT TOP ENVELOPE
C DBOT ---- REVERSED LOADING POINT AT BOTTOM ENVELOPE
C DBOT ---- REVERSED DISP. POINT AT BOTTOM ENVELOPE
C HA,VA ---- FIRST BUCKLING POINT AT TOP ENVELOPE
C DUCMAX ---- FAILURE DUCTILITY
C .GE. 1 FAILURE DUCTILITY CONSIDERED
C .LT. 1 FAILURE DUCTILITY NOT CONSIDERED
C
C
C -----
C = CHECK FAILURE DUCTILITY ==
C
C
IF( DUC(1) .GE. DUCMAX.AND. DUCMAX .GE. 1 ) THEN
  P=0
  STIF=0.001
  RETURN
ENDIF
C
IF( RV .EQ. 1 ) HIGH=.FALSE.
IF( RV .EQ. 0 ) HIGH=.TRUE.
C
C = DEFINE ENVELOPE CONTROL POINTS =
C
C
IF( MOPT.EQ. 2 ) THEN
  PTOP=VA
  PBOT=-VA
  DTOP=HA
  DBOT=-HA
ENDIF
C
IF( P.GT. 0 ) HIGH=.TRUE.
IF( P.LT. 0 ) HIGH=.FALSE.
IF( HIGH) RV=0
IF( .NOT. HIGH) RV=1
C
C = DEFINE ENVELOPE SLOPE =
C
SLOP1=VA/HA
SLOP2=RATIO*SLOP1
C
IF( PRINT) THEN
  WRITE(6,567)RULE,HA,VA,
& SLOP1,SLOP2
567 FORMAT(5X,'PINO-SUAREZ TOWER SHORT DIR. M-R LOOP CONTROL POINTS.',
& 5X,'-----',1X,G10.4,/,
& 5X,'RULE -----',1X,G10.4,/,
& 5X,'(HA, VA) -----',1X,'( ',F10.4,', ',F10.4,' )',/,
& 5X,'SLOP1 -----',1X,G10.4,/,
& 5X,'SLOP2 -----',1X,G10.4,/)
ENDIF
C
C = HYSTERESIS LOOP RULES =
C
C ---RULE 1:
IF( RULE.EQ. 1 ) THEN
  IF( P.GT. PTOP .OR. P.LE. PBOT ) THEN
    RULE=2
    STIF=SLOP2
    *** UNBALANCED FORCE ***
    IF( P.GE. PTOP ) THEN
      UP1=P-PTOP
      UD=UP1/SLOP1
      UP2=-UD*SLOP2
      P=P-(UP1+UP2)
    ELSE IF( P.LE. PBOT ) THEN
      DP=PTOP-P
      UD=DP/SLOP1
      UP2=-UD*SLOP2
      P=P-(UP1+UP2)
    ENDIF
  ELSE
    RULE=1
    STIF=SLOP1
  ENDIF
  IF( DEMAX.LT. ABS(X) ) THEN
    DEMAX=ABS(X)
    DUC(1)=DEMAX/HA
  ENDIF
  RETURN
ENDIF
C
C ---RULE 2:
IF( RULE.EQ. 2 ) THEN
  IF( HIGH.AND. DX.GT. 0 ) THEN
    RULE=2
    STIF=SLOP2
  ELSE IF( .NOT. HIGH.AND. DX.LT. 0 ) THEN
    RULE=2
    STIF=SLOP2
  ELSE IF( HIGH.AND. DX.LE. 0 ) THEN
    RULE=1
    DTOP=XLA
    RULE=1
    PBOT=PTOP - 2*VA
    DBOT=DTOP - (PTOP-PBOT)/SLOP1
    *** UNBALANCED FORCE ***
    UD=X-DTOP
    UP=UD*SLOP1
    P=P+PTOP+UP
  ELSE IF( .NOT. HIGH.AND. DX.GE. 0 ) THEN
    RULE=1
    PTOP=PLA
    DBOT=XLA
    STIF=SLOP1
    PTOP=PBOT + 2*VA
    DTOP=DBOT + (PTOP-PBOT)/SLOP1
    *** UNBALANCED FORCE ***
    UD=X-DBOT
    UP=UD*SLOP1
    P=P+PTOP+UP
  ENDIF
  IF( DEMAX.LT. ABS(X) ) THEN
    DEMAX=ABS(X)
    DUC(1)=DEMAX/HA
  ENDIF
  RETURN
ENDIF
C
***** HYST12 *****
DEBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
END DEBUG
C
C
SUBROUTINE HYST12(TOPT, NSRG, IS, EM, LIBN, HH, UU,HYST00100
& WW, Z, INEB, INEH, ST, IREV1,IREV2,HYST00110
& IREV3, IREV4,IECOP,ECCX0, ECCTO, NUMP, HYST00120

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4      ELS , SMALL,RATIOX,RATIOY, TOTA, IMD,          HTS00130      BETA(I)=0.          HTS01550
4      ISPE , DELT2,S2 , SP ,ISTART, PRINT,          HTS00140      DO 50 J=1,NUMP      HTS01560
4      IELNO ,ISTIP ,IAUTO,IMATER,                  HTS00150      U(I,J)=UO(J)      HTS01570
4      RATIO3 ,IR , G ,QRNBE ,RNEBE,                HTS00160      V(I,J)=VO(J)      HTS01580
C      C      RNP,R1PM, BETA, BCK, ECT, U, V, UO, VO, GCD,          HTS00180      50 CONTINUE          HTS01590
4      EL, D1, DO, DDO, TL, XLOC, FLOC, FLOB, XGLO,          HTS00190      IF (IECOP .EQ. 1 ) THEN          HTS01600
4      EO, EI, E2, TE, PR, TIR, SEASAT, STP,RMDIAG,          HTS00200      BCK(1)=ECCXO          HTS01610
4      SEASAT, TPJT, DP, DUMMT, PSK, ECX, ECCT,          HTS00210      ECX(2)=ECCXO          HTS01620
4      F1,FACTOR,DCOPT,ASAT,E,DE,                  HTS00220      ECT(1)=ECCXO          HTS01630
4      AO, B ,STRP,DEP, EOP,STRP,TEP,EREP,SREP,FUL,FLP)          HTS00230      ECT(2)=ECCXO          HTS01640
C      C      AKI, DUC,EXCR,DEL, ESE, PSE, PSEOLD,CSE)          HTS00240      ENDF          HTS01650
C      C      IOPT = 1 : FORMULATE THE INITIAL MEMBER STIFFNESS          HTS00250      IF ( PRINT ) THEN          HTS01660
C      C      = 2 : FORMULATE THE NONLINEAR MEMBER STIFFNESS          HTS00260      WRITE(6,543)IELNO          HTS01670
C      C      = INITIAL IMPERFECTION OF THE MEMBER IN X DIRECTION          HTS00270      543 FORMAT('1, /THE STABILITY MEMBER ',15,          HTS01680
C      C      = INITIAL IMPERFECTION OF THE MEMBER IN Y DIRECTION          HTS00280      ' SECTIONAL LOCAL COORDINATE')          HTS01690
C      C      = ECCENTRIC LOAD OPTION          HTS00290      WRITE(6,544)          HTS01700
C      C      = ECCENTRIC DISTANCE IN X DIRECTION          HTS00300      544 FORMAT('X, ' SEC. ELEMENT U V AREA          HTS01710
C      C      = ECCENTRIC DISTANCE IN Y DIRECTION          HTS00310      DO 545 I=1,NUMP          HTS01720
C      C      = TOTAL SECTIONAL POINT ON THE SEGMENT CROSS-SECTION          HTS00320      WRITE(6,546)I,UO(I),VO(I),AO(I),B(I)          HTS01730
C      C      = MEMBER GLOBAL STIFFNESS MATRIX          HTS00330      546 FORMAT('15,2X,JG10.4,2X,1P,G10.4)          HTS01740
C      C      = CONDENSED MATRIX          HTS00340      545 CONTINUE          HTS01750
C      C      = RESIDUAL MATRIX          HTS00350      ENDF          HTS01760
C      C      = MEMBER'S DISPLACEMENT INCREMENT AT EACH D.O.F.          HTS00360      C READ EACH NODAL POINTS' GLOBAL COORDINATES          HTS01780
C      C      = TOTAL NO OF SEGMENTS IN THE MEMBER          HTS00370      DO 203 I=1,NSEG          HTS01790
C      C      = TOTAL D.O.F. OF THE MEMBER          HTS00400      DO 203 J=1,2          HTS01800
C      C      = TOTAL LENGTH OF THE MEMBER          HTS00410      DO 203 K=1,3          HTS01820
C      C      = THE CROSS-SECTION LIBRARY NO.          HTS00420      GCD(I,J,K)=0.          HTS01830
C      C      = FLAG FOR JUDGING LOCAL BUCKLING          HTS00430      203 CONTINUE          HTS01840
C      C      = 1-4 LOCAL BUCKLING OCCURRED          HTS00440      TEMP=0          HTS01850
C      C      = THE STABILITY MEMBER NO.          HTS00450      DO 307 I=1,NSEG          HTS01860
C      C      = SYSTEM MEMBER NO.          HTS00460      IF(NSEG .EQ. 1 ) THEN          HTS01870
C      C      = INDEX FOR LIMITING NORM OF DISPLACEMENT          HTS00470      SEL=ELS          HTS01880
C      C      = 1 : EEE .LT. QRNBE          HTS00480      GCD(I,1,3)=TEMP          HTS01890
C      C      = 1 : EEE .GT. QRNBE          HTS00490      GCD(I,2,3)=TEMP+SEL          HTS01900
C      C      = DISTANCE FROM REFERENCE COORDINATE (U,V,W) ORIGIN TO          HTS00510      ELSE IF ( I .EQ. 1 .OR. I .EQ. ((NSEG-2)/2)+1          HTS01920
C      C      = ECCENTRIC LOAD LOCATION IN U DIRECTION          HTS00520      .OR. I .EQ. ((NSEG-2)/2)+2 .OR. I .EQ. NSEG          HTS01930
C      C      = DISTANCE FROM REFERENCE COORDINATE (U,V,W) ORIGIN TO          HTS00530      .AND. NSEG .NE. 1 ) THEN          HTS01940
C      C      = ECCENTRIC LOAD LOCATION IN V DIRECTION          HTS00540      SEL=SMALL          HTS01950
C      C      = 1 : EEE .LT. QRNBE          HTS00550      EL(I)=SEL          HTS01960
C      C      = 1 : EEE .GT. QRNBE          HTS00560      GCD(I,1,3)=TEMP          HTS01970
C      C      = DISTANCE FROM REFERENCE COORDINATE (U,V,W) ORIGIN TO          HTS00580      GCD(I,2,3)=TEMP+SEL          HTS01980
C      C      = ECCENTRIC LOAD LOCATION IN U DIRECTION          HTS00590      ELSE IF ( I .NE. 1 .OR. I .NE. ((NSEG-2)/2)+1          HTS02000
C      C      = ECCENTRIC LOAD LOCATION IN V DIRECTION          HTS00600      .OR. I .NE. ((NSEG-2)/2)+2 .OR. I .NE. NSEG          HTS02010
C      C      = 1 : EEE .LT. QRNBE          HTS00610      .AND. NSEG .NE. 1 ) THEN          HTS02020
C      C      = 1 : EEE .GT. QRNBE          HTS00620      SEL=SMALL/(NSEG-4)          HTS02030
C      C      = DISTANCE FROM REFERENCE COORDINATE (U,V,W) ORIGIN TO          HTS00630      EL(I)=SEL          HTS02040
C      C      = ECCENTRIC LOAD LOCATION IN U DIRECTION          HTS00640      GCD(I,1,3)=TEMP          HTS02050
C      C      = ECCENTRIC LOAD LOCATION IN V DIRECTION          HTS00650      GCD(I,2,3)=TEMP+SEL          HTS02060
C      C      = 1 : EEE .LT. QRNBE          HTS00660      TEMP=TEMP+SEL          HTS02070
C      C      = 1 : EEE .GT. QRNBE          HTS00670      ENDF          HTS02080
C      C      DIMENSION NP( 32,12 ), IFM( 198 ), BETA(NSEG),ECX(2),ECT(2)          HTS00680          HTS02090
C      C      DIMENSION U(NSEG,NUMP),V(NSEG,NUMP),UO(NUMP),VO(NUMP)          HTS00690          HTS02100
C      C      DIMENSION GCD(NSEG,2,3),D1((NSEG+1)*6,1),DO((NSEG+1)*6,1),          HTS00700          HTS02110
C      C      DP((NSEG+1)*6,1),ASAT((NSEG+1)*6,(NSEG+1)*6)          HTS00710          HTS02120
C      C      DIMENSION DOO(NSEG+1)*6,11,EL(NSEG), TL(NSEG), XLOC(NSEG,12)          HTS00720          HTS02130
C      C      DIMENSION FLOC(NSEG,12),FLOB(NSEG,12),KGLC(NSEG,12),EO(NSEG,NUMP)          HTS00730          HTS02140
C      C      DIMENSION EI(NSEG,NUMP),E2(NSEG,NUMP),TE(NUMP),PR(NSEG,9)          HTS00740          HTS02150
C      C      DIMENSION E(NUMP),DE(NSEG,NUMP)          HTS00750          HTS02160
C      C      DIMENSION TIR(NSEG,9),SK(12,12),MTRK(12,12),          HTS00760          HTS02170
C      C      SEASAT((NSEG+1)*6,(NSEG+1)*6)          HTS00770          HTS02180
C      C      DIMENSION STP(NSEG+1)*6((NSEG+1)*6+1)/2),STR(NSEG,NUMP)          HTS00780          HTS02190
C      C      DIMENSION MDIAG( 199 ),DCOPT((NSEG+1)*6,1)          HTS00790          HTS02200
C      C      DIMENSION FACTOR((NSEG+1)*6),SEASAT(12,12),TPJT(12,12),TEMP(12)          HTS00800          HTS02210
C      C      DIMENSION DUMMT((NSEG+1)*6,(NSEG+1)*6)          HTS00810          HTS02220
C      C      DIMENSION R(12,12),PSK(NSEG,12,12), PGLO(12,12)          HTS00820          HTS02230
C      C      DIMENSION BCKX(2),ECCT(2),AO(NUMP),B(NUMP),FF((NSEG+1)*6,1)          HTS00830          HTS02240
C      C      DIMENSION DEP(NSEG,NUMP),EOP(NSEG,NUMP),          HTS00840          HTS02250
C      C      STRP(NSEG,NUMP),TEP(NSEG,NUMP),          HTS00850          HTS02260
C      C      EREP(NSEG,NUMP),SREP(NSEG,NUMP)          HTS00860          HTS02270
C      C      DIMENSION DUC(3),EXCR(6),ESE(3)          HTS00870          HTS02280
C      C      DIMENSION RNP(NSEG,12),R1PM((NSEG+1)*6),RMDIAG((NSEG+1)*6+1)          HTS00880          HTS02290
C      C      DIMENSION FUL(NUMP)          HTS00890          HTS02300
C      C      LOCAL PRINT BUGS          HTS00900          HTS02310
C      C      REAL INSB, INSH          HTS00910          HTS02320
C      C      BUGS=.FALSE.          HTS00920          HTS02330
C      C      FLP=0          HTS00930          HTS02340
C      C      GO TO (740, 740, 200),IOPT          HTS00940          HTS02350
C      C      740 CONTINUE          HTS00950          HTS02360
C      C      C      DEFINE PARAMETERS          HTS00960          HTS02370
C      C      IDOF=(NSEG+1)*6          HTS00970          HTS02380
C      C      EY=TS/EM          HTS00980          HTS02390
C      C      C      DEFINE LOCAL BUCKLING FLAG          HTS00990          HTS02400
C      C      DO 666 IK=1,NUMP          HTS01000          HTS02410
C      C      666 FUL(IK)=0          HTS01010          HTS02420
C      C      C      DEFINE SEGMENT GLOBAL AND LOCAL D.O.F. RELATION          HTS01020          HTS02430
C      C      NP(1,1)=IDOF-12+1          HTS01030          HTS02440
C      C      NP(1,2)=IDOF-12+2          HTS01040          HTS02450
C      C      NP(1,3)=IDOF-12+3          HTS01050          HTS02460
C      C      NP(1,4)=IDOF-12+4          HTS01060          HTS02470
C      C      NP(1,5)=IDOF-12+5          HTS01070          HTS02480
C      C      NP(1,6)=IDOF-12+6          HTS01080          HTS02490
C      C      NP(NSEG,7)=IDOF-12+7          HTS01090          HTS02500
C      C      NP(NSEG,8)=IDOF-12+8          HTS01100          HTS02510
C      C      NP(NSEG,9)=IDOF-12+9          HTS01110          HTS02520
C      C      NP(NSEG,10)=IDOF-12+10          HTS01120          HTS02530
C      C      NP(NSEG,11)=IDOF-12+11          HTS01130          HTS02540
C      C      NP(NSEG,12)=IDOF-12+12          HTS01140          HTS02550
C      C      IF ( NSEG .EQ. 1 ) GO TO 3060          HTS01150          HTS02560
C      C      DO 304 I=1,NSEG-1          HTS01160          HTS02570
C      C      DO 304 J=7,12          HTS01170          HTS02580
C      C      K=I+6          HTS01180          HTS02590
C      C      NP(I,J)=K+(J-6)          HTS01190          HTS02600
C      C      304 CONTINUE          HTS01200          HTS02610
C      C      DO 306 I=2,NSEG          HTS01210          HTS02620
C      C      DO 306 J=1,6          HTS01220          HTS02630
C      C      K=I+6          HTS01230          HTS02640
C      C      NP(I,J)=K+(J-6)          HTS01240          HTS02650
C      C      306 CONTINUE          HTS01250          HTS02660
C      C      3060 CONTINUE          HTS01260          HTS02670
C      C      C      DEFINE FORCE-MOMENT INDEX          HTS01270          HTS02680
C      C      DO 9012 I=1,1DOF          HTS01280          HTS02690
C      C      IFM(I)=0          HTS01290          HTS02700
C      C      IFM(IDOF-12+4)=1          HTS01300          HTS02710
C      C      IFM(IDOF-12+5)=1          HTS01310          HTS02720
C      C      IFM(IDOF-12+6)=1          HTS01320          HTS02730
C      C      IFM(IDOF-12+10)=1          HTS01330          HTS02740
C      C      IFM(IDOF-12+11)=1          HTS01340          HTS02750
C      C      IFM(IDOF-12+12)=1          HTS01350          HTS02760
C      C      IF ( IDOF .EQ. 12 ) GO TO 4535          HTS01360          HTS02770
C      C      DO 308 I=1,NSEG-1          HTS01370          HTS02780
C      C      J=I+6          HTS01380          HTS02790
C      C      IFM(J-2)=1          HTS01390          HTS02800
C      C      IFM(J-1)=1          HTS01400          HTS02810
C      C      IFM(J)=1          HTS01410          HTS02820
C      C      308 CONTINUE          HTS01420          HTS02830
C      C      4535 CONTINUE          HTS01430          HTS02840
C      C      C      DEFINE SECTION POINTS AND SECTIONAL LOCAL COORDINATES          HTS01440          HTS02850
C      C      CALL LIB(LIB,HH,UU,WW,25,INWB,INEN,ST,          HTS01450          HTS02860
C      C      TREV1,TREV2,TREV3,TREV4,IECOP,IELNO,          HTS01460          HTS02870
C      C      UO,VO,AO,B,ECXCO,ECCTO,NUMP)          HTS01470          HTS02880
C      C      DO 50 I=1,NSEG          HTS01480          HTS02890
C      C      BETA(I)=0.          HTS01490          HTS02900
C      C      DO 325 J=1,NUMP          HTS01500          HTS02910
C      C      EO(I,J)=0          HTS01510          HTS02920
C      C      B1(I,J)=EY          HTS01520          HTS02930
C      C      B2(I,J)=-EY          HTS01530          HTS02940
C      C      B3(I,J)=0          HTS01540          HTS02950
C      C      B4(I,J)=0          HTS01550          HTS02960

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```
TE(J)=EM
325 CONTINUE
DO 112 I=1,NSEG
C FINE THE ROTATION MATRIX DUE TO INITIAL IMPERFECTION EFFECT
PR(I,1)=1.
PR(I,2)=0.
PR(I,3)=0.
PR(I,4)=0.
PR(I,5)=1.
PR(I,6)=0.
PR(I,7)=0.
PR(I,8)=0.
PR(I,9)=1.
XA=D1(NP(I,1),1)+GCD(I,1,1)+D00(NP(I,1),1)
YA=D1(NP(I,2),1)+GCD(I,1,2)+D00(NP(I,2),1)
ZA=D1(NP(I,3),1)+GCD(I,1,3)+D00(NP(I,3),1)
ROXA=D1(NP(I,4),1)+D00(NP(I,4),1)
ROTA=D1(NP(I,5),1)+D00(NP(I,5),1)
ROZA=D1(NP(I,6),1)+D00(NP(I,6),1)
XB=D1(NP(I,7),1)+GCD(I,2,1)+D00(NP(I,7),1)
YB=D1(NP(I,8),1)+GCD(I,2,2)+D00(NP(I,8),1)
ZB=D1(NP(I,9),1)+GCD(I,2,3)+D00(NP(I,9),1)
ROXB=D1(NP(I,10),1)+D00(NP(I,10),1)
ROYB=D1(NP(I,11),1)+D00(NP(I,11),1)
ROZB=D1(NP(I,12),1)+D00(NP(I,12),1)
DXA=DD(NP(I,1),1)
DYA=DD(NP(I,2),1)
DZA=DD(NP(I,3),1)
DROXA=DD(NP(I,4),1)
DROTA=DD(NP(I,5),1)
DROZA=DD(NP(I,6),1)
DXB=DD(NP(I,7),1)
DYB=DD(NP(I,8),1)
DZB=DD(NP(I,9),1)
DROXB=DD(NP(I,10),1)
DROYB=DD(NP(I,11),1)
DROZB=DD(NP(I,12),1)
IF(PRINT) THEN
WRITE(6,8088)
WRITE(6,8089)XA,YA,ZA,ROXA,ROTA,ROZA,
XB,YB,ZB,ROXB,ROYB,ROZB,
ROMA,RDMD,VXO,VTO,V50,W50,WTO,W50,I,
R,PR,BETA,TL,PRINT,NSEG,I,ELNO)
ENDIF
C VARIABLES FOR THE ROTATIONAL MATRIX DUE TO INITIAL IMPERFECTION
UXO=PR(I,1)
UYO=PR(I,2)
UZO=PR(I,3)
VXO=PR(I,4)
VYO=PR(I,5)
VZO=PR(I,6)
WYO=PR(I,7)
WTO=PR(I,8)
WZO=PR(I,9)
C CALCULATE ROTATIONAL MATRIX OF SEGMENT
CALL ROMA (XA, YA, ZA, ROXA, ROTA, ROZA, DXA, DYA, DZA, DROXA, DROTA, DROZA,
XB, YB, ZB, ROXB, ROYB, ROZB, DXB, DYB, DZB, DROXB, DROYB, DROZB,
ROMA, RDMD, VXO, VTO, V50, W50, WTO, W50, I,
R, PR, BETA, TL, PRINT, NSEG, I, ELNO)
C TIR(I,1)=R(1,1)
TIR(I,2)=R(1,2)
TIR(I,3)=R(1,3)
TIR(I,4)=R(2,1)
TIR(I,5)=R(2,2)
TIR(I,6)=R(2,3)
TIR(I,7)=R(3,1)
TIR(I,8)=R(3,2)
TIR(I,9)=R(3,3)
C GENERATE THE INITIAL ROTATIONAL MATRIX OR PREVIOUS ROTATIONAL MATRIX
C FIND SEGMENTS' PRINCIPAL AXIS AND MATERIAL PROPERTIES
C ( SUBROUTINE STRATE AND JUDEL ARE CALLED BY PRINC )
CALL PRINC( D, D, I, PRINT, ST, TOTA, LIBN,
EM, ET, G, TS, IOPT, NUMP, NSEG,
EA, EIU, EIV, GMT,
IREV1, IREV2, IREV3, IREV4, UCO, VCO, IELNO, IMATER,
AG, B, BETA, ED, EI, E2, U, V, UO, VO, TS, BS, DS,
INB, INEH, FJL,
RATIO3, IR, STR, DEP, BDP, STRP, TEP, BREP, SREP, FLP)
C CALCULATE ECCENTRICITIES OF TWO END-SEGMENTS
IF( I .EQ. 1 .OR. I .EQ. NSEG ) THEN
IF( IBCOP .EQ. 1 ) THEN
CALL ECCENT(NSEG,UCO,VCO,I,O,D,BCX,
B,CCY,CCX,CCZ,PRINT )
ENDIF
ENDIF
C CALCULATE INITIAL ROTATION MATRIX OF SEGMENT
XA=D1(NP(I,1),1)+GCD(I,1,1)+D00(NP(I,1),1)
YA=D1(NP(I,2),1)+GCD(I,1,2)+D00(NP(I,2),1)
ZA=D1(NP(I,3),1)+GCD(I,1,3)+D00(NP(I,3),1)
ROXA=D1(NP(I,4),1)+D00(NP(I,4),1)
ROTA=D1(NP(I,5),1)+D00(NP(I,5),1)
ROZA=D1(NP(I,6),1)+D00(NP(I,6),1)
XB=D1(NP(I,7),1)+GCD(I,2,1)+D00(NP(I,7),1)
YB=D1(NP(I,8),1)+GCD(I,2,2)+D00(NP(I,8),1)
ZB=D1(NP(I,9),1)+GCD(I,2,3)+D00(NP(I,9),1)
ROXB=D1(NP(I,10),1)+D00(NP(I,10),1)
ROYB=D1(NP(I,11),1)+D00(NP(I,11),1)
ROZB=D1(NP(I,12),1)+D00(NP(I,12),1)
DXA=DD(NP(I,1),1)
DYA=DD(NP(I,2),1)
DZA=DD(NP(I,3),1)
DROXA=DD(NP(I,4),1)
DROTA=DD(NP(I,5),1)
DROZA=DD(NP(I,6),1)
DXB=DD(NP(I,7),1)
DYB=DD(NP(I,8),1)
DZB=DD(NP(I,9),1)
DROXB=DD(NP(I,10),1)
DROYB=DD(NP(I,11),1)
DROZB=DD(NP(I,12),1)
IF(PRINT) THEN
WRITE(6,8088)
WRITE(6,8089)XA, YA, ZA, ROXA, ROTA, ROZA,
XB, YB, ZB, ROXB, ROYB, ROZB,
ROMA, RDM, D, D, I, PRINT, ST, TOTA, LIBN,
EM, ET, G, TS, IOPT, NUMP, NSEG,
EA, EIU, EIV, GMT,
IREV1, IREV2, IREV3, IREV4, UCO, VCO, IELNO, IMATER,
AG, B, BETA, ED, EI, E2, U, V, UO, VO, TS, BS, DS,
INB, INEH, FJL,
RATIO3, IR, STR, DEP, BDP, STRP, TEP, BREP, SREP, FLP)
ENDIF
C VARIABLES FOR THE INITIAL ROTATIONAL MATRIX
UXO=TIR(I,1)
UYO=TIR(I,2)
UZO=TIR(I,3)
VXO=TIR(I,4)
VYO=TIR(I,5)
VZO=TIR(I,6)
WYO=TIR(I,7)
WTO=TIR(I,8)
WZO=TIR(I,9)
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HTSO2970 VTO=TIR(I,5) HTSO4390
HTSO2980 V50=TIR(I,6) HTSO4400
HTSO2990 WYO=TIR(I,7) HTSO4410
HTSO3000 WYO=TIR(I,8) HTSO4420
HTSO3010 WZO=TIR(I,9) HTSO4430
HTSO3020 C CALCULATE ROTATIONAL MATRIX OF SEGMENT HTSO4440
HTSO3030 CALL ROMA (XA, YA, ZA, ROXA, ROTA, ROZA, DXA, DYA, DZA, DROXA, DROTA, DROZA, HTSO4450
* XB, YB, ZB, ROXB, ROYB, ROZB, DXB, DYB, DZB, DROXB, DROYB, DROZB, HTSO4460
* ROMA, RDMD, VXO, VTO, V50, W50, WTO, W50, I, HTSO4470
* R, PR, BETA, TL, PRINT, NSEG, I, ELNO) HTSO4480
DEC=0. HTSO4490
DCU=0. HTSO4500
DCV=0. HTSO4510
HTSO3100 HTSO4520
HTSO3110 HTSO4530
HTSO3120 HTSO4540
HTSO3130 HTSO4550
HTSO3140 IF(PRINT) THEN HTSO4560
HTSO3150 WRITE(6,5321) HTSO4570
HTSO3160 532 FORMAT('X, SEGMENT ',I5,' THE INITIAL ROTATIONAL MATRIX: ') HTSO4580
HTSO3170 WRITE(6,533)(TIR(I,J),J=1,9) HTSO4590
HTSO3180 533 FORMAT(3G15.6) HTSO4600
HTSO3190 ENDF HTSO4610
HTSO3200 C FORMULATE THE SEGMENT'S LOCAL STIFFNESS MATRIX HTSO4620
HTSO3210 CALL ELESTI( NSEG,I,IELNO,PRINT,ISTIF, HTSO4630
* EA,EIU,EIV,GKT,EL,FLOC,PSK,SK ) HTSO4640
HTSO3220 C TRANSFER SEGMENT'S LOCAL STIFFNESS TO GLOBAL STIFFNESS HTSO4650
HTSO3230 DO 707 II=1,12 HTSO4660
HTSO3240 DO 707 JJ=1,12 HTSO4670
HTSO3250 DUMMY(II,JJ)=0 HTSO4680
HTSO3260 DO 707 KK=1,12 HTSO4690
HTSO3270 DUMMY(II,JJ)=DUMMY(II,JJ)+SK(II,KK)*R(KK,JJ) HTSO4700
HTSO3280 HTSO4710
HTSO3290 DO 708 II=1,12 HTSO4720
HTSO3300 DO 708 JJ=1,12 HTSO4730
HTSO3310 RTSKR(II,JJ)=0 HTSO4740
HTSO3320 DO 708 KK=1,12 HTSO4750
HTSO3330 RTSKR(II,JJ)=RTSKR(II,JJ)+R(KK,II)*DUMMY(KK,JJ) HTSO4760
HTSO3340 HTSO4770
HTSO3350 C ASSEMBLING SEGMENT GLOBAL STIFFNESS TO MEMBER HTSO4780
HTSO3360 C CENTROID STIFFNESS MATRIX HTSO4790
HTSO3370 DO 85 J=1,12 HTSO4800
HTSO3380 DO 85 JJ=1,12 HTSO4810
HTSO3390 ASAT(NP(I,J),NP(I,JJ))=ASAT(NP(I,J),NP(I,JJ))+RTSKR(J,J) HTSO4820
HTSO3400 85 CONTINUE HTSO4830
HTSO3410 112 CONTINUE HTSO4840
HTSO3420 C PRINT OUT THE STABILITY MEMBER STIFFNESS MATRIX HTSO4850
HTSO3430 IF( PRINT ) THEN HTSO4860
HTSO3440 WRITE(6,1200)IELNO HTSO4870
HTSO3450 1200 FORMAT('1, THE STABILITY MEMBER',I3,' ASAT MATRIX: ') HTSO4880
HTSO3460 DO 1300 I=1,IDOF HTSO4890
HTSO3470 WRITE(6,1340)(ASAT(I,J),J=1,IDOF) HTSO4900
HTSO3480 1340 FORMAT(IX,6G12.6) HTSO4910
HTSO3490 1300 CONTINUE HTSO4920
HTSO3500 ENDF HTSO4930
HTSO3510 C MEMORIZE THE STABILITY ELEMENT STIFFNESS HTSO4940
HTSO3520 DO 986 I=1,IDOF HTSO4950
HTSO3530 DO 986 J=1,IDOF HTSO4960
HTSO3540 SEASAT(I,J)=ASAT(I,J) HTSO4970
HTSO3550 986 CONTINUE HTSO4980
HTSO3560 C CONVERT ASAT MATRIX TO HALF STORAGE AND BANDED MODE HTSO4990
HTSO3570 NDOST=IDOF HTSO5000
HTSO3580 JSTOR=NDOST*(NDOST+1)/2 HTSO5010
HTSO3590 CALL SGT(ASAT,STF,NDOST,JSTOR,IDOF,MDIAG,IMD) HTSO5020
HTSO3600 C FIND CONDENSED STIFFNESS MATRIX CORRESPONDING TO TWO-END 12 DOF HTSO5030
HTSO3610 IF( IDOF .EQ. 12 ) GO TO 4532 HTSO5040
HTSO3620 L1=1 HTSO5050
HTSO3630 L2=IDOF-12 HTSO5060
HTSO3640 CALL GAUSS2(1, PRINT, IDOF, IMD, NDOST, L1, L2, MDIAG, STF, DRCPDT, HTSO5070
$ FACTOR, JSTOR) HTSO5080
HTSO3650 4532 CONTINUE HTSO5090
HTSO3660 C CONDENSE ASAT MATRIX TO GET CONDENSED MATRIX CORRESPONDING TO HTSO5100
HTSO3670 TWO ENDS TWELVE DOF DIRECTIONS HTSO5110
HTSO3680 HTSO5120
HTSO3690 L1=IDOF-12+1 HTSO5130
HTSO3700 L2=IDOF HTSO5140
HTSO3710 CALL CONVER(PRINT, STF, SEASAT, NDOST, JSTOR, IDOF, IMD, HTSO5150
$ L1, L2, MDIAG) HTSO5160
HTSO3720 HTSO5170
HTSO3730 IF( IBCOP .EQ. 1 ) THEN HTSO5180
HTSO3740 CALL TRANK(EASAT, BCCK, ECCT, TPJT) HTSO5190
HTSO3750 IF(PRINT) THEN HTSO5200
HTSO3760 WRITE(6,'*1,') THE FINAL SEASAT MATRIX AFTER CALL TRANK HTSO5210
HTSO3770 CALL PRIN( EASAT, 12, 12, 12) HTSO5220
HTSO3780 ENDF HTSO5230
HTSO3790 ENDF HTSO5240
HTSO3800 DO 365 I=1, NSEG HTSO5250
HTSO3810 DO 365 J=1, 12 HTSO5260
HTSO3820 RNP(I, J)=NP(I, J) HTSO5270
HTSO3830 DO 326 I=1, (NSEG+1)*6 HTSO5280
HTSO3840 RIFM(I)=IFM(I) HTSO5290
HTSO3850 DO 327 I=1, (NSEG+1)*6+1 HTSO5300
HTSO3860 RMDIAG(I)=MDIAG(I) HTSO5310
HTSO3870 C RETURN HTSO5320
HTSO3880 HTSO5330
HTSO3890 HTSO5340
HTSO3900 HTSO5350
HTSO3910 HTSO5360
HTSO3920 HTSO5370
HTSO3930 HTSO5380
HTSO3940 HTSO5390
HTSO3950 HTSO5400
HTSO3960 HTSO5410
HTSO3970 HTSO5420
HTSO3980 HTSO5430
HTSO3990 HTSO5440
HTSO4000 HTSO5450
HTSO4010 DO 365 I=1, NSEG HTSO5460
HTSO4020 DO 365 J=1, 12 HTSO5470
HTSO4030 RNP(I, J)=NP(I, J) HTSO5480
HTSO4040 DO 326 I=1, (NSEG+1)*6 HTSO5490
HTSO4050 RIFM(I)=IFM(I) HTSO5500
HTSO4060 DO 327 I=1, (NSEG+1)*6+1 HTSO5510
HTSO4070 RMDIAG(I)=MDIAG(I) HTSO5520
HTSO4080 HTSO5530
HTSO4090 C RETURN HTSO5540
HTSO4100 HTSO5550
HTSO4110 200 CONTINUE HTSO5560
HTSO4120 HTSO5570
HTSO4130 HTSO5580
HTSO4140 C DEFINE PARAMETERS HTSO5590
HTSO4150 IDOF=(NSEG+1)*6 HTSO5600
HTSO4160 NDOP=IDOF HTSO5610
HTSO4170 NDOST=IDOF HTSO5620
HTSO4180 JSTOR=NDOST*(NDOST+1)/2 HTSO5630
HTSO4190 ETS=ET/EM HTSO5640
HTSO4200 RNEB=EB HTSO5650
HTSO4210 IF( ISTART .LT. 5 ) THEN HTSO5660
HTSO4220 ISTART=ISTART+1 HTSO5670
HTSO4230 ELSE HTSO5680
HTSO4240 ISTART=5 HTSO5690
HTSO4250 ENDF HTSO5700
HTSO4260 DO 425 I=1, NSEG HTSO5710
HTSO4270 DO 425 J=1, 12 HTSO5720
HTSO4280 NP(I, J)=RNP(I, J) HTSO5730
HTSO4290 DO 426 I=1, (NSEG+1)*6 HTSO5740
HTSO4300 FACTOR(I)=0 HTSO5750
HTSO4310 IFW(I)=RIFW(I) HTSO5760
HTSO4320 DO 427 I=1, (NSEG+1)*6+1 HTSO5770
HTSO4330 MDIAG(I)=RMDIAG(I) HTSO5780
HTSO4340 HTSO5790
HTSO4350 HTSO5800
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C -----
C == INITIALIZE THE MEMBER'S ASAT MATRIX ==
C -----
C
C DO I=1,NDOF
C DO J=1,NDOF
C ASAT(I,J)=0.
C 17 CONTINUE
C
C DISPLACEMENT INCREMENT TRANSFORMATION DUE TO ECCENTRIC LOADS
C
C IF (IECOP.EQ.1) THEN
C DO I=1,12
C TEMP(I)=DD(I,1)
C 913 CONTINUE
C DO I=1,12
C DD(I,1)=0.
C DO I=1,12
C DD(I,1)=DD(I,1)+TPJT(I,KK)*TEMP(KK)
C 525 CONTINUE
C ENDDIF
C
C CONVERT MEMBER END DISPLACEMENT INCREMENT TO THE CORRESPONDING
C MEMBER'S DOF DIRECTION
C
C DO I=1,12
C TEMP(I)=DD(I,1)
C 908 CONTINUE
C DO I=1,12
C DD(IDOF-12+I,1)=TEMP(I)
C 837 CONTINUE
C
C BACK SUBSTITUTE TO GET MEMBER'S INTERNAL DOF DISPLACEMENT INCREMENT
C
C L1=IDOF-12+1
C L2=IDOF
C
C DO I=L1,L2
C FACTOR(I)=DD(I,1)
C IMD=MDIAG(IDOF+1)-1
C
C CALL GAUSS2(4, PRINT, IDOF, IMD, NDOST, 1, L1, L2, MDIAG, STP,
C DPCOY, FACTOR, JSTOR)
C
C DO I=L1,L2
C DP(I,1)=0
C DPCOY(I,1)=0
C 901 CONTINUE
C DO I=L1,L2
C DP(I,1)=DPCOY(I,1)
C 902 CONTINUE
C
C CALL GAUSS2(1, PRINT, IDOF, IMD, NDOST, 1, L1, L2, MDIAG, STP,
C DPCOY, FACTOR, JSTOR)
C
C CALL GAUSS2(3, PRINT, IDOF, IMD, NDOST, 1, L1, L2, MDIAG, STP,
C DPCOY, FACTOR, JSTOR)
C
C DO I=L1,L2
C FACTOR(I)=DD(I,1)
C 848 CONTINUE
C
C IF (IDOF.EQ.12) GO TO 4544
C
C =====
C == BACK SUBSTITUTE BEGIN ==
C =====
C
C L1=1
C L2=IDOF-12
C
C CALL GAUSS2(5, PRINT, IDOF, IMD, NDOST, 1, L1, L2, MDIAG, STP,
C DPCOY, FACTOR, JSTOR)
C
C 4544 CONTINUE
C
C DO I=1,12
C DD(I,1)=DD(I,1)+TEMP(I)
C 903 CONTINUE
C
C DO I=1,NSEGC
C DO J=1,12
C XGLO(I,J)=DD(NP(I,J),1)
C 219 CONTINUE
C
C ADD GLOBAL DISPLACEMENT RATE TO TOTAL DISPLACEMENT
C
C DO I=1,12
C DI(I,1)=DI(I,1)+DD(I,1)
C 1109 CONTINUE
C
C PRINT OUT STABILITY MEMBER INTERNAL DISPLACEMENT
C
C IF (PRINT) THEN
C WRITE(6,8855)IELNO
C 8855 FORMAT('1', 'THE STABILITY MEMBER ', '13', ' INTERNAL DISPLACEMENT: ')
C WRITE(6,873)(DI(I,1), I=1, IDOF)
C 873 FORMAT(1X, 6G13.6)
C ENDDIF
C
C FIND EACH SEGMENT INTERNAL FORCE IN GLOBAL DIRECTIONS
C
C ADJUST THE INITIAL IMPERFECTION DEFLECTION FOR NEW CYCLE, ISPE=1
C
C IF (ISPE.EQ.1) THEN
C DO I=1,NDOF
C DOO(I,1)=0.
C 210 CONTINUE
C
C DO I=1,NSEGC
C SZA=D(NP(I,3),1)+GCD(I,1,3)
C S2B=D(NP(I,9),1)+GCD(I,2,3)
C EL(I)=S2B-SZA
C 211 CONTINUE
C
C =====
C ASSUME INITIAL IMPERFECTION RATIO BASED ON MEMBER MIDDLE POINT
C DISPLACEMENT
C =====
C
C IDUM=(NSEG/2)+1
C RATIX=D(IDUM,1)/EL5
C RATIYO=D(IDUM+1,1)/EL6
C
C DO I=1,NSEGC
C DOO(NP(I,1),1)=(RATIX*EL5)*SIN(PI*
C 6 (D(NP(I,3),1)+GCD(I,1,3)-D(NP(I,9),1))/EL5)
C DOO(NP(I,7),1)=(RATIX*EL5)*SIN(PI*
C 6 (D(NP(I,9),1)+GCD(I,2,3)-D(NP(I,3),1))/EL5)
C DOO(NP(I,2),1)=(RATIX*EL5)*SIN(PI*
C 6 (D(NP(I,3),1)+GCD(I,1,3)-D(NP(I,9),1))/EL5)
C DOO(NP(I,8),1)=(RATIX*EL5)*SIN(PI*
C 6 (D(NP(I,9),1)+GCD(I,2,3)-D(NP(I,3),1))/EL5)
C 215 CONTINUE
C
C IF (PRINT) THEN
C WRITE(6,534)
C WRITE(6,734)
C DO I=1,12
C WRITE(6,536)I,DOO(I,1)
C 235 CONTINUE
C ENDDIF
C
C GENERATE THE SEGMENT LENGTH (INCLUDING THE INITIAL IMPERFECTION
C
C DO I=1,NSEGC
C EL(I)=SQRT(EL(I)**2+(DOO(NP(I,8),1)
C
HY505810 5 -DOO(NP(I,2),1))**2
HY505820 =DOO(NP(I,7),1)-DOO(NP(I,1),1))**2 )
HY505830 6 TL(I)=EL(I)
HY505840 922 CONTINUE
HY505850 WRITE(6,915)
HY505860 WRITE(6,916)
HY505870 WRITE(6,917)(I,EL(I),I=1,NSEGC)
HY505880 ENDDIF
C
C -----
C FIND EACH SEGMENT INTERNAL FORCE IN GLOBAL DIRECTIONS
C
C DO I=1,NSEGC
C CALL CLDAP(R,PR,I,NSEG,XGLO,PSK,SK,
C 6 FGL0,FGL0B,PRINT)
C
C FIND SEGMENT DEFORMATION AND INCREMENTS IN MEMBER COORDINATES
C DIRECTIONS
C
C IF (ISPE.EQ.1) THEN
C KA=0. +GCD(I,1,1)+DOO(NP(I,1),1)
C TA=0. +GCD(I,1,2)+DOO(NP(I,2),1)
C ZA=D(NP(I,3),1)+GCD(I,1,3)+DOO(NP(I,3),1)
C ROXA=0. +DOO(NP(I,4),1)
C ROYA=0. +DOO(NP(I,5),1)
C ROZA=D(NP(I,6),1)+DOO(NP(I,6),1)
C XB=0. +GCD(I,2,1)+DOO(NP(I,7),1)
C YB=0. +GCD(I,2,2)+DOO(NP(I,8),1)
C ZB=D(NP(I,9),1)+GCD(I,2,3)+DOO(NP(I,9),1)
C ROXB=0. +DOO(NP(I,10),1)
C ROYB=0. +DOO(NP(I,11),1)
C ROZB=D(NP(I,12),1)+DOO(NP(I,12),1)
C DXA=0.
C DYA=0.
C DZA=DD(NP(I,3),1)
C DRXA=0.
C DRYA=0.
C DRZA=DD(NP(I,6),1)
C DXB=0.
C DFB=0.
C DZB=DD(NP(I,9),1)
C DROXB=0.
C DROYB=0.
C DROZB=DD(NP(I,12),1)
C DI(NP(I,1),1)=0
C DI(NP(I,2),1)=0
C DI(NP(I,4),1)=0
C DI(NP(I,5),1)=0
C DI(NP(I,7),1)=0
C DI(NP(I,8),1)=0
C DI(NP(I,10),1)=0
C DI(NP(I,11),1)=0
C ELSE IF (ISPE.EQ.0) THEN
C XA=D(NP(I,3),1)+GCD(I,1,1)+DOO(NP(I,1),1)
C YA=D(NP(I,2),1)+GCD(I,1,2)+DOO(NP(I,2),1)
C ZA=D(NP(I,3),1)+GCD(I,1,3)+DOO(NP(I,3),1)
C ROXA=D(NP(I,4),1)+DOO(NP(I,4),1)
C ROYA=D(NP(I,5),1)+DOO(NP(I,5),1)
C ROZA=D(NP(I,6),1)+DOO(NP(I,6),1)
C XB=D(NP(I,7),1)+GCD(I,2,1)+DOO(NP(I,7),1)
C YB=D(NP(I,8),1)+GCD(I,2,2)+DOO(NP(I,8),1)
C ZB=D(NP(I,9),1)+GCD(I,2,3)+DOO(NP(I,9),1)
C ROXB=D(NP(I,10),1)+DOO(NP(I,10),1)
C ROYB=D(NP(I,11),1)+DOO(NP(I,11),1)
C ROZB=D(NP(I,12),1)+DOO(NP(I,12),1)
C DXA=DD(NP(I,1),1)
C DYA=DD(NP(I,2),1)
C DZA=DD(NP(I,3),1)
C DRXA=DD(NP(I,4),1)
C DRYA=DD(NP(I,5),1)
C DRZA=DD(NP(I,6),1)
C DXB=DD(NP(I,7),1)
C DFB=DD(NP(I,8),1)
C DZB=DD(NP(I,9),1)
C DROXB=DD(NP(I,10),1)
C DROYB=DD(NP(I,11),1)
C DROZB=DD(NP(I,12),1)
C ENDDIF
C
C IF (PRINT) THEN
C WRITE(6,808)
C 808B FORMAT('1', 'XA, YA, ZA, ROXA, ROYA, ROZA, ',
C ' XB, YB, ZB, ROXB, ROYB, ROZB, ',
C ' DXA, DYA, DZA, DROXA, DROYA, DROZA, ',
C ' DXB, DFB, DZB, DROXB, DROYB, DROZB /')
C WRITE(6,8089)XA, YA, ZA, ROXA, ROYA, ROZA,
C XB, YB, ZB, ROXB, ROYB, ROZB,
C DXA, DYA, DZA, DROXA, DROYA, DROZA,
C DXB, DFB, DZB, DROXB, DROYB, DROZB
C 8089 FORMAT(1X, 6F15.6)
C ENDDIF
C
C VARIABLES FOR THE INITIAL ROTATIONAL MATRIX
C
C UXO=TIR(I,1)
C UYO=TIR(I,2)
C UXO=TIR(I,3)
C VZO=TIR(I,4)
C VTO=TIR(I,5)
C VZO=TIR(I,6)
C WZO=TIR(I,7)
C WZO=TIR(I,8)
C WZO=TIR(I,9)
C
C CALCULATE SEGMENT CURVATURE RATE IN PRINCIPAL AXIS DIRECTIONS
C
C CALL CURVA( NSEG, I, DEC, DCU, DCV, PRINT, R, EL, PR,
C 6 DXA, DYA, DZA, DROXA, DROYA, DROZA,
C 6 DXB, DFB, DZB, DROXB, DROYB, DROZB )
C WRITE(6,*)I, ' SEGMENT', DEC, DCU, DCV, ' , DEC, DCU, DCV
C
C CALCULATE EACH SEGMENT'S PRINCIPAL AXIS AND MATERIAL PROPERTIES
C
C CALL PRINC(DEC, DCU, DCV, I, PRINT, ST, TOTA, LIBN,
C 6 EA, ST, G, YS, IOP, NUMP, NSEG,
C 6 EA, I, EV, GRT,
C 6 IREV1, IREV2, IREV3, IREV4, UCO, VCO, IELNO, IMATER,
C 6 AD, B, BETA, ED, EI, E2, U, V, UO, VO, TE, E, DE,
C 6 INEB, INEH, FJL,
C 6 RATIO3, IR, STR, DEP, EOP, STAP, TEP, EREF, SREF, FLP)
C
C CALCULATE ECCENTRICITIES OF TWO END-SEGMENTS
C
C IF (I.EQ.1 .OR. I.EQ.NSEG) THEN
C IF (IECOP.EQ.1) THEN
C 888 FORMAT(1X, I, G15.4)
C CALL ECCENT(NSEG, UCO, VCO, I, ROZA, ROZB, ECX,
C 6 ECT, ECCK, ECCT, PRINT)
C ENDDIF
C
C CALCULATE SEGMENT CURRENT ROTATIONAL MATRIX
C
C CALL ROMA (XA, YA, ZA, ROXA, ROYA, ROZA, DXA, DYA, DZA, DROXA, DROYA, DROZA,
C XB, YB, ZB, ROXB, ROYB, ROZB, DXB, DFB, DZB, DROXB, DROYB, DROZB,
C ROMAO, ROMBO, VZO, VTO, WZO, WFO, WFO, I,
C R, PR, BETA, TL, PRINT, NSEG, IELNO)
C
C CALCULATE EACH SEGMENT LOCAL FORCE
C
C CALL CSLF(R, NSEG, I, FLOC, FGL0, PRINT)
C
C CALCULATE ELEMENT STIFFNESS MATRIX

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C
DEC=(DOB(3,1)-DDA(3,1))/EL(ISEG)
DCU=(DOB(4,1)-DDA(4,1))/EL(ISEG)
DCV=(DOB(5,1)-DDA(5,1))/EL(ISEG)
C
RETURN
END
C
C-pa
DEBUG UNIT(6),SUBCHR
END DEBUG
C
SUBROUTINE PRINC(DOB,DCU,DCV,ISEG,PRINT,ST,TOTA,LIBN,
EM,ET,G,YS,IOPT,NUMP,NSEG,
EA,EU,EV,GT,
IREV1,IREV2,IREV3,IREV4,UCO,VCC,IELNO,IMATER,
AO,B,BETA,EO,E1,E2,U,V,UO,VO,TE,E,DE,
INEB,INEM,FJL,
RATIO3,IR,STR,DEP,EXP,STRP,TEP,EREF,SREF,FLP)
C
SUBROUTINE PRINC FOR CALCULATE PRINCIPAL AXES
C
C ECTX = X- DISTANCE FROM ECCENTRIC LOAD TO INSTANCE SHAPE-CENTER
C ECCT = Y- DISTANCE FROM ECCENTRIC LOAD TO INSTANCE SHAPE-CENTER
C SCX = X- DISTANCE FROM ECCENTRIC LOAD TO ORIGINAL SHAPE-CENTER
C SCY = Y- DISTANCE FROM ECCENTRIC LOAD TO ORIGINAL SHAPE-CENTER
C
DIMENSION AO(NUMP),B(NUMP),BETA(NSEG),DE(NSEG,NUMP)
DIMENSION EI(NUMP),EO(NSEG,NUMP),E1(NSEG,NUMP),E2(NSEG,NUMP)
DIMENSION TE(NUMP),U(NSEG,NUMP),V(NSEG,NUMP)
DIMENSION UO(NUMP),VO(NUMP),STR(NSEG,NUMP),FJL(NUMP)
DIMENSION DEP(NSEG,NUMP),EXP(NSEG,NUMP),
STRP(NSEG,NUMP),TEP(NSEG,NUMP),
EREF(NSEG,NUMP),SREF(NSEG,NUMP)
C
LOGICAL PRINT
REAL INEB, INEM
IELAS=1
C
CALCULATE STRAIN RATE OF EACH SECTION ELEMENT
C
DO 50 I=1,NUMP
DE(ISEG,I)=DEC+V(ISEG,I)*DCU-U(ISEG,I)*DCV
E(I)=EO(ISEG,I)+DE(ISEG,I)
EO(ISEG,I)=E(I)
50 CONTINUE
IF (IMATER.EQ.0)
CALL JUDEL(IELAS,NSEG,NUMP,ISEG,PRINT,IELNO,
EM,ET,RATIO3,YS,
DE,E,EO,E1,E2,TE,STR)
IF (IMATER.EQ.1)
CALL JUDEL2(IELAS,NSEG,NUMP,ISEG,PRINT,IELNO,
EM,ET,RATIO3,YS,
DE,E,EO,E1,E2,TE,STR)
IF (IMATER.EQ.2)
CALL JUDEL3(IELAS,NSEG,NUMP,ISEG,PRINT,IELNO,
EM,ET,IR,YS,
DE,E,EO,E1,E2,TE,
DEP,EXP,STRP,STR,TEP,EREF,SREF,IOPT)
IF (IMATER.EQ.3)
CALL JUDEL4(IELAS,NSEG,NUMP,ISEG,PRINT,IELNO,
EM,ET,RATIO3,YS,
DE,E,EO,E1,E2,TE,STR)
C
CHECK SECTION REACH TO PLASTIC HINGE OR NOT
C
IPL=1
DO 566 I=1,NUMP
IF (TE(I).EQ.EM) THEN
IPL=0
GO TO 996
ENDIF
566 CONTINUE
996 IF (PRINT) THEN
IF (PRINT.AND.IPL.EQ.0) THEN
WRITE(6,567)IELNO,ISEG
FORMAT(1X,'** THE STABILITY MEMBER ',15,
', SEGMENT ',15,' REACH TO PLASTIC HINGE **')
ENDIF
32 WRITE(6,32)IELNO,ISEG
FORMAT(1X,' TENGENT STIFFNESS FOR MEMBER ',13,
', SEGMENT ',15,' ')
WRITE(6,34)(TE(I),I=1,NUMP)
34 FORMAT(1X,5G15.6)
WRITE(6,321)
FORMAT(4X,'SEG. E1', DE EO)
321 WRITE(6,44)(I,E1(ISEG,I),E2(ISEG,I),DE(ISEG,I),
EO(ISEG,I),I=1,NUMP)
44 FORMAT(1X,15,4G15.6)
ENDIF
C
JUDGE LOCAL BUCKLING FOR BOX SECTION
C
IF (LIBN.EQ.1.AND.IREV1.NE.0) THEN
CALL BOXLOC(LIBN,FJL,STR,NSEG,NUMP,ISEG,INEB,INEM,
TE,ST,EM,UCO,VO,FLP)
ENDIF
C
CALCULATE THE SECTIONAL PROPERTIES ABOUT SECTIONAL REFERENCE
C AXES WITH ORIGIN AT (0,0)
C
EAO=0.
ESVD=0.
ESUD=0.
EIUD=0.
EIVD=0.
EIUV=0.
DO 60 I=1,NUMP
IF (FJL(I).EQ.0) THEN
EAO=EAO+TE(I)*AO(I)
ESVD=ESVD+TE(I)*VO(I)*AO(I)
ESUD=ESUD+TE(I)*UO(I)*AO(I)
EIUD=EIUD+TE(I)*VO(I)*VO(I)*AO(I)
EIVD=EIVD+TE(I)*UO(I)*UO(I)*AO(I)
EIUV=EIUV+TE(I)*UO(I)*VO(I)*AO(I)
ELSE IF (FJL(I).EQ.1) THEN
EAO=EAO+RATIO3*EM*AO(I)
ESVD=ESVD+RATIO3*EM*VO(I)*AO(I)
ESUD=ESUD+RATIO3*EM*UO(I)*AO(I)
EIUD=EIUD+RATIO3*EM*VO(I)*VO(I)*AO(I)
EIVD=EIVD+RATIO3*EM*UO(I)*UO(I)*AO(I)
EIUV=EIVD+RATIO3*EM*UO(I)*VO(I)*AO(I)
ENDIF
60 CONTINUE
IF (PRINT) THEN
WRITE(6,8856)IELNO,ISEG
8856 FORMAT(1X,' THE STABILITY MEMBER ',13,' SECTION ',13)
WRITE(6,901)
901 FORMAT(1X,'EAO,ESVD,ESUD,EIUD,EIVD')
WRITE(6,902)EAO,ESVD,ESUD,EIUD,EIVD
902 FORMAT(1X,1P,6G15.5)
ENDIF
C
THE NEW CENTROID LOCATION CORRESPONDING TO SECTION REFERENCE AXES
C
UCO=ESUD/EAO
VCO=ESVD/EAO
C
THE SECTIONAL PROPERTIES ABOUT SECTIONAL REFERENCE AXES WITH
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HTS11490 C ORIGIN AT ( UCO,VCO )
HTS11500 C
HTS11510 EIUI=EIUD-VCO*VCO+EAO
HTS11520 EIVI=EIVD-UCO*UCO+EAO
HTS11530 EIUVI=EIUV-UCO*VCO+EAO
HTS11540
HTS11550 IF( EIUI.LE.0..OR.EIVI.LE.0.) THEN
HTS11560 WRITE(6,*) '** INEFFICIENT SECTION ELEMENT ARRANGEMENT **'
HTS11570 WRITE(6,*) '-----ERROR-----ERROR-----'
HTS11580 WRITE(6,*) '** EIUI OR EIVI IS NEGATIVE **'
HTS11590 ENDFI
HTS11600
HTS11610 IF (PRINT) THEN
HTS11620 WRITE(6,109)
109 FORMAT(1X,'THE UCO ,VCO,EIUI,EIVI,EIUVI')
HTS11630 WRITE(6,867)UCO,VCO,EIUI,EIVI,EIUVI
867 FORMAT(1P,5G15.6)
HTS11640 ENDFI
C
HTS11680 IF( ABS((EIVI-EIUI)/EIUI) .LE. 0.00001 .AND.
HTS11690 ABS(EIUI/EM) .LE. 0.00001 ) THEN
HTS11700 BETA(ISEG)=0
HTS11710 ELSE IF( ABS((EIVI-EIUI)/EIUI) .LE. 0.00001 .AND.
HTS11720 ABS(EIUI/EM) .GT. 0.00001 ) THEN
HTS11730 BETA(ISEG)=0.5*ATAN2(2.*EIUI,0.)
HTS11740 ELSE IF( ABS((EIVI-EIUI)/EIUI) .GT. 0.00001 .AND.
HTS11750 ABS(EIUI/EM) .LE. 0.00001 ) THEN
HTS11760 BETA(ISEG)=0
HTS11770 ELSE IF( ABS((EIVI-EIUI)/EIUI) .GT. 0.00001 .AND.
HTS11780 ABS(EIUI/EM) .GT. 0.00001 ) THEN
HTS11790 BETA(ISEG)=0.5*ATAN2(2.*EIUI,(EIVI-EIUI))
HTS11800 ENDFI
C
HTS11820 -----
HTS11830 --- LIMIT BETA TO (90 DEGREE TO -90 DEGREE) ---
HTS11840 -----
HTS11850 P180=3.1415926
HTS11860 P90=3.1415926/2.
HTS11870 IF( BETA(ISEG) .GT. P90
HTS11880 .AND. BETA(ISEG) .LE. P180 ) THEN
HTS11890 BETA(ISEG)=BETA(ISEG)-P180
HTS11900 ELSE IF( BETA(ISEG) .GE. -P180
HTS11910 .AND. BETA(ISEG) .LT. -P90 ) THEN
HTS11920 BETA(ISEG)=BETA(ISEG)+P180
HTS11930 ENDFI
C
HTS11940 -----
HTS11950 --- AVOID UNSTABLE BETA ANGLE OCCURRED ---
HTS11960 --- NEAR 90, -90, AND 0 DEGREE ---
HTS11970 -----
HTS11980 IF( ABS(BETA(ISEG))-P90 .LT. 0.03 ) THEN
HTS11990 IF(BETA(ISEG) .LT. 0 ) BETA(ISEG)=-P90
HTS12000 IF(BETA(ISEG) .GT. 0 ) BETA(ISEG)=P90
HTS12010 ELSE IF( ABS(BETA(ISEG)) .LT. 0.03 ) THEN
HTS12020 BETA(ISEG)=0
HTS12030 ENDFI
C
HTS12060 IF( BETA(ISEG) .EQ. 0 ) THEN
HTS12070 EA=EAO
HTS12080 EIU=EIUI
HTS12090 EIV=EIVI
HTS12100 ELSE IF( BETA(ISEG) .NE. 0 ) THEN
HTS12110 EA=EAO
HTS12120 EIU=0.5*(EIUI+EIVI)-(EIUVI/SIN(2.*BETA(ISEG)))
HTS12130 EIV=0.5*(EIVI-EIUI)+(EIUVI/SIN(2.*BETA(ISEG)))
HTS12140 ENDFI
HTS12150
HTS12160 IF (PRINT) THEN
HTS12170 WRITE(6,309)
309 FORMAT(1X,'THE BETA,EA,EIU,EIV')
HTS12180 WRITE(6,867)BETA(ISEG),EA,EIU,EIV
HTS12190 ENDFI
C
C SECTION RIGIDITIES WITH RESPECT TO THE INSTANTANEOUS PRINCIPAL
C AXES U AND V
C
CALL SECRI(LIBN,ST,TOTA,BJ,IREV1,IREV2,IREV3,IREV4,IELNO,
B,NUMP)
GTG=BJ
C
CALCULATE PRESENT SECTIONAL ELEMENT COORDINATE
C ACCORDING TO INSTANTANEOUS AXES
C
DO 52 J=1,NUMP
U(ISEG,J)=COS(BETA(ISEG))*
(UO(J)-UCO)+SIN(BETA(ISEG))*
(VO(J)-VCO)
V(ISEG,J)=-SIN(BETA(ISEG))*
(UO(J)-UCO)+COS(BETA(ISEG))*
(VO(J)-VCO)
52 CONTINUE
IF (PRINT) THEN
WRITE(6,7776)
7776 FORMAT(1X,'THE U AND V IN PRINC SUBROUTINES')
WRITE(6,7778)(U(ISEG,J),V(ISEG,J),J=1,NUMP)
7778 FORMAT(1X,1P,2G15.6)
ENDIF
RETURN
END
C
HTS12500
HTS12510
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HTS12570
HTS12580
HTS12590
HTS12600
HTS12610
HTS12620
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HTS13900
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      SK(12,12)=SK(6,6)
      SK(7,11)=-SK(1,5)
      SK(8,10)=-SK(4,4)
      SK(1,7)=-SK(1,1)
      SK(2,8)=-SK(2,2)
      SK(3,9)=-SK(3,3)
      SK(4,10)=-SK(4,4)
      SK(5,11)=-SK(5,5)
      SK(6,12)=-SK(6,6)
      SK(1,11)=SK(1,5)
      SK(2,10)=SK(2,4)
      SK(4,8)=-SK(2,10)
      SK(5,7)=-SK(1,11)
      HYS19590 DO 710 JJ=1,12
      HYS19600 FLOCAL(11, JJ)=0
      HYS19610 DO 710 KK=1,12
      HYS19620 FLOCAL(11, JJ)=FLOCAL(11, JJ)+R(11, KK)*FGLO(KK, JJ)
      HYS19630 C
      HYS19640 DO 54 J=1,12
      HYS19650 FLOCAL(ISEG, J)=FLOCAL(J, 1)
      HYS19660 C
      HYS19670 IF(PRINT) THEN
      HYS19680 WRITE(6, 80) ISEG
      HYS19690 C 80
      HYS19700 FORMAT('1', 'ELEMENT', '13', ' LOCAL FORCES')
      HYS19710 HYS16(12) FLOC( ISEG, 11), I1=1, 12)
      HYS19720 C 72
      HYS19730 FORMAT(1X, 6G15.6)
      HYS19740 C
      HYS19750 ENDP
      HYS19760 RETURN
      HYS19770 END
      HYS19780 C-pa
      HYS19790 C
      HYS19800 C
      HYS19810 C
      HYS19820 C
      HYS19830 C
      HYS19840 C
      HYS19850 C
      HYS19860 C
      HYS19870 C
      HYS19880 C
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      HYS19990 C
      HYS20000 C
      HYS20010 C
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      HYS21440 C
      HYS21450 C
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      HYS21910 C
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      HYS21930 C
      HYS21940 C
      HYS21950 C
      HYS21960 C
      HYS21970 C
      HYS21980 C
      HYS21990 C
      HYS22000 C
      HYS22010 C
      HYS22020 C
      HYS22030 C
      HYS22040 C
      HYS22050 C
      HYS22060 C
      HYS22070 C
      HYS22080 C
      HYS22090 C
      HYS22100 C
      HYS22110 C
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      HYS22150 C
      HYS22160 C
      HYS22170 C
      HYS22180 C
      HYS22190 C
      HYS22200 C
      HYS22210 C
      HYS22220 C
      HYS22230 C
      HYS22240 C
      HYS22250 C
      HYS22260 C
      HYS22270 C
      HYS22280 C
      HYS22290 C
      HYS22300 C
      HYS22310 C
      HYS22320 C
      HYS22330 C
      HYS22340 C
      HYS22350 C
      HYS22360 C
      HYS22370 C
      HYS22380 C
      HYS22390 C
      HYS22400 C
      HYS22410 C
      HYS22420 C

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SUBROUTINE CONVER(PRINT,A,B,DOF,ISTOR,NOP,IMD,IS,L1,L2,MDIAG)
C *****
C *** CONVERT STIFFNESS MATRIX TO FULL STORAGE MODE ***
C NOTE : CONVERT ONLY TAKES PLACE FROM L1 TO L2
C A : INPUT HALF STORAGE BANDED MATRIX
C B : CONVERTED MATRIX
C ISTOR : SIZE OF A MATRIX
C NOP : DOF OF A MATRIX
C MDIAG : A MATRIX MAIN DIAGONAL TERM ADDRESS
C IS : B MATRIX SIZE
C
LOGICAL PRINT
DIMENSION A(ISTOR),B(IS,IS),MDIAG( 199 )
DO 200 J=L1,L2,+1
DO 200 I=J,L1,-1
IJ=MDIAG(J)+J-1
IF(IJ.LT.,MDIAG(J+1)).OR. IJ.EQ.,MDIAG(J)) THEN
B(I-L1+1,J-L1+1)=A(IJ)
B(J-L1+1,I-L1+1)=B(I-L1+1,J-L1+1)
ELSE
B(I-L1+1,J-L1+1)=0
B(J-L1+1,I-L1+1)=0
ENDIF
200 CONTINUE
IF (PRINT) THEN
WRITE(6,*) 'CONVERT HALF STORAGE BANDED MODE TO '
$ 'FULL STORAGE MODE:'
WRITE(6,*) 'THE INPUT MATRIX:'
WRITE(6,7F)(A(I),I=1,IMD)
FORMAT(1X,F15.6)
WRITE(6,*) 'CONVERT ONLY TAKE PLACE FROM ',L1,' TO ',L2
DO 77 I=L1,L2,L1+1
WRITE(6,7F)(B(I,J),J=L1-L1+1)
FORMAT(1X,F15.6)
77 CONTINUE
ENDIF
RETURN
END
C
C.PA
C PROCESS SOLUP OPT(3) COSTMT XREF
C
C DEBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
C END DEBUG
C
C MATRIX SOLUTION OF EQUATION AX=P, WHERE THE UPPER TRIANGULAR MATRIX
C OF THE SYMMETRIC MATRIX A IS STORED BY SKYLINE, P IS STORED AS
C A FULL MATRIX. THE MATRIX P IS REPLACED BY X AFTER BACK
C SUBSTITUTION IS COMPLETE. BOTH A AND P ARE ALTERED.
C
C GUYAN OPTION: IF GUYAN IS .TRUE, MASS MATRIX IS ALSO CONDENSED
C
SUBROUTINE GAUSS2(IOPT,PRINT,N,IMD,LROW,NLC,L1,L2,MD,A,P,FACTOR,
$
DIMENSION A(ISTOR),FACTOR(LROW),P(LROW,NLC),MD( 199 )
LOGICAL STST,ABORT,PRINT
C
C IJI(I,J)=MD(J)+J-I
C
C MATRIX STORAGE SCHEME:
C -THE MATRIX A IS BANDED AND SYMMETRIC. THUS ONLY THE SKYLINE ABOVE
C AND THE MAIN DIAGONAL NEEDS TO BE STORED.
C -THE MATRIX A IS STORED IN A LINEAR ARRAY BY COLUMNS. THE VALUE ON
C THE MAIN DIAGONAL IS STORED IN THE LINEAR ARRAY FIRST.
C -ADDRESSES OF THE LINEAR ARRAY ELEMENTS CONTAINING THE MAIN DIAGONAL
C TERMS ARE STORED IN MD.
C
C EXAMPLE: LET B BE THE 5X5 FULL SYMMETRIC MATRIX BELOW...
C
C 1 3 12
C 2 5 11
C 3 4 7 10
C 4 6 9
C 5 8
C
C B=
C
C STYM. MD= 1, 2, 4, 6, 8, 13
C
C THUS, B(3,3)=A(MD(3))=A(4)
C B(I,J)=A(MD(J)+J-I) IF J>I AND (MD(J)+J-I)<MD(J+1)
C =0 IF J<I AND (MD(J)+J-I)>MD(J+1)
C -MATRIX P IS NOT SYMMETRIC, AND IS STORED AS A FULL MATRIX
C
C VARIABLE TABLE:
C IOPT = OPTION NUMBER
C N = NUMBER OF DEGREES OF FREEDOM OF A MATRIX
C NLC = NUMBER OF LOAD CASES FOR THE P MATRIX
C L1 = FIRST ROW TO BE WORKED WITH
C L2 = LAST ROW TO BE WORKED WITH
C MD = ADDRESS OF THE MAIN DIAGONAL TERMS OF A
C P = LOAD MATRIX ON INPUT, SOLN (X) OF AX=P ON COMPLETION OF
C IOPT=4
C FACTOR= TEMPORARY STORAGE MATRIX
C L = ROW NUMBER FOR ELIMINATION
C I = ROW NUMBER
C J = COLUMN NUMBER
C
C L1=MAX(L1,1)
C L2=MIN(L2,N)
C
C GO TO (10,110,210,310,410),IOPT
C
10 CONTINUE
IOPT=1, REDUCE A AND P
GAUSSIAN ELIMINATION IS USED TO REDUCE THE A AND P MATRICES
C FROM ROW L1 TO ROW L2. IF L1 IS NOT 1, IT IS ASSUMED THAT
C A AND P ARE ALLREADY REDUCED FROM 1 TO L1...
C
IF (PRINT) THEN
WRITE(6,500) 'IOPT=1, REDUCE STIFFNESS AND LOADS',L1,L2,N
CALL LMATRIX(A,MD,N,IMD,'INPUT STIFFNESS')
CALL WMATRIX(P,N,NLC,'INPUT LOAD')
ENDIF
DO 100 L=L1,L2
IF (L.EQ.N) GO TO 100
C
C TEST FOR ZERO PIVOT.....
IF (A(MD(L)).EQ.0) THEN
WRITE(6,2000) L,MD(L),A(MD(L))
CALL ERRRA
ABORT=.TRUE.
RETURN
ENDIF
C
DO 100 L=L1,L2
IF (L.EQ.N) GO TO 100
C
C 215 CONTINUE
IOPT=2, REDUCE P ONLY
GAUSSIAN ELIMINATION IS USED TO REDUCE THE P MATRIX
C FROM ROW L1 TO ROW L2. IF L1 IS NOT 1, IT IS ASSUMED THAT
C A AND P ARE ALLREADY REDUCED FROM 1 TO L1...
C
IF (PRINT) THEN
WRITE(6,500) 'IOPT=2, REDUCE LOADS ONLY',L1,L2,N
CALL LMATRIX(A,MD,N,MD(N+1),'REDUCED STIFFNESS')
CALL WMATRIX(P,N,NLC,'NEW LOAD MATRIX')
ENDIF
DO 100 L=L1,L2
IF (A(MD(L)).EQ.0) THEN
WRITE(6,2000) L,MD(L),A(MD(L))
CALL ERRRA
ABORT=.TRUE.
RETURN
ENDIF
C
C LOOP FOR EACH COLUMN (J)
DO 150 J=L1,NLC
IF (P(L,J).EQ.0) GO TO 150
DO 120 I=L+1,N
IF (I.LT.MD(I+1)) P(I,J)=P(I,J)-A(I,L)*P(L,J)/A(MD(L))
120 CONTINUE
150 CONTINUE
CALL WMATRIX(P,N,NLC,'REDUCED LOAD')
RETURN
C
210 CONTINUE
IOPT=3, BACK SUBSTITUTION
C
C BACK SUBSTITUTION IS USED TO SOLVE FOR X IN SX=V, WHERE S IS
C THE REDUCE UPPER TRIANGULAR FACTOR OF THE A MATRIX, AND V
C IS THE REDUCED FORM OF THE P MATRIX. THE RESULTS (X) ARE
C STORED IN P.
C
C BACK SUBSTITUTION ONLY TAKES PLACE BETWEEN ROWS L1 AND L2.
C
IF (PRINT) THEN
WRITE(6,500) 'IOPT=3, BACK SUBSTITUTION',L1,L2,N
CALL LMATRIX(MD,N,MD(N+1),'REDUCED STIFFNESS')
CALL WMATRIX(P,N,NLC,'REDUCED LOAD')
ENDIF
DO 250 L=L2,L1,-1
/*ORIGINAL
/*ORIGINAL
C IF (A(MD(L)).EQ.0) THEN
WRITE(6,2000) L,MD(L),A(MD(L))
/*ORIGINAL
/*ORIGINAL
CALL ERRRA
/*ORIGINAL
/*ORIGINAL
ABORT=.TRUE.
/*ORIGINAL
/*ORIGINAL
RETURN
/*ORIGINAL
/*ORIGINAL
ENDIF
/*ORIGINAL
/*ORIGINAL
C CALCULATE X FOR EACH LOAD CASE, STORE IN P
DO 250 J=L2,L1,-1
IF (A(MD(J)).EQ.0) THEN
WRITE(6,2000) J,MD(J),A(MD(J))
CALL ERRRA
ABORT=.TRUE.
RETURN
ENDIF
DO 215 LC=1,NLC
P(J,LC)=P(J,LC)/A(MD(J))
C
IF (J.GT.1) THEN
IJ=MD(J+1)
NI=J-(MD(J+1)-MD(J))+1
WRITE(6,*) 'NI,IJ',NI,IJ
DO 220 I=NI,J-1,+1
DO 220 LC=1,NLC
WRITE(6,*) 'I',I,'J',J,'LC1',LC,'IJ',IJ
WRITE(6,*) 'P(I,LC)',P(I,LC)
WRITE(6,*) 'A(IJ)',A(IJ)
P(I,LC)=P(I,LC)-P(IJ,LC)*A(IJ)
WRITE(6,*) 'Q(I,LC)',P(I,LC)
220 CONTINUE
ENDIF
250 CONTINUE
DO 215 J=L1,L2,L1-1
/*
/*
C DO 215 J=L1,L2,L1-1
/*
/*
C P(L,J)=P(L,J)/A(MD(L))
/*
/*
C MODIFY THE REMAINDER OF V
/*
/*
C DO 220 I=L1,L2-1
/*
/*
C DO 220 J=L1,NLC
/*
/*
C IL=IJI(I,L)
/*
/*
C IF (I.LT.MD(L+1)) P(I,J)=P(I,J)-A(I,L)*P(L,J)
/*
/*
C 220 CONTINUE
/*
/*
C 250 CONTINUE
/*
/*
C IF (PRINT) CALL WMATRIX(P,N,NLC,'DISPLACEMENT')
C
RETURN
C
310 CONTINUE
IOPT=4, REDUCED MULTIPLICATION
C
C SOLVE FOR V IN S*D=P WHERE S HAS BEEN REDUCED BY GAUSSIAN
C ELIMINATION.
C THE RESULTING LOAD IS STORED IN P.
C THE DISPLACEMENTS ARE INPUT IN D.
C MULTIPLICATION ONLY TAKES PLACE BETWEEN ROWS L1 AND L2.
C
IF (PRINT) THEN
WRITE(6,500) 'IOPT=4, REDUCED MULTIPLICATION',L1,L2,N
CALL LMATRIX(A,MD,N,MD(N+1),'REDUCED STIFFNESS')
CALL WMATRIX(FACTOR,N,1,'DISPLACEMENT')
ENDIF
C
C ONLY ONE LOAD CASE...
LC=1
C
C ZERO P
C
DO 320 J=L1,L2
IF (A(MD(J)).EQ.0) THEN
WRITE(6,2000) J,MD(J),A(MD(J))
CALL ERRRA
ABORT=.TRUE.
RETURN
ENDIF
320 P(J,LC)=0

```

```

C
C----- MULTIPLY S_RED * DISP - P
DO 340 I=L1,L2,+1
DO 340 J=L1,L2,+1
IF ( I .EQ. J ) IO=MD(J)+J-1
IF ( I .GT. J ) IO=MD(I)+I-1
340 P(I,LC)=P(I,LC)+A(I,J)*FACTOR(J)
IF (PRINT) CALL WMATRX(P,N ,1, 'REDUCED LOAD')
C
400 CONTINUE
C
IF (PRINT) CALL WMATRX(P,N ,1, 'LOAD')
C
RETURN
410 CONTINUE
IOPT=5, BACK SUBSTITUTION
SOLVE FOR D' IN S'D=P'
C
L1-->
L2-->
L3-->
WHERE S IS THE CONDENSED MATRIX BY GAUSS ELEMINATION,
AND IT IS A NON POSITIVE DEFINITIVE MATRIX
D AND P ARE THE REDUCED DISPLACEMENT AND LOAD MATRIX
P' ARE INPUT IN ARRAY P
D ARE INPUT IN ARRAY FACTOR
C-----
IF (PRINT) THEN
WRITE (6,500) 'IOPT=5, BACK SUBSTITUTION',L1,L2,N
CALL LMATRX(A,MD,N,MD(N+1), 'REDUCED STIFFNESS')
CALL WMATRX(FACTOR,N ,1, 'INPUT DISPLACEMENT')
CALL WMATRX(P,N ,NLC, 'INPUT LOAD')
ENDIF
C
ONLY ONE LOAD CASE....
LC=1
DO 720 J=L1,L2
IF (A(MD(J)),.EQ.0) THEN
WRITE (6,2000) J,MD(J),A(MD(J))
CALL ERRTRA
ABORT=.TRUE.
RETURN
ENDIF
720 CONTINUE
C-----CALCULATE DISPLACEMENT MATRIX , BACK SUBSTITUTION ONLY
TAKES PLACE BETWEEN L1 AND L2
L3=N
DO 640 I=L1,L2
FACTOR(I)=0.
DO 350 J=L2,L1,-1
DO 350 J=L3,+1
IO=MD(J)+J-1
IF ( I .EQ. MD(I) ) THEN
FACTOR(I)=FACTOR(I)+P(I,LC)/A(MD(I))
ELSE IF ( IJ .LT. MD(J+1) ) THEN
FACTOR(I)=FACTOR(I)- A(IJ)*FACTOR(J)/A(MD(I))
ENDIF
350 CONTINUE
IF (PRINT) THEN
CALL WMATRX(P,N,LC, 'OUTPUT LOAD')
WRITE(6,*) 'PRINTING MATRIX: OUTPUT DISPLACEMENT'
WRITE(6,79)(J,FACTOR(J),J=L1,N)
FORMAT(IX,'ROW',15,3X,F15.6)
79 ENDF
RETURN
500 FORMAT (/2X,A,2X,'L1',15,' L2',15,' N',15)
2000 FORMAT (///IX,50(' '),
+ ' PIVOT IS LE 0 ----- ROW',15, T51,' '
+ ' STORAGE LCN',15, T51,' '
+ ' PIVOT',1P,G15.6,T51,' '
+ ' SOLUTION IS ABORTED ', T51,' '
+ IX,50(' '))
END
C
C.PA
SUBROUTINE SKT ( ASAT,STIFF,IDF,ISTOR,NOP,MDIAG,IMD )
C== CONVERT STIFFNESS MATRIX TO HALF STORAGE AND BANDED MODE ==
C-----
DIMENSION ASAT(IDF,IDF),STIFF(ISTOR),MDIAG(IDF+1)
DIMENSION ASAT(IDF,IDF),STIFF(ISTOR),MDIAG(199)
C
CONVERT STIFFNESS MATRIX TO HALF STORAGE BANDED MODE - STIFF
C THE MAIN DIAGONAL ADDRESS ARE STORED IN MDIAG ARRAY - MDIAG
L=0
DO 70 J=1,NOP
DO 80 I=1,NOP
WRITE(6,*) 'I=',I,' J=',J
IF ( ASAT(I,J) .EQ. 0 .AND. I .NE. J ) GO TO 80
IF ( ASAT(I,J) .EQ. 0 .AND. I .EQ. J ) THEN
WRITE(6,*) 'THE STIFFNESS ',I,'TH DIAGONAL TERM ARE ZERO'
// - SOLUTION ABORTED'
RETURN
ENDIF
M=I
GO TO 100
80 CONTINUE
100 DO 70 I=J,M,-1
L=L+1
IF ( I .EQ. J ) MDIAG(J)=L
IF ( ASAT(I,J) .EQ. 0 .AND. I .EQ. J ) THEN
WRITE(6,*) 'THE STIFFNESS ',I,'TH DIAGONAL TERM ARE ZERO'
// - SOLUTION ABORTED'
RETURN
ENDIF
STIFF(L)=ASAT(I,J)
IPT=L
70 CONTINUE
MDIAG(NOP+1)=IPT+1
IMD=MDIAG(NOP+1)-1
C
RETURN
END
C
C.PA
DEBUC UNIT(6),SUBCHK
END DEBUC
SUBROUTINE ENORM(IOPTT,SR,BR,DSR,DBR,TK,DELTP,DEN,NOP,IFM,
+ MM,LL,JJ,NN)
C-----
CALCULATE NORM OF LOAD INCREMENT AND NORM OF DISPLACEMENT INCREMENT
C
IOPT : 1 - CALCULATE DELTP FOR NORM OF LOAD AND DEN
IOPT : 2 - CALCULATE DELTP FOR NORM OF DISPLACEMENT
C
SR : DISPLACEMENT VECTOR
BR : LOAD VECTOR
DSR : DISPLACEMENT INCREMENT VECTOR
DBR : LOAD VECTOR INCREMENT VECTOR
TK : STIFFNESS MATRIX
C DELTP: NORM OF LOAD OR DISPLACEMENT

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HY533750
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HY533770
HY533780
C DEN = DEN + TRANSPOSE OF [DSR]*TK+[DSR]
C IFM = TRANSVERSE AND ROTATION DCP IDENTIFICATION INDEX
C 0 FOR TRANSVERSE
C 1 FOR ROTATION
C
C NOTE: SR(MM,LL), BR(MM,JJ), DSR(MM,LL), DBR(MM,LL), TK(MM,MM)
C IFM(NN)
C DSR AND DSRTK ARRAY ARE BASED ON 32 SEGMENTS, IF SEGMENT'S
C NUMBER IS LARGER THAN 32, THE DIMENSION OF DSR AND DSRTK
C NEED TO MODIFIED
C-----
DIMENSION DSR(MM,LL),DBR(MM,LL),SR(MM,JJ),BR(MM,JJ),TK(MM,MM),
+ DSRTK(1,198),DSRTK(1,198),IFM(NN)
C
C SKIM STIFFNESS TERMS
DO 80 I=1,NOP
DO 80 J=1,NOP
IF (ABS(TK(I,J)) .LT. 1.E-7) TK(I,J)=0
80 CONTINUE
C
GO TO (100,200),IOPTT
C----- IOPTT=1 -----
C
SEARCH MAXIMUM FORCE OR DISPLACEMENT INCREMENT
100 CONTINUE
FMAX=0.
TMAX=0.
DO 25 I=1,NOP
IF (IFM(I) .EQ. 0) THEN
IF (ABS(BR(I,1)) .GT. ABS(FMAX)) THEN
FMAX=BR(I,1)
ENDIF
ELSE IF (IFM(I) .NE. 0) THEN
IF (ABS(BR(I,1)) .GT. ABS(TMAX)) THEN
TMAX=BR(I,1)
ENDIF
ENDIF
25 CONTINUE
C
FIND NORM OF DBR
DEBT=0.
DO 32 I=1,NOP
IF (IFM(I) .EQ. 0) THEN
IF (IFM(I) .EQ. 0) GO TO 32
DELT=DELT+(ABS(DBR(I,1)/FMAX))**2
ELSE IF (IFM(I) .NE. 0) THEN
IF (TMAX .EQ. 0) GO TO 32
DELT=DELT+(ABS(DBR(I,1)/TMAX))**2
ENDIF
32 CONTINUE
DELT=SQRT(DELT/NOP)
C
FIND [DSR]*TK+[DSR]
DO 67 I=1,NOP
DSRT(1,I)=DSR(1,I)
67 CONTINUE
C
[DSRT]*TK+[DSRT]
DO 68 J=1,NOP
DSRTK(1,J)=DSRT(1,J)+DSRT(1,K)*TK(K,J)
68 CONTINUE
C
[DSRTK]*[DSR]=[DSRT]
DSRT(1,1)=0.
DO 78 K=1,NOP
DSRT(1,1)=DSRT(1,1)+DSRTK(1,K)*DSR(K,1)
78 CONTINUE
DEB=DSRT(1,1)
C
RETURN
C----- IOPTT=2 -----
200 CONTINUE
FMAX=0.
TMAX=0.
DO 45 I=1,NOP
IF (IFM(I) .EQ. 0) THEN
IF (ABS(BR(I,1)) .GT. ABS(FMAX)) THEN
FMAX=BR(I,1)
ENDIF
ELSE IF (IFM(I) .NE. 0) THEN
IF (ABS(BR(I,1)) .GT. ABS(TMAX)) THEN
TMAX=BR(I,1)
ENDIF
ENDIF
45 CONTINUE
C
FIND NORM OF DSR
DELT=0.
DO 42 I=1,NOP
IF (IFM(I) .EQ. 0) THEN
IF (IFM(I) .EQ. 0) GO TO 42
DELT=DELT+(ABS(DSR(I,1)/FMAX))**2
ELSE IF (IFM(I) .NE. 0) THEN
IF (TMAX .EQ. 0) GO TO 42
DELT=DELT+(ABS(DSR(I,1)/TMAX))**2
ENDIF
42 CONTINUE
DELT=SQRT(DELT/NOP)
END
C
C.PA
CPROCESS SUBLOC(LIBN,FUL,STR,NSEG,NUMP,ISEG,INEB,INEH,
+ TE,ST,EM,UO,VO,FLP)
C-----
SUBROUTINE BOXLOC(LIBN,FUL,STR,NSEG,NUMP,ISEG,INEB,INEH,
+ TE,ST,EM,UO,VO,FLP)
C
JUDGE LOCAL BUCKLING STRESS FOR BOX SECTION
C
NOTE: FLP=0 : NO LOCAL BUCKLING OCCURRED
FLP=1-5 : LOCAL BUCKLING OCCURRED
C
DIMENSION STR(NSEG,NUMP),UO(NUMP),VO(NUMP)
DIMENSION FUL(NUMP),TE(NUMP)
REAL INEB, INEH
INEB=INEB
INEH=INEH
C
IF ( LIBN .NE. 1 ) RETURN
C
DO 60 I=1,NUMP
FUL(I)=0
60
IPT=1
INEB=INEB-INEH
IPT=INEB-1
IPT=2*INEB+1
IPT=3*INEB+1
C
CALCULATE SECTION ELEMENT LENGTH
ELW=ABS(VO(2)-VO(3))

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```

IF( J.EQ. 0 ) THEN
  SAVE=0
  SCR=999
ELSE IF( J.NE. 0 ) THEN
  SAVE=ABS(TOTST/J)
  SCR=0.425*(3.14159*3.14159)*EM/(12*0.91*(W/ST)**2)
ENDIF
C
IF( SAVE.GT. SCR ) THEN
  DO 30 I=INEQ+1,INEQ
    IF( STR(ISEG,I).LT. 0.AND. FJL(I).NE. 1 ) FJL(I)=1
  DO 130 I=2*INEQ+INEH+1,3*INEQ
    IF( STR(ISEG,I).LT. 0.AND. FJL(I).NE. 1 ) FJL(I)=1
  FLP=4
  ENDF
  W=0
  TOTST=0
  DO 14 I=3*INEQ+INEH+1,4*INEQ
    IF( STR(ISEG,I).LT. 0 ) THEN
      J=J+1
      W=W+ELB
      TOTST=TOTST+STR(ISEG,I)
    ENDF
  CONTINUE
  DO 16 I=INEQ+INEH+1,2*INEQ
    IF( STR(ISEG,I).LT. 0 ) THEN
      J=J+1
      W=W+ELB
      TOTST=TOTST+STR(ISEG,I)
    ENDF
  CONTINUE
C
  FIND AVERAGE STRESS AND CRITICAL STRESS SCR
  IF( J.EQ. 0 ) THEN
    SAVE=0
    SCR=999
  ELSE IF( J.NE. 0 ) THEN
    SAVE=ABS(TOTST/J)
    SCR=0.425*(3.14159*3.14159)*EM/(12*0.91*(W/ST)**2)
  ENDF
C
  IF( SAVE.GT. SCR ) THEN
    DO 10 I=INEQ+INEH+1,2*INEQ
      IF( STR(ISEG,I).LT. 0.AND. FJL(I).NE. 1 ) FJL(I)=1
    DO 11 I=3*INEQ+INEH+1,4*INEQ
      IF( STR(ISEG,I).LT. 0.AND. FJL(I).NE. 1 ) FJL(I)=1
    FLP=4
    ENDF
  ENDF
  ENDF
  RETURN
  END
C
CPROCESS SDUMP GOSTMT XREF
*****
C
  DUC=DE/DEL
  EXCR(6),TRACC, SUBCHK,INIT, SUBTRACC
  END DEBUG
C
GOEL TRUSS HYSTERESIS MODEL FOR SIMULATED LOCAL BUCKLING
CT BOX SECTION
NOTES: IT IS ASSUMED THAT OVERALL BUCKLING WILL NOT OCCURRED
C
SUBROUTINE HYST13(P,PLA,DE,DELA,V,VLA,EL,SLK,
  EM,AREA,RADG,YS,PI,
  CC,CYCLE,RULE,STIF,IRV,IDUC,
  HA,VA,HC,VC,RSN,DEMAX,REGIN,PREGIN,
  PBOCT,DBOCT,PTOP,DTOP,PRINT,
  EXC,EXCR,DELT,ESSE,PSE,PSEOLD,CSE)
  DIMENSION DUC(3), EXCR(6), ESE(3)
  LOGICAL REVRSC,PRINT
  REAL*4 IRV, IDUC
  IF( IRV.EQ. 0 ) REVRSC=.FALSE.
  IF( IRV.EQ. 1 ) REVRSC=.TRUE.
  PI=3.1415926
  ES=AREA*EM/EL
  DELT=P/ES
  PL=P/PI
  ESR=SLK*EL/RADG
  SLOP2=STIF/ES
  DA=ABS(DE)
  DP=P-PLA
  DDE=DE-DELA
  DDEL=DE/DELT
  C
  ***** CALCULATE EXCURSION RATE AT EVERY ZERO CROSSING *****
  IF( P-PLA.LB. 0 ) THEN
    CALL ONE(0, DUC,EXCR,DELT, DA,ESSE,PSE,PSEOLD,CSE)
  ENDF
  C
  -----
  DUC ---- ELEMENT EXCURSION RATE
  EXCR ---- ELEMENT EXCURSION RATE
  DELT ---- ELEMENT YIELDING DISPLACEMENT
  ESSE ---- ELEMENT ELASTIC STRAIN ENERGY
  PSE ---- ELEMENT PLASTIC STRAIN ENERGY
  PSEOLD ---- ELEMENT PLASTIC STRAIN ENERGY AT LAST ZERO CROSSING
  CSE ---- ELEMENT CONSTANT STRAIN ENERGY
  C
  DP ---- ELEMENT AXIAL LOAD INCREMENT (KIPS)
  ESR ---- EFFECTIVE SLENDER RATIO (KL/R)
  ES ---- YIELDING STRESS
  AREA ---- CROSS SECTION AREA (A)
  EM ---- ELASTIC MODULUS (E)
  PL ---- ELEMENT AXIAL LOAD INCREMENT (KIPS)
  P ---- CURRENT AXIAL LOAD (KIPS)
  C ---- P - PT RATIO (P/PT)
  PLA ---- LAST AXIAL LOAD
  DDEL ---- DISPLACEMENT INCREMENT (IN)
  DE ---- CURRENT AXIAL DISPLACEMENT (IN)
  DEMAX ---- MAXIMUM AXIAL DISPLACEMENT (IN)
  DA ---- CURRENT ABSOLUTE AXIAL DISPLACEMENT (IN)
  DELA ---- LAST AXIAL DISPLACEMENT
  DDE ---- ELEMENT DISPLACEMENT INCREMENT
  DDEL ---- DEL/DE/DELT
  DELT ---- YIELDING DISPLACEMENT
  V ---- CURRENT VELOCITY
  ES ---- ELASTIC STIFFNESS
  EXI ---- UNLOADING EQUIVALENT STIFFNESS FOR CALCULATING ESSE
  PL ---- TENSION YIELDING LOAD
  EY ---- ELEMENT LENGTH (IN)
  CC ---- C SUB-C CONSTANT
  PHAX ---- BUCKLING LOAD (KIPS)
  DELMAX ---- BUCKING DISPLACEMENT (IN)
  HA,HB,HC,HD,HE,HF ---- P/PY - DE/DELT PLOT'S DE/DELT COORDINATE
  VA,VB,VC,VD,VE,VF ---- P/PY - DE/DELT PLOT'S P/PT COORDINATE
  C
  RESIDUAL STRAIN
  RULE ---- RULE NUMBER
  CYCLE ---- NO. OF CYCLES
  STIF ---- ELEMENT STIFFNESS
  REVRSC ---- LOGICAL INDEX FOR REVERSING CURVE OCCURRED
  IDUC ---- 1/ HC LESS THAN 12, 0:HC=12
  REGIN ---- REGIN 1: DE > HA
  2: DE = (HB, HA)
  3: DE = (HB, HH)
  4: DE = (HC, HB)
  C
  HYST13(6,567)CYCLE,RULE,HA,VA,HB,VB,HC,VC,HD,VD,
  HD,VD,HD1,VD1,HH,VH,HE,VE
  567 FORMAT(5X,'GOEL HYSTERESIS ENVELOPE CONTROL POINTS...')
  & 5X,'-----'
  & 5X,'CYCLE'
  & 5X,'RULE'
  & 5X,'(HA,VA)'
  & 5X,'(HB,VB)'
  & 5X,'(HC,VC)'
  & 5X,'(HD,VD)'
  & 5X,'(HE,VE)'
  & 5X,'(HD1,VD1)'
  & 5X,'(HH,VH)'
  & 5X,'(HE,VE)'
  ENDF
  C
  GOEL'S HYSTERESIS LOOP RULES

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C
C (1) RULE 1
IF( RULE .EQ. 1.) THEN
  IF( P. GE. PMAX .AND. P .LT. PY ) THEN
    STIP=SLOP1*ES
    EKI=SLOP1*ES
    IF( DEMAX .LT. 0 .AND. DE .LE. DEMAX ) THEN
      RSN=((0.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC
    ENDIF
  ELSE IF( P. GE. PY ) THEN
    STIP=SLOP5
    RULE=6
    RSN=0
    EKI=SLOP1*ES
    ELSE IF( P. LT. PMAX ) THEN
      HA=DEL
      VA=PL
      SLOP2=(VB-VA)/(HB-HA)
      STIF=SLOP2*ES
      RULE=2
      IF( DEMAX .LT. 0 .AND. DE .LE. DEMAX ) THEN
        RSN=((0.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC
      ENDIF
      EKI=SLOP1*ES
    ENDIF
    CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
    RETURN
  ENDIF
C (2) RULE 2
IF( RULE .EQ. 2.) THEN
  IF( DEL .GE. HX ) THEN
    IF( DDE .LE. 0.) THEN
      IF( PREGIN .NE. REGIN ) REVRSC= .FALSE.
      STIP=SLOP2*ES
      IF( DEMAX .LT. 0 .AND. DE .LE. DEMAX ) THEN
        RSN=((0.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC
      ELSE IF( DEMAX .GT. 0 ) THEN
        RSN=((0.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC
      ENDIF
      IF( .NOT. REVRSC ) THEN
        EKI=SLOP1*ES
      ELSE IF( REVRSC ) THEN
        EKI=ES*( (PTOP-PL)/(DTOP-DEL) )
      ENDIF
    ELSE IF( DDE .GT. 0.) THEN
      IF( .NOT. REVRSC ) THEN
        RULE=7
        STIP=SLOP1*ES
        PLA=PL-SLOP2*DDEL
        DELA=DEL-DDEL
        DBOT=DLA
        PBOT=PLA
        PL=PLA+SLOP1*DDEL
        P=PL*PY
        PTOP=VD+(SLOP5*(PBOT-VE-SLOP1*DBOT
          +SLOP1*HD)/(SLOP5-SLOP1))
        DTOP=(PBOT-VE-SLOP5*HD-SLOP1*DBOT)
          / (SLOP5-SLOP1)
        EKI=ES*SLOP1
      ELSE IF( REVRSC ) THEN
        RULE=12
        DBOT=DEL
        PBOT=P/PY
        SLOP7=(PTOP-PBOT)/(DTOP-DBOT)
        STIF=SLOP7*ES
        EKI=ES*SLOP7
      ENDIF
    ENDIF
  ELSE IF( DEL .LT. HX .AND. DEL .GE. HB ) THEN
    IF( DDE .LE. 0.) THEN
      IF( PREGIN .NE. REGIN ) REVRSC= .FALSE.
      STIP=SLOP2*ES
      IF( DEMAX .LT. 0 .AND. DE .LE. DEMAX ) THEN
        RSN=((0.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC
      ELSE IF( DEMAX .GT. 0 ) THEN
        RSN=((0.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC
      ENDIF
      IF( .NOT. REVRSC ) THEN
        EKI=SLOP1*ES
      ELSE IF( REVRSC ) THEN
        EKI=ES*( (PTOP-PL)/(DTOP-DEL) )
      ENDIF
    ELSE IF( DDE .GT. 0.) THEN
      IF( .NOT. REVRSC ) THEN
        RULE=8
        STIP=SLOP1*ES
        PLA=PL-SLOP2*DDEL
        DELA=DEL-DDEL
        DBOT=DLA
        PBOT=PLA
        PL=PLA+SLOP1*DDEL
        P=PL*PY
        PTOP=VD+(SLOP4*(PBOT-VE-SLOP1*DBOT
          +SLOP1*HD)/(SLOP4-SLOP1))
        DTOP=(PBOT-VE-SLOP4*HD-SLOP1*DBOT)
          / (SLOP4-SLOP1)
        EKI=ES*SLOP1
      ELSE IF( REVRSC ) THEN
        RULE=11
        DBOT=DEL
        PBOT=P/PY
        PTOP=VD
        DTOP=HD
        SLOP7=(PTOP-PBOT)/(DTOP-DBOT)
        STIF=SLOP7*ES
        EKI=ES*SLOP7
      ENDIF
    ENDIF
  ELSE IF( DEL .LT. HB ) THEN
    IF( PREGIN .NE. REGIN ) REVRSC= .FALSE.
    RULE=3
    EKI=ES*SLOP1
  ENDIF
  IF( REVRSC ) IRV=1.
  IF( .NOT. REVRSC ) IRV=0.
  CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
  IF( ABS(DEMAX) .LT. ABS(DE) ) THEN
    CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
    DEMAX=DE
  ENDIF
  RETURN
ENDIF
C (3) RULE 3
IF( RULE .EQ. 3.) THEN
  IF( DEL .GT. HC ) THEN
    IF( DDE .LE. 0.) THEN
      IF( PREGIN .NE. REGIN ) REVRSC= .FALSE.
      STIF=SLOP3*ES
    ENDIF
  ENDIF
  IF( .NOT. REVRSC ) IRV=1.
  IF( .NOT. REVRSC ) IRV=0.
  CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
  IF( ABS(DEMAX) .LT. ABS(DE) ) THEN
    CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
    DEMAX=DE
  ENDIF
  RETURN
ENDIF
C (4) RULE 4
IF( RULE .EQ. 4.) THEN
  IF( DEL .GT. DBOT .AND. DEL .LT. DTOP .AND. P .GT. PMAX ) THEN
    STIP=SLOP1*ES
    EKI=ES*SLOP1
  ELSE IF( DEL .LT. DBOT .OR. P .LE. PMAX ) THEN
    IF( P .LE. PMAX ) THEN
      REVRSC= .FALSE.
      PTOP=0
      DTOP=0
      DBOT=PL
      DBOT=DEL
      HA=DEL
      VA=PL
      SLOPE2=(VB-VA)/(HB-HA)
    ENDIF
    STIP=SLOP2*ES
    RULE=2
    PL=PBOT-(DBOT-DEL)*SLOP2
    P=PL*PT
    EKI=ES*( (PTOP-PL)/(DTOP-DEL) )
  ELSE IF( DEL .GT. DTOP .AND. PL .LT. VE ) THEN
    STIP=SLOP5*ES
    RULE=5
    EKI=ES*SLOP1
  ELSE IF( DEL .GT. DTOP .AND. PL .GE. VE ) THEN
    STIF=SLOP6*ES
    RULE=6
    EKI=ES*SLOP1
  ENDIF
  IF( REVRSC ) IRV=1.
  IF( .NOT. REVRSC ) IRV=0.
  CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
  CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
  RETURN
ENDIF
C (5) RULE 5
IF( RULE .EQ. 5.) THEN
  IF( DEL .GT. DBOT .AND. DEL .LT. DTOP ) THEN
    STIF=SLOP1*ES
    EKI=ES*SLOP1
  ELSE IF( DEL .LT. DBOT ) THEN
    STIF=SLOP2*ES
    RULE=2
    PL=PBOT-(DBOT-DEL)*SLOP2
    P=PL*PT
    EKI=ES*( (PTOP-PL)/(DTOP-DEL) )
  ELSE IF( DEL .GT. DTOP ) THEN
    STIF=SLOP4*ES
    EKI=ES*SLOP4
  ENDIF
  IF( REVRSC ) IRV=1.
  IF( .NOT. REVRSC ) IRV=0.
  CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
  CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
  RETURN
ENDIF
C (6) RULE 6
IF( REVRSC ) IRV=1.
IF( .NOT. REVRSC ) IRV=0.
CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
RETURN
ENDIF
C (7) RULE 7
IF( REVRSC ) IRV=1.
IF( .NOT. REVRSC ) IRV=0.
CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
RETURN
ENDIF
C (8) RULE 8
IF( RULE .EQ. 8.) THEN
  IF( DEL .GT. DBOT .AND. DEL .LT. DTOP ) THEN
    STIF=SLOP1*ES
    EKI=ES*SLOP1
  ELSE IF( DEL .LT. DBOT ) THEN
    STIF=SLOP2*ES
    RULE=2
    PL=PBOT-(DBOT-DEL)*SLOP2
    P=PL*PT
    EKI=ES*( (PTOP-PL)/(DTOP-DEL) )
  ELSE IF( DEL .GT. DTOP ) THEN
    STIF=SLOP4*ES
    EKI=ES*SLOP4
  ENDIF
  IF( REVRSC ) IRV=1.
  IF( .NOT. REVRSC ) IRV=0.
  CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
  CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
  RETURN
ENDIF
C (9) RULE 9
IF( REVRSC ) IRV=1.
IF( .NOT. REVRSC ) IRV=0.
CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
CALL DNE(1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
RETURN
ENDIF

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IF( RULE .EQ. 9.) THEN
  IF( DEL .GT. DBOT .AND. DEL .LT. DTOP ) THEN
    STIF=SLOP1*ES
    EKI=ES*SLOP1
  ELSE IF( DEL .LT. DBOT ) THEN
    STIF=SLOP3*ES
    RULE=3
    PL=PBOT-(DBOT-DEL)*SLOP3
    P=PL*PT
    EKI=ES*((PTOP-PL)/(DTOP-DEL))
  ELSE IF( DEL .GT. DTOP ) THEN
    STIF=SLOP4*ES
    RULE=4
    EKI=ES*SLOP1
  ENDDIF
  IF( REVRSC ) IRV=1.
  IF ( .NOT. REVRSC ) IRV=0.
  CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
  CALL DNE( 1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
  RETURN
  ENDDIF
(7) RULE 4
IF( RULE .EQ. 4.) THEN
  IF( DEL .GE. HCL .AND. DEL .LT. HD ) THEN
    IF( DDE .GE. 0. ) THEN
      STIF=SLOP4*ES
      EKI=ES*SLOP1
    ELSE IF( DDE .LT. 0. ) THEN
      PREF=VD*(SLOP4*(VB-VD-SLOP1*HB)
      SLOP1*HD)/(SLOP4-SLOP1))
      DREF=(VB-VD-SLOP1*HB-SLOP1*HD)/(SLOP4-SLOP1)
      IF( DEL .LT. DREF ) THEN
        PTOP=P/PT
        DTOP=DEL
        DBOT=(SLOP3*HB-VB+PTOP-SLOP1*DEL)/(SLOP3-SLOP1)
        PBOT=VB-SLOP3*(DBOT-HB)
        STIF=SLOP1*ES
        RULE=3
        REVRSC=.TRUE.
        EKI=ES*SLOP1
      ELSE IF( DEL .GE. DREF ) THEN
        PTOP=P/PT
        DTOP=DEL
        DBOT=(SLOP2*HA-VB+PTOP-SLOP1*DEL)/(SLOP2-SLOP1)
        PBOT=VA-SLOP2*(DBOT-HA)
        STIF=SLOP1*ES
        RULE=8
        REVRSC=.TRUE.
        EKI=ES*SLOP1
      ENDDIF
    ELSE IF( DEL .GT. HD .AND. DDE .GT. 0. ) THEN
      STIF=SLOP5*ES
      RULE=5
      EKI=ES*SLOP1
    ELSE IF( DEL .LT. HCL .AND. DEL .GE. HC ) THEN
      RULE=4
      STIF=SLOP4*ES
      EKI=ES*SLOP1
    ELSE IF( DEL .LT. HC ) THEN
      REVRSC=.FALSE.
      RULE=3
      HC=DEL
      VC=PL
      IDC=1.
      STIF=SLOP6*ES
      EKI=ES*((VD-VC)/(HD-HC))
    ENDDIF
    IF( REVRSC ) IRV=1.
    IF ( .NOT. REVRSC ) IRV=0.
    CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
    IF( ABS(DEMAX) .LT. ABS(DE) ) THEN
      CALL DNE( 1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
      DEMAX=DE
    ENDDIF
  RETURN
  ENDDIF
(8) RULE 5
IF( RULE .EQ. 5.) THEN
  IF( DEL .LT. HE ) THEN
    IF( DDE .GE. 0. ) THEN
      STIF=SLOP5*ES
      EKI=ES*SLOP1
    ELSE IF( DDE .LT. 0. ) THEN
      BRP=VA-SLOP1*HA
      HRP=(VD-SLOP5*HD-BRP)/(SLOP1-SLOP5)
      IF( DEL .GE. HRP ) CYCLE=CYCLE+1
      PTOP=P/PT
      DTOP=DEL
      DBOT=(SLOP2*HA-VB+PTOP-SLOP1*DEL)/(SLOP2-SLOP1)
      PBOT=VA-SLOP2*(DBOT-HA)
      STIF=SLOP1*ES
      RULE=1
      REVRSC=.TRUE.
      EKI=ES*SLOP1
    ENDDIF
    ELSE IF( DEL .GE. HE .AND. DDE .GT. 0. ) THEN
      STIF=SLOP6
      RULE=6
      EKI=ES*SLOP1
    ENDDIF
  IF( REVRSC ) IRV=1.
  IF ( .NOT. REVRSC ) IRV=0.
  CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
  CALL DNE( 1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
  DEMAX=DE
  ENDDIF
  RETURN
  ENDDIF
(9) RULE 6
IF( RULE .EQ. 6.) THEN
  IF( DDE .GE. 0. ) THEN
    STIF=SLOP6
    EKI=ES*SLOP1
  ELSE IF( DDE .LT. 0. ) THEN
    STIF=SLOP1*ES
    RULE=1
    CYCLE=CYCLE+1
    REVRSC=.FALSE.
    EKI=ES*SLOP1
  ENDDIF
  IF( REVRSC ) IRV=1.
  IF ( .NOT. REVRSC ) IRV=0.
  CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
  CALL DNE( 1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
  DEMAX=DE
  ENDDIF
  RETURN
  ENDDIF
(10) RULE 10
IF( RULE .EQ. 10.) THEN
  IF( DEL .GT. DBOT .AND. DEL .LT. DTOP ) THEN
    STIF=SLOP7*ES
    EKI=ES*SLOP7
  ELSE IF( DEL .LT. DBOT ) THEN
    RULE=3
    REVRSC=.FALSE.
    PTOP=0
    DTOP=0
    PBOT=0
    EKI=ES*((PTOP-PL)/(DTOP-DEL))
  ELSE IF( DEL .GT. DTOP ) THEN
    STIF=SLOP4*ES
    RULE=4
    EKI=ES*SLOP1
  ENDDIF
  IF( REVRSC ) IRV=1.
  IF ( .NOT. REVRSC ) IRV=0.
  CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
  CALL DNE( 1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
  RETURN
  ENDDIF
(11) RULE 11
IF( RULE .EQ. 11.) THEN
  IF( DEL .GT. DBOT .AND. DEL .LT. DTOP ) THEN
    STIF=SLOP7*ES
    EKI=ES*SLOP7
  ELSE IF( DEL .LT. DBOT ) THEN
    RULE=2
    REVRSC=.FALSE.
    PTOP=0
    DTOP=0
    PBOT=0
    EKI=ES*((VD-PL)/(HD-DEL))
  ELSE IF( DEL .GT. DTOP ) THEN
    STIF=SLOP4*ES
    RULE=4
    EKI=ES*SLOP1
  ENDDIF
  IF( REVRSC ) IRV=1.
  IF ( .NOT. REVRSC ) IRV=0.
  CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
  CALL DNE( 1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
  RETURN
  ENDDIF
(12) RULE 12
IF( RULE .EQ. 12.) THEN
  IF( DEL .GT. DBOT .AND. DEL .LT. DTOP ) THEN
    STIF=SLOP7*ES
    EKI=ES*SLOP7
  ELSE IF( DEL .LT. DBOT ) THEN
    RULE=2
    REVRSC=.FALSE.
    PTOP=0
    DTOP=0
    PBOT=0
    EKI=ES*((PTOP-PL)/(DTOP-DEL))
  ELSE IF( DEL .GT. DTOP ) THEN
    STIF=SLOP5*ES
    RULE=5
    EKI=ES*SLOP1
  ENDDIF
  IF( REVRSC ) IRV=1.
  IF ( .NOT. REVRSC ) IRV=0.
  CALL STRENG( ESE, PSE, PSEOLD, P, PLA, DE, DELA, STIF, EKI )
  CALL DNE( 1, DUC, EXCR, DELT, DA, ESE, PSE, PSEOLD, CSE )
  RETURN
  ENDDIF
C-----
C DEBU UNIT(6), TRACE, SUBCHK, INIT, SUBTRACE
C ENDB DEBU
C-----
C THIS HYSTERESIS SUBROUTINE FOR PERFORATED SHEAR WALL... HYST14...
C-----
SUBROUTINE HYST14( FPP, DP, FPP, DPP, H,
  & OFF, ODC, OFY, ODT, OFU, ODU,
  & AD, AV, MOQ, OSOC, OSCT, OSOT,
  & OGTU, OSUF, OSCU, OSOB, OSOE,
  & K, LRULE, KKL, DSINC, DMAX, DM,
  & FM, RP, SOC, SCT, SOT, SFU,
  & SUP, SCU, SOB, SOE, FUD, DUO,
  & KEYDS1, SECN0, E2Y, DU17, AREAL,
  & CYCLE, CSE, FU00, DU00, DKP, TNPTFG,
  & POSTFG, COL, RAT, RAT1, S96, SR,
  & DPT2, SRELAG, USE, IOPT2, IOPT,
  & PS06, KLAST, DPTT, DPT, ESE, PSE,
  & PSEOLD, EXCR, FC, FY, FU, FF,
  & DC, DT, DU, DF, DSINCL, UPOS,
  & UNEG )
  INTEGER CYCLE, LRULE, LRULST, IOPT
  REAL K, RP, DU0(4), FU0(4), KLAST, KEYDS1, KEYDS2, MQD, LWP
  LOGICAL SRELAG, TNPTFG, POSTFG
C-----
C H ---- WALL SHEAR SPAN, FROM LOADING POINT TO THE BOTTOM OF WALL
C WID ---- WALL WIDTH
C HD ---- THE HEIGHT OF THE OPENING
C H ---- THE HEIGHT OF THE WALL
C AV ---- THE NET AREA OF DIAGONAL STEEL BARS
C AD ---- THE NET AREA OF VERTICAL STEEL BARS
C HC, H ABOVE JUST USED FOR CALCULATION OF FAILURE DISPL. (STIFF SUBRTN)
C ODP ---- INITIAL CRACKING LOAD
C ODY ---- INITIAL YIELDING DISPL.
C ODU ---- INITIAL ULTIMATE DISPL.
C ODF ---- INITIAL FAILURE DISPL.
C OCP ---- INITIAL CRACKING LOAD
C OFY ---- INITIAL YIELDING LOAD
C OFU ---- INITIAL ULTIMATE LOAD
C ODP ---- INITIAL FAILURE LOAD
C DC ---- CRACKING DISPL.
C DY ---- YIELDING DISPL.
C DU ---- ULTIMATE DISPL.
C DF ---- FAILURE DISPL.
C FC ---- CRACKING LOAD
C FY ---- YIELDING LOAD
C FU ---- ULTIMATE LOAD
C FF ---- FAILURE LOAD
C DMAX ---- MAX. DISPL. FOR THE LOADING SENSE IN THE CURRENT CYCLE
C DNP ---- CRACKING DISPL. IN THE NEGATIVE LOADING SENSE
C DTP ---- YIELDING DISPL. IN THE NEGATIVE LOADING SENSE
C DKP ---- SEE DFORCE SUBRTN; WHICH EQUAL TO CURRENT DP AND ASSIGNED TO
C DPP ---- SEE DFORCE SUBRTN; WHICH EQUAL TO CURRENT DP AND ASSIGNED TO
C FCP ---- CRACKING LOAD IN THE NEGATIVE LOADING SENSE
C-----
HYST0000
HYST0001
HYST0002
HYST0003
HYST0004
HYST0005
HYST0006
HYST0007
HYST0008
HYST0009
HYST0010
HYST0011
HYST0012
HYST0013
HYST0014
HYST0015
HYST0016
HYST0017
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HYST0038
HYST0039
HYST0040
HYST0041
HYST0042
HYST0043
HYST0044
HYST0045
HYST0046
HYST0047
HYST0048
HYST0049
HYST0050
HYST0051
HYST0052
HYST0053
HYST0054
HYST0055
HYST0056
HYST0057
HYST0058
HYST0059
HYST0060
HYST0061
HYST0062

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IF(POSTFG) FP=PPP*(DUO(2)-DPP)*KLAST*(DP-DUO(2))*K
IF(.NOT. POSTFG) FP=PPP*(DUO(1)-DPP)*KLAST*(DP-DUO(1))*K
END IF
LAULE=53
CALL DINTNR(FC, FT, FU, FP, DC, DT, DU, DP, DPP, SOC, SCT, STU, SUP,
K, FP, DP, DSINC, KKL, LAULE, POSTFG, DUO, FUO)
END IF
DMAX=MAX(ABS(DP), DMAX)
DSINCL=DSINC
IOPT2=1
CALL DDISSI(IOPT2, ESE, PSE, PSEOLD, DPP, FPP, DP, FP, AREAL,
K, USE, KKL, FC, DC, FT, DT, DM, DMAX, ODC, SOB, FUOO, LAULE)
RETURN
END IF
C----- CHECK IF PASS TURNING POINT
23 CALL DCURAD(KKL, KEYDS1, DSINC, FP, FPP, DM, FC, FT, FU, FP,
K, DP, DMAX, DPP, DC, DT, DU, DP, DPP, CYCLEN, AREAL, PMAI, TNPTFG,
LAULE, MQD, ODC, ODT, ODU, OFU, DP, HO, H, LWP, OSOC, POSTFG,
OSCT, SOC, SCT, STU, SUP, SR, SOB, SOE, SQ6, OFC, OFT,
SRLAG, COL, PSQ6, KLAST, DPT, DPTT, KKLST, USE,
OSUF, PSEOLD, UPOS, UNEG, ESE, PSE, CSE, EXCR, IOPT, IOPT2)
C
IF((K.EQ.0).AND.(FP.EQ.0).AND.(FP.EQ.FP)) THEN
GO TO 77
END IF
C
-- TO CALCULATE THE STIFFNESS IN EACH LOAD STEP
CALL DNCKUL(FC, FT, FU, FP, DC, DT, DU, DP, FPP, DM, FM, KP,
SOC, SCT, STU, SUP, SCU, SOB, SOE, KKL, LAULE, FPP, DPP,
FUO, DUO, DSINC, KEYDS1, K, S2T, DU17, FU17, CYCLEN, OSOB,
AREAL, HS, MID, AV, AD, FUOO, DUOO, POSTFG, PSEOLD, PSE, ESE,
DAP, HO, H, RAT, RAI1, OFC, OFT, OFU, ODC, ODT, ODU,
SQ6, OSOC, OSCT, ODT, OSUF, MQD, LWP, SR, DP10, KEYDS2,
SRLAG, COL, PSQ6, KLAST, DPT, DPTT, FPT, TNPTFG,
KKLST, LRLST, DSINCL, IOPT2, USE)
C
IF(K.EQ.0.) RETURN
77 IF(TNPTFG) THEN
IF(KKL.EQ.1 .OR. KKL.EQ.8 .OR. KKL.EQ.10 .OR. KKL.EQ.3
.OR. KKL.EQ.7 .OR. KKL.EQ.9 .OR. KKL.EQ.5)
.OR. KKL.EQ.6) THEN
IF(ABS(FP).LE.(0.7*FT) .AND. KKL.NE.1 .AND. KKL.NE.3) THEN
USE=K
IOPT2=3
IF(ABS(FP).GT.(0.7*FT)) IOPT2=1
CALL DDISSI(IOPT2, ESE, PSE, PSEOLD, DPT, FPP, DP, FP, AREAL,
K, USE, KKL, FC, DC, FT, DT, DM, DMAX, ODC, SOB, FUOO,
LAULE)
ELSE IF(KKL.EQ.2 .OR. KKL.EQ.4) THEN
IOPT2=3
CALL DDISSI(IOPT2, ESE, PSE, PSEOLD, DPT, FPP, DP, FP, AREAL,
K, USE, KKL, FC, DC, FT, DT, DM, DMAX, ODC, SOB, FUOO,
LAULE)
END IF
TNPTFG=.FALSE.
ELSE IF(KKL.NE.5 .AND. KKL.NE.6) .OR.
((KKL.EQ.5 .OR. KKL.EQ.8) .AND. ((DPP*FP).GT.0.)) THEN
IF(ABS(FP).LE.(0.7*FT) .AND. KKL.NE.1 .AND. KKL.NE.3) THEN
USE=K
IOPT2=3
END IF
IF(ABS(FP).GT.(0.7*FT)) IOPT2=1
CALL DDISSI(IOPT2, ESE, PSE, PSEOLD, DPT, FPP, DP, FP, AREAL,
K, USE, KKL, FC, DC, FT, DT, DM, DMAX, ODC, SOB, FUOO, LAULE)
ELSE IF(KKL.EQ.5 .OR. KKL.EQ.6) .AND. ((DPP*FP).LT.0.) THEN
USE=K
IOPT2=3
CALL DDISSI(IOPT2, ESE, PSE, PSEOLD, DPT, FPP, DP, FP, AREAL,
K, USE, KKL, FC, DC, FT, DT, DM, DMAX, ODC, SOB, FUOO, LAULE)
END IF
DSINCL=DSINC
C CALC DUCTILITY
IF(KKL.EQ.1 .OR. KKL.EQ.8 .OR. KKL.EQ.10) THEN
DLI=MAX(DP, DPP)
CALL DNE(1, UPOS, EXCR, ODT, DLI, ESE(2), PSE, PSEOLD, CSE)
END IF
IF(KKL.EQ.3 .OR. KKL.EQ.7 .OR. KKL.EQ.9) THEN
DLI=MIN(DP, DPP)
CALL DNE(1, UNEG, EXCR, ODT, DLI, ESE(3), PSE, PSEOLD, CSE)
END IF
66 RETURN
END
C
C DEBUG UNIT(6), TRACE, SUBCHK, INIT, SUBTRACE
C END DBUG
C-----
C= SUBROUTINE DNCKUL SUBRTN (FIND STIFFNESS AND FORCE IN EACH STEP) ==HY604340
C-----
SUBROUTINE DNCKUL(FC, FT, FU, FP, DC, DT, DU, DP, FPP, DM, FM, KP,
SOC, SCT, STU, SUP, SCU, SOB, SOE, KKL, LAULE, FPP, DPP,
FUO, DUO, DSINC, KEYDS1, K, S2T, DU17, FU17, CYCLEN, OSOB,
AREAL, HS, MID, AV, AD, FUOO, DUOO, POSTFG, PSEOLD, PSE, ESE,
DAP, HO, H, RAT, RAI1, OFC, OFT, OFU, ODC, ODT, ODU,
SQ6, OSOC, OSCT, ODT, OSUF, MQD, LWP, SR, DP10, KEYDS2,
SRLAG, COL, PSQ6, KLAST, DPT, DPTT, FPT, TNPTFG,
KKLST, LRLST, DSINCL, IOPT2, USE)
C
REAL X, DUO(2), FUO(2), KP, DSINC, MQD, LWP, KLAST, KEYDS1, KEYDS2
INTEGER CYCLEN
DIMENSION ESE(3)
LOGICAL SRLAG, TNPTFG, POSTFG
C 1. .... LOAD .....-RULE 1.0
IF((KKL.EQ.1).OR.(KKL.EQ.3)) THEN
LAULE=10
IF(ABS(DP).LE.DC) THEN
K=SOC
USE=K
IOPT2=3
RETURN
ELSE IF(ABS(DP).GT.DC) .AND. (ABS(DP).LE.DT) THEN
DDT=SIGN(DC, DP)
K=SCT
IF(ABS(FP).LE.(0.7*FT)) IOPT2=2
IF(ABS(FP).GT.(0.7*FT)) IOPT2=1
ELSE IF(ABS(DP).GT.DT) .AND. (ABS(DP).LE.DU) THEN
DDT=SIGN(DT, DP)
K=STU
IOPT2=1
ELSE IF(ABS(DP).GT.DU .AND. ABS(DP).LE.(20*DT)) THEN
DDT=SIGN(DU, DP)
K=STU
IOPT2=1
ELSE IF(ABS(DP).GT.DF .AND. ABS(DP).LE.(20*DT)) THEN
DDT=SIGN(DU, DP)
K=(0.1*FU)-FP/((20*DT)-DP)
USE=OSOC
ELSE IF(ABS(DP).GT.(20*DT)) THEN
DDT=SIGN(20*DT, DP)
K=-0.0001*SCT
USE=OSOC
END IF
IF(KLAST.EQ.K) RETURN
IF(TNPTFG) THEN
FPP=PPP
DPT=PPP*(DOT-DPP)*KLAST+(DP-DOT)*K
FPP=PPP*(DOT-DPP)*KLAST+(DP-DOT)*K
DPT=PPP*(DOT-DPP)*KLAST
DPT=DOT
END IF
RETURN
END IF
C 2. .... UNLOAD ....
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IF(KKL.EQ.2).OR.(KKL.EQ.4) THEN
IOPT2=3
IF(DM.LT.ODC .AND. DMAX.LT.ODC) THEN
C .....-RULE 2.1
LAULE=21
K=SOC
IF(KKLST.EQ.5 .OR. KKLST.EQ.6 .OR. KKLST.EQ.7
.OR. KKLST.EQ.8) THEN
DPI=DP
FPI=FP
CALL DDISSI(3, ESE, PSE, PSEOLD, DPP, FPP, DPT, O., AREAL,
K, USE, KKL, FC, DC, FT, DT, DM, DMAX, ODC, SOB, FUOO,
LAULE)
FPP=0. .K*DPTT
DPP=DPT
FPP=0
DPI=DP
FPI=FP
END IF
IOPT2=3
RETURN
END IF
C
IF(LAULE.EQ.54 .OR. LAULE.EQ.23) THEN
K=KLAST
RETURN
END IF
C
IF(KKLST.EQ.5 .OR. KKLST.EQ.6 .OR. KKLST.EQ.7 .OR.
KKLST.EQ.8 .OR. KKLST.EQ.9 .OR. KKLST.EQ.10) THEN
SK1=(0.-FPP)/(DPP/3-DPP)
K=SK1
IF((DM.GE.ODC) .AND. (ABS(KEYDS2).LE.DC)) THEN
C .....
IF(KKLST.EQ.7 .OR. KKLST.EQ.8 .OR.
KKLST.EQ.9 .OR. KKLST.EQ.10) THEN
K=MAX(KP, SK1)
END IF
FPP=PPP*(DP-DPP)*K
C .....-RULE 5.4
LAULE=54
IF((FPP*PPP).LT.0.) THEN
IF(FPP*GT.0) K=FC/(DC-DPT)
IF(FPP.LT.0) K=-FC/(-DC-DPP)
FPP=0. .K*DPTT
IF(KKL.EQ.2) KKL=5
IF(KKL.EQ.4) KKL=6
END IF
RETURN
END IF
IF((ABS(FPP).LE.(0.7*FT)) .AND. (KKLST.EQ.7 .OR.
KKLST.EQ.8 .OR. KKLST.EQ.9 .OR. KKLST.EQ.10)) THEN
K=MAX(K, SK1)
C .....-RULE 2.3
LAULE=23
USE=K
FPP=PPP*(DP-DPP)*K
IF((FPP*PPP).LT.0.) THEN
DPT=DPP+(0.-FPP)/K
CALL NEWSTI(MQD, ODC, ODT, ODU, OFC, OFT, OFU, AREAL, DM, FC, FT,
FU, FC, DT, DU, DF, HO, H, LWP, OSOC, SRLAG, OSCT,
SOC, SCT, STU, SUP, SR, SOB, SOE, SQ6, DPP, COL, PSQ6,
DPT, SDP, OSUF)
IF(SQ6.LE.0.3 .AND. SQ6.GT.0.) THEN
DJP=SIGN(DC, -DPT)
FJP=SIGN(FC, DJP)
K=FJP/(DJP-DPT)
ELSE
K=SR
END IF
IOPT2=3
IF(KKL.EQ.2) KKL=5
IF(KKL.EQ.4) KKL=6
DPP=DPT
FPP=0.
FPP=0.+(DP-DPT)*K
RETURN
END IF
IOPT2=3
RETURN
END IF
IF((FPP*PPP).LT.0) THEN
IF(FPP.GT.0) K=FC/(DC-DPT)
IF(FPP.LT.0) K=-FC/(-DC-DPP)
FPP=0. .K*DPTT
IF(KKL.EQ.2) KKL=5
IF(KKL.EQ.4) KKL=6
END IF
IF(DSINC.GT.0.) THEN
FPP=0. .GGGG
FU17=FFFF/6
END IF
IF(DSINC.GT.0.) THEN
FPP=0. .GGGG
FU17=FFFF/6
END IF
END IF
IF(ABS(FP).LE.(0.7*FT)) THEN
FPP=PPP*(DP-DPP)*K
DPT=SIGN(DC, -DPP)
FPP=0. .K*DPTT
IF(KKL.EQ.2) KKL=5
IF(KKL.EQ.4) KKL=6
IF(KKLST.EQ.7 .OR. KKLST.EQ.8 .OR. KKLST.EQ.9
.OR. KKLST.EQ.10) .AND. (ABS(FPP).LT.ABS(FUOO)) THEN
FPP=PPP/6
FU17=FFFF/6
END IF
IF(ABS(FU17)*6).LE.(0.70*FT)) THEN
IF(ABS(FP).GT.FC/3) .AND. (ABS(FP).LE.(0.70*FT)) THEN
C .....-RULE 2.2
LAULE=23
K=OSOC
IF(K.NE.KLAST) THEN
FPP=PPP*(DP-DPP)*K
END IF
IF(ABS(FP).LE.FC/3) THEN
C .....-RULE 2.3
LAULE=23
IF(LAULE.NE.LRLST) TNPTFG=.TRUE.
IF(TNPTFG) THEN
IF(ABS(DP).GT.(20*DT)) THEN
K=SOC
ELSE
FPP=SIGN(FC/3, FPP)
DPT=DPP+(FPT-FPP)/KLAST
DPT=SIGN(DC, -DPP)
FPP=SIGN(FC, DCP)
K=(FC-FPP)/(DCP-DPT)
IF(K.GT.OSOC) THEN
FPP=0
DCP=DPP/3
K=(0.-FPP)/(DCP-DPT)
USE=K

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HY604890
HY604900
HY604910
HY604920
HY604930
HY604940
HY604950
HY604960
HY604970
HY604980
HY604990
HY605000
HY605010
HY605020
HY605030
HY605040
HY605050
HY605060
HY605070
HY605080
HY605090
HY605100
HY605110
HY605120
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HY605200
HY605210
HY605220
HY605230
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HY605270
HY605280
HY605290
HY605300
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HY606180
HY606190
HY606200
HY606210
HY606220
HY606230
HY606240
HY606250
HY606260
HY606270
HY606280
HY606290
HY606300
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      END IF
      PPT=PPP+(DPT-DPP)*KLAST
      PP=PPP+(DPT-DPP)*KLAST+(DP-DPT)*K
    END IF
    RETURN
  END IF
  END IF
  RETURN
ELSE
  IF((ABS(PP).GT.ABS(FU17)).AND.(ABS(PP).LE.ABS(FU17)*6
  &
  .OR. KLAST.LT.0.)) THEN
    FFF=FU09
  C .....RULE 2.5
  IF(LRULE.NE.LRULE6) THEN
    DPPP=SIGN(DC,-DPP)
    FPPP=SIGN(FC,DPPP)
    SKK1=(FPPP-FFF)/(DPPP-DPP)
    K=MAX(SKK1,OSOB)
    FP=PPP+(DP-DPP)*K
  END IF
  IF((ABS(ABS(PP)-ABS(FU17)).LT.ABS(K*DSINC)).AND.
  &
  (ABS(PP).LT.ABS(FU17))) THEN
    PPT=FU17
    DPT=DPP+(PPT-PPP)/K
    TNPTFG=.TRUE.
  END IF
  IF((TNPTFG).OR.(ABS(PP).LE.ABS(FU17))) THEN
  C .....RULE 2.6
  LRULE=26
  IF(TNPTFG) THEN
    FTP=SIGN(FY,-DP)
    DTP=SIGN(DY,-DP)
    K=(FTP-FPT)/(DTP-DPT)
    FP=PPP+(DPT-DPP)*KLAST+(DP-DPT)*K
    PPT=PPP+(DPT-DPP)*KLAST
    IF((FP*PPP).LT.0.) THEN
      DPT=DPP+(D.-PPP)/K
      CALL NEWST(MQD,ODC,ODF,ODU,OPC,OPT,OPU,AREAL,DM,
      &
      FC,FY,FU,FF,DC,DT,DU,DF,HO,H,LWP,OSOC,
      &
      SRFLAG,OSCT,SOC,STU,SUF,SR,SOB,SOE,
      &
      SQ6,DP,COI,PSQ6,DPT,EDP,OSUF)
      IF(SQ6.LE.0.3.AND.SQ6.GT.0.) THEN
        DJP=SIGN(DC,-DPT)
        FJP=SIGN(FC,DJP)
        K=FJP/(DJP-DPT)
      ELSE
        K=FR
      END IF
      IOPT2=3
      IF(KKL.EQ.2) KKL=5
      IF(KKL.EQ.4) KKL=6
      DPP=DPT
      PPP=0.
      FP=0.+(DP-DPT)*K
      RETURN
    END IF
  END IF
  RETURN
  END IF
  RETURN
  END IF
  END IF
  C 3. ....REVERSAL LOADING .....
  IF((KKL.EQ.5).OR.(KKL.EQ.8)) THEN
    IF((SQ6.LE.COI).OR.(LRULE.EQ.31)) THEN
      LRULE=31
      IF(DM.LT.DC.AND.DMAX.LT.DC) THEN
        K=KLAST
        PSE=0.
        DPP=0.
        FPP=0.
        USE=K
        IOPT2=3
        IF(KKL.EQ.5) KKL=3
        IF(KKL.EQ.6) KKL=1
      RETURN
    END IF
    IF(KKL.NE.KKLLST) THEN
      DJP=SIGN(DC,-DPT)
      FJP=SIGN(FC,DJP)
      IF(DPT.NE.0) THEN
        K=FJP/(DJP-DPT)
      ELSE
        K=FJP/(DJP-DPP)
      END IF
      CALL ODISSI(IOPT2,ESE,PSE,PSOLD,DPP,FPP,DPT,0.,AREAL,
      &
      K,USE,KKL,FC,DC,FT,DT,DM,DMAX,ODC,SOB,FU09,
      &
      LRULE)
      FP=0.+K*DPT
      DPT=0.
      DPP=DPT
      FPP=0.
    END IF
    END IF
    IF((DP*PP).GT.0.) THEN
      USE=(0.-FP)/(DP/3-DP)
    END IF
    IOPT2=3
    RETURN
  END IF
  C .....RULE 3.2
  LRULE=32
  IF((FP*PPP).LT.0.) THEN
    K=SR
  END IF
  IF(DPT.NE.0) THEN
    IOPT2=3
    CALL ODISSI(IOPT2,ESE,PSE,PSOLD,DPP,FPP,DPT,0.,AREAL,
    &
    K,USE,KKL,FC,DC,FT,DT,DM,DMAX,ODC,SOB,FU09,
    &
    LRULE)
    FP=0.+K*DPT
    DPT=0.
    DPP=DPT
    FPP=0.
  END IF
  END IF
  IF((DP*PP).GT.0.) THEN
    USE=(0.-FP)/(DP/3-DP)
  END IF
  IOPT2=3
  RETURN
  END IF
  C4....RELOADING AFTER UNLOADING.....
  IF((KKL.EQ.8).OR.(KKL.EQ.9)) THEN
    IF(DM.LT.DC.AND.DMAX.LT.DC) THEN
      K=KLAST
      USE=K
      IOPT2=3
      IF(KKL.EQ.7) KKL=3
      IF(KKL.EQ.8) KKL=1
    RETURN
  END IF
  IF(KKL.NE.KKLLST) THEN
    IF(POSTEG) THEN
      FFF=FU0(1)
      GGG=FU0(2)
      DDD=DU0(1)
      EEE=DU0(2)
    ELSE

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C      * ->IDPTT 1<-
C      * -> OSINC 1<-
C
IF((FP*PPP).LT. 0.) THEN
DPT=DPP-PPP/K
DPT=DP-DPT
CALL DMS(0,UPOS,EXCR,ODY,DP,ESE,PSE,PSEOLD,CSE)
C ----- IF K<SUP AND REACH FORCE=0 THEN SET FORCE=0 AND DP=DPT
C .AFTER THAT K=0
IF(KL.EQ.(-0.0001*SCT)) THEN
DP=DP
FP=0.
K=0.
STOP
END IF
IF(DM.LT.ODC .AND. DMAX.LT.ODC) THEN
IOPT2=3
ELSE
IOPT2=2
END IF
C
CALL DDISSI(IOPT2,ESE,PSE,PSEOLD,DPP,PPP,DPT,0.,AREAL,
K,USE,KCL,FC,DC,FT,DT,DM,DMAX,ODC,SOB,FUOO,LRULE)
IF((POSTFG).AND.(PPP.LT.0)) THEN
CYCLEN=CYCLEN+1
FMAX=0.
DMAX=ABS(DP)
END IF
IF(.NOT. POSTFG .AND. PPP.GT.0) THEN
CYCLEN=CYCLEN+1
FMAX=0.
DMAX=ABS(DP)
END IF
END IF
IF(DM.GT.ODC) THEN
CALL NEWSTI(MOD,ODC,ODY,ODU,OPC,OPF,OPU,AREAL,DM,FC,FT,
FU,FF,DC,DT,DU,DF,HO,H,LMP,OSOC,SRLFLAG,OSCT,SOC,
SCT,STU,SUF,SR,SOB,SOE,SOQ,DDP,COL,PSQ,6,DPT,
SDP,OSUP)
END IF
IF(KL.EQ.2) KKL=5
IF(KL.EQ.4) KKL=6
END IF
IF((KL.EQ.7).AND.(KKLST.EQ.4)).OR.
((KL.EQ.8).AND.(KKLST.EQ.2)) THEN
CYCLEN=CYCLEN+1
DM=MAX(DM,DMAX)
END IF
RETURN
END
C-----
C DEBUT UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
C END DEBUT
C-----
C == NEWSTI SUBRTN (FIND STIFFNESS WHEN PASSING HORIZ. AXIS (FORCE=0)
C-----
SUBROUTINE NEWSTI(MOD,ODC,ODY,ODU,OPC,OPF,OPU,AREAL,DM,FC,FT,
FU,FF,DC,DT,DU,DF,HO,H,LMP,OSOC,SRLFLAG,OSCT,SOC,
SCT,STU,SUF,SR,SOB,SOE,SOQ,DDP,COL,PSQ,6,DPT,
SDP,OSUP)
REAL MOD,LMP
LOGICAL SRLFLAG
SQ6=AREAL/(OFU*ODU)
TOL=LE-10
IF(ABS(DM).LT.ODC .AND. SQ6.LT.TOL) SQ6=0.
PSDP=SDP
SDP=DPT
ALPHA=HO/H
IF((SQ6.GT.0.).AND.(SQ6.LE.0.3)) THEN
DC=DC
FC=FC
ELSE
B=0.2543*SQ6
BR=(0.27798*10**B)*OSOC
DC=ABS(SDP)*0.25
DC=MAX(DC,ODC)
IF(DC.GT.(1.5*ODC)) THEN
DC=1.5*ODC
END IF
IF(ABS(SDP).GT.DU) THEN
FC=0.98*FC
SR=FC/(ABS(SDP)+DC)
END IF
C
FC=SR*(ABS(SDP)+DC)
END IF
IF(SQ6.NE.0.) THEN
B=0.20102*SQ6
BB=1.1578*10**B
IF(BB.GT.1.) BB=1
SCT=BB*OSCT
END IF
COB1=(OPF-OPC)/(ODY-ODC)
COB2=(OPU-OPF)/(ODU-ODY)
STU=(COB2/COB1)*SCT
IF(SQ6.LE.0.) THEN
DT=DT
ELSE
IF((SQ6.GT.0.).AND.(SQ6.LE.0.312)) THEN
DT=ODY
ELSE
DT=(1.5299+0.4546*LOG(SQ6))*ODT
END IF
IF(SQ6.EQ.0.) THEN
DU=DU
ELSE
DU=DT*(ODU/ODT)
DU=MAX(DU,1.5*DT)
END IF
FT=FC*(DT-DC)*SCT
FU=FU*(DU-DT)*STU
C ASSUME REFERENCE FAILURE POINT F
DU=DU*(FP-FU)/SUP
C 7/11/92 10:10 SAT. BASED ON OBSERVATION, SUP=-0.5*6CT
SUF=-0.5*SCT
FF=(FC+FU)/2
SOE=FC/DC
SCT=FU/DT
SCU=(FU-FC)/(DU-DC)
XA=FU/SOY
XB=(XA-DI)/2
SOB=FT/XB
XD=FU/SOY
XE=(XD+DU)/2
SCE=FU/XE
SQ6=SQ6
RETURN
END
C-----
C DEBUT UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
C END DEBUT
C-----
C == SUBROUTINE DDISEI SUBRTN (AREAL=ENERGY DISSIPATION)
C == CALCULATE THE ELASTIC STRAIN ENERGY(BEB(1)) AND
C == PLASTIC STRAIN ENERGY (PSE)
C-----
SUBROUTINE DDISEI(IOPT2,ESE,PSE,PSEOLD,DPP,PPP,DP,FP,AREAL,
K,USE,KCL,FC,DC,FT,DT,DM,DMAX,ODC,SOB,FUOO,
LRULE)
INTEGER LRULE
HTS09150 REAL K HTS10570
HTS09160 DIMENSION ESE(3) HTS10580
HTS09170 C AREA1 REPRESENTING AREA ENCLOSED BY THE HYSTERESIS RESPONSE HTS10590
C TEE,ESE(1),ESE(2),ESE(3),PSE,PSEOLD SEE STRENG SUBRTN HTS10600
C USE = EQUIVALENT UNLOADING STIFFNESS HTS10610
C----- HTS10620
IF(IOPT2.EQ.1 .AND. K.NE.0.) THEN
C ... CALCULATE THE EQUIVALENT UNLOADING STIFFNESS
IF((ABS(FUOO).GT.0.7*FT).OR.(KL.EQ.7 .OR. KKL.EQ.8 .OR.
KCL.EQ.9 .OR. KKL.EQ.10)) THEN
DDPP=SIGN(DC,-DP)
FP=SIGN(FC,DDPP)
SKL1=(FP-PPP-FUOO*0.85)/(DDPP-DP)
SK1=MAX(SKL1,SOB)
DP1=DP*(FP/6-FP*0.85)/SK1
FP=SIGN(FT,-DP1)
DTP=SIGN(DT,FP)
DP2=DP1*(0.-FP/6)/SK2
AA=0.5*(0.85*FP+FP/6)*(DP-DP1)+0.5*FP/6*(DP1-DP2)
USE=(FP*2)/(2*ABS(AA))
ELSE IF((ABS(FP).LE.0.7*FT) THEN
SK1 = FC/DC
FP1=SIGN(FC/3,FP)
DP1=DP*(FP1-FC)/SK1
DCP = SIGN(DC,-DP)
DCP = SIGN(DCP,DP)
SK2 = (FCP-FP1)/(DCP-DP1)
DP2 = DP1 + (0.-FP1)/SK2
AA=0.5*(FP1+FP)*(DP-DP1)+0.5*FP1*(DP1-DP2)
USE = (FP*2)/(2*ABS(AA))
END IF
END IF
IF(IOPT2.EQ.2 .AND. K.NE.0.) THEN
IF(DM.LE.ODC .AND. DMAX.LE.ODC .AND. (KKL.EQ.1 .OR.
KCL.EQ.3 .OR. KKL.EQ.5 .OR. KKL.EQ.6)) THEN
USE=K
ELSE IF((DP*FP).GT.0.) THEN
USE=(0.-FP)/(DP/3-DP)
END IF
CALL STRENG(ESE,PSE,PSEOLD,FP,PPP,DP,OPP,K,USE)
AREAL=ESE(1)+PSE
RETURN
END
C-----
C DEBUT UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
C END DEBUT
C-----
C == SUBROUTINE DINTER SUBRTN (CHECK IF NEW CURRENT TRACK INTERSECT
C == IF YES, FIND CONNECT PT. (UPDATED CURRENT PT.)
C-----
SUBROUTINE DINTER(PC,FT,EU,FF,PPP,DP,DU,DF,DDP,SOC,SCT,STU,
SUF,K,FP,DP,OSINC,KKL,LRULE,POSTFG,DUO,FUD)
REAL K,KJ,DUO(4),FUO(4)
LOGICAL POSTFG
IF(ABS(DP).LT.DC) THEN
KJ=FP/DP
IF(KJ.LT.SOC) RETURN
IF(KJ.EQ.SOC) THEN
IF(OSINC.GT.0) KKL=1
IF(OSINC.LT.0) KKL=3
RETURN
END IF
IF(KJ.GT.SOC .AND. K.GT.SOC) THEN
IF(LRULE.EQ.51 .OR. LRULE.EQ.52) THEN
K=(K*DDP-PPP)/(K-SOC)
T=**SOC
ADP=SIGN(X,DDP)
AFP=SIGN(Y,DDP)
PP=AFP+SOC*(DP-ADP)
IF(OSINC.GT.0) KKL=1
IF(OSINC.LT.0) KKL=3
K=**SOC
END IF
C ----- FOR RELOAD DURING REVERSAL STAGE
IF(LRULE.EQ.41 .OR. LRULE.EQ.42) THEN
IF(DP.GT.0) THEN
IF(POSTFG) THEN
FFFF=FUO(1)
GGGG=DUO(1)
ELSE
FFFF=FUO(2)
GGGG=DUO(2)
END IF
ELSE
IF(POSTFG) THEN
FFFF=FUO(2)
GGGG=DUO(2)
ELSE
FFFF=FUO(1)
GGGG=DUO(1)
END IF
END IF
IF(ABS(DP).GT.ABS(GGGG) .AND. (DP*GGGG).GT.0) THEN
T2=PPP+(GGGG-DPP)*K
SK=(ABS(FC)-ABS(T2))/(ABS(DC)-ABS(GGGG))
PP=T2+(DP-GGGG)*SK
IF(DP.GT.0) KKL=6
IF(DP.LT.0) KKL=5
K=SK
RETURN
END IF
END IF
IF(ABS(DPP).LT.0 .AND. (LRULE.EQ.41 .OR. LRULE.EQ.42)
.AND. (DP*GGGG).GT.0) THEN
T2=PPP+(GGGG-DPP)*K
SK=(ABS(FC)-ABS(T2))/(ABS(DC)-ABS(GGGG))
PP=T2+(DP-GGGG)*SK
IF(DP.GT.0) KKL=6
IF(DP.LT.0) KKL=5
K=SK
RETURN
END IF
IF(DP.LT.0) PP=T2+(DC-GGGG)*SK*(DP-DC)*SCT
IF(OSINC.GT.0) KKL=1
IF(OSINC.LT.0) KKL=3
K=**SCT
RETURN
END IF
KJ=(ABS(FP)-FC)/(ABS(DP)-DC)
IF(KJ.LT.SCT) RETURN
IF(KJ.EQ.SCT) THEN
IF(DSINC.GT.0) KKL=1
IF(DSINC.LT.0) KKL=3
END IF
KJ=(ABS(FP)-FC)/(ABS(DP)-DC)
IF(KJ.LT.SCT) RETURN
IF(KJ.EQ.SCT) THEN
IF(DSINC.GT.0) KKL=1
IF(DSINC.LT.0) KKL=3
END IF

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RETURN
END IF
IF (KJ,GT, SCT) THEN
X=(K*ABS(DPP)-DC*SCT-ABS(FPP)+FC)/(K-SCT)
Y=ABS(FPP)+K*ABS(X)-K*ABS(DPP)
IF (ABS(X),LT,DC) THEN
X=(K*DDP-FPP)/(K-SOC)
Y=K*SOC
ADP=SIGN(X, DPP)
APP=SIGN(Y, DPP)
IF (DP,GT,0) FP=APP+SOC*(DC-ADP)+SCT*(DP-DC)
IF (DP,LT,0) FP=APP+SOC*(-DC-ADP)+SCT*(DP-DC)
IF (DSINC,GT,0) KKL=1
IF (DSINC,LT,0) KKL=3
K=SCT
RETURN
END IF
ADP=SIGN(X, DPP)
APP=SIGN(Y, DPP)
FP=APP+SCT*(DP-ADP)
IF (DSINC,GT,0) KKL=1
IF (DSINC,LT,0) KKL=3
K=SCT
END IF
RETURN
END IF
IF ((ABS(DP),GE,DT) .AND. ABS(DP),LT,DU) THEN
KJ=(ABS(FP)-FT)/(ABS(DP)-DT)
IF (KJ,LT,STU) RETURN
IF (KJ,GT,STU) THEN
IF (DSINC,GT,0) KKL=1
IF (DSINC,LT,0) KKL=3
RETURN
END IF
IF (KJ,GT,STU) THEN
X=(FT-DT*STU+K*ABS(DPP)-ABS(FPP))/(K-STU)
Y=ABS(FPP)+K*ABS(X)-K*ABS(DPP)
IF (ABS(X),LT,DT) THEN
X=(K*ABS(DPP)-DC*SCT-ABS(FPP)+FC)/(K-SCT)
Y=ABS(FPP)+K*ABS(X)-K*ABS(DPP)
ADP=SIGN(X, DPP)
APP=SIGN(Y, DPP)
IF (DP,GT,0) FP=APP+SCT*(DT-ADP)+STU*(DP-DT)
IF (DP,LT,0) FP=APP+SCT*(-DT-ADP)+STU*(DP-DT)
IF (DSINC,GT,0) KKL=1
IF (DSINC,LT,0) KKL=3
K=STU
RETURN
END IF
ADP=SIGN(X, DPP)
APP=SIGN(Y, DPP)
FP=APP+STU*(DP-ADP)
IF (DSINC,GT,0) KKL=1
IF (DSINC,LT,0) KKL=3
K=STU
END IF
RETURN
END IF
IF (ABS(DP),GE,DU .AND. ABS(DP),LT,DF) THEN
KJ=(ABS(FP)-FU)/(ABS(DP)-DU)
IF (KJ,LT,0) .AND. (KJ,LT,SUF) RETURN
IF (KJ,GT,SUF) THEN
IF (DSINC,GT,0) KKL=1
IF (DSINC,LT,0) KKL=3
RETURN
END IF
IF (KJ,GT,SUF) THEN
X=(FU-DU*SUF+K*ABS(DPP)-ABS(FPP))/(K-SUF)
Y=ABS(FPP)+K*ABS(X)-K*ABS(DPP)
IF (ABS(X),LT,DU) THEN
X=(K*ABS(DPP)-DC*SCT-ABS(FPP)+FC)/(K-SCT)
Y=ABS(FPP)+K*ABS(X)-K*ABS(DPP)
ADP=SIGN(X, DPP)
APP=SIGN(Y, DPP)
IF (DP,GT,0) FP=APP+SCT*(DU-ADP)+SUF*(DP-DU)
IF (DP,LT,0) FP=APP+SCT*(-DU-ADP)+SUF*(DP-DU)
IF (DSINC,GT,0) KKL=1
IF (DSINC,LT,0) KKL=3
K=SUF
RETURN
END IF
ADP=SIGN(X, DPP)
APP=SIGN(Y, DPP)
FP=APP+SUF*(DU-ADP)
IF (DSINC,GT,0) KKL=1
IF (DSINC,LT,0) KKL=3
K=SUF
END IF
RETURN
END IF
IF (ABS(DP),GE,DF .AND. ABS(DP),LT,(20*DT)) THEN
KJ=(ABS(FP)-FF)/(ABS(DP)-DF)
DG=20*DT
SFG=(0.1*FU-FF)/(DG-DF)
IF (KJ,GT,SFG) THEN
IF (DSINC,GT,0) KKL=1
IF (DSINC,LT,0) KKL=3
RETURN
END IF
IF (KJ,GT,SFG) THEN
X=(0.1*FU-DG*SFG+K*ABS(DPP)-ABS(FPP))/(K-SFG)
Y=ABS(FPP)+K*ABS(X)-K*ABS(DPP)
IF (ABS(X),LT,DU) THEN
X=(K*ABS(DPP)-DC*SCT-ABS(FPP)+FC)/(K-SCT)
Y=ABS(FPP)+K*ABS(X)-K*ABS(DPP)
ADP=SIGN(X, DPP)
APP=SIGN(Y, DPP)
IF (DP,GT,0) FP=APP+SCT*(DU-ADP)+SUF*(DP-DU)
IF (DP,LT,0) FP=APP+SCT*(-DU-ADP)+SUF*(DP-DU)
IF (DSINC,GT,0) KKL=1
IF (DSINC,LT,0) KKL=3
K=SFG
RETURN
END IF
ADP=SIGN(X, DPP)
APP=SIGN(Y, DPP)
FP=APP+SUF*(DU-ADP)
IF (DSINC,GT,0) KKL=1
IF (DSINC,LT,0) KKL=3
K=SFG
END IF
RETURN
END IF
IF (ABS(X),LT,DF .AND. ABS(X),GT,DU) THEN
X=(FF-DP*SUF+K*ABS(DPP)-ABS(FPP))/(K-SUF)
Y=ABS(FPP)+K*ABS(X)-K*ABS(DPP)
ADP=SIGN(X, DPP)
APP=SIGN(Y, DPP)
IF (DP,GT,0) FP=APP+SUF*(DF-ADP)+SFG*(DP-DF)
IF (DP,LT,0) FP=APP+SUF*(-DF-ADP)+SFG*(DP-DF)
IF (DSINC,GT,0) KKL=1
IF (DSINC,LT,0) KKL=3
K=SFG
RETURN
END IF
ADP=SIGN(X, DPP)
APP=SIGN(Y, DPP)
FP=APP+SUF*(DF-ADP)
IF (DSINC,GT,0) KKL=1
IF (DSINC,LT,0) KKL=3
K=SFG
END IF
RETURN
END IF
ADP=SIGN(X, DPP)

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READ(5,*)ISP1
IF(ISP1.EQ.0) THEN
  WRITE(6,*)
  WRITE(6,*) NOT INCLUDE STATICS CASE EXTERNAL DEFORMATIONS
  WRITE(6,*)
ENDIF
IF(ISP1.EQ.1) THEN
  WRITE(6,*)
  WRITE(6,*) STATICS CASE EXTERNAL DEFORMATIONS ARE INCLUDED
  WRITE(6,*)
ENDIF
C*
IF(ISP1.EQ.1) THEN
  DO 49 I=1,NDOF
    DISP(I)=SGD(I,2)
    VEL(I)=0
    ACC(I)=0
    FORC(I)=0
49 CONTINUE
ENDIF
C*
INPUT: NODE,IGEN,INC,OPTION,DX,DT,DS,RX,RY,RZ
C
IF(ISP1.NE.1) THEN
  READ(5,*) NODE,IGEN,INC,OPTION,(A(I),I=1,6)
  IF(NODE.LE.0) GO TO 500
  IF(HEAD) THEN
    HEAD=.FALSE.
    WRITE(6,5) TITLE(1),TITLE(2)
  ENDIF
  WRITE(6,7) NODE,IGEN,INC,OPTION,(A(I),I=1,6)
  5 FORMAT(1) STRUCTURE.....:A//
    SOLUTION.....:A//
    INITIAL ACCELERATIONS, VELOCITIES DISPLACEMENTS,
    AND LOADS //
    *****
    4 4X,'NODE',2X,'IGEN',3X,'INC',1X,'COMPONENT',
    4 74,'TX',10X,'TY',10X,'TZ',
    4 10X,'RX',10X,'RY',10X,'RZ')
    7 FORMAT(2X,316,1X,A1P,SG12.4)
C----- DETERMINE IF DISPLACEMENT, VELOCITY, ACCELERATION OR LOAD
IOP=0
IF( (TEST(OPTION,'DISP',.FALSE.)) THEN
  IOP=ISSET(IOP,1)
ELSE IF( (TEST(OPTION,'VEL',.FALSE.)) THEN
  IOP=ISSET(IOP,2)
ELSE IF( (TEST(OPTION,'ACC',.FALSE.)) THEN
  IOP=ISSET(IOP,3)
ELSE IF( (TEST(OPTION,'LOA',.FALSE.)) THEN
  IOP=ISSET(IOP,4)
ELSE IF( (TEST(OPTION,'FOR',.FALSE.)) THEN
  IOP=ISSET(IOP,4)
ELSE IF( (TEST(OPTION,'END',.FALSE.)) THEN
  GO TO 500
ELSE
  WRITE(6,*) 'INVALID OPTION:',OPTION
  GO TO 10
ENDIF
C----- DETERMINE INTERNAL JOINT NUMBER
20 JOINT=IQUICK(NODE,IGEN)
IF(JOINT.LE.0) THEN
  WRITE(6,*) 'NODE #',NODE,
  6 WAS NOT FOUND, CHECK INPUT, INITIAL VALUE IS OMITTED
  GO TO 10
ENDIF
C----- PUT INITIAL VALUES IN PROPER ARRAYS
DO 30 I=1,6
  IF(A(I).EQ.0) GO TO 30
  II=IDOF(JOINT,I)
  IF( (BTEST(JPLG(JOINT),I+1)) THEN
    WRITE(6,*) 'CONSTRAINED DOF #',I, ' COMPONENT IS SKIPPED'
  ELSE IF( (BTEST(IOP,1)) THEN
    DISP(II)=A(I)
  ELSE IF( (BTEST(IOP,2)) THEN
    VEL(II)=A(I)
  ELSE IF( (BTEST(IOP,3)) THEN
    ACC(II)=A(I)
  ELSE IF( (BTEST(IOP,4)) THEN
    FORC(II)=A(I)
  ENDIF
30 CONTINUE
C----- GENERATE VALUES AT OTHER NODES
IF(IGEN.GT.3) THEN
  IGEN=IGEN-1
  NODE=NODE+INC
  GO TO 20
ENDIF
C----- LOOP TO READ ADDITIONAL VALUES
GO TO 10
ENDIF
500 CONTINUE
ISP1=0
C----- PRINT OUT NON ZERO INITIAL VALUES
FLAG=.TRUE.
DO 520 I=1,NDOF
  IF(DISP(I).NE.0 .OR. VEL(I).NE.0 .OR. ACC(I).NE.0 .OR.
  4 FORC(I).NE.0) THEN
    IF(FLAG) WRITE(6,510)
    WRITE(6,530) I,FORC(I),DISP(I),VEL(I),ACC(I)
    FLAG=.FALSE.
  ENDIF
520 CONTINUE
C
510 FORMAT(//5X,'INITIAL, NON-ZERO VALUES'/
  6 7X,'DOF',7X,'FORCES',10X,'DISP',11X,'VEL',11X,'ACC ')
530 FORMAT(110,4G15.6)
RETURN
C
SUBROUTINE INTPO(PERIOD,RSV,NAP,RSNP)
DIMENSION RSNP(2,NAP)
C== INTERPOLATION OF RESPONSE SPECTRUM VALUE
C-----
IF(PERIOD.LT.RSNP(1,1)) THEN
  RSV=RSNP(2,1)
RETURN
ELSE IF(PERIOD.GT.RSNP(1,NAP)) THEN
  RSV=RSNP(2,NAP)
RETURN
ENDIF
I=0
300 I=I+1
J=I+1
IF(PERIOD.GE.RSNP(1,I).AND.PERIOD.LT.RSNP(1,J)) THEN
  COEF=(RSNP(2,J)-RSNP(2,I))/(RSNP(1,J)-RSNP(1,I))
  RSV=(PERIOD-RSNP(1,I))*COEF+RSNP(2,I)
ELSE IF(PERIOD.GE.RSNP(1,J)) THEN
  GO TO 300
ENDIF
C
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INT00260 RETURN
INT00270 END
INT00280 C - INTERSECTION OF TWO LINES IN POINT SLOPE FORM .....
INT00290 C DEBUG UNIT(6),SUBCHK,SUBTRACE
INT00300 C END DEBUG
INT00310 C X = X COORDINATE OF INTERSECTION
INT00320 C Y = Y COORDINATE OF INTERSECTION
INT00330 C X1 = X COORDINATE OF LINE 1
INT00340 C Y1 = Y COORDINATE OF LINE 1
INT00350 C S1 = SLOPE OF LINE 1
INT00360 C X2 = X COORDINATE OF LINE 2
INT00370 C Y2 = Y COORDINATE OF LINE 2
INT00380 C S2 = SLOPE OF LINE 2
INT00390
INT00400 SUBROUTINE INTSCT(X,Y,X1,Y1,S1,X2,Y2,S2)
INT00410 IMPLICIT REAL(A-H,O-Z)
INT00420 IMPLICIT INTEGER(L-N)
INT00430 LOGICAL BTEST
INT00440 IF(S1.NE.S2) THEN
INT00450 B1= Y1 - S1*X1
INT00460 B2= Y2 - S2*X2
INT00470 T=(B1-B2)/(S2-S1)
INT00480 T=(B1-S2*B2+S1)/(S2-S1)
INT00490 ELSE
INT00500 WRITE(6,10) X1,Y1,S1,X2,Y2,S2
INT00510 10 FORMAT(//***** ER* ROR IN INTSCT....//,1X,
INT00520 4 ' S1=S2, NO SOLN, THUS SET X=T*0. //
INT00530 4 ' X1=',1P,G15.5, ' Y1=',G15.5, ' S1=',G15.5//
INT00540 4 ' X2=',1P,G15.5, ' Y2=',G15.5, ' S2=',G15.5//)
INT00550 X=0
INT00560 Y=0
INT00570 CALL ERRTRA
INT00580 ENDF
INT00590 CC WRITE(6,5) X1,Y1,S1,X2,Y2,S2,B2,X,Y
INT00600 CC 5 FORMAT(// IN INTSCT....//
INT00610 CC 4 ' X1=',1P,G15.5, ' Y1=',G15.5, ' S1=',G15.5//
INT00620 CC 4 ' X2=',1P,G15.5, ' Y2=',G15.5, ' S2=',G15.5//
INT00630 CC 4 ' X=',1P,G15.5, ' Y=',G15.5//)
INT00640 RETURN
INT00650 END
C-----
INT00670 C DEBUG UNIT(6),SUBCHK,SUBTRACE
INT00680 C END DEBUG
INT00690 C INTEGER FUNCTION IQUICK(TARGET,ID,N)
INT00700 C INTEGER TARGET,ID(N)
INT00710 C LOGICAL BTEST
C
IF(ID(1).GT.TARGET .OR. ID(N).LT.TARGET) GO TO 100
IF(ID(IQUICK).EQ.TARGET) RETURN
IQUICK=N
IF(ID(IQUICK).EQ.TARGET) RETURN
J=(N+1)/2
IQUICK=J
10 J= J /2
IF(IQUICK.LT.1) IQUICK=1
IF(IQUICK.GT.N) IQUICK=N
IF(ID(IQUICK).EQ.TARGET) RETURN
IF(ID(IQUICK).GT.TARGET .AND. J.NE.0) THEN
  IQUICK=IQUICK-J
  GO TO 10
ELSE IF(ID(IQUICK).LT.TARGET .AND. J.NE.0) THEN
  IQUICK=IQUICK+J
  GO TO 10
ENDIF
C
I=MAX(IQUICK-2,1)
J=MIN(I+4,N)
DO 20 IQUICK=I,J,1
  IF(ID(IQUICK).EQ.TARGET) RETURN
20 CONTINUE
C
DO 30 IQUICK=1,N
  IF(ID(IQUICK).EQ.TARGET) RETURN
30 CONTINUE
100 WRITE(6,110) TARGET,IQUICK,ID(IQUICK)
110 FORMAT(' TARGET',18,' NOT FOUND, LAST ID(',16,')',18)
IQUICK=0
RETURN
END
C-----
INT00880 C END DEBUG UNIT(6),SUBCHK,SUBTRACE
INT00890 C END DEBUG
INT00900 C
INT00910 C SUBROUTINE ISORT(ID,INDEX,N)
INT00920 C INTEGER ID(N)
INT00930 C INTEGER INDEX(N)
INT00940 C LOGICAL FLAG
INT00950 C LOGICAL BTEST
INT00960 C DO 10 I=1,N
INT00970 C INDEX(I)=I
10 CONTINUE
C
WRITE(6,*) I,INDEX(I),ID(I)
10 CONTINUE
C
DO 30 I=N,2,-1
  FLAG=.TRUE.
  DO 20 J=2,I
    IF( (ID(INDEX(J)).LT.ID(INDEX(J-1))) THEN
      K=INDEX(J)
      INDEX(J)=INDEX(J-1)
      INDEX(J-1)=K
      FLAG=.FALSE.
    ENDIF
  20 CONTINUE
  IF(FLAG) RETURN
30 CONTINUE
RETURN
END
C#PROCESS SDUMP GOSTMT XREF
SUBROUTINE ITERS(PI,TMXI,TMJ,TMPX,TMPY,TMPJ,TMPXJ,
  4 TMI,TMJ,TMPJ,TMPYJ,IAFAG)
C-----
C THIS SUBROUTINE IS FOR JUDGING BOX SECTION STRENGTH CRITERIA
C BASED ON W.P.C.'S PAPER
C NOTE:
C PI : CURRENT AXIAL LOAD
C PJ : AXIAL YIELDING LOAD CAPACITY
C TMXI : CURRENT MOMENT IN X DIRECTION AT I END
C TMXJ : CURRENT MOMENT IN X DIRECTION AT J END
C TMI : CURRENT MOMENT IN Y DIRECTION AT I END
C TMIJ : CURRENT MOMENT IN Y DIRECTION AT J END
C TMPX : FULLY PLASTIC MOMENT IN X DIRECTION
C TMPY : FULLY PLASTIC MOMENT IN Y DIRECTION
C TMPXJ : CURRENT REDUCED PLASTIC MOMENT IN X DIRECTION AT I END
C TMPXJ : CURRENT REDUCED PLASTIC MOMENT IN X DIRECTION AT J END
C TMPYJ : CURRENT REDUCED PLASTIC MOMENT IN Y DIRECTION AT I END
C TMPYJ : CURRENT REDUCED PLASTIC MOMENT IN Y DIRECTION AT J END
C IAFAG : FLAG FOR JUDGING FULLY PLASTIC SECTION
C 1: MEMBER TWO ENDS STILL ARE NOT FULLY PLASTIC
C 2: MEMBER A END IS IN FULLY PLASTIC
C 3: MEMBER B END IS IN FULLY PLASTIC
C 4: MEMBER A AND B ENDS ARE IN FULLY PLASTIC
C-----
IF(PI.GE.0) RETURN
SP=ABS(PI/PI)
IF(SP.EQ.0) THEN
  RETURN
ELSE IF(SP.LT.1 .AND. SP.GT.0) THEN
  A1=1.7-(SP/LOG(SP))
ELSE IF(SP.GE.1) THEN
  A1=1.7
RETURN
END
```

```
INT00250
INT00260
INT00010
INT00020
INT00030
INT00040
INT00050
INT00060
INT00070
INT00080
INT00090
INT00100
INT00110
INT00120
INT00130
INT00140
INT00150
INT00160
INT00170
INT00180
INT00190
INT00200
INT00210
INT00220
INT00230
INT00240
INT00250
INT00260
INT00270
INT00280
INT00290
INT00300
INT00310
INT00320
INT00330
INT00340
INT00350
INT00360
INT00370
INT00380
INT00390
INT00400
INT00410
INT00420
INT00430
INT00440
INT00450
INT00460
INT00470
INT00480
INT00490
INT00500
INT00510
INT00520
INT00530
INT00540
INT00550
INT00560
INT00570
INT00580
INT00590
INT00600
INT00610
INT00620
INT00630
INT00640
INT00650
INT00660
INT00670
INT00680
INT00690
INT00700
INT00710
INT00720
INT00730
INT00740
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INT00770
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INT00800
INT00810
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INT00880
INT00890
INT00900
INT00910
INT00920
INT00930
INT00940
INT00950
INT00960
INT00970
INT00980
INT00990
INT01000
INT01010
INT01020
INT01030
INT01040
INT01050
INT01060
INT01070
INT01080
INT01090
INT01100
INT01110
INT01120
INT01130
INT01140
INT01150
INT01160
INT01170
INT01180
INT01190
INT01200
INT01210
INT01220
INT01230
INT01240
INT01250
INT01260
INT01270
INT01280
INT01290
INT01300
INT01310
INT01320
INT01330
INT01340
INT01350
INT01360
INT01370
INT01380
INT01390
INT01400
INT01410
INT01420
INT01430
INT00010
INT00020
INT00030
INT00040
INT00050
INT00060
INT00070
INT00080
INT00090
INT00100
INT00110
INT00120
INT00130
INT00140
INT00150
INT00160
INT00170
INT00180
INT00190
INT00200
INT00210
INT00220
INT00230
INT00240
INT00250
INT00260
INT00270
INT00280
INT00290
INT00300
INT00310
INT00320
INT00330
```

```
IAPAG=4
RETURN
ENDIF
TMPCKI=1.2*(1-SP)*TMPX
IF(ABS(TMPCKI).GT.ABS(TMPX)) TMPCKI=TMPX
TMPCKJ=TMPCKI
TMPCKI=1.2*(1-SP)*TMPY
IF(ABS(TMPCKI).GT.ABS(TMPY)) TMPCKI=TMPY
TMPCKJ=TMPCKI
RMKI=ABS(TMKI)
RMKJ=ABS(TMKJ)
RMTI=ABS(TMTI)
RMTJ=ABS(TMTJ)

IF( IAPAG .EQ. 1 ) THEN
IF( (RMKI/TMPCKI) .GE. 1 .OR. (RMTI/TMPCKI) .GE. 1 ) THEN
YIELD1=1
ELSE IF( (RMKJ/TMPCKJ) .LT. 1 .AND. (RMTJ/TMPCKJ) .LT. 1 ) THEN
YIELD1=(RMKI/TMPCKI)**A1 + (RMTI/TMPCKI)**A1
ENDIF
IF( (RMKJ/TMPCKJ) .GE. 1 .OR. (RMTJ/TMPCKJ) .GE. 1 ) THEN
YIELD2=1
ELSE IF( (RMKJ/TMPCKJ) .LT. 1 .AND. (RMTJ/TMPCKJ) .LT. 1 ) THEN
YIELD2=(RMKJ/TMPCKJ)**A1 + (RMTJ/TMPCKJ)**A1
ENDIF
IF( YIELD1 .GE. 1 .AND. YIELD2 .LT. 1 ) THEN
IAPAG=2
ELSE IF( YIELD1 .LT. 1 .AND. YIELD2 .GE. 1 ) THEN
IAPAG=3
ELSE IF( YIELD1 .GE. 1 .AND. YIELD2 .GE. 1 ) THEN
IAPAG=4
ENDIF
ELSE IF( IAPAG .EQ. 2 ) THEN
IF( (RMKJ/TMPCKJ) .GE. 1 .OR. (RMTJ/TMPCKJ) .GE. 1 ) THEN
YIELD2=1
ELSE IF( (RMKJ/TMPCKJ) .LT. 1 .AND. (RMTJ/TMPCKJ) .LT. 1 ) THEN
YIELD2=(RMKJ/TMPCKJ)**A1 + (RMTJ/TMPCKJ)**A1
ENDIF
IF( YIELD2 .GE. 1 ) THEN
IAPAG=4
ENDIF
ELSE IF( IAPAG .EQ. 3 ) THEN
IF( (RMKI/TMPCKI) .GE. 1 .OR. (RMTI/TMPCKI) .GE. 1 ) THEN
YIELD1=1
ELSE IF( (RMKI/TMPCKI) .LT. 1 .AND. (RMTI/TMPCKI) .LT. 1 ) THEN
YIELD1=(RMKI/TMPCKI)**A1 + (RMTI/TMPCKI)**A1
ENDIF
IF( YIELD1 .GE. 1 ) THEN
IAPAG=4
ENDIF
ENDIF
RETURN
END
SUBROUTINE ITERST(PI, PY, TMKI, TMKJ, TMPX,
& TMTI, TMTJ, TMPY,
& TIX, TIY, E, CL, AR, TJ,
& SLEK, SLKX, SLKY, SX, S,
& H, IAPAG)
-----
THIS SUBROUTINE IS FOR JUDGING BOX SECTION STABILITY CRITERIA
BASED ON W.P.C.'S PAPER
NOTE:
PI : CURRENT AXIAL LOAD
PY : AXIAL YIELDING LOAD CAPACITY
TMKI : CURRENT MOMENT IN X DIRECTION AT I END
TMKJ : CURRENT MOMENT IN X DIRECTION AT J END
TMTI : CURRENT MOMENT IN Y DIRECTION AT I END
TMTJ : CURRENT MOMENT IN Y DIRECTION AT J END
TMPX : FULLY PLASTIC MOMENT IN X DIRECTION
TMPY : FULLY PLASTIC MOMENT IN Y DIRECTION
TIX : MOMENT OF INERTIA ABOUT X DIRECTION
TIY : MOMENT OF INERTIA ABOUT Y DIRECTION
B : BOX SECTION WIDTH
H : BOX SECTION HIGH
E : ELASTIC MODULUS OF ELASTICITY
AR : NET AREA OF BOX SECTION
IAPAG : FLAG 0: CRITERIA SATISFIED, 1: CRITERIA .GT. 1
SLEK : SLENDERNESS COEFFICIENT IN X DIRECTION
SLKX : SLENDERNESS COEFFICIENT IN Y DIRECTION
SX : SECTION MODULUS IN X DIRECTION ( STRONG AXIS DIRECTION)
J : TORSIONAL RIGIDITY
VALUE: STABILITY CRITERIA FORMULA
>1 : UNSTABLE, <1 : STABLE
NTYPE: TYPE OF SECTION
1 : FOR BOX SECTION
-----
SKIP TENSION AXIAL LOAD CASE
IF( PI .LT. 0 ) RETURN
PIC=ABS(PI)
PII=3.1415926
PY=PT/AR
RX=SQRT(TIX/AR)
RY=SQRT(TIY/AR)
TNAX=SLKX*CL/RX
TNAY=SLKY*CL/RY
TNAC=TNAX/SQRT(PII*PII*S/PY)
IF( TNAC .LE. 1.5 ) THEN
PN=PI*EXP(-0.419*TNAC*TNAC)
ELSE IF( TNAC .GT. 1.5 ) THEN
PN=0.877*PI*(TNAC*TNAC)
ENDIF
PEX=PII*PII*E*AR/((SLEK*CL/RX)**2)
PEY=PII*PII*E*AR/((SLKY*CL/RY)**2)
CALCULATE COLUMN LATERAL BUCKLING MOMENT
IF( ABS(TMKI) .GT. ABS(TMKJ) ) THEN
TM1=TMKI
TM2=TMKI
ELSE IF( ABS(TMKI) .LE. ABS(TMKJ) ) THEN
TM1=TMKJ
TM2=TMKJ
ENDIF
IF( TM2 .EQ. 0 ) RETURN
CB=1.75+1.05*(TM1/TM2)+0.3*(TM1/TM2)**2
IF( CB .GT. 2.3 ) CB=2.3
TMR=PI*SX
TLR=57000*RT*SQRT(TJ*AR)/TMR
TLR=715*CB*RT*SQRT(TJ*AR)/TMPY
TMR=57000*CB*SQRT(TJ*AR)/(CL/RT)
IF( CL .GT. TLR ) THEN
TMX=TMCR
IF(TMX .GT. (CB*TMR)) TMX=CB*TMR
ELSE IF( TLR .LT. CL .AND. CL .LT. TLR ) THEN
TMX=CB*(TMPX-(TMPX-TMR)*(CL-TLR)/(TLR-TLR))
IF(TMX .GT. TMPX) TMX=TMPX
ELSE IF( CL .LT. TLR ) THEN
TMX=TMPX
ENDIF
CALCULATE REDUCED PLASTIC MOMENT CAPACITY
SP=ABS(PI/PY)
IF( SP .LE. 0.4 ) THEN
AA=0.06
BB=1
ELSE IF( SP .GT. 0.4 ) THEN
AA=0.15
BB=2
ENDIF
BETA=1.7*(SP/LOG(SP))-AA*TNAX*(SP)**BB
IF( BETA .LT. 1.18BETA+1.1
& TMPCKI=TNAX*(1-(PIC/PNI))*(1-(1.25*PIC/(PEX*(B/H)**(1./3))))
& IF(TMPCKI .LT. 0) TMPCKI=0.05*TMX
& IF(TMPCKI .LT. 0) TMPCKI=0.05*TMPY
& IF(ABS(TMTI) .GT. ABS(TMTJ)) THEN
& RMT=ABS(TMTI)
& ELSE IF(ABS(TMTI) .LE. ABS(TMTJ)) THEN
& RMT=ABS(TMTJ)
ENDIF
RMI=ABS(TMTI)
RMI=ABS(TMTJ)
VALUE=(RMI/TMPCKI)**BETA + (RMT/TMPCKI)**BETA
IF( VALUE .GE. 1 ) THEN
IAPAG=1
ENDIF
RETURN
END
C=PROCCSS SOUNP GOSTMT XREF
SUBROUTINE ITERG2(PI, TMKI, TMKJ, TMPX, PT, TMPY, TMPKJ,
& TMTI, TMTJ, TMPY, TMTI, TMTJ, IAPAG)
-----
THIS SUBROUTINE IS FOR JUDGING BOX SECTION STRENGTH CRITERIA
BASED ON W.P.C.'S PAPER
NOTE:
PI : CURRENT AXIAL LOAD
PT : AXIAL YIELDING LOAD CAPACITY
TMKI : CURRENT MOMENT IN X DIRECTION AT I END
TMKJ : CURRENT MOMENT IN X DIRECTION AT J END
TMTI : CURRENT MOMENT IN Y DIRECTION AT I END
TMTJ : CURRENT MOMENT IN Y DIRECTION AT J END
TMPX : FULLY PLASTIC MOMENT IN X DIRECTION
TMPY : FULLY PLASTIC MOMENT IN Y DIRECTION
TMPKI : CURRENT REDUCED PLASTIC MOMENT IN X DIRECTION AT I END
TMPKJ : CURRENT REDUCED PLASTIC MOMENT IN X DIRECTION AT J END
TMTPI : CURRENT REDUCED PLASTIC MOMENT IN Y DIRECTION AT I END
TMTPJ : CURRENT REDUCED PLASTIC MOMENT IN Y DIRECTION AT J END
IAPAG : FLAG FOR JUDGING FULLY PLASTIC SECTION
5: MEMBER TWO ENDS STILL ARE NOT FULLY PLASTIC
6: MEMBER A END IS IN FULLY PLASTIC
8: MEMBER A AND B ENDS ARE IN FULLY PLASTIC
-----
IF( PI .GE. 0 ) RETURN
SP=ABS(PI/PY)
IF( SP .EQ. 0 ) THEN
RETURN
ELSE IF( SP .LT. 1 .AND. SP .GT. 0 ) THEN
ELSE IF( SP .GE. 1 ) THEN
IAPAG=4
RETURN
ENDIF
END
TMPCKI=1.2*(1-SP)*TMPX
IF(ABS(TMPCKI).GT.ABS(TMPX)) TMPCKI=TMPX
TMPCKJ=TMPCKI
TMPCKI=1.2*(1-SP)*TMPY
IF(ABS(TMPCKI).GT.ABS(TMPY)) TMPCKI=TMPY
TMPCKJ=TMPCKI
RMKI=ABS(TMKI)
RMKJ=ABS(TMKJ)
RMTI=ABS(TMTI)
RMTJ=ABS(TMTJ)
IF( IAPAG .EQ. 5 ) THEN
IF( (RMKI/TMPCKI) .GE. 1 .OR. (RMTI/TMPCKI) .GE. 1 ) THEN
YIELD1=1
ELSE IF( (RMKJ/TMPCKJ) .LT. 1 .AND. (RMTJ/TMPCKJ) .LT. 1 ) THEN
YIELD1=(RMKI/TMPCKI)**A1 + (RMTI/TMPCKI)**A1
ENDIF
IF( (RMKJ/TMPCKJ) .GE. 1 .OR. (RMTJ/TMPCKJ) .GE. 1 ) THEN
YIELD2=1
ELSE IF( (RMKJ/TMPCKJ) .LT. 1 .AND. (RMTJ/TMPCKJ) .LT. 1 ) THEN
YIELD2=(RMKJ/TMPCKJ)**A1 + (RMTJ/TMPCKJ)**A1
ENDIF
ENDIF
IF( YIELD1 .GE. 1 .AND. YIELD2 .LT. 1 ) THEN
IAPAG=6
ELSE IF( YIELD1 .LT. 1 .AND. YIELD2 .GE. 1 ) THEN
IAPAG=7
ELSE IF( YIELD1 .GE. 1 .AND. YIELD2 .GE. 1 ) THEN
IAPAG=8
ENDIF
ENDIF
END
NLEN=MIN(LEN(NAME)+1,60)
NC=1
C= LOOP FOR EACH LOAD CASE ==
DO 300 L=1,NLOAD
NDSPL=0
IF( (.NOT. (NCOB.EQ.1 .AND. COSINE(1,1,1).EQ.1 .AND.
& SINE(2,2,1).EQ.1 .AND. COSINE(3,3,1).EQ.1)
& .AND. IOPT.EQ.2) ) GO TO 210
C=PRINT GLOBAL DISPLACEMENTS ==
DIMENSION IDP(MAXNO,6),CONST(MAXNO,5),JTFLG(MAXNO)
DIMENSION DISP(NDOF,NLOAD)
DIMENSION ID(MAXNO)
DIMENSION D(6),G(6),COSINE(3,3,NCOS),JTCOS(MAXNO)
CHARACTER(*) TITLE(2),STEPID,NAME
CHARACTER*15 C0(6)
CHARACTER*60 EQUAL
LOGICAL HEAD,FLAG,BTEST
INTEGER DCF
DATA EQUAL
& /
C=-----
NLEN=MIN(LEN(NAME)+1,60)
NC=1
C= LOOP FOR EACH LOAD CASE ==
DO 300 L=1,NLOAD
NDSPL=0
IF( (.NOT. (NCOB.EQ.1 .AND. COSINE(1,1,1).EQ.1 .AND.
& SINE(2,2,1).EQ.1 .AND. COSINE(3,3,1).EQ.1)
& .AND. IOPT.EQ.2) ) GO TO 210
C=PRINT GLOBAL DISPLACEMENTS ==
DIMENSION IDP(MAXNO,6),CONST(MAXNO,5),JTFLG(MAXNO)
DIMENSION DISP(NDOF,NLOAD)
DIMENSION ID(MAXNO)
DIMENSION D(6),G(6),COSINE(3,3,NCOS),JTCOS(MAXNO)
CHARACTER(*) TITLE(2),STEPID,NAME
CHARACTER*15 C0(6)
CHARACTER*60 EQUAL
LOGICAL HEAD,FLAG,BTEST
INTEGER DCF
DATA EQUAL
& /
C=-----
TRANSFER DISPL TO LOCAL VECTOR
DO 20 J=1,6
D(J)=DISP(I,J,L)
20
C=-----

```

```

C..... ADD ROTATIONS TO DISPL FOR CONSTRAINTS
C..... IF UNCONSTRAINED, CONST(I,J)=0
      D(1) = D(1) + D(5)*CONST(I,3) + D(6)*CONST(I,5)
      D(2) = D(2) + D(4)*CONST(I,3) + D(6)*CONST(I,6)
      D(3) = D(3) + D(4)*CONST(I,2) + D(5)*CONST(I,4)
C
C..... TRANSFER TO GLOBAL DISPLACEMENTS .....
      JCOS=JTCOS(I)
      DO 25 I=1,3
        G(I)=0
        G(I2)=0
        DO 25 J=1,3
          G(I1)=G(I1)+COSINE(J1,11,JCOS)*D(J1)
          G(I2)=G(I2)+COSINE(J1,11,JCOS)*D(J2)
25
C..... WRITE DISPL
      DO 30 J=1,6
        WRITE (CD(J),9) G(J)
30
      FORMAT (1P,G14.6,' ')
      WRITE (6,220) ID(I), (CD(K),K=1,6)
C
C..... IF (ISP1.EQ.2) THEN
      DO 399 K=1,6
        SGD(ID(I),K)=G(K)
      ENDIF
C
      NDSPLS=NDSPLS+1
100 CONTINUE
C
      IF (NDCOS.EQ.1 .AND. COSINE(1,1,1).EQ.1 .AND. COSINE(2,2,1).EQ.1
        .AND. COSINE(3,3,1).EQ.1 ) GO TO 300
C-----
C----- PRINT JOINT DISPLACEMENTS -----
C----- NOTE: DISP MAY CONTAIN DISPLACEMENTS, VELOCITIES, ACCELERATIONS OR
C----- EIGENVALUES...
C----- ALL OF WHICH ARE PRINTED OUT AT EACH NODE....
210 IF (HEAD.AND. NDSPLS.GT.20) WRITE (6,310) TITLE(1),TITLE(2)
      IF (IOP1.NE.2) WRITE (6,316) NAME,L,STEPID,EQUAL(1*INLEN)
      IF (IOP1.EQ.2) WRITE (6,416) NAME, EQUAL(1*INLEN)
      DO 200 I=1,NNODE
C..... TRANSFER GLOBAL DISPL TO LOCAL VECTOR .....
      DO 219 J=1,6
        I=IDOF(J)
        D(J)=DISP(I,J,L)
219
C..... ADD ROTATIONS TO DISPL FOR CONSTRAINTS
C..... IF UNCONSTRAINED, CONST(I,J)=0
      D(1) = D(1) + D(5)*CONST(I,3) + D(6)*CONST(I,5)
      D(2) = D(2) + D(4)*CONST(I,3) + D(6)*CONST(I,6)
      D(3) = D(3) + D(4)*CONST(I,2) + D(5)*CONST(I,4)
C
C..... WRITE DISPL
      DO 230 J=1,6
        WRITE (CD(J),9) D(J)
        I=IDOF(J)
        IF (I.GT.NP) CD(J)(15:15)='*'
230
      WRITE (6,221) ID(I), (CD(K),K=1,6),JTCOS(I)
C
      IF (ISP1.EQ.2) THEN
        DO 388 K=1,6
          SGD(ID(I),K)=D(K)
        ENDIF
C
200 CONTINUE
      WRITE (6,320)
C
300 CONTINUE
      RETURN
C
10 FORMAT (1P,G15.7)
220 FORMAT (5X,15,6A)
221 FORMAT (5X,15,6A,110)
310 FORMAT ('1 STRUCTURE.....1 'A/'
      & ' SOLUTION.....1 'A/'
      & ' ')
315 FORMAT ('1 'GCS 'A', ' LOADING #',15,5X,A /
      & ' 'A'/'
      & ' 6X, 'NODE',7X,'DX',13X,'DY',13X,'DZ',13X,'RX',13X,'RY',13X,'RZ'/'
      & ' ')
316 FORMAT ('1 'JCS 'A', ' LOADING #',15,5X,A /
      & ' 'A'/'
      & ' 6X, 'NODE',7X,'DX',13X,'DY',13X,'DZ',13X,'RX',13X,'RY',13X,'RZ',
      & ' 6X, 'COSINE #'/'
      & ' ')
415 FORMAT ('1 'GCS 'A'/'
      & ' 'A'/'
      & ' 25X, 'NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY'/'
      & ' 6X, 'NODE',7X,'DX',13X,'DY',13X,'DZ',13X,'RX',13X,'RY',13X,'RZ',
      & ' 6X, 'COSINE #'/'
      & ' ')
416 FORMAT ('1 'JCS 'A'/'
      & ' 'A'/'
      & ' 25X, 'NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY'/'
      & ' 6X, 'NODE',7X,'DX',13X,'DY',13X,'DZ',13X,'RX',13X,'RY',13X,'RZ',
      & ' 6X, 'COSINE #'/'
      & ' ')
320 FORMAT ('1X, 'NOTE: * DENOTES A RESTRAINED DEGREE OF FREEDOM')
C
      END
C-----
C----- SUBROUTINE JOILOA(BUG,MAXNOD,NDOF,NNODE,NLOAD,NP,LD,DOF,CONST,
C----- FORCE,DSPLG,JTFLG,TITLE,HEAD)
      DIMENSION ID(MAXNOD,6),CONST(MAXNOD,6),FORCE(NDOF,NLOAD)
      DIMENSION ID(MAXNOD),JTFLG(MAXNOD)
      CHARACTER*(*) TITLE(2)
      CHARACTER=4 DIR
      CHARACTER=1 CX(6)
      CHARACTER=25 SUPT
      CHARACTER=18 DOF
      CHARACTER=14 F(6)
      LOGICAL DSPLG,FIRST,BUG,HEAD
      LOGICAL BTEST
      LOAD=1
      FIRST=.TRUE.
      DSPLG=.FALSE.
5 FORMAT ('1 STRUCTURE.....1 'A/'
      & ' SOLUTION.....1 'A/'
      & ' APPLIED JOINT LOADS'/'
      & ' ')
C-----
C----- INPUT LOADING DATA ==
C-----
10 READ (5,*) LOAD,JOINT,JTGEN,JTINC,DIR,VALUE
      IF (INDEX(DIR,'END'),NE.0) GO TO 100
15 DOF=
      IF (FIRST) THEN
        IF (HEAD) WRITE (6,5) TITLE(1),TITLE(2)
        WRITE (6,6)
        FIRST=.FALSE.
      ENDIF
C----- CHECK FOR A VALID JOINT NUMBER...
      IF (JOINT.LT.ID(1) .OR. JOINT.GT.ID(NNODE) ) THEN

```



```

K=IDOF(I,J)
WRITE (P(1),240) FORCE(K,L)
IF (K.GT.NP) CX(J)=' '
ENDIF
CONTINUE
210 WRITE (6,230) ID(1),(F(J),CX(J),J=1,6)
CONTINUE
220 CONTINUE
200 FORMAT (' APPLIED JOINT LOADS, LOADING #',I5,10X,
' NOTES = DENOTES SUPPORT DISPLACEMENT' /
' *****' /
' 6X,'NODE','7X,'FX','13X,'FY','13X,'FZ','13X,'MX','13X,'MY','13X,'MZ') /
230 FORMAT (5X,15,12A)
240 FORMAT (1P,G14.6)
RETURN
END
C----- KMIN = CALCULATES STIFFNESS BASED ON DX AND DT -----
DEBUG UNIT(6),SUBCHKR,SUBTRACE
END DEBUG
KMIN =
DD = CHANGE IN DISPLACEMENT
DP = CHANGE IN LOAD
SI = MAXIMUM AND DEFAULT STIFFNESS
REAL FUNCTION KMIN(DD,DP,SI)
LOGICAL BTEST
IF ((DD+DP).GT. 0.00) THEN
KMIN=MIN((DP/DD),SI)
ELSE
KMIN=SI
ENDIF
RETURN
END
C-----
DEBUG UNIT(6),SUBCHKR,SUBTRACE,INIT
END DEBUG
SUBROUTINE LINACC(IOMP,BUG,DT,TO,TF,T,THETA,NEWKDY,
& L1,L2,L3,L4,L5,L6,IMDMS,IMD,IMDS,IMDKG,NDOP,NFREE,
& KGLoad,KFORM,
& A ,V ,D ,P
& DA ,DV ,DD ,DP ,WORK
& MASS ,MDMSS,DAMP ,STIFF ,MD
& KG ,MDKG ,ACC ,GRAV
& S ,MDS ,Q ,R ,PBAR
& FDAMP ,FMSS ,CO ,C1 ,C2 ,C3 ,C4 ,C5 ,C6 ,ALPHA ,BETA ,C10 ,C11 ,ABORT)
INTEGER FDAMP ,FMSS
REAL MASS ,KG
LOGICAL BUG ,NEWKDY ,BTEST ,ABORT
DIMENSION A(NDOP),V(NDOP),D(NDOP),DA(NDOP),DV(NDOP),DD(NDOP)
DIMENSION P(NDOP),DP(NDOP),DMP(1),IDUMT(1),WORK(2*NDOP)
DIMENSION PBAR(1:4),Q(1:4),R(1:4)
DIMENSION MDMSS(MDOP+1),MASS(IMDMS)
DIMENSION MDKG(NDOP+1),KG(MDKG)
DIMENSION MD(NDOP+1),STIFF(IMD)
DIMENSION MDS(NDOP+1),S(IMDS),DAMP(3)
GO TO (100,200,300) IOMP
C-----
C= INITIALIZE VALUES ==
C-----
100 CONTINUE
ALPHA=DAMP(1)
BETA =DAMP(2)
CO=3.*ALPHA/DT + 6./(DT**2)
C1=1.0/(1+3.*BETA/DT)
C2=CO*C1
C3=ALPHA-C2*BETA
C4=6./DT
C5=DT/2.
C6=0.
C10=3./DT
C11=6./(DT**2)
IF (BUG) WRITE (6,101) ALPHA,BETA,CO,C1,C2,C3,C4,C5,C6,C10,C11
101 FORMAT (1P,
& ' LINACC: ALPHA=',G12.5,' BETA=',G12.5,' CO=',G12.5,' C1=',G12.5,
& ' LINACC: C2=',G12.5,' C3=',G12.5,' C4=',G12.5,' C5=',G12.5,
& ' LINACC: C6=',G12.5,' C10=',G12.5,' C11=',G12.5)
C----- DEVELOP SKELINE OF DYNAMIC STIFFNESS (S) -----
MDI = ADDRESS OF DYNAMIC STIFF. MAIN DIAGONAL TERM ROW I
MDI(1) = ADDRESS OF STIFFNESS MAIN DIAGONAL TERM ROW I
MDMAS(1) = ADDRESS OF MASS MAIN DIAGONAL TERM ROW I
MDKG(1) = ADDRESS OF GEOMETRIC STIFF. MAIN DIAGONAL TERM ROW I
C----- L = # OF STIFFNESS TERMS IN COLUMN J
LMASS = # OF MASS TERMS IN COLUMN J
LKG = # OF KG TERMS IN COLUMN J
L = # OF S TERMS IN COLUMN J
MDS(1)=1
LKG=0
DO 110 J=2,NFREE+1
J=L+1
LSTIFF=MD(J)-MD(J-1)
LMASS =MDMSS(J)-MDMSS(J-1)
IF (KFORM.EQ.2) LKG = MDKG(J)-MDKG(J-1)
L=L+LSTIFF,LMASS,LKG)
C 110 MDS(J1)=MDS(J1-1)+L
C----- CHECK FOR PROPORTIONAL DAMPING -----
IF (FDAMP.NE.1) THEN
WRITE (6,*) *****
WRITE (6,*) = PROPORTIONAL DAMPING WAS NOT SPECIFIED *
WRITE (6,*) = SOLUTION IS ABORTED... *
WRITE (6,*) *****
STOP
ENDIF
RETURN
C----- SOLVE FOR INCREMENTAL DISPLACEMENTS, VELOCITIES AND ACCELERATIONS -----
200 CONTINUE
IF (BUG) THEN
CALL ERTRA
DO 205 J=L3,L4
205 WRITE (6,206) I,D(I),V(I),A(I),P(I),DP(I)
206 FORMAT (' DOP',I6,' DISP',1P,G12.4,' VEL',G12.4,
& ' ACC',G12.4,1,' LOAD', G12.4,' DP',G12.4)
ENDIF
C----- FORM DYNAMIC STIFFNESS -----
C J = COLUMN NUMBER FOR MASS AND STIFF
C I = ROW NUMBER FOR MASS AND STIFF
C J1 = COLUMN NUMBER FOR S
C I1 = ROW NUMBER FOR S
C JSTIFF = ADDRESS OF ELEMENT I,J IN MATRIX STIFF
C IJMSS = ADDRESS OF ELEMENT I,J IN MATRIX MASS
C IJKG = ADDRESS OF ELEMENT I,J IN GEOMETRIC STIFFNESS MATRIX
C IJS = ADDRESS OF ELEMENT I,J IN MATRIX S
C LSTIFF = ROW # CONTAINING THE FIRST NONZERO TERM IN COLUMN J OF STIFF
C LMASS = ROW # CONTAINING THE FIRST NONZERO TERM IN COLUMN J OF MASS
C LKG = ROW # CONTAINING THE FIRST NONZERO TERM IN COLUMN J OF KG
C LS = ROW # CONTAINING THE FIRST NONZERO TERM IN COLUMN J OF S
C----- FORM DYNAMIC STIFFNESS IF NEWKDY IS TRUE -----
IF (NEWKDY) THEN
C----- DETERMINE THE FACTOR FOR KG -----
IF (KGLoad.EQ.1) THEN

```

```

300 CONTINUE
IF (BUG) WRITE (6,320)
DO 310 I=1,NDOF
A(I)=DM(I)+A(I)
V(I)=DV(I)+V(I)
D(I)=DD(I)+D(I)
P(I)=DP(I)+P(I)
IF (BUG) WRITE (6,330) 1,DD(I),DV(I),DA(I),DP(I),
4 D(I),V(I),A(I),P(I)
310 DP(I)=0.00
320 FORMAT (' DOF',6X,'DD',12X,'DV',12X,'DA',12X,'DP',
3 12X,'D',12X,'V',12X,'A',12X,'P')
330 FORMAT (1X,16,1P,8G14.6)
END
RETURN
END
-----
C DEBUG UNIT(6),SUBCHK,SUBTRACE
END DEBUG
SUBROUTINE LMAX(A,MD,IC,IMD,NAME)
DIMENSION MD(1C+1),A(IMD)
LOGICAL BTEST
CHARACTER*(*) NAME
CHARACTER*20 NUMB,ROW(6)
LOGICAL PT,DMP
I(I,J)=MD(J)+J-1
CALL GETINT(NAME,'DUMP',IDUMP,0.,TRUE.,FALSE.)
IF (IDUMP.NE.0) THEN
DMP=.TRUE.
ELSE
DMP=.FALSE.
ENDIF
DO 1 I=1,6
ROW(I)=' '
IR=IC
WRITE (6,*) ' '
WRITE (6,*) 'PRINTING MATRIX: ',NAME
DO 50 ICG=1,IC,6
IF (ICG.GT.1) WRITE (6,*) ' '
K=NIN((ICG+5),IC)
WRITE (6,10) (J,J=ICG,K)
DO 20 I=1,IR
L=I
PT=.FALSE.
DO 15 J=ICG,K
13=J-I*(I,J)
IF (13.GE.MD(J+1).OR.1.GT.J) THEN
NUMB=' '
ELSE
PT=.TRUE.
WRITE (NUMB,16) A(I,J)
IF (DMP.AND.A(I,J).NE.0)
4 WRITE (IDUMP,*) I,J,A(I,J),NAME
ENDIF
L=L+1
15 IF (PT) WRITE (6,30) I,(ROW(L),L=1,6)
20 CONTINUE
RETURN
10 FORMAT('10,6('',1,' COLUMN',15,'-----'),')
16 FORMAT('20,6)
30 FORMAT(' ROW',15,6A)
END
-----
C DEBUG UNIT(6),SUBCHK,SUBTRACE
END DEBUG
SUBROUTINE LMAX(A,MD,IC,IMD,NAME)
DIMENSION MD(1C+1),A(IMD),B(100,100)
LOGICAL BTEST
CHARACTER*(*) NAME
CHARACTER*20 NUMB,ROW(6)
LOGICAL PT,DMP
DO 90 I=1,100
DO 90 J=1,100
B(I,J)=0
90 DO 50 I=1036,IC
II=I-1035
DO 50 J=1,IC
JJ=J-1035
IF (J.EQ.I) THEN
B(II,JJ)=A(MD(I))
ELSE IF (J.NE.I) THEN
IF (MD(J)+J-1).LT.MD(J+1) B(II,JJ)=A(MD(J)-J-1)
IF (MD(J)+J-1).GT.MD(J+1) B(II,JJ)=0
ENDIF
B(JJ,II)=B(II,JJ)
50 CONTINUE
WRITE(16,*) 'PRINTING MATRIX: ',NAME
DO 40 I=1,60
WRITE (16,30) (B(I,J),J=1,60)
40 CONTINUE
30 FORMAT('F20.6)
RETURN
END
-----
C DEBUG UNIT(6),SUBCHK,SUBTRACE
END DEBUG
SUBROUTINE MASSIN(INMASS,FMASS,MCOND,BUG,IBUG,TITLE,MAX3,MAXD3,
4 MAXNO,NCOS,NNODE,IS1W,IS1D,IS1S,DS,
6 IZMDS,IZMASS,IZLM,IZKE,IZMDAT,
6 NDOF,NMASS,IZCOS,IZIDOF,IZCNST,IZJPLG,IZJCOS,NAME,
6 NCOND,NFREZ,IZMD)
LOGICAL BUG,MCOND
INTEGER FMASS
CHARACTER*(*) NAME,TITLE(2)
DIMENSION Z(MAX3),N3(MAX3),DZ(MAXD3),MD(7)
LOGICAL BTEST
DOUBLE PRECISION DZ
DATA MD/1,2,4,7,11,16,22/
C IF (FMASS.EQ.1) THEN
C INITIALIZE THE DIAGONAL MASS MATRIX
MCOND=.TRUE.
IZMDS=12
N3=12+NDOF+1
CALL SKYLINE(0,1,NDOF,IBUG,TITLE,N3(IZMDS),
4 6,N3(IZLM),2(IZKE),MD,NAME)
C----- INPUT THE MASS DATA
IZMAT=12
I2=12+INMASS=12
IMD=N2(IZMDS+NDOF)
IZMASS=12
CALL MFORM(2,INMASS,NMASS,2(IZMASS),N3(IZMDS),MD,NDOF,IMD,TITLE,
4 NNODE,MAXNO,NCOS,BUG,IBUG,
6 N2(IZID),2(IZCOS),N2(IZIDOF),2(IZCNST),N2(IZJPLG),
6 N2(IZJCOS),2(IZMDAT),2(IZ),N2(IZ+21),2(IZ+27),
6 2(IZ+63),2(IZ+99))
C----- RELEASE UNUSED MASS DATA STORAGE
I2=12-(INMASS+NMASS)*12
C
C MODIFY MASS MATRIX FOR CONDENSATION ***
C-----
C THE CONDENSATION PROCESS MAY EXPAND THE MASS MATRIX'S

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4      Z(IC-1),ESEE,EPSE,DUCT,EXCR,DAMAGE)
ELSE IF (MATTP.EQ.14) THEN
  CALL MAT14
4      (IOPT,IELNO,ERR,FIRST,PRINT,BUG,
4      MAT,SE,P,PL,D,DL,V,VL,LELEM,LMAT,
4      Z(IC-1),ESEE,EPSE,DUCT,EXCR,DAMAGE)
ELSE
  WRITE(6,10) IELNO
  ERR=TRUE
10  FORMAT('ELEMENT','I6,' IS NOT AVAILABLE')
  ENDDIF
  LSTTYP=MATTP
C
C
C  WRITE(6,110) (I,N3(I),Z(I),I=68,91)
110  FORMAT (DP,' ZMATX(I,'I6,')',I15,IP,G20.10)
  MAT = MAT
  WRITE(6,*) 'MAT=',MAT,'MATTP=',MATTP
  RETURN
  END
C-----
C  DEBUG UNIT(6),SUBCHK,SUBTRACE
  END
C
C  SUBROUTINE MAT01
4      (IOPT,ISTOP,ERR,FIRST,PRINT,BUG,
4      MAT,SE,P,PL,D,DL,V,VL,LELEM,LMAT,
4      SMAT,ESEE,EPSE,DUCT,EXCR)
C
C  IMPLICIT REAL(A-H,O-3)
  LOGICAL ERR,FIRST,PRINT,BUG
  LOGICAL BTEST
  DIMENSION SE(100),SMAT(14),DUCT(3),EXCR(6)
  CHARACTER*80 TYPE
C
C  IF (IOPT.EQ.0) THEN
  LELEM=14
  RETURN
  ENDDIF
C-----
C  VARIABLES:
C-----
C----- GLOBAL VARIABLES
C  IOPT = 1, INITIALIZE MATERIAL
C  = 2, GET STIFFNESS
C  SMAT = INTERNAL STORAGE
C  SE = OUTPUT STIFFNESS
C-----
C  GO TO (100,200,300,400),IOPT
100  CONTINUE
C----- INPUT DATA
  E = SMAT(1)
  G = SMAT(2)
  AX = SMAT(3)
  AY = SMAT(4)
  AZ = SMAT(5)
  IX = SMAT(6)
  IY = SMAT(7)
  IS = SMAT(8)
  KII3 = SMAT(9)
  KJJ2 = SMAT(10)
  KIJ3 = SMAT(11)
  KJJT = SMAT(12)
  KIJT = SMAT(13)
  KIJT = SMAT(14)
C
  READ(5,*) TYPE,(SMAT(I),I=1,14)
  READ(5,*) TYPE,(SMAT(I),I=1,8)
  LELEM = 12
  LMAT = 14
C
  IF (FIRST.OR. BUG) WRITE(6,191)
  WRITE(6,192) MAT,(SMAT(K),K=1,14)
  WRITE(6,192) MAT,(SMAT(K),K=1,8)
191  FORMAT(' J=0 ELASTIC BEAM ELEMENT//
  & 4X,'MATL','T14','S','T20','GAMMA','T33','AX','T43','AT','T53','A3',
  & T63,'IX','T73','IY','T83','IZ'//)
191  FORMAT(' J=0 ELASTIC BEAM ELEMENT//
  & 4X,'MATL','T14','S','T20','GAMMA','T33','AX','T43','AT','T53','A3',
  & T63,'IX','T73','IY','T83','IZ','T100','K11','T110','K22','T120','K12'//)
192  FORMAT(4X,14,IP,8G10.3/)
192  FORMAT(4X,14,IP,8G10.3, ' Z-Axis ',3G10.3/
  & 4X,4X,1P,80X, ' Y-Axis ',3G10.3/)
C
  RETURN
C----- GET STIFFNESS TERMS -----
200  CONTINUE
  E = SMAT(1)
  G = SMAT(2)
  AX = SMAT(3)
  AY = SMAT(4)
  AZ = SMAT(5)
  RIX = SMAT(6)
  RIY = SMAT(7)
  RI3 = SMAT(8)
  RKI3 = SMAT(9)
  RKJJ = SMAT(10)
  RKIJ = SMAT(11)
  RKJT = SMAT(12)
  RKIY = SMAT(13)
  RKIJT = SMAT(14)
C
  SE(1)=E*AX
  SE(2)=G*AT
  SE(3)=G*AZ
  SE(4)=G*RIX
  SE(5)=E*RIY
  SE(6)=E*RI3
  SE(7)=RKI3
  SE(8)=RKJJ
  SE(9)=RKIJ
  SE(10)=RKIY
  SE(11)=RKJT
  SE(12)=RKIJT
  SE(13)=E*SE
  SE(14)=E*PE
C
  SE(13)=0
  SE(14)=(P+PL)/2*(D-DL)
C----- TRANSFER ENERGIES FOR BOTH IOPT=3 AND 4.
400  ESEE=SE(13)
  EPSE=SE(14)
C----- TRANSFER D & E FOR BOTH IOPT=3 AND 4.
  DO 410 I=1,3
  DUCT(I)=0
  EXCR(I)=0
410  EXCR(I-3)=0
C
  RETURN
  END
C  DEBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
  END
  END

```

```

SUBROUTINE MAT02
4      (IOPT,IELNO,ERR,FIRST,PRINT,BUG,
4      MAT,SE,P,PL,D,DL,V,VL,LELEM,LMAT,
4      SMAT,ESEE,EPSE,DUCT,EXCR,DAMAGE)
C
C  IMPLICIT REAL(A-H,O-3)
  LOGICAL ERR,FIRST,PRINT,BUG
  LOGICAL BTEST
  DIMENSION SE(100),SMAT(23),DUCT(3),EXCR(6)
  CHARACTER*80 TYPE
C
C----- LENGTH OF ELEMENT DATA REQUIRED...
  LELEM=33
  IF (IOPT.EQ.0) RETURN
C-----
C  VARIABLES:
C-----
C----- GLOBAL VARIABLES
C  IOPT = 1, INITIALIZE MATERIAL
C  = 2, GET STIFFNESS
C  RINPUT = INPUT DATA
C  SMAT = INTERNAL STORAGE
C  SE = OUTPUT STIFFNESS
C-----
C----- AXIUMOD DATA -----
  SMAT(1)=SC, COMPRESSION STIFFNESS
  SMAT(2)=ST1, INITIAL TENSILE STIFFNESS
  SMAT(3)=ST2, POST YIELD TENSILE STIFFNESS
  SMAT(4)=PT, YIELD POINT
  SMAT(5)=ALPHA, AXIUMOD PARAMETER
  SMAT(6)=BETA, AXIUMOD PARAMETER
  SMAT(7)=DTT, AXIUMOD PARAMETER
  SMAT(8)=DTG, AXIUMOD PARAMETER
  SMAT(9)=CSE, CONSTANT STRAIN ENERGY
  SMAT(10)=DMAX, ULTIMATE DISPLACEMENT
  SMAT(11)=BDAM, BETA FOR DAMAGE INDEX
C-----
C----- INPUT DATA FOR AXIUMOD HYSTERESIS MODEL
  THE FOLLOWING DATA IS INPUT ONCE FOR EACH MODEL
C
C  SC = COMPRESSION STIFFNESS
C  ST1 = INITIAL TENSILE STIFFNESS
C  ST2 = POST YIELD TENSILE STIFFNESS
C  PT = YIELD LOAD
C  ALPHA = MODEL COEFFICIENTS
C  BETA = MODEL COEFFICIENTS
C-----
C  GO TO (100,200,300,400,500),IOPT
100  CONTINUE
  READ(5,*) TYPE,(SMAT(I),I=1,6),DUCTMAX,BDAM
  LMAT = 11
  SMAT(7)=SMAT(4)/SMAT(2)
  SMAT(8)=SMAT(4)/SMAT(1)
  SMAT(9)=SMAT(4)*2/(C*SMAT(2))
  SMAT(10)=DUCTMAX*SMAT(7)
  SMAT(11)=BDAM
C
  IF (FIRST.OR. BUG) WRITE(6,192)
  WRITE(6,191) MAT,(SMAT(K),K=1,6),DUCTMAX,BDAM
191  FORMAT(1X,15,8G15.6/)
192  FORMAT('/// AXIUMOD HYSTERESIS MODEL- FOR UNIT LENGTH MEMBER'//
  & 'MATL','SE','SC','10X','ST1','10X','ST2','10X','PT',
  & '10X','ALPHA','10X','BETA',6X,'MAX DUCT','7X','BETA-DI')
C
  GO TO 9999
C----- GET STIFFNESS TERMS -----
200  CONTINUE
C----- AXIUMOD MODEL DATA STORAGE FOR MEMBER -----
  SE(1)=S
  SE(2)=HRN
  SE(3)=PMAX
  SE(4)=DMAX
  SE(5)=PT
  SE(6)=D
  SE(7)=FP
  SE(8)=DP
  SE(9)=KR
  SE(10)=KS
  SE(11)=RT
  SE(12)=NRL
  SE(13)=A
  SE(14)=S EQ UNLOAD - EQUIVALENT UNLOADING STIFFNESS
  SE(15)=UPOS(1)
  SE(16)=UPOS(2)
  SE(17)=UPOS(3)
  SE(18)=UNEG(1)
  SE(19)=UNEG(2)
  SE(20)=UNEG(3)
  SE(21)=EXCR(1)
  SE(22)=EXCR(2)
  SE(23)=EXCR(3)
  SE(24)=EXCR(4)
  SE(25)=EXCR(5)
  SE(26)=EXCR(6)
  SE(27)=ESE(1)
  SE(28)=ESE(2)
  SE(29)=ESE(3)
  SE(30)=PSE
  SE(31)=PSE_OLD
  SE(32)=POS_DMAX
  SE(33)=POS_PMAX
C
  DO 210 I=1,LELEM
  SE(I)=0
  CALL HYST02(P,D,PL,DL,V,VL,SE(1),SE(2),
  & SMAT(1),SMAT(2),SMAT(3),SMAT(4),SMAT(5),SMAT(6),SMAT(7),SMAT(8),
  & SMAT(9))
  & SE(3),SE(4),SE(5),SE(6),SE(7),SE(8),SE(9),SE(10),SE(11),SE(12),
  & SE(13),BUG, SE(14),SE(27),SE(30),SE(31),SE(15),SE(18),SE(21),
  & IDR)
  IF (BUG) WRITE(6,*) 'STIFF='STIFF,' A=',A
  GO TO 9999
C----- GET STIFFNESS TERMS -----
300  CONTINUE
  SE(1)=A
  SE(13)=A
  ICYC=0
  IF (BUG) THEN
  WRITE(6,*) '----- IN MAT02 ---- IELNO=',IELNO,
  & ' WRITE(6,*) '
  WRITE(6,*) 'A1',SE(13)
  WRITE(6,*) 'P,D',P,D
  WRITE(6,*) 'PL,DL',PL,DL, 'VL=',VL
  ENDDIF
  IDR=999
310  STIFF=SE(1)
  P=PL*(D-DL)*STIFF
  A=SE(13)
  IF (A.EQ.0.AND. SE(2).EQ.0) A=P
  ICYC=ICYC+1
  IF (BUG) THEN
  WRITE(6,*) 'STIFF='STIFF,' A=',A, ' IDR=',IDR

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WRITE (6,*) ' P',P ,',ICTC=',ICTC
ENDIF
C
C----- IS P WITHIN LIMITS ?
IF (ICTC.GT.5) THEN
WRITE (6,*) 'MORE THAN 5 CYCLES IN MAT02. IELNO=',IELNO
ELSE IF ((PL.LE.P.AND.P.LT.A).OR.(PL.GE.P.AND.P.GT.A).OR.
(DL.EQ.D)) THEN
CALL STRENG (SE(27),SE(30),SE(31),P,PL,D,DL,SE(1),SE(14))
IF (BUG) WRITE (6,*) 'C----- IS P WITH IN LIMITS ?'
CALL HYST02(P,D,PL,DL,V,VL,SE(1),SE(2))
SMAT(1),SMAT(2),SMAT(3),SMAT(4),SMAT(5),SMAT(6),
SMAT(7),SMAT(8),SMAT(9),SE(3),SE(4),SE(5),SE(6),SE(7),SE(8),
SE(9),SE(10),SE(11),SE(12),SE(13),BUG,
SE(14),SE(27),SE(30),SE(31),SE(15),SE(18),SE(21),IDR )
V=VL
C----- IS P GREATER THAN LIMITS ?
ELSE IF ((PL.LE.A.AND.A.LE.P).OR.(PL.GE.A.AND.A.GE.P)).AND.
(STIFF.NE.D.D)) THEN
DI=DL*(A-PL)/STIFF
PI=A
CALL STRENG (SE(27),SE(30),SE(31),PI,PL,DI,DL,SE(1),SE(14))
DL=DI
PL=PI
IF (ABS(P-PL).GT.0.005*ABS(P+PL)) THEN
PI=PL+(P-PL)*.01
DI=DL+(D-DL)*.01
ELSE
PI=P
DI=D
ENDIF
IF (BUG) THEN
WRITE (6,*) 'C----- IS P GREATER THAN LIMITS ?'
WRITE (6,*) 'PL,DL',PL,DL
WRITE (6,*) 'PI,DI',PI,DI
ENDIF
CALL HYST02(PI,DI,PL,DL,1.,1.,SE(1),SE(2),
SMAT(1),SMAT(2),SMAT(3),SMAT(4),SMAT(5),SMAT(6),
SMAT(7),SMAT(8),SMAT(9),SE(3),SE(4),SE(5),SE(6),SE(7),SE(8),
SE(9),SE(10),SE(11),SE(12),SE(13),BUG,
SE(14),SE(27),SE(30),SE(31),SE(15),SE(18),SE(21),IDR )
V=VL
GO TO 310
C----- IS P LESS THAN LIMITS ?
ELSE IF ((P.LT.PL.AND.PL.LE.A).OR.(P.GT.PL.AND.PL.GE.A)).AND.
(ABS(P-PL).GT.0.005*ABS(P+PL)) THEN
PI=PL+(P-PL)*.01
DI=DL+(D-DL)*.01
ELSE
PI=P
DI=D
ENDIF
IF (BUG) THEN
WRITE (6,*) 'C----- IS P LESS THAN LIMITS ?'
WRITE (6,*) 'PI,DI',PI,DI
ENDIF
CALL HYST02(PI,DI,PL,DL,1.,1.,SE(1),SE(2),
SMAT(1),SMAT(2),SMAT(3),SMAT(4),SMAT(5),SMAT(6),
SMAT(7),SMAT(8),SMAT(9),SE(3),SE(4),SE(5),SE(6),SE(7),SE(8),
SE(9),SE(10),SE(11),SE(12),SE(13),BUG,
SE(14),SE(27),SE(30),SE(31),SE(15),SE(18),SE(21),IDR )
V=VL
GO TO 310
ENDIF
C----- SAVE MAXIMUM PEAK POSITIVE DISPL
IF (D.GT.SE(32)) THEN
SE(32)=D
SE(33)=P
ENDIF
C
IF (BUG) WRITE (6,*) 'STIFF','STIFF',A',SE(13)
C----- TRANSFER ENERGIES FOR BOTH IOPT=J AND 4.
400 ESE=SE(27)
EPE=SE(30)
C----- TRANSFER DUCTILITIES AND EXTRUSION RATIOS FOR BOTH IOPT=J AND 4.
DO 410 I=1,3
DUCT(I)=MAX(SE(14+I),SE(17+I))
EXCR(I)=SE(20+I)
410 EXCR(I+3)=SE(20+I+3)
C
RETURN
C----- DAMAGE INDEX -----
500 CONTINUE
ESE=SE(27)
EPE=SE(30)
BDAM=SMAT(11)
D=SE(32)
P=SE(33)
DAMAGE=ABS(D)/SMAT(10) + BDAM / ( SMAT(4)*SMAT(10)) + EPESE
9999 CONTINUE
RETURN
END
C
C----- INPUT DATA FOR BENDI HYSTERESIS MODEL -----
C
C NSEG = NUMBER OF BACKBONE SEGMENTS
C NI = NUMBER OF SMALL LOOP POINTS
C
C----- BENDI DATA -----
SMAT(1)= NSEG, NUMBER OF BACKBONE SEGMENTS
SMAT(2)= NI, NUMBER OF SMALL LOOP POINTS
SMAT(3)= CSE, CONSTANT STRAIN ENERGY...
SMAT(4)= DT, YIELD POINT FOR DUCTILITIES
SMAT(5)= BDAM, BETA FOR DAMAGE INDEX
SMAT(6)= A, BACKBONE MOMENT FOR POINT I
SMAT(7)= NSEG, DP, BACKBONE ROTATION FOR POINT I
GO TO (100,200,300,400,500),IOPT
100 CONTINUE
C----- INPUT DATA FOR BENDI HYSTERESIS MODEL -----
C
C NSEG = NUMBER OF BACKBONE SEGMENTS
C NI = NUMBER OF SMALL LOOP POINTS

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WRITE (6,*) ' P=' ,P , 'ICYC=' ,ICYC
ENDIF
C
C----- IS P WITHIN LIMITS ?
IF (ICYC.GT.10) THEN
WRITE (6,*) 'MORE THAN 10 CYCLES IN MAT03. IELN0',IELN0
ELSE IF ((PL.LE.P.AND. P.LT.A) .OR. (PL.GE.P.AND. P.GT.A) .OR.
(DL.EQ.D)) THEN
IF (BUG) WRITE (6,*) 'C----- IS P WITH IN LIMITS ?'
CALL STRENG (SE(30),SE(33),SE(34),P,PL,D,DL,SE(1),SE(17))
CALL HYST03(P,D,PL,DL,DVLE,NSEGG,NI,
& SMAT(6),SMAT(J1),SMAT(6),SMAT(J1),SI,SMAT(3),
& SE(1),SE(2),SE(3),SE(4),SE(5),SE(6),
& SE(7),SE(8),SE(9),
& SE(10),SE(11),SE(12),SE(13),SE(14),SE(15),
& SE(16),SE(35),SE(J3),
& SE(24),BUG,
& SE(17),SMAT(4),SE(30),SE(33),SE(34),SE(18),SE(21),SE(24))
DVL=1
C----- IS P GREATER THAN LIMITS ?
ELSE IF (((PL.LE.A.AND. A.LE.P) .OR. (PL.GE.A.AND. A.GE.P)) .AND.
(STIFF.NE.0.00)) THEN
DI=DL*(A-PL)/STIFF
PI=A
CALL STRENG (SE(30),SE(33),SE(34),PI,PL,D1,DL,SE(1),SE(17))
DL=D1
PI=PI
PI=PL*(P-PL)*.001
DI=DL*(D-PL)*.001
IF (ABS(P-PL).GT. 0.0005*ABS(P+PL)) THEN
PI=PL*(P-PL)*.001
DI=DL*(D-PL)*.001
ELSE
PI=P
DI=D
ENDIF
IF (BUG) THEN
WRITE (6,*) 'C----- IS P GREATER THAN LIMITS ?'
WRITE (6,*) 'PL,DL',PL,DL
WRITE (6,*) 'PI,DI',PI,DI
ENDIF
CALL HYST03(PI,DI,PL,D1,.00 ,NSEGG,NI,
& SMAT(6),SMAT(J1),SMAT(6),SMAT(J1),SI,SMAT(3),
& SE(1),SE(2),SE(3),SE(4),SE(5),SE(6),
& SE(7),SE(8),SE(9),
& SE(10),SE(11),SE(12),SE(13),SE(14),SE(15),
& SE(16),SE(35),SE(J3),
& SE(24),BUG,
& SE(17),SMAT(4),SE(30),SE(33),SE(34),SE(18),SE(21),SE(24))
GO TO 310
C----- IS P LESS THAN LIMITS ?
ELSE IF ((P.LT.PL.AND. PL.LE.A) .OR. (P.GT.PL.AND. PL.GE.A)) THEN
DI=DL*(D-PL)*.001
IF (ABS(P-PL).GT. 0.0005*ABS(P+PL)) THEN
PI=PL*(P-PL)*.001
DI=DL*(D-PL)*.001
ELSE
PI=P
DI=D
ENDIF
IF (BUG) THEN
WRITE (6,*) 'C----- IS P LESS THAN LIMITS ?'
WRITE (6,*) 'PI,DI',PI,DI
ENDIF
CALL HYST03(PI,DI,PL,D1,.00 ,NSEGG,NI,
& SMAT(6),SMAT(J1),SMAT(6),SMAT(J1),SI,SMAT(3),
& SE(1),SE(2),SE(3),SE(4),SE(5),SE(6),
& SE(7),SE(8),SE(9),
& SE(10),SE(11),SE(12),SE(13),SE(14),SE(15),
& SE(16),SE(35),SE(J3),
& SE(24),BUG,
& SE(17),SMAT(4),SE(30),SE(33),SE(34),SE(18),SE(21),SE(24))
GO TO 310
ENDIF
C----- SAVE MAXIMUM DISPL AND IT'S LOAD
IF (ABS(D).GT. ABS(ES(36+3*NI))) THEN
ES(36+3*NI)=D
ES(36+3*NI)=D
ENDIF
C
IF (BUG) WRITE (6,*) 'STIFF',SE(1), ' A=',SE(4)
C----- TRANSFER ENERGIES FOR BOTH IOPT=3 AND 4.
400 ESSE=SE(30)
EPSE=SE(33)
NSEGG=SMAT(1)
NI=SMAT(2)
DMAX=SMAT(5+2*NSEGG)
DY=SMAT(4)
BDAM=SMAT(5)
P=SE(35+3*NI)*P
D=SE(36+3*NI)
DI=SMAT(6+NSEGG)
IF (DT.LE.D1) THEN
PT=SMAT(6)*DT/D1
ELSE
DO 510 I=2,NBEGG
P1=SMAT(4+I)
P2=SMAT(5+I)
D1=SMAT(4+I+NSEGG)
D2=SMAT(5+I+NSEGG)
IF (DT.GE.D1 .AND. DT.LE.D2) THEN
PT=P1*(DT-D1)*(P2-P1)/(D2-D1)
GO TO 520
ENDIF
CONTINUE
510 ENDF
520 DAMAGE=ABS(D)/DMAX + BDAM / (PT*DMAX) * EPSE
RETURN
END
C----- DEBU UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
END DEBU
C----- SUBROUTINE MAT004
& (IOPT,IELNO,ERR,FIRST,PRINT,BUG,
& MAT,SE,P,PL,D,DL,V,VL,LELEM,LMAT,
& SMAT,ESSE,EPSE,DUCT,EXCR,DAMAGE)
C
IMPLICIT REAL(A-H,O-Z)
LOGICAL ERR,FIRST,PRINT,BUG
DIMENSION SE(100),SMAT(20),DUCT(3),EXCR(6)
CHARACTER*80 TYPE
C
IF (IOPT.NE.1) LELEM=32+3*SMAT(2) +2

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DVEL=V*VL
SET VELOCITY FLAG TO 1 TO REMOVE INFL OF HIGH FREQ VELOC
DVEL=1
ICTC=0
IF (BUG) THEN
  WRITE (6,*) 'IN MAT04'
  WRITE (6,*) 'P,D',P,D
  WRITE (6,*) 'PL,DL',PL,DL
ENDIF
310 STIFF=SE(1)
P=PL*(D-DL)*STIFF
A=SE(4)
IF (A.EQ.0 .AND. SE(2).EQ.0) THEN
  A=SIGN(SMAT(5),P)
  IF (P.LT.PL .AND. PL.GT.0 .AND. P.GT.0) A=0
  IF (P.GT.PL .AND. PL.LT.0 .AND. P.LT.0) A=0
ENDIF
ICTC=ICTC+1
IF (BUG) THEN
  WRITE (6,*) 'STIFF',STIFF,' A',A
  WRITE (6,*) 'P',P,' D',D,' ITC',ICTC
ENDIF
C-----
IF ITC WITHIN LIMITS ?
  WRITE (6,*) 'MORE THAN 10 CYCLES IN MAT04, IELNO',IELNO
  ELSE IF (PL.LE.P .AND. P.LT.A) .OR. (PL.GE.P .AND. P.GT.A) .OR.
  (DL.EQ.D) THEN
    IF (BUG) WRITE (6,*) 'C----- IS P WITHIN LIMITS ?'
    CALL HYST04 (P, D, PL, DL, DVEL, NBSG, NI, BUG,
    & SMAT(5), SMAT(11), SMAT(3), SMAT(1), SMAT(3), SMAT(4),
    & SE(1), SE(2), SE(3), SE(4), SE(5), SE(6),
    & SE(7), SE(8), SE(9), SE(10), SE(11), SE(12),
    & SE(13), SE(14), SE(15), SE(16), SE(19), SE(22),
    & SE(28), SE(31), SE(32), SE(33), SE(33), SE(34))
  C-----
  IS P GREATER THAN LIMITS ?
  ELSE IF ((PL.LE.A .AND. A.LE.P) .OR. (PL.GE.A .AND. A.GE.P)) .AND.
  (STIFF.NE.0.001) THEN
    DI=DL+(A-PL)/STIFF
    PI=A
    CALL STRENG (SE(28), SE(31), SE(32), PI, PL, DI, DL, SE(15))
    DI=DI
    PI=PI
    DI=DI+(P-PL)*.001
    IF (ABS(P-PL).GT.0.0005*ABS(P+PL)) THEN
      DI=DI+(D-DL)*.001
    ELSE
      PI=P
      DI=D
    ENDIF
    IF (BUG) THEN
      WRITE (6,*) 'C----- IS P GREATER THAN LIMITS ?'
      WRITE (6,*) 'PI,DI',PI,DI
    ENDIF
    CALL HYST04 (PI, DI, PL, DL, 1.00, NBSG, NI, BUG,
    & SMAT(5), SMAT(11), SMAT(3), SMAT(1), SE(1), SE(2), SE(3), SMAT(4),
    & SE(1), SE(2), SE(3), SE(4), SE(5), SE(6),
    & SE(7), SE(8), SE(9), SE(10), SE(11), SE(12),
    & SE(13), SE(14), SE(15), SE(16), SE(19), SE(22),
    & SE(28), SE(31), SE(32), SE(33), SE(33), SE(34))
  GO TO 310
C-----
  IS P LESS THAN LIMITS ?
  ELSE IF ((P.LT.PL .AND. PL.LE.A) .OR. (P.GT.PL .AND. PL.GE.A)) THEN
    PI=PL+(P-PL)*.001
    DI=DL+(D-DL)*.001
    IF (ABS(P-PL).GT.0.0005*ABS(P+PL)) THEN
      DI=DL+(P-PL)*.001
    ELSE
      PI=P
      DI=D
    ENDIF
    IF (BUG) THEN
      WRITE (6,*) 'C----- IS P LESS THAN LIMITS ?'
      WRITE (6,*) 'PI,DI',PI,DI
    ENDIF
    CALL HYST04 (PI, DI, PL, DL, 1.00, NBSG, NI, BUG,
    & SMAT(5), SMAT(11), SMAT(3), SMAT(1), SE(1), SE(2), SE(3), SMAT(4),
    & SE(1), SE(2), SE(3), SE(4), SE(5), SE(6),
    & SE(7), SE(8), SE(9), SE(10), SE(11), SE(12),
    & SE(13), SE(14), SE(15), SE(16), SE(19), SE(22),
    & SE(28), SE(31), SE(32), SE(33), SE(33), SE(34))
  GO TO 310
ENDIF
IF (BUG) WRITE (6,*) 'STIFF',STIFF,' A',SE(4)
C-----
SAVE MAXIMUM DISPL AND LOAD
IF (ABS(D).GT.ABS(SE(34)*NI)) THEN
  SE(33)=3*NI*P
  SE(34)=3*NI*D
ENDIF
C-----
TRANSFER ENERGIES FOR BOTH IOPT=3 AND 4.
400 EBSB=SE(28)
EPSE=SE(31)
C-----
TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT=3 AND 4.
DO 410 I=1,
  DUCT(I)=MAX(SE(15)+1, SE(18)+1))
  EXCR(I)=SE(21+I)
410 EXCR(I+3)=SE(21+I+3)
  RETURN
C-----
DAMAGE INDEX -----
500 EBSB =SE(28)
  EPSE =SE(31)
  NBSG=SMAT(1)
  NI =SMAT(2)
  DMAX =SMAT(4+2*NBSG)
  DY =SMAT(4)
  BDAM =SMAT(5+2*NBSG)
  P=SE(33+ NI*P)
  D=SE(34+ NI*D)
  IF (DY.LE.SMAT(5+NBSG)) THEN
    PT=SMAT(5)*DY/SMAT(5+NBSG)
  ELSE
    DO 510 I=2, NBSG
      P1=SMAT(3+I)
      P2=SMAT(4+I)
      D1=SMAT(3+NBSG)
      D2=SMAT(4+NBSG)
      IF (DY.GE.D1 .AND. DT.LE.D2) THEN
        PT=P1+(DY-D1)*(P2-P1)/(D2-D1)
      GO TO 520
    ENDIF
    CONTINUE
  510
  ENDIF
  520 DAMAGE=ABS(D)/DMAX + BDAM / (PT*DMAX) * EPSE
  RETURN
C1111 FORMAT (1P,
  & ' S1A=',G13.5, ' D=',G13.5, ' K=',G18.9, ' RULE=',G13.5,
  & ' DIR=',F6.3, ' A=',G13.5, ' IR=',I3)
C1112 FORMAT (1P,
  & ' S1B=',G13.5, ' D=',G13.5, ' K=',G18.9, ' RULE=',G13.5,
  & ' DIR=',F6.3, ' A=',G13.5, ' IR=',I3)
C1113 FORMAT (1P,

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MAT01590 C 6' SIC= P',G13.5, ' D',G13.5, ' K',G18.9, ' RULE=',G13.5,
MAT01600 C 6' DIR=',F6.3, ' A',G13.5, ' IR=',I3)
MAT01610 C -----
MAT01620 C DEBBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
MAT01630 C -----
MAT01640 C END DEBBUG
MAT01650 C -----
MAT01660 C SUBROUTINE MAT06
MAT01670 C & IOPT ,IELNO ,ERR ,FIRST ,PRINT ,BUG,
MAT01680 C & MAT ,SE ,P,PL,D,DL,V,VL,LELEM,LMAT,
MAT01690 C & SMAT ,EBSB,EPSE,DUCT,EXCR,DAMAGE)
MAT01700 C -----
MAT01710 C IMPLICIT REAL(A-H,O=8)
MAT01720 C LOGICAL ERR,FIRST,PRINT,BUG
MAT01730 C LOGICAL TBEET
MAT01740 C DIMENSION SE(100),SMAT(13),DUCT(3),EXCR(8)
MAT01750 C -----
MAT01760 C LELEM=49
MAT01770 C IF (IOPT.EQ.0) RETURN
MAT01780 C -----
MAT01790 C -----
MAT01800 C VARIABLES:
MAT01810 C -----
MAT01820 C GLOBAL VARIABLES
MAT01830 C IOPT = 1, INITIALISE MATERIAL
MAT01840 C SE = GET STIFFNESS
MAT01850 C RINPUT = INPUT DATA
MAT01860 C SMAT = INTERNAL STORAGE
MAT01870 C SE = OUTPUT STIFFNESS
MAT01880 C -----
MAT01890 C TAKEDA DATA ----
MAT01900 C SMAT(1)=E1, ELASTIC STIFFNESS
MAT01910 C SMAT(2)=PC, CRACKING LOAD
MAT01920 C SMAT(3)=DC, CRACKING DISPLACEMENT
MAT01930 C SMAT(4)=PT, YIELD LOAD
MAT01940 C SMAT(5)=DY, YIELD DISPLACEMENT
MAT01950 C SMAT(6)=PU, ULTIMATE LOAD
MAT01960 C SMAT(7)=DU, ULTIMATE DISPLACEMENT
MAT01970 C SMAT(8)=OC, INITIAL STIFFNESS
MAT01980 C SMAT(9)=CY, CRACKED STIFFNESS
MAT01990 C SMAT(10)=TU, ULTIMATE STIFFNESS
MAT02000 C SMAT(11)=CT, REGADING STIFFNESS
MAT02010 C SMAT(12)=CSB, CONSTANT STRAIN ENERGY AT YIELD
MAT02020 C SMAT(13)=BDAM, BETA FOR DAMAGE INDEX
MAT02030 C -----
MAT02040 C INPUT DATA FOR TAKEDA HYSTERESIS MODEL
MAT02050 C THE FOLLOWING DATA IS INPUT ONCE FOR EACH MODEL
MAT02060 C -----
MAT02070 C EI = MOMENT OF INERTIA
MAT02080 C G = MOMENT GRADIENT
MAT02090 C PC,DC = CRACKING LOAD AND DISPLACEMENT
MAT02100 C PT,DY = YIELD LOAD AND DISPLACEMENT
MAT02110 C PU,DU = ULTIMATE LOAD AND DISPLACEMENT
MAT02120 C -----
MAT02130 C GO TO (100,200,300,400,500),IOPT
MAT02140 C -----
MAT02150 C 100 CONTINUE
MAT02160 C READ (5,*) TYPE ,SMAT(1),SMAT(12), (SMAT(1),I=2,7)
MAT02170 C READ (5,*) TYPE ,SMAT(1), (SMAT(1),I=2,7),BDAM
MAT02180 C -----
MAT02190 C LMAT = 13
MAT02200 C SMAT(8)=SMAT(2) / SMAT(3)
MAT02210 C SMAT(9)=(SMAT(4)-SMAT(2))/(SMAT(5)-SMAT(3))
MAT02220 C SMAT(10)=(SMAT(6)-SMAT(4))/(SMAT(7)-SMAT(5))
MAT02230 C SMAT(11)=(SMAT(2)+SMAT(4))/(SMAT(3)+SMAT(5))
MAT02240 C SMAT(1)=SMAT(1)
MAT02250 C SMAT(12)=(SMAT(2)*SMAT(3)/2 +.5*(SMAT(2)+SMAT(4))*(SMAT(5)-SMAT(3))
MAT02260 C SMAT(13)=BDAM
MAT02270 C -----
MAT02280 C IF (FIRST .OR. BUG) WRITE (6,5)
MAT02290 C WRITE (6,6) MAT,BDAM,SMAT(1), SMAT(2),SMAT(4),SMAT(5),
MAT02300 C SMAT(3),SMAT(5),SMAT(7)
MAT02310 C 5 FORMAT('//// TAKEDA HYSTERESIS MODEL- FOR UNIT LENGTH MEMBER-')
MAT02320 C & ' MAT, BETA EI'
MAT02330 C & 'X',CRACK',10X,'YIELD',2X,3G15.6, /39X,3G15.6)
MAT02340 C 6 FORMAT ('/X,15,G15.6,1X,2X,3G15.6, /39X,3G15.6)
MAT02350 C WRITE (6,6) MAT,SMAT(1),SMAT(12),SMAT(2),SMAT(4),SMAT(6),
MAT02360 C & SMAT(3),SMAT(5),SMAT(7)
MAT02370 C 5 FORMAT('//// TAKEDA HYSTERESIS MODEL- FOR UNIT LENGTH MEMBER-')
MAT02380 C & ' MAT, ELASTIC EI MOMT. GRADIENT',
MAT02390 C & 'X',CRACK',10X,'YIELD',10X,'ULTIMATE',///)
MAT02400 C 6 FORMAT ('X,15,G15.6,1X,G15.6,2X,3G15.6, /39X,3G15.6)
MAT02410 C -----
MAT02420 C RETURN
MAT02430 C -----
MAT02440 C GET STIFFNESS TERMS -----
MAT02450 C 200 CONTINUE
MAT02460 C SE(1)=B TAKEDA MODEL DATA STORAGE FOR MEMBER ----
MAT02470 C SE(2)=HRN
MAT02480 C SE(3)=A
MAT02490 C SE(4)=B
MAT02500 C SE(5)=IDRO
MAT02510 C SE(6)=RF
MAT02520 C SE(7)=RNRN
MAT02530 C SE(8)=UM1
MAT02540 C SE(9)=UM2
MAT02550 C SE(10)=UM3
MAT02560 C SE(11)=UM4
MAT02570 C SE(12)=S1
MAT02580 C SE(13)=S0
MAT02590 C SE(14)=X0UM
MAT02600 C SE(15)=X0Y
MAT02610 C SE(16)=U0D
MAT02620 C SE(17)=U0
MAT02630 C SE(18)=X1UM
MAT02640 C SE(19)=U1
MAT02650 C SE(20)=U1D
MAT02660 C SE(21)=X2U0
MAT02670 C SE(22)=U2
MAT02680 C SE(23)=X3U1
MAT02690 C SE(24)=U3
MAT02700 C SE(25)=DY
MAT02710 C SE(26)=HARFEG
MAT02720 C SE(27)=ACHNG
MAT02730 C SE(28)=E_Q_UNLOAD
MAT02740 C SE(29)=EBSB
MAT02750 C SE(30)=EPSE
MAT02760 C SE(31)=EBSB
MAT02770 C SE(32)=EPSE
MAT02780 C SE(33)=PSE_OLD
MAT02790 C SE(34)=UPOS(1)
MAT02800 C SE(35)=UPOS(2)
MAT02810 C SE(36)=UPOS(3)
MAT02820 C SE(37)=UNEG(1)
MAT02830 C SE(38)=UNEG(2)
MAT02840 C SE(39)=UNEG(3)
MAT02850 C SE(40)=EXCR(1)
MAT02860 C SE(41)=EXCR(2)
MAT02870 C SE(42)=EXCR(3)
MAT02880 C SE(43)=EXCR(4)
MAT02890 C SE(44)=EXCR(5)
MAT02900 C SE(45)=EXCR(6)
MAT02910 C SE(46)=P_MAX
MAT02920 C SE(47)=D_MAX
MAT02930 C SE(48)=EXCR(8)
MAT02940 C SE(49)=SMAT(1)
MAT02950 C -----
MAT02960 C EI=SMAT(1)
MAT02970 C GRAD=SMAT(12)

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DO 210 I=1,47
210 SE(I)=0
CALL HYST06(P,D,PL,DL,V,VL,
& SE(1),SE(2),SE(3),SE(4),SE(5),SE(6),
& SMAT(2),SMAT(3),SMAT(4),SMAT(5),SMAT(6),SMAT(7),SMAT(8),
& SMAT(9),SMAT(10),SMAT(11),SMAT(12),
& SE(7),SE(8),SE(9),SE(10),SE(11),SE(12),SE(13),SE(14),SE(15),
& SE(16),SE(17),SE(18),SE(19),SE(20),SE(21),SE(22),SE(23),SE(24),
& SE(25),SE(26),SE(27),BUG,
& SE(28),SE(29),SE(32),SE(33),SE(34),SE(37),SE(40))
C
SE(48)=SMAT(8)
SE(49)=EI
C
RETURN
C----- GET STIFFNESS TERMS -----
300 CONTINUE
SE(1)=STIFF
SE(2)=A
SE(28)=S.EQ.UNLOAD
ICTC=0
IF (BUG) THEN
WRITE (6,*) '----- IN MAT06 -----'
WRITE (6,*) 'P,D',P,D,' V',V
WRITE (6,*) 'PL,DL',PL,DL,' VL',VL
ENDIF
310 STIFF=SE(1)
P=PL+(D-DL)*STIFF
A=SE(27)
IF (A.EQ.0 .AND. SE(7).EQ.0) A=SMAT(2)
ICTC=ICTC+1
IF (BUG) THEN
WRITE (6,*) 'STIFF=',STIFF,' A=',A
WRITE (6,*) 'P=',P,' ICTC=',ICTC
ENDIF
C----- IS P WITHIN LIMITS ?
IF (ICTC.GT.5) THEN
WRITE (6,*) 'MORE THAN 5 CYCLES IN MAT06'
CALL STRENG(P,D,PL,DL,V,VL,
& SE(1),SE(2),SE(3),SE(4),SE(5),SE(6),
& SMAT(2),SMAT(3),SMAT(4),SMAT(5),SMAT(6),
& SMAT(7),SMAT(8),SMAT(9),SMAT(10),SMAT(11),SMAT(12),
& SE(7),SE(8),SE(9),SE(10),SE(11),SE(12),SE(13),
& SE(14),SE(15),SE(16),SE(17),SE(18),SE(19),SE(20),
& SE(21),SE(22),SE(23),SE(24),SE(25),SE(26),SE(27),BUG,
& SE(28),SE(29),SE(32),SE(33),SE(34),SE(37),SE(40))
V=VL
C----- IS P GREATER THAN LIMITS ?
ELSE IF ((PL.LE.A .AND. A.LT.P) .OR. (PL.GE.A .AND. A.GE.P)) .AND.
& (STIFF.NE.0.00) THEN
DI=DL+(A-PL)/STIFF
PI=A
CALL STRENG(SE(29),SE(32),SE(33),PI,PL,DI,DL,SE(1),SE(28))
IF (BUG) THEN
WRITE (6,*) 'C----- IS P GREATER THAN LIMITS ?'
WRITE (6,*) 'PL,DL',PL,DL
WRITE (6,*) 'PI,DI',PI,DI
ENDIF
CALL HYST06(PI,DI,PL,DL,VL,
& SE(1),SE(2),SE(3),SE(4),SE(5),SE(6),
& SMAT(2),SMAT(3),SMAT(4),SMAT(5),SMAT(6),
& SMAT(7),SMAT(8),SMAT(9),SMAT(10),SMAT(11),SMAT(12),
& SE(7),SE(8),SE(9),SE(10),SE(11),SE(12),SE(13),
& SE(14),SE(15),SE(16),SE(17),SE(18),SE(19),SE(20),
& SE(21),SE(22),SE(23),SE(24),SE(25),SE(26),SE(27),BUG,
& SE(28),SE(29),SE(32),SE(33),SE(34),SE(37),SE(40))
DL=DI
PL=PI
C----- IS P LESS THAN LIMITS ?
ELSE IF ((P.LT.PL .AND. PL.LE.A) .OR. (P.GT.PL .AND. PL.GE.A)) .AND.
& (ABS(D-PL).GT.0.0001*ABS(P+PL)) THEN
PI=PL+(P-PL)*.001
DI=DL+(D-PL)*.001
ELSE
PI=P
DI=D
ENDIF
PI=PL+(P-PL)*.001
DI=DL+(D-PL)*.001
IF (BUG) THEN
WRITE (6,*) 'C----- IS P LESS THAN LIMITS ?'
WRITE (6,*) 'PI,DI',PI,DI
ENDIF
CALL HYST06(PI,DI,PL,DL,VL,
& SE(1),SE(2),SE(3),SE(4),SE(5),SE(6),
& SMAT(2),SMAT(3),SMAT(4),SMAT(5),SMAT(6),
& SMAT(7),SMAT(8),SMAT(9),SMAT(10),SMAT(11),SMAT(12),
& SE(7),SE(8),SE(9),SE(10),SE(11),SE(12),SE(13),
& SE(14),SE(15),SE(16),SE(17),SE(18),SE(19),SE(20),
& SE(21),SE(22),SE(23),SE(24),SE(25),SE(26),SE(27),BUG,
& SE(28),SE(29),SE(32),SE(33),SE(34),SE(37),SE(40))
GO TO 310
ENDIF
IF (BUG) WRITE (6,*) 'STIFF=',STIFF,' A.B=',SE(3),SE(4),SE(27)
C----- MAXIMUM DISPLACEMENT
IF (ABS(D).GT.ABS(SE(47))) THEN
SE(46)=P
SE(47)=D
ENDIF
C----- TRANSFER ENERGIES FOR BOTH IOPT=3 AND 4.
400 EPSE=SE(29)
EPSE=SE(32)
C----- TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT=3 AND 4
DO 410 I=1,3
DUCT(I)=MAX(SE(33+I),SE(36+I))
EXCR(I)=SE(39+I)
410 EXCR(I+3)=SE(42+I)
RETURN
C----- DAMAGE INDEX -----
500 EBSE=SE(29)

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MAT01400 EPSE=SE(32)
MAT01410 PT = SMAT(4)
MAT01420 PU = SMAT(6)
MAT01430 DU = SMAT(7)
MAT01440 P = SE(46)
MAT01450 D = SE(47)
MAT01460 BDAM=SMAT(13)
MAT01470 DAMAGE=ABS(D)/DU + BDAM/(PT*DU) * EPSE
C
MAT01480 RETURN
MAT01490 END
MAT01500 C-----
MAT01510 C DEBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
MAT01520 C END DEBUG
MAT01530 MAT00010
MAT01540 MAT00020
MAT01550 MAT00030
MAT01560 MAT00040
MAT01570 MAT00050
MAT01580 MAT00060
MAT01590 MAT00070
MAT01600 MAT00080
MAT01610 MAT00090
MAT01620 MAT00100
MAT01630 MAT00110
MAT01640 MAT00120
MAT01650 MAT00130
MAT01660 MAT00140
MAT01670 MAT00150
MAT01680 MAT00160
MAT01690 MAT00170
MAT01700 MAT00180
MAT01710 MAT00190
MAT01720 MAT00200
MAT01730 MAT00210
MAT01740 MAT00220
MAT01750 MAT00230
MAT01760 MAT00240
MAT01770 MAT00250
MAT01780 MAT00260
MAT01790 MAT00270
MAT01800 MAT00280
MAT01810 MAT00290
MAT01820 MAT00300
MAT01830 MAT00310
MAT01840 MAT00320
MAT01850 MAT00330
MAT01860 MAT00340
MAT01870 MAT00350
MAT01880 MAT00360
MAT01890 MAT00370
MAT01900 MAT00380
MAT01910 MAT00390
MAT01920 MAT00400
MAT01930 MAT00410
MAT01940 MAT00420
MAT01950 MAT00430
MAT01960 MAT00440
MAT01970 MAT00450
MAT01980 MAT00460
MAT01990 MAT00470
MAT02000 MAT00480
MAT02010 MAT00490
MAT02020 MAT00500
MAT02030 MAT00510
MAT02040 MAT00520
MAT02050 MAT00530
MAT02060 MAT00540
MAT02070 MAT00550
MAT02080 MAT00560
MAT02090 MAT00570
MAT02100 MAT00580
MAT02110 MAT00590
MAT02120 MAT00600
MAT02130 MAT00610
MAT02140 MAT00620
MAT02150 MAT00630
MAT02160 MAT00640
MAT02170 MAT00650
MAT02180 MAT00660
MAT02190 MAT00670
MAT02200 MAT00680
MAT02210 MAT00690
MAT02220 MAT00700
MAT02230 MAT00710
MAT02240 MAT00720
MAT02250 MAT00730
MAT02260 MAT00740
MAT02270 MAT00750
MAT02280 MAT00760
MAT02290 MAT00770
MAT02300 MAT00780
MAT02310 MAT00790
MAT02320 MAT00800
MAT02330 MAT00810
MAT02340 MAT00820
MAT02350 MAT00830
MAT02360 MAT00840
MAT02370 MAT00850
MAT02380 MAT00860
MAT02390 MAT00870
MAT02400 MAT00880
MAT02410 MAT00890
MAT02420 MAT00900
MAT02430 MAT00910
MAT02440 MAT00920
MAT02450 MAT00930
MAT02460 MAT00940
MAT02470 MAT00950
MAT02480 MAT00960
MAT02490 MAT00970
MAT02500 MAT00980
MAT02510 MAT00990
MAT02520 MAT01000
MAT02530 MAT01010
MAT02540 MAT01020
MAT02550 MAT01030
MAT02560 MAT01040
MAT02570 MAT01050
MAT02580 MAT01060
MAT02590 MAT01070
MAT02600 MAT01080
MAT02610 MAT01090
MAT02620 MAT01100
MAT02630 MAT01110
MAT02640 MAT01120
MAT02650 MAT01130
MAT02660 MAT01140
MAT02670 MAT01150
MAT02680 MAT01160
MAT02690 MAT01170
MAT02700 MAT01180
MAT02710 MAT01190
MAT02720 MAT01200
MAT02730 MAT01210
MAT02740 MAT01220
MAT02750 MAT01230
MAT02760 MAT01240
MAT02770 MAT01250
MAT02780 MAT01260
MAT02790 MAT01270
MAT02800 MAT01280
MAT02810 MAT01290

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C ----- DAMAGE INDEX ----- MAT01320      4 SE(8),SE(9),SE(10),SE(11),BUG, MAT01290
C ----- MAT01330      4 SE(12),SE(13),SE(16),SE(22),SE(23),SE(26),SE(27),SE(28) ) MAT01300
500 BESE=SE(6) MAT01340      SE(28)=SE(22)*SMAT(5)/2 MAT01310
EPSE=SE(9) MAT01350      4 SMAT(1),SMAT(2),SMAT(3),SMAT(4),SMAT(5),SMAT(9), MAT01320
PT = SMAT( 2) MAT01360      SE(29)=SE(22)*SMAT(7) MAT01330
DT = SMAT( 4) MAT01370      SE(30)=0 MAT01340
DMAX = SMAT( 6) MAT01380      SE(31)=0 MAT01350
BDAM = SMAT( 7) MAT01390      RETURN MAT01360
P = SE(23) MAT01400      300 CONTINUE MAT01370
D = SE(24) MAT01410      C ----- CALL HYST07 TO CALC NEW STIFFNESS STIF MAT01380
DAMAGE=ABS(D)/DMAX + BDAM / (PT*DMAX) * EPSE MAT01420      CALL HYST08(P,PL,D,DL,V,VL,SE(5),SE(2), MAT01400
C RETURN MAT01430      4 SMAT(6),SE(3),SE(4),SE(1),SE(6),SE(7), MAT01410
C ----- GET STIFFNESS TERMS ----- MAT01420
C ----- CALL HYST07 TO CALC NEW STIFFNESS STIF MAT01390
C ----- CALL HYST08(P,PL,D,DL,V,VL,SE(5),SE(2), MAT01410
4 SMAT(1),SMAT(2),SMAT(3),SMAT(4),SMAT(5),SMAT(9), MAT01420
4 SE(32),SE(33),SE(34),SE(35),SE(36),SE(37),SE(38),SE(39), MAT01430
4 SE(8),SE(9),SE(10),SE(11),BUG, MAT01440
4 SE(12),SE(13),SE(16),SE(22),SE(23),SE(26),SE(27),SE(28) ) MAT01450
C --- STORE MAXIMUM DISPLACEMENT AND CORRESPONDING LOAD MAT01460
IF(ABS(D)-GT. ABS(SE(30))) THEN MAT01480
SE(30)=D MAT01490
SE(31)=P MAT01500
C ENDFI MAT01510
C ----- GET ENERGIES, DUCTILITY AND EXCURSION FOR BOTH IOPT=3 AND 4 ----- MAT01520
C 400 CONTINUE MAT01530
C --- TRANSFER ENERGIES FOR BOTH IOPT=3 AND 4. MAT01540
BESE=SE(23) MAT01550
EPSE=SE(26) MAT01560
C --- TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT=3 AND 4. MAT01570
DO 410 I=1,3 MAT01580
DUCT(I)=SE(12+I) MAT01590
EXCR(I+3)=SE(15+I+3) MAT01600
410 EXCR(I)=SE(15+I) MAT01610
C RETURN MAT01620
C ----- DAMAGE INDEX ----- MAT01630
500 CONTINUE MAT01640
BESE=SE(23) MAT01650
EPSE=SE(26) MAT01660
DMAX=SE(29) MAT01670
D=SE(30) MAT01700
P=SE(31) MAT01710
PT=SMAT(5) MAT01720
BDAM=SMAT(8) MAT01730
DAMAGE=ABS(D)/DMAX + BDAM / (PT*DMAX) * EPSE MAT01740
C RETURN MAT01750
C ----- INTERPERTATE INPUT DATA ----- MAT01760
C B = SMAT( 1) MAT01770
C A = SMAT( 2) MAT01780
C R = SMAT( 3) MAT01790
C TS = SMAT( 4) MAT01800
C PT = SMAT( 5) MAT01810
C CC = SMAT( 6) MAT01820
C DMAX = SMAT( 7) : ULTIMATE DISPLACEMENT = DUCMAX*DELT MAT01830
C BDAM = SMAT( 8) : BATA PARAMETER MAT01840
C SHAPE = SMAT( 9) : CROSS SECTION SHAPE MAT01850
1: FOR BOX OR ANGLE SECTION MAT01860
2: FOR I SHAPE SECTION MAT01870
3: FOR BOX SECTION CONSIDERING MAT01880
LOCAL BUCKLING. THIS IS ONLY FOR MEMBER MAT01890
WITH KL/R LESS THAN 40 CASE MAT01900
C ----- MAT01910
C READ (5,*) TYPE ,(SMAT(I),I=1,4), SHAPE, DUCMAX, BDAM MAT01920
C ----- MAT01930
C LMAT = 9 MAT01940
C PI=3.1415926 MAT01950
C SMAT(5)=SMAT(2)*SMAT(4) MAT01960
C SMAT(6)=SQRT(2-PI*PI*SMAT(1)/SMAT(4) ) MAT01970
C SMAT(7)=DUCMAX MAT01980
C SMAT(8)=BDAM MAT01990
C SMAT(9)=SHAPE MAT02000
C ----- MAT02010
C IF (FIRST .OR. BUG) WRITE (6,191) MAT02020
C WRITE (6,192) MAT,(SMAT(K),K=1,4),SHAPE,DUCMAX,BDAM MAT02030
C ----- MAT02040
191 FORMAT (/// GOEL HYSTERESIS MODEL PARAMETERS'// MAT02050
& ' MAT,' ,3X, ' B',6X, ' A',6X, ' R',6X, ' TS', MAT02060
& ' 6X, ' SHAPE',6X, ' MAX DUC', 6X, ' BETA' )'// MAT02070
192 FORMAT (1X,15,7G12.6/) MAT02080
C RETURN MAT02090
C ----- INITIALISE STIFFNESS TERMS ----- MAT02100
200 CONTINUE MAT02110
C ----- GOEL HYSTERESIS MODEL DATA STORAGE FOR MEMBER ----- MAT02120
C SE( 1) = STIFMEMBER AXIAL STIFFNESS MAT02130
C SE( 2) = K EFFECT SLENDER COEFFICIENT (TRANSFERED FROM ELE08) MAT02140
C SE( 3) = NOCT NO. OF CYCLE MAT02150
C SE( 4) = NRULE RULE NUMBER MAT02160
C SE( 5) = EL MEMBER LENGTH (TRANSFERED FROM ELE08) MAT02170
C SE( 6) = ITRV 0:REVRSC=.FALSE. 1:REVRSC=.TRUE. MAT02180
C SE( 7) = LDUC 0: HC=-12 1: HC .LT. -12 MAT02190
C SE( 8) = PBO7 MAT02200
C SE( 9) = DBO7 MAT02210
C SE(10) = PTOP MAT02220
C SE(11) = DTOP MAT02230
C SE(12) = REQ UNLOADING EQUIVALENT STIFFNESS MAT02240
C SE(13) = DUCT(1) MAT02250
C SE(14) = DUCT(2) MAT02260
C SE(15) = DUCT(3) MAT02270
C SE(16) = EXCR(1) MAT02280
C SE(17) = EXCR(2) MAT02290
C SE(18) = EXCR(3) MAT02300
C SE(19) = EXCR(4) MAT02310
C SE(20) = EXCR(5) MAT02320
C SE(21) = EXCR(6) MAT02330
C SE(22) = DELT YIELDING DISPLACEMENT MAT02340
C SE(23) = BESE(1) MAT02350
C SE(24) = BESE(2) MAT02360
C SE(25) = BESE(2) MAT02370
C SE(26) = EPSE MAT02380
C SE(27) = PSBOLD MAT02390
C SE(28) = CSE MAT02400
C SE(29) = DMAX ULTIMATE DISPLACEMENT MAT02410
C SE(30) = D_MAX MAXIMUM ELEMENT DISPLACEMENT FOR DAMAGE INDEX MAT02420
C SE(31) = P_MAX ELEMENT LOAD CORRESPONDING TO D_MAX MAT02430
C SE(32) = HA CURRENT CONTROL POINT HA MAT02440
C SE(33) = VA CURRENT CONTROL POINT VA MAT02450
C SE(34) = HC CURRENT CONTROL POINT HC MAT02460
C SE(35) = VC CURRENT CONTROL POINT VC MAT02470
C SE(36) = RSN CURRENT RESIDUAL STRAIN VALUE MAT02480
C SE(37) = DEMAX CURRENT MAXIMUM AXIAL DISPLACEMENT FOR DUCTILITIES MAT02490
C SE(38) = REGIN CURRENT REGINE ZONE NUMBER MAT02500
C SE(39) = PREGIN PREVIOUS REGIN ZONE NUMBER MAT02510
C ----- MAT02520
C SE(1)=1 MAT02530
C SE(2)=1 MAT02540
C SE(4)=1 MAT02550
C DO 210 I=6,28 MAT02560
210 SE(I)=0 MAT02570
C SE(36)=1 MAT02580
C SE(39)=1 MAT02590
C ----- MAT02600
C CALL HYST08(0.,0.,0.,0.,V,VL,SE(5),SE(2), MAT02610
& SMAT(1),SMAT(2),SMAT(3),SMAT(4),SMAT(5),SMAT(9), MAT02620
& SMAT(6),SE(3),SE(4),SE(1),SE(6),SE(7), MAT02630
& SE(32),SE(33),SE(34),SE(35),SE(36),SE(37),SE(38),SE(39), MAT02640
C ----- MAT02650
4 SE(8),SE(9),SE(10),SE(11),BUG, MAT02660
4 SE(12),SE(13),SE(16),SE(22),SE(23),SE(26),SE(27),SE(28) ) MAT02670
C RETURN MAT02680
C ----- GET STIFFNESS TERMS ----- MAT02690
C ----- CALL HYST07 TO CALC NEW STIFFNESS STIF MAT02700
C ----- CALL HYST08(P,PL,D,DL,V,VL,SE(5),SE(2), MAT02710
4 SMAT(1),SMAT(2),SMAT(3),SMAT(4),SMAT(5),SMAT(9), MAT02720
4 SE(32),SE(33),SE(34),SE(35),SE(36),SE(37),SE(38),SE(39), MAT02730
4 SE(8),SE(9),SE(10),SE(11),BUG, MAT02740
4 SE(12),SE(13),SE(16),SE(22),SE(23),SE(26),SE(27),SE(28) ) MAT02750
C --- STORE MAXIMUM DISPLACEMENT AND CORRESPONDING LOAD MAT02760
IF(ABS(D)-GT. ABS(SE(30))) THEN MAT02770
SE(30)=D MAT02780
SE(31)=P MAT02790
C ENDFI MAT02800
C ----- GET ENERGIES, DUCTILITY AND EXCURSION FOR BOTH IOPT=3 AND 4 ----- MAT02810
C 400 CONTINUE MAT02820
C --- TRANSFER ENERGIES FOR BOTH IOPT=3 AND 4. MAT02830
BESE=SE(23) MAT02840
EPSE=SE(26) MAT02850
C --- TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT=3 AND 4. MAT02860
DO 410 I=1,3 MAT02870
DUCT(I)=SE(12+I) MAT02880
EXCR(I+3)=SE(15+I+3) MAT02890
410 EXCR(I)=SE(15+I) MAT02900
C RETURN MAT02910
C ----- DAMAGE INDEX ----- MAT02920
500 CONTINUE MAT02930
BESE=SE(23) MAT02940
EPSE=SE(26) MAT02950
DMAX=SE(29) MAT02960
D=SE(30) MAT02970
P=SE(31) MAT02980
PT=SMAT(5) MAT02990
BDAM=SMAT(8) MAT03000
DAMAGE=ABS(D)/DMAX + BDAM / (PT*DMAX) * EPSE MAT03010
C RETURN MAT03020
C ----- INTERPERTATE INPUT DATA ----- MAT03030
C B = SMAT( 1) MAT03040
C A = SMAT( 2) MAT03050
C R = SMAT( 3) MAT03060
C TS = SMAT( 4) MAT03070
C PT = SMAT( 5) MAT03080
C CC = SMAT( 6) MAT03090
C DMAX = SMAT( 7) : ULTIMATE DISPLACEMENT = DUCMAX*DELT MAT03100
C BDAM = SMAT( 8) : BATA PARAMETER MAT03110
C SHAPE = SMAT( 9) : CROSS SECTION SHAPE MAT03120
1: FOR BOX OR ANGLE SECTION MAT03130
2: FOR I SHAPE SECTION MAT03140
3: FOR BOX SECTION CONSIDERING MAT03150
LOCAL BUCKLING. THIS IS ONLY FOR MEMBER MAT03160
WITH KL/R LESS THAN 40 CASE MAT03170
C ----- MAT03180
C READ (5,*) TYPE ,(SMAT(I),I=1,4), SHAPE, DUCMAX, BDAM MAT03190
C ----- MAT03200
C LMAT = 9 MAT03210
C PI=3.1415926 MAT03220
C SMAT(5)=SMAT(2)*SMAT(4) MAT03230
C SMAT(6)=SQRT(2-PI*PI*SMAT(1)/SMAT(4) ) MAT03240
C SMAT(7)=DUCMAX MAT03250
C SMAT(8)=BDAM MAT03260
C SMAT(9)=SHAPE MAT03270
C ----- MAT03280
C IF (FIRST .OR. BUG) WRITE (6,191) MAT03290
C WRITE (6,192) MAT,(SMAT(K),K=1,4),SHAPE,DUCMAX,BDAM MAT03300
C ----- MAT03310
191 FORMAT (/// GOEL HYSTERESIS MODEL PARAMETERS'// MAT03320
& ' MAT,' ,3X, ' B',6X, ' A',6X, ' R',6X, ' TS', MAT03330
& ' 6X, ' SHAPE',6X, ' MAX DUC', 6X, ' BETA' )'// MAT03340
192 FORMAT (1X,15,7G12.6/) MAT03350
C RETURN MAT03360
C ----- INITIALISE STIFFNESS TERMS ----- MAT03370
200 CONTINUE MAT03380
C ----- GOEL HYSTERESIS MODEL DATA STORAGE FOR MEMBER ----- MAT03390
C SE( 1) = STIFMEMBER AXIAL STIFFNESS MAT03400
C SE( 2) = K EFFECT SLENDER COEFFICIENT (TRANSFERED FROM ELE08) MAT03410
C SE( 3) = NOCT NO. OF CYCLE MAT03420
C SE( 4) = NRULE RULE NUMBER MAT03430
C SE( 5) = EL MEMBER LENGTH (TRANSFERED FROM ELE08) MAT03440
C SE( 6) = ITRV 0:REVRSC=.FALSE. 1:REVRSC=.TRUE. MAT03450
C SE( 7) = LDUC 0: HC=-12 1: HC .LT. -12 MAT03460
C SE( 8) = PBO7 MAT03470
C SE( 9) = DBO7 MAT03480
C SE(10) = PTOP MAT03490
C SE(11) = DTOP MAT03500
C SE(12) = REQ UNLOADING EQUIVALENT STIFFNESS MAT03510
C SE(13) = DUCT(1) MAT03520
C SE(14) = DUCT(2) MAT03530
C SE(15) = DUCT(3) MAT03540
C SE(16) = EXCR(1) MAT03550
C SE(17) = EXCR(2) MAT03560
C SE(18) = EXCR(3) MAT03570
C SE(19) = EXCR(4) MAT03580
C SE(20) = EXCR(5) MAT03590
C SE(21) = EXCR(6) MAT03600
C SE(22) = DELT YIELDING DISPLACEMENT MAT03610
C SE(23) = BESE(1) MAT03620
C SE(24) = BESE(2) MAT03630
C SE(25) = BESE(2) MAT03640
C SE(26) = EPSE MAT03650
C SE(27) = PSBOLD MAT03660
C SE(28) = CSE MAT03670
C SE(29) = DMAX ULTIMATE DISPLACEMENT MAT03680
C SE(30) = D_MAX MAXIMUM ELEMENT DISPLACEMENT FOR DAMAGE INDEX MAT03690
C SE(31) = P_MAX ELEMENT LOAD CORRESPONDING TO D_MAX MAT03700
C SE(32) = HA CURRENT CONTROL POINT HA MAT03710
C SE(33) = VA CURRENT CONTROL POINT VA MAT03720
C SE(34) = HC CURRENT CONTROL POINT HC MAT03730
C SE(35) = VC CURRENT CONTROL POINT VC MAT03740
C SE(36) = RSN CURRENT RESIDUAL STRAIN VALUE MAT03750
C SE(37) = DEMAX CURRENT MAXIMUM AXIAL DISPLACEMENT FOR DUCTILITIES MAT03760
C SE(38) = REGIN CURRENT REGINE ZONE NUMBER MAT03770
C SE(39) = PREGIN PREVIOUS REGIN ZONE NUMBER MAT03780
C ----- MAT03790
C SE(1)=1 MAT03800
C SE(2)=1 MAT03810
C SE(4)=1 MAT03820
C DO 210 I=6,28 MAT03830
210 SE(I)=0 MAT03840
C SE(36)=1 MAT03850
C SE(39)=1 MAT03860
C ----- MAT03870
C CALL HYST08(0.,0.,0.,0.,V,VL,SE(5),SE(2), MAT03880
& SMAT(1),SMAT(2),SMAT(3),SMAT(4),SMAT(5),SMAT(9), MAT03890
& SMAT(6),SE(3),SE(4),SE(1),SE(6),SE(7), MAT03900
& SE(32),SE(33),SE(34),SE(35),SE(36),SE(37),SE(38),SE(39), MAT03910

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C SE(26)= HBP
SE(27)= VBP
C SE(28)= SEHD
SE(29)= SEVD
C SE(30)= SEHB
SE(31)= SEVE
SE(32)= SESSPP
SE(33)= SESSOTP
SE(34)= SESSPPT
SE(35)= SESSDBT
SE(36)= SHD
SE(37)= SVD
SE(38)= SVE
SE(39)= SWB
SE(40)= KBQ UNLOADING EQUIVALENT STIFFNESS
SE(41)= DUCT(1)
SE(42)= DUCT(2)
SE(43)= DUCT(3)
SE(44)= EXCR(1)
SE(45)= EXCR(2)
SE(46)= EXCR(3)
SE(47)= EXCR(4)
SE(48)= EXCR(5)
SE(49)= EXCR(6)
SE(50)= DELT YIELDING DISPLACEMENT
SE(51)= SESE(1)
SE(52)= SESE(2)
SE(53)= SESE(2)
SE(54)= EPSE
SE(55)= PSEBOLD
SE(56)= HA
SE(57)= DMAX ULTIMATE DISPLACEMENT
SE(58)= D_MAX MAXIMUM ELEMENT DISPLACEMENT FOR DAMAGE INDEX
SE(59)= P_MAX ELEMENT LOAD CORRESPONDING TO D_MAX
SE(60)= DEMAX CURRENT MAXIMUM AXIAL DISPLACEMENT FOR DUCTILITIES
SE(61)= INITIAL STIFFNESS
SE(62)= DISP FACTOR FACH
SE(63)= MOMENT FACTOR FACV
SE(64)= ROTATION STIFFNESS FACTOR=FACV/FACH
SE(65)= HA
SE(66)= VA
-----
DO 210 I=1,61
  SE(1)=SMAT(1)
  SE(5)=1
  SE(65)= SMAT(1)
  SE(66)= SMAT(2)
C
MOPVT=IOPT
CALL HYST09(MOPVT,SE(1),0,0,0,0,SE(2),SE(3),
  & SE(4),SE(5),SE(6),SE(7),SE(8),SE(9),
  & SE(10),SE(11),SE(12),SE(13),
  & SE(14),SE(15),SE(16),SE(17),
  & SE(65),SE(66),SE(18),SE(19),SE(20),SE(21),SE(22),SE(23),
  & SE(24),SE(25),SE(26),SE(27),
  & SE(28),SE(29),SE(30),SE(31),
  & SE(32),SE(33),SE(34),SE(35),
  & SE(36),SE(37),SE(38),SE(39),BUG,
  & SE(40),SE(41),SE(44),SE(50),SE(51),SE(54),SE(55),SE(56),
  & SE(60),SE(62),SE(63),SE(64),SMAT(3) )
  SE(56)=SE(50)+SMAT(2)/2
  SE(57)=SE(22)+SMAT(3)
  SE(58)=0
  SE(59)=0
  SE(61)=SE(1)
C
RETURN
----- GET STIFFNESS TERMS -----
300 CONTINUE
  SE(2)=D-PL
  SE(3)=D-DL
C
CALL HYST09 TO CALC NEW STIFFNESS STIF
MOPVT=IOPT
CALL HYST09(MOPVT,SE(1),P,D,PL,DL,SE(2),SE(3),
  & SE(4),SE(5),SE(6),SE(7),SE(8),SE(9),
  & SE(10),SE(11),SE(12),SE(13),
  & SE(14),SE(15),SE(16),SE(17),
  & SE(65),SE(66),SE(18),SE(19),SE(20),SE(21),SE(22),SE(23),
  & SE(24),SE(25),SE(26),SE(27),
  & SE(28),SE(29),SE(30),SE(31),
  & SE(32),SE(33),SE(34),SE(35),
  & SE(36),SE(37),SE(38),SE(39),BUG,
  & SE(40),SE(41),SE(44),SE(50),SE(51),SE(54),SE(55),SE(56),
  & SE(60),SE(62),SE(63),SE(64),SMAT(3) )
C
STORE MAXIMUM DISPLACEMENT AND CORRESPONDING LOAD
IF(ABS(D).GT.ABS(SE(58))) THEN
  SE(58)=D
  SE(59)=P
ENDIF
C
GET ENERGIES, DUCTILITY AND EXCURSION FOR BOTH IOPT=3 AND 4
400 CONTINUE
C
TRANSFER ENERGIES FOR BOTH IOPT=3 AND 4.
  EBSE=SE(51)
  EPSE=SE(54)
C
TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT=3 AND 4.
DO 410 I=1,3
  DUCT(I)=SE(40+I)
  EXCR(I+3)=SE(43+I+3)
410 EXCR(I)=SE(43+I)
C
RETURN
----- DAMAGE INDEX -----
500 CONTINUE
C
DAMAGE INDEX IS NOT AVAILABLE
  EBSE=SE(51)
  EPSE=SE(54)
  DMAX=SE(57)
  D=SE(58)
  P=SE(59)
  PT=SMAT(2)
  BDAM=SMAT(4)
  DAMAGE=ABS(D)/DMAX + BDAM / (PT*DMAX) * EPSE
  DAMAGE=0
C
RETURN
END
C
PROCESS SDUMP GOSTMT XREF MAP
-----
C
DEBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
C
END DEBUG
-----
SUBROUTINE MAT10
  & (IOPT,IELNO,ERR,FIRST,PRINT,BUG,
  & MAT,SE,PP,D,DL,V,VL,LELEM,LMAT,
  & SMAT,EBSE,EPSE,DUCT,EXCR,DAMAGE)
C
IMPLICIT REAL(A-H,O-Z)
LOGICAL ERR,FIRST,PRINT,BUG
LOGICAL FIRST
DIMENSION SE(100),SMAT(9),DUCT(3),EXCR(6)
CHARACTER*80 TYPE
C
RESERVE STORAGE FOR ELEMENT
LELEM=5
IF (IOPT.EQ.0) RETURN

```

```

MAT00910
MAT00920
MAT00930
MAT00940
MAT00950
MAT00960
MAT00970
MAT00980
MAT00990
MAT01000
MAT01010
MAT01020
MAT01030
MAT01040
MAT01050
MAT01060
MAT01070
MAT01080
MAT01090
MAT01100
MAT01110
MAT01120
MAT01130
MAT01140
MAT01150
MAT01160
MAT01170
MAT01180
MAT01190
MAT01200
MAT01210
MAT01220
MAT01230
MAT01240
MAT01250
MAT01260
MAT01270
MAT01280
MAT01290
MAT01300
MAT01310
MAT01320
MAT01330
MAT01340
MAT01350
MAT01360
MAT01370
MAT01380
MAT01390
MAT01400
MAT01410
MAT01420
MAT01430
MAT01440
MAT01450
MAT01460
MAT01470
MAT01480
MAT01490
MAT01500
MAT01510
MAT01520
MAT01530
MAT01540
MAT01550
MAT01560
MAT01570
MAT01580
MAT01590
MAT01600
MAT01610
MAT01620
MAT01630
MAT01640
MAT01650
MAT01660
MAT01670
MAT01680
MAT01690
MAT01700
MAT01710
MAT01720
MAT01730
MAT01740
MAT01750
MAT01760
MAT01770
MAT01780
MAT01790
MAT01800
MAT01810
MAT01820
MAT01830
MAT01840
MAT01850
MAT01860
MAT01870
MAT01880
MAT01890
MAT01900
MAT01910
MAT01920
MAT01930
MAT01940
MAT01950
MAT01960
MAT01970
MAT01980
MAT01990
MAT02000
MAT02010
MAT02020
MAT02030
MAT02040
MAT02050
MAT02060
MAT02070
MAT02080
MAT02090
MAT02100
MAT02110
MAT02120
MAT020010
MAT020020
MAT020030
MAT020040
MAT020050
MAT020060
MAT020070
MAT020080
MAT020090
MAT020100
MAT020110
MAT020120
MAT020130
MAT020140
MAT020150
MAT020160
MAT020170
MAT020180
MAT020190
MAT020200
C
VARIABLES:
-----
GLOBAL VARIABLES
C
IOPT = 1, INITIALIZE MATERIAL
      = 2, INITIALIZE STIFFNESS
      = 3, GET STIFFNESS TERM
      = 4, GET ENERGIES, DUCTILITY AND EXCURSION
      = 5, CALCULATE DAMAGE INDEX
C
RINPUT = INPUT DATA
C
SMAT = INTERNAL STORAGE
C
SE = OUTPUT STIFFNESS
-----
GO TO (100,200,300,400,500),IOPT
C
100 CONTINUE
C
INTERPRETATE INPUT DATA
  ELAS = SMAT( 1 ) : 0 FOR ELASTIC ANALYSIS
                  : 1 FOR INELASTIC ANALYSIS
                  : 2 FOR SIMULATING PINO-SUAREZ TOWER COLUMN
                  : 3 FOR INELASTIC ANALYSIS AND CHECK BOX
                    COLUMN INTERACTIVE STRENGTH CRITERIA
  SP = SMAT( 2 ) : RATIO FOR INELASTIC STIFFNESS TO INITIAL
                  STIFFNESS
  E = SMAT( 3 ) : ELASTIC MODULUS
  TI = SMAT( 4 ) : SECTION MOMENT OF INERTIA
  TMP = SMAT( 5 ) : PLASTIC MOMENT CAPACITY
  DUCHMAX = SMAT( 6 ) : FAILURE DUCTILITY
                  : SE, 1 FAILURE DUCTILITY CONSIDERED
                  : LT, 1 FAILURE DUCTILITY NOT CONSIDERED
  BDAM = SMAT( 7 ) : PARAMETER FOR ANG'S DAMAGE INDEX
  REDUCE = SMAT( 8 ) : RESIDUAL STRENGTH FOR ELAS=2
                  : SMAT( 9 ) : DUMPT VARIABLE
-----
READ (5,*) TYPE ,(SMAT(1),I=1,5),DUCHMAX,BDAM
SMAT(6)=DUCHMAX
SMAT(7)=BDAM
C
IF (SMAT(1).EQ.2) THEN
  READ(5,*)REDUCE
  SMAT(8)=REDUCE
ENDIF
C
LMAT = 9
C
IF (FIRST.OR.BUG) WRITE (6,191)
WRITE (6,192) MAT,(SMAT(K),K=1,5),SMAT(6),SMAT(7),REDUCE
191 FORMAT (///' BILINEAR INTERACTIVE MATERIAL PROPERTIES'//
  & '-----'//
  & ' MAT, 'JK, ' ELAS, '6X, ' SP, '
  & ' '6X, ' E, '6X, ' TI, '6X, ' MP, '
  & ' '6X, ' MAX DUC, '6X, ' BETA, '6X, ' REDU P, ' //)
192 FORMAT (1X,15,8G12.6/)
C
RETURN
----- INITIALIZE STIFFNESS TERMS -----
200 CONTINUE
C
BILINEAR INTERACTIVE MATERIAL DATA STORAGE FOR MEMBER
  SE( 1 ) = EI SECTION RIGIDITY
  SE( 2 ) = P1 CURRENT YIELDING LOAD FOR POSITIVE LOAD
  SE( 3 ) = P2 CURRENT YIELDING LOAD FOR NEGATIVE LOAD
  SE( 4 ) = YIELD 0: ELASTIC, 1: INELASTIC
  SE( 5 ) = ELAS
  SE( 6 ) = SP
  SE( 7 ) = FAILURE DUCTILITY
  SE( 8 ) = PARAMETER FOR ANG'S DAMAGE INDEX
  SE( 9 ) = RESIDUAL STRENGTH FOR ELAS=2
  SE(10) = CURRENT DUCTILITY
  SE(11) = SMAT(4)
  SE(12) = TMP*SMAT(5)
  SE(13) = DELT YIELDING DISPLACEMENT
            FOR ROTATION SE(13)=TMP*LENGTH/(6EI)
            FOR AXIAL SE(13)=TMP*LENGTH/EI
            FOR TORSION SE(13)=TMP*LENGTH/EI
  SE(14) = DEMAX CURRENT MAXIMUM DISPLACEMENT FOR DUCTILITY
  SE(15) = YIELDING DISPLACEMENT INDEX:
            0: DELT HAS NOT BEEN DETERMINED
            1: DELT HAS BEEN DETERMINED
-----
DO 210 I=1,15
  SE(1)=SMAT(3)+SMAT(4)
  SE(2)=SMAT(5)
  SE(3)=-SMAT(5)
  SE(4)=0
  SE(5)=SMAT(1)
  SE(6)=SMAT(2)
  SE(7)=SMAT(6)
  SE(8)=SMAT(7)
  SE(9)=SMAT(8)
  SE(10)=SMAT(4)
  SE(11)=SMAT(4)
  SE(12)=SMAT(5)
  SE(13)=0
  SE(14)=0
  SE(15)=0
C
RETURN
----- GET STIFFNESS TERMS -----
300 CONTINUE
C
CHECK FAILURE DUCTILITY
  DUCT(1)=SE(10)
  IF (DUCT(1).GE.SMAT(6).AND.SMAT(6).GE.1) THEN
    P=0
    YIELD=1
    SE(4)=YIELD
    RETURN
  ENDIF
  ELAS = SMAT( 1 )
  SP = SMAT( 2 )
  E = SMAT( 3 )
  TI = SMAT( 4 )
  TMP = SMAT( 5 )
  P1 = SE(2)
  P2 = SE(3)
  YIELD = SE(4)
C
IF (ELAS.EQ.0) THEN
  YIELD = 0
  SE(4)=YIELD
  SE(14)=0
  DUCT(1)=0
  RETURN
ENDIF
PPPPP
C
IF (ELAS.EQ.3) THEN
C
FOR ELASTIC REGION
  IF (YIELD.EQ.0) THEN
    IF (P.GE.P1.OR.P.LE.P2) THEN
      YIELD=1
      IF (P.GE.P1) P=P1
      IF (P.LE.P2) P=P2
    ENDIF
  ENDIF
  DETERMINE ELEMENT DUCTILITY UNIT
  NOTE: ROTATION DUCTILITY UNIT IS DEFINED WHEN
  SE(15)=1, IF (P.GE.TMP.AND.D.GE.TMP*LENGTH/(6EI))
  ELSE SE(15)=0

```

```

IF( SE(15) .EQ. 0 ) THEN
  IF( ABS(D) .GE. ABS(SE(13)) ) THEN
    SE(13)=ABS(D)
    SE(15)=1
  ENDIF
ELSE IF( P .LT. P1 .AND. P .GT. P2 ) THEN
  YIELD=0
ENDIF
C ** FOR INELASTIC REGION
ELSE IF( YIELD .EQ. 1 ) THEN
  IF( P .GE. P1 .OR. P .LE. P2 ) THEN
    YIELD=1
    IF( P .GE. P1 ) P=P1
    IF( P .LE. P2 ) P=P2
    DETERMINE ELEMENT DUCTILITY UNIT
    NOTE: ROTATION DUCTILITY UNIT IS DEFINED WHEN
    SE(15)=1, IF (P .GE. TMP .AND. D .GE. TMP*LENGTH/(6EI))
    ELSE SE(15)=0
  IF( SE(15) .EQ. 0 ) THEN
    IF( ABS(D) .GE. ABS(SE(13)) ) THEN
      SE(13)=ABS(D)
      SE(15)=1
    ENDIF
    ELSE IF( P .LT. P1 .AND. PP .GE. 0 ) THEN
      YIELD=0
      *** UNBALANCED FORCE ***
      IF( (D-DL) .NE. 0 ) THEN
        SM2=(P-PP)/(D-DL)
        SM1=SM2/SP
        UD=D-DL
        UP=SM1*UD
        P=PP+UP
        IF( P .LE. P2 ) THEN
          P=P2
          YIELD=1
        ENDIF
      ENDIF
    ELSE IF( P .GT. P2 .AND. PP .LE. 0 ) THEN
      YIELD=0
      *** UNBALANCED FORCE ***
      IF( (D-DL) .NE. 0 ) THEN
        SM2=(P-PP)/(D-DL)
        SM1=SM2/SP
        UD=D-DL
        UP=SM1*UD
        P=PP+UP
        IF( P .GE. P1 ) THEN
          P=P1
          YIELD=1
        ENDIF
      ENDIF
    ENDIF
    GO TO 399
  ENDIF
  C --- FOR ELAS=1 AND 3 CASES:
  ELASTIC REGION
  IF( YIELD .EQ. 0 ) THEN
    IF( P .GE. P1 .OR. P .LE. P2 ) THEN
      YIELD=1
      *** UNBALANCED FORCE ***
      IF( P .GE. P1 ) THEN
        IF( (D-DL) .NE. 0 .AND. (P-PP) .NE. 0 ) THEN
          SM1=(P-PP)/(D-DL)
          /* SM2 IS ESTIMATED VALUE */
          SM2=SM1*SP
          UP1=P-P1
          UD=UP1/SM1
          UP2=UD*SM2
          P=P-(UP1+UP2)
        ENDIF
        IF( D .EQ. 0 .AND. DL .EQ. 0 ) P=P
        P=P1
      ELSE IF( P .LE. P2 ) THEN
        IF( (D-DL) .NE. 0 .AND. (P-PP) .NE. 0 ) THEN
          SM1=(P-PP)/(D-DL)
          /* SM2 IS ESTIMATED VALUE */
          SM2=SM1*SP
          UP1=P-P2
          UD=UP1/SM1
          UP2=UD*SM2
          P=P-(UP1+UP2)
        ENDIF
        IF( D .EQ. 0 .AND. DL .EQ. 0 ) P=P2
        P=P2
      ENDIF
    DETERMINE ELEMENT DUCTILITY UNIT
    NOTE: ROTATION DUCTILITY UNIT IS DEFINED WHEN
    SE(15)=1, IF (P .GE. TMP .AND. D .GE. TMP*LENGTH/(6EI))
    ELSE SE(15)=0
  IF( SE(15) .EQ. 0 ) THEN
    IF( ABS(D) .GE. ABS(SE(13)) ) THEN
      SE(13)=ABS(D)
      SE(15)=1
    ENDIF
  ENDIF
  ELSE
    YIELD=0
  ENDIF
  GO TO 399
ENDIF
C --- INELASTIC REGION
IF( YIELD .EQ. 1 ) THEN
  IF( P .GT. P1 ) THEN
    IF( P .GE. PP ) THEN
      YIELD=1
      IF( SE(15) .EQ. 0 ) THEN
        IF( ABS(D) .GE. ABS(SE(13)) ) THEN
          SE(13)=ABS(D)
          SE(15)=1
        ENDIF
      ELSE IF( P .LT. PP ) THEN
        YIELD=0
        P1=PP
        P2=PP+2*TMP
        *** UNBALANCED FORCE ***
        IF( (D-DL) .NE. 0 ) THEN
          SM2=(P-PP)/(D-DL)
          SM1=SM2/SP
          UD=D-DL
          UP=SM1*UD
          P=PP+UP
        ENDIF
      ELSE IF( P .LT. P2 ) THEN
        IF( P .LE. PP ) THEN
          YIELD=1
          IF( SE(15) .EQ. 0 ) THEN
            IF( ABS(D) .GE. ABS(SE(13)) ) THEN
              SE(13)=ABS(D)
              SE(15)=1
            ENDIF
          ENDIF
        ELSE
          YIELD=0
          *** UNBALANCED FORCE ***
          IF( (D-DL) .NE. 0 ) THEN
            SM2=(P-PP)/(D-DL)
            SM1=SM2/SP
            UD=D-DL
            UP=SM1*UD
            P=PP+UP
          ENDIF
        ENDIF
      ENDIF
    ENDIF
    IF( SE(15) .EQ. 0 ) THEN
      IF( ABS(D) .GE. ABS(SE(13)) ) THEN
        SE(13)=ABS(D)
        SE(15)=1
      ENDIF
    ENDIF
  ENDIF
  GO TO 399
ENDIF
IF( SE(15) .EQ. 0 ) THEN
  IF( ABS(D) .GE. ABS(SE(13)) ) THEN
    SE(13)=ABS(D)
    SE(15)=1
  ENDIF
ENDIF
399 SE(2)=P1
SE(3)=P2
SE(4)=YIELD
IF( SE(15) .EQ. 1 ) THEN
  IF( SE(14) .LE. ABS(D) ) THEN
    DUCT(1)=SE(14)/SE(13)
    SE(10)=DUCT(1)
  ELSE IF( SE(15) .EQ. 0 ) THEN
    DUCT(1)=0
    SE(10)=DUCT(1)
  ENDIF
  C IF( P .GT. P1 .AND. P .GT. P2 ) THEN
  C DDDD=P-PPPP
  C WRITE(6,979) P,PPPP,DDDD
  C 879 FORMAT(1X,4G15.6)
  C ENDDIF
  C----- GET DUCTILITY IOPT=3 AND 4 -----
  400 CONTINUE
  C --- TRANSFER ENERGIES FOR BOTH IOPT=3 AND 4.
  BESE=0
  EPSE=0
  ELAS = SMAT(1)
  C --- TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT=3 AND 4.
  500 CONTINUE
  DUCT(I)=0
  EXCR(I+3)=0
  410 EXCR(I)=0
  IF( ELAS .EQ. 0 ) THEN
    DUCT(1)=0
    SE(10)=DUCT(1)
  ELSE IF( ELAS .NE. 0 ) THEN
    IF( SE(15) .EQ. 0 ) THEN
      DUCT(1)=0
      SE(10)=DUCT(1)
    ELSE IF( SE(15) .EQ. 1 ) THEN
      DUCT(1)=SE(14)/SE(13)
      SE(10)=DUCT(1)
    ENDIF
  ENDIF
  C WRITE(40,777)SE(13),SE(14),DUCT(1),SE(15)
  C 777 FORMAT(1X,4G15.6)
  RETURN
C----- DAMAGE INDEX -----
  C---DAMAGE INDEX IS NOT AVAILABLE
  DAMAGE=0
  C RETURN
  END
C@PROCESS SOUNP OPT(0) GOSTMT KREF MAP
  C-----
  C DBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE
  C-----
  C-----
  SUBROUTINE MAT11
  6 (IOPT, IELNO, ERR, FIRST, PRINT, BUG,
  6 MAT, SE, P, PL, D, DL, V, VL, LELEM, LMAT,
  6 SMAT, BESE, EPSE, DUCT, EXCR, DAMAGE)
  C IMPLICIT REAL(A-H,O-S)
  LOGICAL ERR, FIRST, PRINT, BUG
  LOGICAL BESE
  DIMENSION SE(100),SMAT(5),DUCT(3),EXCR(6)
  CHARACTER*80 TPTS
  C----- RESERVE STORAGE FOR ELEMENT
  LELEM=31
  IF( IOPT .EQ. 0 ) RETURN
  C-----
  C VARIABLES:
  C-----
  C----- GLOBAL VARIABLES
  IOPT = 1, INITIALIZE MATERIAL
  = 2, INITIALIZE STIFFNESS
  = 3, GET STIFFNESS TERM
  = 4, GET ENERGIES, DUCTILITY AND EXCURSION
  = 5, CALCULATE DAMAGE INDEX
  C RINPUT = INPUT DATA
  C SMAT = INTERNAL STORAGE
  C SE = OUTPUT STIFFNESS
  C-----
  GO TO (100,200,300,400,500),IOPT
  100 CONTINUE
  C----- INTERPERATE INPUT DATA
  HA = SMAT( 1 )
  VA = SMAT( 2 )
  RATIO = SMAT( 3 ) ; STRAIN HARDEN RATIO TO ELASTIC STIFFNESS
  DUCMAX = SMAT( 4 ) ; FAILURE DUCTILITY
  BDAM = SMAT( 5 ) ; BATA PARAMETER
  C READ (5,*) TYPE ,(SMAT(I),I=1,3), DUCMAX, BDAM
  C LMAT = 5
  SMAT(4)=DUCMAX
  SMAT(5)=BDAM
  C IF( FIRST .OR. BUG ) WRITE (6,191)
  WRITE (6,192) MAT,(SMAT(K),K=1,3),DUCMAX,BDAM
  191 FORMAT (////, PING-SQUARES TOWER SHORT DIR. HYST. MODEL PARAMETERS'//
  6 ' MAT',3X,' HA',6X,' VA',6X,' RATIO',
  6 ' 6X,' MAX DUC', 6X,' BETA')
  192 FORMAT (1X,13,2X,6G12.6/)
  C RETURN
  C----- INITIALIZE STIFFNESS TERMS -----

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6 IX,EM =,IP,G10.3,9X,LIBN =,IP,G10.3/ MAT01830
6 IX,BOX HEIGHT =,IP,G10.3,9X,BOX WILTH =,IP,G10.3/ MAT01840
6 IX,SECTO =,IP,G10.3,9X,ECCYO =,IP,G10.3/ MAT01850
6 IX,H.W.SEC.NO. =,IP,G10.3,9X,H.H.SEC.NO. =,IP,G10.3/ MAT01860
6 IX,THICKNESS =,IP,G10.3,9X,L.B. FLAG =,IP,G10.3/ MAT01870
6 IX,IREV2 =,IP,G10.3,9X,IREV3 =,IP,G10.3/ MAT01880
6 IX,IREV4 =,IP,G10.3,9X,IRECOP =,IP,G10.3/ MAT01890
6 IX,NUMP =,IP,G10.3,9X,SMALL =,IP,G10.3/ MAT01900
6 IX,RATIO =,IP,G10.3,9X,RATIO =,IP,G10.3/ MAT01910
6 IX,TOTA =,IP,G10.3,9X,IAUTO =,IP,G10.3/ MAT01920
6 IX,IMATER =,IP,G10.3,9X,RATIO3 =,IP,G10.3/ MAT01930
6 IX,IR =,IP,G10.3,9X,G =,IP,G10.3/ MAT01940
6 IX,QRNEE =,IP,G10.3,9X,ISTIF =,IP,G10.3// MAT01950
192 FORMAT (IX,MAT. NO. =,I3.2X, TUBE SECTION'/ MAT01960
6 IX,NSEG =,IP,G10.3,9X,YS =,IP,G10.3/ MAT01970
6 IX,EM =,IP,G10.3,9X,LIBN =,IP,G10.3/ MAT01980
6 IX,RADIUS =,IP,G10.3,9X,DUMY VAR. =,IP,G10.3/ MAT02000
6 IX,ECCYO =,IP,G10.3,9X,ECCYO =,IP,G10.3/ MAT02010
6 IX,LAG LENGTH =,IP,G10.3,9X,DUMY VAR. =,IP,G10.3/ MAT02020
6 IX,THICKNESS =,IP,G10.3,9X,IREV1 =,IP,G10.3/ MAT02030
6 IX,IREV2 =,IP,G10.3,9X,IREV3 =,IP,G10.3/ MAT02040
6 IX,IREV4 =,IP,G10.3,9X,IRECOP =,IP,G10.3/ MAT02050
6 IX,NUMP =,IP,G10.3,9X,SMALL =,IP,G10.3/ MAT02060
6 IX,RATIO =,IP,G10.3,9X,RATIO =,IP,G10.3/ MAT02070
6 IX,TOTA =,IP,G10.3,9X,IAUTO =,IP,G10.3/ MAT02080
6 IX,IMATER =,IP,G10.3,9X,RATIO3 =,IP,G10.3/ MAT02090
6 IX,IR =,IP,G10.3,9X,G =,IP,G10.3/ MAT02100
6 IX,QRNEE =,IP,G10.3,9X,ISTIF =,IP,G10.3// MAT02110
193 FORMAT (IX,MAT. NO. =,I3.2X, ANGLE SECTION'/ MAT02120
6 IX,NSEG =,IP,G10.3,9X,YS =,IP,G10.3/ MAT02130
6 IX,EM =,IP,G10.3,9X,LIBN =,IP,G10.3/ MAT02140
6 IX,LAG LENGTH =,IP,G10.3,9X,DUMY VAR. =,IP,G10.3/ MAT02150
6 IX,ECCYO =,IP,G10.3,9X,ECCYO =,IP,G10.3/ MAT02160
6 IX,LAG.SEC.NO. =,IP,G10.3,9X,DUMY VAR. =,IP,G10.3/ MAT02180
6 IX,THICKNESS =,IP,G10.3,9X,N.R.IN U DIR=,IP,G10.3/ MAT02190
6 IX,N.C.IN U DIR=,IP,G10.3,9X,N.R.IN V DIR=,IP,G10.3/ MAT02200
6 IX,N.C.IN V DIR=,IP,G10.3,9X,IRECOP =,IP,G10.3/ MAT02210
6 IX,NUMP =,IP,G10.3,9X,SMALL =,IP,G10.3/ MAT02220
6 IX,RATIO =,IP,G10.3,9X,RATIO =,IP,G10.3/ MAT02230
6 IX,TOTA =,IP,G10.3,9X,IAUTO =,IP,G10.3/ MAT02240
6 IX,IMATER =,IP,G10.3,9X,RATIO3 =,IP,G10.3/ MAT02250
6 IX,IR =,IP,G10.3,9X,G =,IP,G10.3/ MAT02260
6 IX,QRNEE =,IP,G10.3,9X,ISTIF =,IP,G10.3// MAT02270
194 FORMAT (IX,MAT. NO. =,I3.2X, RECTANGULAR SECTION'/ MAT02280
6 IX,NSEG =,IP,G10.3,9X,YS =,IP,G10.3/ MAT02300
6 IX,EM =,IP,G10.3,9X,LIBN =,IP,G10.3/ MAT02310
6 IX,SEC HEIGHT =,IP,G10.3,9X,SEC WILTH =,IP,G10.3/ MAT02320
6 IX,ECCYO =,IP,G10.3,9X,ECCYO =,IP,G10.3/ MAT02330
6 IX,UU.SEC.NO. =,IP,G10.3,9X,HH.SEC.NO. =,IP,G10.3/ MAT02340
6 IX,DUMY VAR. =,IP,G10.3,9X,IREV1 =,IP,G10.3/ MAT02350
6 IX,IREV2 =,IP,G10.3,9X,IREV3 =,IP,G10.3/ MAT02360
6 IX,IREV4 =,IP,G10.3,9X,IRECOP =,IP,G10.3/ MAT02370
6 IX,NUMP =,IP,G10.3,9X,SMALL =,IP,G10.3/ MAT02380
6 IX,RATIO =,IP,G10.3,9X,RATIO =,IP,G10.3/ MAT02390
6 IX,TOTA =,IP,G10.3,9X,IAUTO =,IP,G10.3/ MAT02400
6 IX,IMATER =,IP,G10.3,9X,RATIO3 =,IP,G10.3/ MAT02410
6 IX,IR =,IP,G10.3,9X,G =,IP,G10.3/ MAT02420
6 IX,QRNEE =,IP,G10.3,9X,ISTIF =,IP,G10.3// MAT02430
195 FORMAT (IX,MAT. NO. =,I3.2X, ANGLE WITH COVER PL. '/ MAT02440
6 IX,NSEG =,IP,G10.3,9X,YS =,IP,G10.3/ MAT02450
6 IX,EM =,IP,G10.3,9X,LIBN =,IP,G10.3/ MAT02470
6 IX,LAG LENGTH =,IP,G10.3,9X,COV.WIDTH =,IP,G10.3/ MAT02480
6 IX,COV.THICKNESS =,IP,G10.3,9X,DUMY VAR. =,IP,G10.3/ MAT02490
6 IX,N.R. COV. =,IP,G10.3,9X,N.C. COV. =,IP,G10.3/ MAT02500
6 IX,THICKNESS =,IP,G10.3,9X,N.R.IN U DIR=,IP,G10.3/ MAT02510
6 IX,N.C.IN U DIR=,IP,G10.3,9X,N.R.IN V DIR=,IP,G10.3/ MAT02520
6 IX,N.C.IN V DIR=,IP,G10.3,9X,IRECOP =,IP,G10.3/ MAT02530
6 IX,NUMP =,IP,G10.3,9X,SMALL =,IP,G10.3/ MAT02540
6 IX,RATIO =,IP,G10.3,9X,RATIO =,IP,G10.3/ MAT02550
6 IX,TOTA =,IP,G10.3,9X,IAUTO =,IP,G10.3/ MAT02560
6 IX,IMATER =,IP,G10.3,9X,RATIO3 =,IP,G10.3/ MAT02570
6 IX,IR =,IP,G10.3,9X,G =,IP,G10.3/ MAT02580
6 IX,QRNEE =,IP,G10.3,9X,ISTIF =,IP,G10.3// MAT02590
196 FORMAT (IX,MAT. NO. =,I3.2X, WIDE-FLANGE SECTION'/ MAT02600
6 IX,NSEG =,IP,G10.3,9X,YS =,IP,G10.3/ MAT02620
6 IX,EM =,IP,G10.3,9X,LIBN =,IP,G10.3/ MAT02630
6 IX,BOX HEIGHT =,IP,G10.3,9X,BOX WILTH =,IP,G10.3/ MAT02640
6 IX,ECCYO =,IP,G10.3,9X,ECCYO =,IP,G10.3/ MAT02650
6 IX,WEB THICK. =,IP,G10.3,9X,DUMY VAR. =,IP,G10.3/ MAT02660
6 IX,FLA.THICK. =,IP,G10.3,9X,IREV1 =,IP,G10.3/ MAT02670
6 IX,IREV2 =,IP,G10.3,9X,IREV3 =,IP,G10.3/ MAT02680
6 IX,IREV4 =,IP,G10.3,9X,IRECOP =,IP,G10.3/ MAT02690
6 IX,NUMP =,IP,G10.3,9X,SMALL =,IP,G10.3/ MAT02700
6 IX,RATIO =,IP,G10.3,9X,RATIO =,IP,G10.3/ MAT02710
6 IX,TOTA =,IP,G10.3,9X,IAUTO =,IP,G10.3/ MAT02720
6 IX,IMATER =,IP,G10.3,9X,RATIO3 =,IP,G10.3/ MAT02730
6 IX,IR =,IP,G10.3,9X,G =,IP,G10.3/ MAT02740
6 IX,QRNEE =,IP,G10.3,9X,ISTIF =,IP,G10.3// MAT02750
199 FORMAT (IX,MAT. NO. =,I3, 'IG NOT AVAILABLE', MAT02760
6 ' LIBN=',G10.3, ' CHECK LIBN '/) MAT02770
C
11=29+1 MAT02780
12=11+NSEG*12 MAT02790
13=12+(NSEG+1)*6 MAT02800
14=13+NSEG MAT02810
15=14+2 MAT02820
16=15+2 MAT02830
17=16+NSEG*NUMP MAT02840
18=17+NSEG*NUMP MAT02850
19=18+NUMP MAT02860
110=19+NUMP MAT02870
111=110+NSEG*2+3 MAT02880
112=111+NSEG MAT02890
113=112+(NSEG+1)*6 MAT02900
114=113+(NSEG+1)*6 MAT02910
115=114+(NSEG+1)*6 MAT02920
116=115+NSEG MAT02930
117=116+NSEG*12 MAT02940
118=117+NSEG*12 MAT02950
119=118+NSEG*12 MAT02960
120=119+NSEG*12 MAT02970
121=120+NSEG*NUMP MAT02980
122=121+NSEG*NUMP MAT02990
123=122+NSEG*NUMP MAT03000
124=123+NUMP MAT03010
125=124+NSEG*9 MAT03020
126=125+NSEG*9 MAT03030
127=126+(NSEG+1)*6**2 MAT03040
128=127+(NSEG+1)*6*(NSEG+1)*6+1)/2 MAT03050
129=128+(NSEG+1)*6 + 1 MAT03060
130=129+144 MAT03070
131=130+144 MAT03080
132=131+(NSEG+1)*6 MAT03090
133=132+(NSEG+1)*6**2 MAT03100
134=133+NSEG*144 MAT03110
135=134+2 MAT03120
136=135+2 MAT03130
137=136+(NSEG+1)*6 MAT03140
138=137+(NSEG+1)*6 MAT03150
139=138+(NSEG+1)*6 MAT03160
140=139+(NSEG+1)*6**2 MAT03170
141=140+NUMP MAT03180
142=141+NSEG*NUMP MAT03190
143=142+NUMP MAT03200
144=143+NUMP MAT03210
145=144+NSEG*NUMP MAT03220
C
I46=I45+NSEG*NUMP MAT03230
I47=I46+NSEG*NUMP MAT03240
I48=I47+NSEG*NUMP MAT03250
I49=I48+NSEG*NUMP MAT03260
I50=I49+NSEG*NUMP MAT03270
I51=I50+NSEG*NUMP MAT03280
LMAT= I51 + NUMP - 1 MAT03290
SMAT(29)=LMAT MAT03300
C MAT03310
C MAT03320
C MAT03330
C MAT03340
C MAT03350
C MAT03360
C----- INITIALISE STIFFNESS TERMS ----- MAT03370
200 CONTINUE MAT03380
C----- STABILITY ELEMENT MATERIAL DATA STORAGE FOR MEMBER ----- MAT03390
C SE(1- 78)= SEASAT MAT03400
C SE( 79)=ELS MAT03410
C SE( 80)=LMD MAT03420
C SE( 81)=ISPE MAT03430
C SE( 82)=DELTFP2 MAT03440
C SE( 83)=G2 MAT03450
C SE( 84)=G6 MAT03460
C SE( 85)=ISTART MAT03470
C SE( 86)=QRNEE MAT03480
C SE( 87)=ESEE MAT03500
C SE( 88)=EPSE MAT03510
C SE( 89)=DUCT(1) MAT03520
C SE( 90)=DUCT(2) MAT03530
C SE( 91)=DUCT(3) MAT03540
C SE( 92)=EXCR(1) MAT03550
C SE( 93)=EXCR(2) MAT03560
C SE( 94)=EXCR(3) MAT03570
C SE( 95)=EXCR(4) MAT03580
C SE( 96)=EXCR(5) MAT03590
C SE( 97)=EXCR(6) MAT03600
C SE( 98)=FLP FLAG FOR JUDGING LOCAL BUCKLING MAT03610
C SE( 99)= 0 1 NO LOCAL BUCKLING OCCURRED MAT03620
C SE( 99)= 1-41 LOCAL BUCKLING OCCURRED MAT03630
C SE( 99)= 1-41 LOCAL BUCKLING OCCURRED MAT03640
C SE( 99)=FLAG FOR JUDGING LOCAL BUCKLING HAS OCCURRED OR NOT MAT03650
C SE( 99)= 0 1 NO LOCAL BUCKLING HAS OCCURRED MAT03660
C SE( 99)= 1 1 LOCAL BUCKLING HAS OCCURRED MAT03670
C SET COMPRESSION STIFFNESS=0.001 MAT03680
C----- MAT03690
NSEG = SMAT( 1) MAT03700
YS = SMAT( 2) MAT03710
EM = SMAT( 3) MAT03720
LIBN = SMAT( 4) MAT03730
HH = SMAT( 5) MAT03740
UU = SMAT( 6) MAT03750
WN = SMAT( 7) MAT03760
ZZ = SMAT( 8) MAT03770
INEB = SMAT( 9) MAT03780
INEN = SMAT(10) MAT03790
ST = SMAT(11) MAT03800
IREV1 = SMAT(12) MAT03810
IREV2 = SMAT(13) MAT03820
IREV3 = SMAT(14) MAT03830
IREV4 = SMAT(15) MAT03840
IRECOP = SMAT(16) MAT03850
NUMP = SMAT(17) MAT03860
SMALL = SMAT(18) MAT03870
RATIO = SMAT(19) MAT03880
RATIO3 = SMAT(20) MAT03890
TOTA = SMAT(21) MAT03900
IAUTO = SMAT(22) MAT03910
IMATER = SMAT(23) MAT03920
RATIO3 = SMAT(24) MAT03930
IR = SMAT(25) MAT03940
G = SMAT(26) MAT03950
QRNEE = SMAT(27) MAT03960
ISTIF = SMAT(28) MAT03970
ISPE=0 MAT03980
ISTART=0 MAT03990
ELB=SE(79) MAT04000
SE=99+1 MAT04010
17=11+NSEG*12 MAT04020
13=12+(NSEG+1)*6 MAT04030
14=13+NSEG MAT04040
15=14+2 MAT04050
16=15+2 MAT04060
17=16+NSEG*NUMP MAT04070
18=17+NSEG*NUMP MAT04080
19=18+NUMP MAT04090
110=19+NUMP MAT04100
111=110+NSEG*2+3 MAT04110
112=111+NSEG MAT04120
113=112+(NSEG+1)*6 MAT04130
114=113+(NSEG+1)*6 MAT04140
115=114+(NSEG+1)*6 MAT04150
116=115+NSEG MAT04160
117=116+NSEG*12 MAT04170
118=117+NSEG*12 MAT04180
119=118+NSEG*12 MAT04190
120=119+NSEG*12 MAT04200
121=120+NSEG*NUMP MAT04210
122=121+NSEG*NUMP MAT04220
123=122+NSEG*NUMP MAT04230
124=123+NUMP MAT04240
125=124+NSEG*9 MAT04250
126=125+NSEG*9 MAT04260
127=126+(NSEG+1)*6**2 MAT04270
128=127+(NSEG+1)*6*(NSEG+1)*6+1)/2 MAT04280
129=128+(NSEG+1)*6 + 1 MAT04290
130=129+144 MAT04300
131=130+144 MAT04310
132=131+(NSEG+1)*6 MAT04320
133=132+(NSEG+1)*6**2 MAT04330
134=133+NSEG*144 MAT04340
135=134+2 MAT04350
136=135+2 MAT04360
137=136+(NSEG+1)*6 MAT04370
138=137+(NSEG+1)*6 MAT04380
139=138+(NSEG+1)*6 MAT04390
140=139+(NSEG+1)*6**2 MAT04400
141=140+NUMP MAT04410
142=141+NSEG*NUMP MAT04420
143=142+NUMP MAT04430
144=143+NUMP MAT04440
145=144+NSEG*NUMP MAT04450
C
DO 210 I=1,78 MAT04500
210 SE(I)=0 MAT04510
ELB=SE( 79) MAT04520
SE( 99)=0 MAT04530
C-----CALIBRATE TWO-END DISPLACEMENT INCREMENT BEFORE CALL HYST12 MAT04540
DO 66 I=1,10,3 MAT04550
SMAT(I3+I-1)=D(2+I)-DL(2+I-1) MAT04560
SMAT(I3+I)=D(3+I)-DL(3+I-1) MAT04570
SMAT(I3+I+1)=D(1+I)-DL(1+I-1) MAT04580
C
66 CONTINUE MAT04590
C DO 66 I=1,12 MAT04600
C 66 SMAT(I3+I-1)=D(I)-DL(I) MAT04610
MAT04620
MAT04630
MAT04640
MAT04650
MAT04660

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MOPT=IOPT
CALL HYST12( MOPT , NSEG , TS , EM , LIBN , HH , UU , MAT04670
4      NN , Z , INEB , INEH , ST , IREV1 , IREV2 , MAT04680
4      IREV3 , IREV4 , IECCOP , ECCXO , ECCYO , NUMP , MAT04700
4      ELS , SMALL , RATIXO , RATIFO , TOTL , IMD , MAT04710
4      ISPE , DELTLP2 , S2 , SP , ISTART , BUG , MAT04720
4      IELNO , ISTIF , IAUTO , IMATER , MAT04730
4      RATIO3 , IR , G , QRNEE , RNEEE , MAT04740
C
4  SMAT(11),SMAT(12),SMAT(13),SMAT(14),SMAT(15),SMAT(16),SMAT(17),
4  SMAT(18),SMAT(19),SMAT(10),SMAT(11),SMAT(12),SMAT(13),
4  SMAT(14),SMAT(15),SMAT(16),SMAT(17),SMAT(18),SMAT(19),
4  SMAT(120),SMAT(121),SMAT(122),SMAT(123),SMAT(124),SMAT(125),
4  SMAT(126),SMAT(127),SMAT(128),SMAT(129),SMAT(130),SMAT(131),
4  SMAT(132),SMAT(133),SMAT(134),SMAT(135),SMAT(136),SMAT(137),
4  SMAT(138),SMAT(139),SMAT(140),SMAT(141),SMAT(142),SMAT(143),
4  SMAT(144),SMAT(145),SMAT(146),SMAT(147),SMAT(148),SMAT(149),
4  SMAT(150),SMAT(151),SE(98) )
C
K=0
DO 60 I=1,12
DO 60 J=1,12
K=K+1
60  EASAT(I,J)=SMAT(I29+K-1)
C
-----CONVERT STABILITY ELEMENT TWO-END STIFFNESS INTO STANDARD DOP
C
DIRECTION
DO 68 I=1,12
DO 68 J=1,12
68  A(I,J)=0
C
DO 69 I=1,4
A(1+I-1)*3,3+(I-1)*3)=1
A(2+I-1)*3,1+(I-1)*3)=1
A(3+I-1)*3,2+(I-1)*3)=1
69  CONTINUE
C
DO 90 I=1,12
DO 90 J=1,12
SAT(I,J)=0
DO 90 K=1,12
90  SAT(I,J)=SAT(I,J)+EASAT(I,K)*A(J,K)
C
DO 91 I=1,12
DO 91 J=1,12
EASAT(I,J)=0
DO 91 K=1,12
91  EASAT(I,J)=EASAT(I,J)+A(I,K)*SAT(K,J)
C
K=0
DO 80 I=1,12
DO 80 J=1,12
K=K+1
80  SE(K)=EASAT(I,J)
SE( 79)=ELS
SE( 80)=IMD
SE( 81)=ISPE
SE( 82)=DELTLP2
SE( 83)=S2
SE( 84)=SP
SE( 85)=ISTART
SE( 86)=RNEEE
C
RETURN
----- GET STIFFNESS TERMS -----
300 CONTINUE
C
ELS = SE( 79)
IMD = SE( 80)
ISPE = SE( 81)
DELTLP2 = SE( 82)
S2 = SE( 83)
SP = SE( 84)
ISTART = SE( 85)
RNEEE = SE( 86)
C
C-- CALL HYST12 TO CALC NEW STIFFNESS STIP
NSEG = SMAT( 1)
TS = SMAT( 2)
EM = SMAT( 3)
LIBN = SMAT( 4)
HH = SMAT( 5)
UU = SMAT( 6)
NN = SMAT( 7)
Z = SMAT( 8)
INEB = SMAT( 9)
INEH = SMAT(10)
ST = SMAT(11)
IREV1 = SMAT(12)
IREV2 = SMAT(13)
IREV3 = SMAT(14)
IREV4 = SMAT(15)
IECCOP = SMAT(16)
NUMP = SMAT(17)
SMALL = SMAT(18)
RATIXO = SMAT(19)
RATIFO = SMAT(20)
TOTL = SMAT(21)
IAUTO = SMAT(22)
IMATER = SMAT(23)
RATIO3 = SMAT(24)
IR = SMAT(25)
G = SMAT(26)
QRNEE = SMAT(27)
ISTIF = SMAT(28)
ISPD=0
EL6=SE(79)
I1=29+1
I2=I1+NSEG+12
I3=I2+(NSEG+1)*6
I4=I3+NSEG
I5=I4+2
I6=I5+2
I7=I6+NSEG+NUMP
I8=I7+NSEG+NUMP
I9=I8+NUMP
I10=I9+NUMP
I11=I10+NSEG+2*3
I12=I11+NSEG
I13=I12+(NSEG+1)*6
I14=I13+(NSEG+1)*6
I15=I14+(NSEG+1)*6
I16=I15+NSEG
I17=I16+NSEG+12
I18=I17+NSEG+12
I19=I18+NSEG+12
I20=I19+NSEG+12
I21=I20+NSEG+NUMP
I22=I21+NSEG+NUMP
I23=I22+NSEG+NUMP
I24=I23+NUMP
I25=I24+NSEG+9
I26=I25+NSEG+9
I27=I26+((NSEG+1)*6)**2
I28=I27+((NSEG+1)*6*(NSEG+1)*6+1)/2
I29=I28+(NSEG+1)*6 + 1
I30=I29+I44
I31=I30+I44
I32=I31+(NSEG+1)*6
I33=I32+(NSEG+1)*6**2
I34=I33+NSEG+I44
I35=I34+2
136=I35+2
137=I36+(NSEG+1)*6
138=I37+(NSEG+1)*6
139=I38+(NSEG+1)*6
140=I39+(NSEG+1)*6**2
141=I40+NUMP
142=I41+NSEG+NUMP
143=I42+NUMP
144=I43+NUMP
145=I44+NSEG+NUMP
146=I45+NSEG+NUMP
147=I46+NSEG+NUMP
148=I47+NSEG+NUMP
149=I48+NSEG+NUMP
150=I49+NSEG+NUMP
151=I50+NSEG+NUMP
C
C-----CALIBRATE TWO-END DISPLACEMENT INCREMENT BEFORE CALL HYST12
DO 46 I=1,10,3
SMAT(I13+I-1)=D(2+I-1)-DL(2+I-1)
SMAT(I13+I) =D(3+I-1)-DL(3+I-1)
SMAT(I13+I+1)=D(1+I-1)-DL(1+I-1)
46  CONTINUE
C
DO 18 I=1,12
C 18  SMAT(I13+I-1)=D(I)-DL(I)
C
MOPT=IOPT
CALL HYST12( MOPT , NSEG , TS , EM , LIBN , HH , UU , MAT04670
4      NN , Z , INEB , INEH , ST , IREV1 , IREV2 , MAT04680
4      IREV3 , IREV4 , IECCOP , ECCXO , ECCYO , NUMP , MAT04700
4      ELS , SMALL , RATIXO , RATIFO , TOTL , IMD , MAT04710
4      ISPE , DELTLP2 , S2 , SP , ISTART , BUG , MAT04720
4      IELNO , ISTIF , IAUTO , IMATER , MAT04730
4      RATIO3 , IR , G , QRNEE , RNEEE , MAT04740
C
4  SMAT(11),SMAT(12),SMAT(13),SMAT(14),SMAT(15),SMAT(16),SMAT(17),
4  SMAT(18),SMAT(19),SMAT(10),SMAT(11),SMAT(12),SMAT(13),
4  SMAT(14),SMAT(15),SMAT(16),SMAT(17),SMAT(18),SMAT(19),
4  SMAT(120),SMAT(121),SMAT(122),SMAT(123),SMAT(124),SMAT(125),
4  SMAT(126),SMAT(127),SMAT(128),SMAT(129),SMAT(130),SMAT(131),
4  SMAT(132),SMAT(133),SMAT(134),SMAT(135),SMAT(136),SMAT(137),
4  SMAT(138),SMAT(139),SMAT(140),SMAT(141),SMAT(142),SMAT(143),
4  SMAT(144),SMAT(145),SMAT(146),SMAT(147),SMAT(148),SMAT(149),
4  SMAT(150),SMAT(151),SE(98) )
C
K=0
DO 65 I=1,12
DO 65 J=1,12
K=K+1
65  EASAT(I,J)=SMAT(I29+K-1)
C
-----CONVERT STABILITY ELEMENT TWO-END STIFFNESS INTO STANDARD DOP
C
DIRECTION
DO 38 I=1,12
DO 38 J=1,12
38  A(I,J)=0
C
DO 39 I=1,4
A(1+I-1)*3,3+(I-1)*3)=1
A(2+I-1)*3,1+(I-1)*3)=1
A(3+I-1)*3,2+(I-1)*3)=1
39  CONTINUE
C
DO 40 I=1,12
DO 40 J=1,12
SAT(I,J)=0
DO 40 K=1,12
40  SAT(I,J)=SAT(I,J)+EASAT(I,K)*A(J,K)
C
DO 41 I=1,12
DO 41 J=1,12
EASAT(I,J)=0
DO 41 K=1,12
41  EASAT(I,J)=EASAT(I,J)+A(I,K)*SAT(K,J)
C
IF( SE(99) .EQ. 1 ) THEN
IF( P(7) .LE. 0 ) THEN
XLL=DL(7)-DL(1)
XCC=D(7)-D(1)
XDD=XCC
IF( XDD .LT. 0 ) THEN
DO 339 J=1,12
EASAT(I,J)=0
EASAT(7,J)=0
EASAT(J,1)=0
EASAT(J,7)=0
339  CONTINUE
EASAT(1,1)=-0.001
EASAT(7,7)=0.001
EASAT(1,7)=-0.001
EASAT(7,1)=-0.001
ENDIF
ENDIF
ENDIF
IF( SE(99) .EQ. 0 ) THEN
IF( SE(98) .GT. 0 ) SE(99)=1
ENDIF
C
K=0
DO 85 I=1,12
DO 85 J=1,12
K=K+1
85  SE(K)=EASAT(I,J)
SE( 79)=ELS
SE( 80)=IMD
SE( 81)=ISPE
SE( 82)=DELTLP2
SE( 83)=S2
SE( 84)=SP
SE( 85)=ISTART
SE( 86)=RNEEE
C
RETURN
END
C
C--- GET ENERGIES, DUCTILITY AND EXCURSION FOR BOTH IOPT=3 AND 4 ----
400 CONTINUE
C
C --- TRANSFER ENERGIES FOR BOTH IOPT=3 AND 4.
EPESE=0
EPSE=0
DAMAGE=0
C
C --- TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT=3 AND 4.
DO 410 I=1,3
DUCT(I)=0
EXCR(I)=0
410  EXCR(I)=0
C
RETURN
C
----- DAMAGE INDEX -----
500 CONTINUE
EPESE=0
EPSE=0
DAMAGE=0
C
RETURN
END
C
C#PROCESS SDUMP OPT(0) GOSTMT XREF MAP
C
C----- DEBUG UNIT (6), TRACE, SUBCHK, INIT, SUBTRACE
C
C----- END DEBUG
C-----
SUBROUTINE MAT13

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```

4 (IOPT, IELNO, ERR, FIRST, PRINT, BUG,
MAT, SE, P, PL, D, DL, V, VL, LELM, LMAT,
SMAT, BESE, EPSE, DUCT, EXCR, DAMAGE)
C
IMPLICIT REAL(A-H,O-S)
LOGICAL ERR, FIRST, PRINT, BUG
LOGICAL STSET
DIMENSION SE(100), SMAT(8), DUCT(3), EXCR(6)
CHARACTER*80 TYPE
C
----- RESERVE STORAGE FOR ELEMENT
LELEM=39
IF (IOPT.EQ.0) RETURN
C
-----
C VARIABLES:
C-----
C----- GLOBAL VARIABLES
C IOPT = 1, INITIALIZE MATERIAL
      = 2, GET STIFFNESS
C RINPUT = INPUT DATA
C SMAT = INTERNAL STORAGE
C SE = OUTPUT STIFFNESS
C-----
GO TO (100,200,300,400,500),IOPT
C
100 CONTINUE
C----- INTERPATE INPUT DATA
E = SMAT(1)
A = SMAT(2)
R = SMAT(3)
YS = SMAT(4)
PT = SMAT(5)
CC = SMAT(6)
DMAX = SMAT(7) : ULTIMATE DISPLACEMENT = DUCMAX*DELY
BDAM = SMAT(8) : BATA PARAMETER
C
READ (5,*) TYPE, (SMAT(I),I=1,4), DUCMAX, BDAM
C
LMAT = 8
PI=3.1415926
SMAT(5)=SMAT(2)*SMAT(4)
SMAT(6)=SQRT(2.*PI*PI*SMAT(1)/SMAT(4))
SMAT(7)=DUCMAX
SMAT(8)=BDAM
C
IF (FIRST .OR. BUG) WRITE (6,191)
WRITE (6,192) MAT, (SMAT(K),K=1,4), DUCMAX, BDAM
C
191 FORMAT (/// ' GOEL HYSTERESIS MODEL FOR BOX LOCAL BUCKLING' //
& ' MAT, ' ,XJ, ' B, ' ,XK, ' A, ' ,XK, ' R, ' ,XK, ' YS, '
& ' ,XK, ' MAX DUC, ' ,XK, ' BETA' )
192 FORMAT (1X,15,6G12.6/)
C
RETURN
C
----- INITIALIZE STIFFNESS TERMS -----
200 CONTINUE
C----- GOEL HYSTERESIS MODEL DATA STORAGE FOR MEMBER -----
SE(1)=K STIFFMEMBER AXIAL STIFFNESS
SE(2)=K-1 EFFECT SLENDER COEFFICIENT
SE(3)=NOCT NO. OF CYCLE
SE(4)=NRULE RULE NUMBER
SE(5)=EL MEMBER LENGTH (TRANSFERRED FROM ELEM8)
SE(6)=IRV CRVREARC=.FALSE. 1:REVRSC=.TRUE.
SE(7)=IDUC 0:HC=-12 1:HC .LT. -12
SE(8)=PSOT
SE(9)=DBOT
SE(10)=PTOP
SE(11)=DTOP
SE(12)=KEQ UNLOADING EQUIVALENT STIFFNESS
SE(13)=DUCT(1)
SE(14)=DUCT(2)
SE(15)=DUCT(3)
SE(16)=EXCR(1)
SE(17)=EXCR(2)
SE(18)=EXCR(3)
SE(19)=EXCR(4)
SE(20)=EXCR(5)
SE(21)=EXCR(6)
SE(22)=DELT YIELDING DISPLACEMENT
SE(23)=BESE(1)
SE(24)=BESE(2)
SE(25)=BESE(3)
SE(26)=EPSE
SE(27)=PSBOLD
SE(28)=CSB
SE(29)=DMAX ULTIMATE DISPLACEMENT
SE(30)=D_MAX MAXIMUM ELEMENT DISPLACEMENT FOR DAMAGE INDEX
SE(31)=P_MAX ELEMENT LOAD CORRESPONDING TO D_MAX
SE(32)=HC CURRENT CONTROL POINT HA
SE(33)=VA CURRENT CONTROL POINT VA
SE(34)=VC CURRENT CONTROL POINT VC
SE(35)=RGN CURRENT RESIDUAL STRAIN VALUE
SE(37)=DENAX CURRENT MAXIMUM AXIAL DISPLACEMENT FOR DUCTILITIES
SE(38)=RGIN CURRENT REGINE SOME NUMBER
SE(39)=PREGIN PREVIOUS REGINE SOME NUMBER
C-----
SE(2)=1
SE(3)=1
SE(4)=1
DO 210 I=6,28
SE(I)=0
210 SE(37)=0
SE(38)=1
SE(39)=1
C
CALL HYST13(0.,0.,0.,0.,V,VL,SE(5),SE(2),
& SMAT(1),SMAT(2),SMAT(3),SMAT(4),SMAT(5),
& SMAT(6),SE(3),SE(4),SE(1),SE(5),SE(7),
& SE(32),SE(33),SE(34),SE(35),SE(36),SE(37),SE(38),SE(39),
& SE(8),SE(9),SE(10),SE(11),BUG,
& SE(12),SE(13),SE(16),SE(22),SE(23),SE(26),SE(27),SE(28) )
SE(28)=SE(22)*SMAT(5)/7
SE(39)=SE(22)*SMAT(7)
SE(30)=0
SE(31)=0
C
RETURN
C----- GET STIFFNESS TERMS -----
300 CONTINUE
C----- CALL HYST13 TO CALC NEW STIFFNESS STIF
CALL HYST13(P,PL,D,DL,V,VL,SE(5),SE(2),
& SMAT(1),SMAT(2),SMAT(3),SMAT(4),SMAT(5),
& SMAT(6),SE(3),SE(4),SE(1),SE(5),SE(7),
& SE(32),SE(33),SE(34),SE(35),SE(36),SE(37),SE(38),SE(39),
& SE(8),SE(9),SE(10),SE(11),BUG,
& SE(12),SE(13),SE(16),SE(22),SE(23),SE(26),SE(27),SE(28) )
C
--- STORE MAXIMUM DISPLACEMENT AND CORRESPONDING LOAD
IF (ABS(D) .GT. ABS(SE(30))) THEN
SE(30)=D
SE(31)=P
ENDIF
C
--- GET ENERGIES, DUCTILITY AND EXCURSION FOR BOTH IOPT=3 AND 4 ---
400 CONTINUE

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MAT00070 C
MAT00080 C --- TRANSFER ENERGIES FOR BOTH IOPT=3 AND 4.
MAT00090 BESE=SE(23)
MAT00100 EPSE=SE(26)
MAT00110 C
MAT00120 C --- TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT=3 AND 4.
MAT00130 DO 410 I=1,3
MAT00140 DUCT(I)=SE(12+I)
MAT00150 EXCR(I+3)=SE(15+I+3)
MAT00160 EXCR(I)=SE(15+I)
MAT00170 C
MAT00180 RETURN
MAT00190 C----- DAMAGE INDEX -----
MAT00200 500 CONTINUE
MAT00210 BESE=SE(23)
MAT00220 EPSE=SE(26)
MAT00230 DMAX=SE(29)
MAT00240 D=SE(30)
MAT00250 P=SE(31)
MAT00260 PT=SMAT(5)
MAT00270 BDAM=SMAT(8)
MAT00280 DAMAGE=ABS(D)/DMAX + BDAM / (PT*DMAX) * EPSE
MAT00290 C
MAT00300 RETURN
MAT00310 END
MAT00320 C----- DBUG UNIT(6),TRACE,SUBCHK,INIT,SUBTRACE -----
MAT00330 DBUG
MAT00340 C-----
MAT00350 SUBROUTINE MAT14
MAT00360 & (IOPT, IELNO, ERR, FIRST, PRINT, BUG,
MAT00370 & MAT, SE, P, PL, D, DL, V, VL, LELM, LMAT,
MAT00380 & SMAT, BESE, EPSE, DUCT, EXCR, DAMAGE)
MAT00390 C
MAT00400 LOGICAL ERR, FIRST, PRINT, BUG
MAT00410 LOGICAL STSET
MAT00420 DIMENSION SE(100), SMAT(40), DUCT(3), EXCR(6)
MAT00430 CHARACTER*80 TYPE
MAT00440 REAL KEY D62, MOD, LWP
MAT00450 IWBGER SEBNO, CYCLEM
MAT00460 REAL K, EP, SUB(4), FUG(4), ELAST
MAT00470 LOGICAL TNPTFG, SRFLAG, POSTFG
MAT00480 C
MAT00490 IF (IOPT.NE.1) LELM=80
MAT00500 IF (IOPT.EQ.0) RETURN
MAT00510 C-----
MAT00520 C VARIABLES:
MAT00530 C-----
MAT00540 C----- GLOBAL VARIABLES
MAT00550 C IOPT = 1, INITIALIZE MATERIAL
MAT00560      = 2, GET STIFFNESS
MAT00570 C RINPUT = INPUT DATA
MAT00580 C SMAT = INTERNAL STORAGE
MAT00590 C SE = OUTPUT STIFFNESS
MAT00600 C-----
MAT00610 C----- SHEAR DATA -----
MAT00620 SMAT(1)=NSEGS, NUMBER OF BACKBONE SEGMENTS
MAT00630 SMAT(2)=CSE, CONSTANT STRAIN ENERGY...
MAT00640 SMAT(3)=H, HEIGHT OF SHEAR WALL
MAT00650 SMAT(4)=WD, WIDTH OF SHEAR WALL
MAT00660 SMAT(4+I)=PP, BACKBONE SHEAR FOR POINT I
MAT00670 SMAT(4+I+NSEG)=DP, BACKBONE DISPLACEMENT FOR POINT I
MAT00680 SMAT(5+I+NSEG)=BDAM, BETA FOR DAMAGE INDEX
MAT00690 SMAT(6+I+NSEG)=HO, HEIGHT OF OPENING
MAT00700 SMAT(7+I+NSEG)=HS, SHEAR SPAN LENGTH
MAT00710 SMAT(8+I+NSEG)=LWP,
MAT00720 LWP=L'/W1
MAT00730 C
MAT00740 C
MAT00750 C
MAT00760 C
MAT00770 C
MAT00780 C
MAT00790 C
MAT00800 C
MAT00810 C
MAT00820 C
MAT00830 C
MAT00840 C
MAT00850 C
MAT00860 C
MAT00870 C
MAT00880 C
MAT00890 C
MAT00900 C
MAT00910 C
MAT00920 C
MAT00930 C
MAT00940 C
MAT00950 C
MAT00960 C
MAT00970 C
MAT00980 C
MAT00990 C
MAT01000 C----- INPUT DATA FOR SHEAR HYSTERESIS MODEL -----
MAT01010 CHAT00700
MAT01020 C NSEG = NUMBER OF BACKBONE SEGMENTS
MAT01030 CHAT00710
MAT01040 C DP = BACKBONE DISPLACEMENT
MAT01050 CHAT00730
MAT01060 C FOR THIS MATERIAL, NSEG SHOULD BE 4.
MAT01070 CHAT00740
MAT01080 READ (3,*) TYPE, NSEG, BDAM
MAT01090 READ (5,*) ( SMAT(4+I), I=1, NSEG)
MAT01100 READ (5,*) ( SMAT(4+I+NSEG), I=1, NSEG)
MAT01110 LELM = 80
MAT01120 LMAT = 28
MAT01130 SMAT(1)=NSEG
MAT01140 SMAT(2)=CSE; CSE= CONSTANT STRAIN ENERGY
MAT01150 SMAT(2)=SMAT(5)*SMAT(9)/2+(SMAT(5)+SMAT(6))*(SMAT(10)-SMAT(9))/2
MAT01160 C TO CORRECT FAILURE DISPLACEMENT
MAT01170 CC IF(SMAT(8).NE.0.) THEN
MAT01180 CC STI=(SMAT(8)-SMAT(7))/(SMAT(12)-SMAT(11))
MAT01190 CC SMAT(12)=SMAT(11)+(0.-SMAT(7))/STI
MAT01200 CC SMAT(8)=0.
MAT01210 C
MAT01220 C THE ARRAT SMAT IN BETWEEN SEES THE READ STATEMENTS ABOVE.
MAT01230 C SMAT(4+I), I=1, NSEG AND SMAT(4+I+NSEG), I=1, NSEG
MAT01240 SMAT(5+I+NSEG)=BDAM
MAT01250 SMAT(6+I+NSEG)=HO
MAT01260 SMAT(7+I+NSEG)=HS
MAT01270 SMAT(8+I+NSEG)=LWP
MAT01280 SMAT(9+I+NSEG)=AD
MAT01290 SMAT(10+I+NSEG)=AV
MAT01300 C
MAT01310 C----- CALC CONSTANT STRAIN ENERGY -----
MAT01320 C D0
MAT01330 C PL=0
MAT01340 C DO 10 I=1, NSEG
MAT01350 P=SMAT(4+I)
MAT01360 D=SMAT(4+I+NSEG)
MAT01370 IF (D.LT.DF) THEN
MAT01380 I=DF/D
MAT01390 CSE=CSE + 0.5*(P+PL)*(D-DL)
MAT01400 PL=P
MAT01410 DL=D
MAT01420 ELSE IF (D.EQ.DL) THEN
MAT01430 GO TO 20
MAT01440 ELSE
MAT01450 PF=PL*(DF-DL)+(P-PL)/(D-DL)
MAT01460 CSE=CSE + 0.5*(PF+PL)*(DF-DL)
MAT01470 GO TO 20
MAT01480 ENDF

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MAT01490
MAT01500
MAT01510
MAT01520
MAT01530
MAT01540
MAT01550
MAT01560
MAT01570
MAT01580
MAT01590
MAT01600
MAT01610
MAT01620
MAT01630
MAT01640
MAT01650
MAT01660
MAT01670
MAT01680
MAT01690
MAT01700
MAT01710
MAT01720
MAT01730
MAT00010
MAT00020
MAT00030
MAT00040
MAT00050
MAT00060
MAT00070
MAT00080
MAT00090
MAT00100
MAT00110
MAT00120
MAT00130
MAT00140
MAT00150
MAT00160
MAT00170
MAT00180
MAT00190
MAT00200
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MAT00220
MAT00230
MAT00240
MAT00250
MAT00260
MAT00270
MAT00280
MAT00290
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MAT00990
MAT01000
MAT01010
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MAT01060
MAT01070
MAT01080
MAT01090
MAT01100
MAT01110
MAT01120
MAT01130
MAT01140
MAT01150
MAT01160
MAT01170

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C 10 CONTINUE
C 20 SMAT(2)=CSE
C
IF (FIRST .OR. BUG) WRITE (6,65)
WRITE (6,67) MAT, BDAM, (SMAT(4+K), SMAT(4+K+NSEG), K=1, NSEG)
WRITE (6,*)
65 FORMAT('/// SHEAR HYSTERESIS MODEL DATA - ',
/ ' FOR WHOLE LENGTH MEMBER'
/ '-----'
/ ' BACKBONE CURVE POINTS - MAT. ',
/ ' 5X, BETA', 6X, ' 3X, STRESS', 8X, ' STRAIN')
67 FORMAT ( '( 28X, I4, 2X, JG15.6) / (49X, ZG15.6) )
C
RETURN
C----- GET STIFFNESS TERMS -----
300 CONTINUE
SHEARI MODEL DATA STORAGE FOR MEMBER ----
C
SE(1)=K
SE(2)=LRULE
SE(3)=KKL
SE(4)=DSINC
SE(5)=DMAX
SE(6)=DM
SE(7)=PM
SE(8)=KP
SE(9)=SOC
SE(10)=SCT
SE(11)=SOT
SE(12)=STU
SE(13)=SUP
SE(14)=SCU
SE(15)=SOB
SE(16)=SOE
SE(17)=FUO(I, I=1,4)
SE(21)=DUO(I, I=1,4)
SE(25)=KBTDS2
SE(26)=SECN0
SE(27)=S2T
SE(28)=DU17
SE(29)=FU17
SE(30)=AREAL
SE(31)=CYCLEN
SE(32)=CSE
SE(33)=FU00
SE(34)=DU00
SE(35)=DKP
SE(36)=TNPTFG
SE(37)=POSTFG
SE(38)=CO1
SE(39)=RAT
SE(40)=RAT1
SE(41)=SQ6
SE(42)=SR
SE(43)=DP20
SE(44)=SRFLAG
--USE = EQUIVALENT UNLOADING STIFFNESS
SE(45)=USE
SE(46)=IOPT2
--IOPT2 = USED IN HYST14 SUBRTN (DNCRUL, DOISSI SUBRTN IN HYST14)
SE(47)=IOPT
SE(48)=SQ6
SE(49)=KLAST
SE(50)=DPTT
SE(51)=DPT
SE(52)=ESE
SE(53)=ESE
SE(54)=ESE
SE(55)=PSE
SE(56)=PSE OLD
SE(57)=EXCR(1)
SE(58)=EXCR(2)
SE(59)=EXCR(3)
SE(60)=SE(51), SE(52) SEE ONE SUBRTN
SE(61)=EXCR(4)
SE(62)=EXCR(5)
SE(63)=FC
SE(64)=FT
SE(65)=FU
SE(66)=FF
SE(67)=DC
SE(68)=DT
SE(69)=DU
SE(70)=DF
SE(71)=DSINCL
SE(72)=UPOS(1)
SE(73)=UPOS(2)
SE(74)=UPOS(3)
SE(75)=UNEG(1)
SE(76)=UNEG(2)
SE(77)=UNEG(3)
SE(78)=LENGTH
SE(79)=WIDTH
SE(80)=SHEAR SPAN LENGTH RATIO SMAT(11+2*NSEG) OR MQD
C
INITIALISE -----
SRFLAG=.FALSE.
POSTFG=.TRUE.
TNPTFG=.FALSE.
AREAL=0.
FP=0.
DP=0.
FM=0.
DM=0.
FPP=0.
DPP=0.
FMAX=0.
DMAX=0.
KKL=1.
DUO=0.
SECN0=0.
CYCLEN=1.
DO J1 L=1,4
FUO(L)=0.
DO J2 L=1,4
DUO(L)=0.
SE(3)=KKL
SE(31)=CYCLEN
NSEG=SMAT(1)
SMAT(3)=SE(78)
SMAT(4)=SE(79)
SMAT(11+2*NSEG)=SMAT(7+2*NSEG)/SMAT(4)
SE(32)=0.5*SMAT(5)+SMAT(9)+
0.5*(SMAT(5)+SMAT(6))*(SMAT(10)-SMAT(9))
SE(80)=SMAT(11+2*NSEG)
SE(32)=SMAT(2)
MQD=SE(80)
C FIND CO1, THIS COEFF. IS OBSERVED FROM EXPERIMENTAL DATA
CO1=0.3
SE(38)=CO1
IF(TNPTFG) SE(36)=1
IF(.NOT. TNPTFG) SE(36)=0
IF(POSTFG) SE(37)=1
IF(.NOT. POSTFG) SE(37)=0
IF(SRFLAG) SE(44)=1
IF(.NOT. SRFLAG) SE(44)=0
C
IF(SMAT(28), EQ. 0) THEN
CALL DPNSTF(SMAT(5), SMAT(6), SMAT(7), SMAT(8),
SMAT(9), SMAT(10), SMAT(11), SMAT(12),
SOC, SCT, SOT, STU, SUP, SCU, SOB, SOE)
C COMPUTE STIFFNESS S(OC)
SOC=FC/DC

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C COMPUTE STIFFNESS S(C1)
  SCY=(FY-FC)/(DY-DC)
C COMPUTE STIFFNESS S(O1)
  SOY=FY/DY
C COMPUTE STIFFNESS S(U)
  STU=(FU-FY)/(DU-DY)
C COMPUTE STIFFNESS S(UF)
  SUF=(FU-FU)/(DU-DU)
C COMPUTE STIFFNESS S(CU)
  SCU=(FU-FC)/(DU-DC)
C COMPUTE STIFFNESS S(OB) ; B IS MIDPT. OF PT. A AND PT. Y
C PT. A (INTERSECTION OF EXTENSION OF OC WITH HORIZONTAL LINE THRU Y
  XA=FY/SOC
  XB=(XA+O1)/2
  SOB=FY/XB
C COMPUTE STIFFNESS S(OE) ; E IS MIDPT. OF PT. D AND PT. U
C PT. D (INTERSECTION OF EXTENSION OF OC WITH HORIZONTAL LINE THRU U
  XD=FU/SOT
  XE=(XD+DU)/2
  SOE=FU/XE
  RETURN
  END
C-----
C      DEBUG UNIT(6),TRACE,SUBCKK,INIT,SUBTRACE
C      END DEBUG
C-----
SUBROUTINE MFORM(IOPT,INMASS,NMASS,MASS,MDM,MD,NDOF,IMD,TITLE,
  & NNODE,MAXNO,NCOS,BUG,IBUG,
  & ID,COSINE,IDOF,CONST,JTFLG,
  & JTCOS,DATA,SMASS,LM,A,S,SAT)
  DIMENSION ID(MAXNO), JTFLG(MAXNO), JTCOS(MAXNO),
  & IDOF(MAXNO,6), COSINE(3,3,NCOS),CONST(MAXNO,6),
  & DIMENSION A(6,6),S(6,6),SAT(6,6),MDM(NDOF+1),MD(7)
  REAL MASS(IMD)
  LOGICAL BTEST
  DIMENSION DATA(12,INMASS),SMASS(21),LM(6)
  CHARACTER*130 ELNO
  CHARACTER*(*) TITLE(2)
  LOGICAL BUG
C-----
C      PRINT HEADER AND INPUT MASS DATA
  IF (IOPT.EQ.1) THEN
    WRITE (6,5) TITLE(1),TITLE(2)
    5 FORMAT ('1 STRUCTURE.....',A,
  & ' SOLUTION.....',A,
  & ' LUMPED NODE MASSES',
  & '
  & '
  & ' 4X, NODES',5X, 'MX',10X, 'MY',10X, 'MZ',
  & ' 9X, 'IX', 9X, 'IY', 9X, 'IZ',
  & ' 9X, 'ITX', 9X, 'ITY', 9X, 'ITZ',5X, ' IGEN INC')
C INPUT NODE, PX, PY, PZ, RX, RT, RZ, RXY, RKX, RYZ, IGEN, INC
  6 READ (5,*) (DATA(I,NMASS),I=1,12)
  NODE = DATA(1,NMASS)
  IGEN = DATA(11,NMASS)
  INC = DATA(12,NMASS)
  IF (NODE.GT.0) THEN
    WRITE (6,7) NODE,(DATA(I,NMASS),I=2,10),IGEN,INC
    IF (NMASS.LT.INMASS) THEN
      NMASS=NMASS+1
      GO TO 6
    ELSE
      NMASS=NMASS-1
    ENDIF
  ELSE
    NMASS=NMASS-1
  ENDIF
  7 FORMAT ('2X,15,1P,9G12.4,216)
  ELSE IF (IOPT.EQ.2) THEN
C-----
C      DO 100 IMASS=1,NMASS
  NODE=DATA(1,IMASS)
  PX = DATA(2,IMASS)
  PY = DATA(3,IMASS)
  PZ = DATA(4,IMASS)
  RX = DATA(5,IMASS)
  RT = DATA(6,IMASS)
  RZ = DATA(7,IMASS)
  RXY = DATA(8,IMASS)
  RKX = DATA(9,IMASS)
  RYZ = DATA(10,IMASS)
  IGEN=DATA(11,IMASS)
  INC = DATA(12,IMASS)
C-----
C      9 JOINT=IQUICK(NODE,IN,NNODE)
  IF (JOINT.LE.0) THEN
    WRITE (6,*) 'NODE #',NODE,
    & ' WAS NOT FOUND, CHECK INPUT. MASS IS OMITTED'
    GO TO 90
  ENDIF
C-----
C      ICOS =JTCOS(JOINT)
C-----
C      SET UP NODE MASS MATRIX IN ITS COORDINATE SYSTEM
C      SET UP TRANSFORMATION MATRIX, A = (CONST)
  DO 10 I=1,6
  DO 10 J=1,6
  10 A(I,J)=0
  20 A(I,1)=CONST(JOINT,1)
  A(4,2)=CONST(JOINT,2)
  A(4,3)=CONST(JOINT,2)
  A(5,1)=CONST(JOINT,3)
  A(5,3)=CONST(JOINT,4)
  A(6,1)=CONST(JOINT,5)
  A(6,2)=CONST(JOINT,6)
C-----
C      SET UP NODE MASS MATRIX
  DO 30 I=1,6
  DO 30 J=I+1,6
  30 S(I,J)=0
C-----
C      S(1,1)=PX
  S(2,2)=PY
  S(3,3)=PZ
  S(4,4)=RX
  S(5,5)=RT
  S(6,6)=RZ
  S(4,5)=RXY
  S(4,6)=RXX
  S(5,6)=RZY
C-----
C      DO 40 I=1,6
  DO 40 J=I+1,6
  40 S(J,I)=S(I,J)
C-----
C      TRANSFORM MASS MATRIX TO ACCOUNT FOR CONSTRAINTS
  DO 50 I=1,6
  DO 50 J=1,6
  SAT(I,J)=0
  DO 50 K=1,6
  50 SAT(I,J)=SAT(I,J) + S(I,K)*A(J,K)
C-----
C      DO 60 I=1,6
  DO 60 J=1,6
  S(I,J)=0
  DO 60 K=1,6
  60 S(I,J)=S(I,J) + A(I,K)*SAT(K,J)
C-----
C      TRANSFER MASS MATRIX TO HALF STORAGE MODE
  IJ=0
  DO 70 J=1,6
  DO 70 I=J+1,6
  IJ=IJ+1
  70 SMASS(IJ)=S(I,J)
C-----
C      INITIALIZE THE DIAGONAL MASS MATRIX
  DO 80 I=1,6
  LM(I)=IDOF(JOINT,I)
C-----
C      IF (IOPT.EQ.1) THEN
C-----
C      CALL SKTLIN TO RESERVE SPACE FOR THIS NODE'S MASS
  IF (BUG) THEN
    WRITE (ELNO,915) NODE,LM(I),I=1,6
    CALL LMATRX(SMASS,MD,6,21,ELNO)
  ENDIF
  CALL SKTLIN(6,2,NDOF,IBUG,TITLE,MDM,6,
  & LM,SMASS,MD,'GLOBAL MASS')
  ELSE IF (IOPT.EQ.2) THEN
  IF (BUG) THEN
    WRITE (ELNO,915) NODE,LM(I),I=1,6
    915 FORMAT ('MASS AT NODE #',I6,' LM',6I6)
    CALL LMATRX(SMASS,MD,6,21,ELNO)
  ENDIF
  CALL FORML(MASS,MDM,NDOF,IMD,SMASS,MD,6,21,LM,1,0)
  90 CONTINUE
  IF (IGEN.GT.0) THEN
    IGEN=IGEN-1
    NODE=NODE+INC
    GO TO 9
  ENDIF
  100 CONTINUE
  RETURN
  END
#PROCESS SDUMP OPT(3) GOSTMT XREP
C-----
C      DEBUG UNIT(6),TRACE,SUBCKK,INIT,SUBTRACE
C      END DEBUG
C-----
C      MATRIX SOLUTION OF EQUATION AX=P, WHERE THE UPPER TRIANGULAR MATRIX
  OF THE MATRIX A IS STORED BY SKTLINES, P IS STORED AS
  C A FULL MATRIX. THE MATRIX P IS REPLACED BY X AFTER BACK
  C SUBSTITUTION IS COMPLETE. BOTH A AND P ARE ALTERED....
  C GUTAN OPTION: IF GUTAN IS .TRUE, MASS MATRIX IS ALSO CONDENSED
C-----
C      SUBROUTINE MSOLL(IOPT,PRINT,N,IND,LRW,NLC,L1,L2,MD,A,P,FACTOR,
  & GUTAN,MASS,MDMASS,IMDMAS,
  & KGCND,K,MDKG,IMDKG,ABORT)
  DIMENSION A(LMD),FACTOR(N),P(LRW,NLC),MD(N+1)
  DIMENSION MDMASS(N+1),MDKG(N+1)
  LOGICAL BTEST,ABORT
  REAL MASS(IMDMAS),KG(IMDKG)
  LOGICAL GUTAN,PRINT,KGCND
C-----
C      IJ(I,J)=MD(J)+J-I
  IJ(I,J)=MDMASS(J)+J-I
  IJ(K,I,J)=MDKG(J)+J-I
C-----
C      MATRIX STORAGE SCHEME
C-----
C      THE MATRIX A IS BANDED AND SYMMETRIC. THUS ONLY THE SKYLINE ABOVE
  C AND THE MAIN DIAGONAL NEEDS TO BE STORED.
C-----
C      THE MATRIX A IS STORED IN A LINEAR ARRAY BY COLUMNS. THE VALUE ON
  C THE MAIN DIAGONAL IS STORED IN THE LINEAR ARRAY FIRST.
C-----
C      ADDRESSES OF THE LINEAR ARRAY ELEMENTS CONTAINING THE MAIN DIAGONAL
  C TERMS ARE STORED IN MD.
C-----
C      EXAMPLES LET B BE THE 5X5 FULL SYMMETRIC MATRIX BELOW...
  B =
  1 2 3 4 5
  4 7 10 11 12
  6 9 12 13 14
  8 11 14 17 18
  10 13 16 19 22
  STYM.
  MD = 1, 2, 4, 6, 8, 13
C-----
C      THUS, B(1,3)=A(MD(3)) = A(4)
  B(I,J)=A(MD(J)+J-I) IF J>I AND (MD(J)+J-I)<MD(J+1)
  =0 IF J>I AND (MD(J)+J-I)>MD(J+1)
C-----
C      MATRIX P IS NOT SYMMETRIC, AND IS STORED AS A FULL MATRIX
C-----
C      VARIABLE TABLE:
C-----
C      IOPT = OPTION NUMBER
C      N = NUMBER OF DEGREES OF FREEDOM OF A MATRIX
C      NLC = NUMBER OF LOAD CASES FOR THE P MATRIX
C      L1 = FIRST ROW TO BE WORKED WITH
C      L2 = LAST ROW TO BE WORKED WITH
C      MD = ADDRESS OF THE MAIN DIAGONAL TERMS OF A
C      P = LADD MATRIX ON INPUT, SOLN (X) OF AX=P ON COMPLETION OF
C-----
C      FACTOR= TEMPORARY STORAGE MATRIX
  GUTAN = GUTAN REDUCTION FLAG FOR MASS MATRIX
  MASS = MASS MATRIX
  MDMASS = MASS MATRIX ADDRESSES OF MAIN DIAGONAL ELEMENTS
  KGCND= FLAG FOR CONDENSING OUT KG
  KG = GEOMETRIC STIFFNESS MATRIX
  MDKG = KG MATRIX ADDRESSES OF MAIN DIAGONAL ELEMENTS
  L = ROW NUMBER FOR ELIMINATION
  I = ROW NUMBER
  J = COLUMN NUMBER
C-----
C      L1=MAX(L1,1)
  L2=MIN(L2,N)
C-----
C      GO TO (10,110,210,310),IOPT
  10 CONTINUE
C-----
C      IOPT=1, REDUCE A AND P
C-----
C      GAUSSIAN ELIMINATION IS USED TO REDUCE THE A AND P MATRICES
  FROM ROW L1 TO ROW L2. IF L1 IS NOT 1, IT IS ASSUMED THAT
  C A AND P ARE ALLREADY REDUCED FROM 1 TO L1...
C-----
C      IF (PRINT) THEN
  WRITE (6,500) 'IOPT=1, REDUCE STIFFNESS AND LOADS',L1,L2,N
  CALL LMATRX(A,MD,N,IND,'INPUT STIFFNESS')
  CALL WMATRX(P,N,NLC,'INPUT LOAD')
  IF (GUTAN) CALL LMATRX(MASS,MDMASS,N,IMDMAS,'INPUT MASS')
  IF (KGCND) CALL LMATRX(KG,MDKG,N,IMDKG,'INPUT KG')
  WRITE (6,*) 'MD-----'
  WRITE (6,*) (MD(I),I=1,N+1)
  IF (GUTAN) WRITE (6,*) 'MDMASS-----'
  IF (GUTAN) WRITE (6,*) (MDMASS(I),I=1,N+1)
  IF (KGCND) WRITE (6,*) 'MDKG-----'
  IF (KGCND) WRITE (6,*) (MDKG(I),I=1,N+1)
  ENDIF
  DO 100 L=L1,L2
  IF (L.EQ.N) GO TO 100
C-----
C      TEST FOR ZERO PIVOT.....
  IF (A(MD(L),EQ.0) THEN
    WRITE (6,200) L,MD(L),A(MD(L))
    CALL ERRTRA
    STOP '*** ABNORMAL END IN SUBROUTINE MSOLL ***'
    RETURN .TRUE.
  ENDIF
  ENDIF

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C      END DEBUG                                N0000040                                N0001460
SUBROUTINE NODDOP(NDOF,NCON,NFRSE,NREST,MAXNOD,NNODE,NCDS, N0000050                                N0001470
& SCALE,NSUPT,NCONST, N0000060                                N0001480
& ID,DOF,INDEX,COORD,COSINE,CONST, N0000070                                N0001490
& JTFLG,JTCOS,BUG,MSTRJT) N0000080                                N0001500
C      DIMENSION COORD(MAXNOD,3),ID(MAXNOD),INDEX(MAXNOD),JFM(6), N0000090                                N0001510
& IDOF(MAXNOD,6),COSINE(3,3,NCDS),CONST(MAXNOD,6), N0000100                                N0001520
& JTFLG(MAXNOD),JTCOS(MAXNOD),MSTRJT(MAXNOD,6) N0000110                                N0001530
C      DIMENSION VX(3),VT(3),VZ(3) N0000120                                N0001540
LOGICAL FLAG,PT,BUG,TBSET,BTSET N0000130                                N0001550
CHARACTER*15 TYPE N0000140                                N0001560
CHARACTER*2 CX(6) N0000150                                N0001570
C----- INPUT NODE DOF'S TO BE CONDENSED OUT... N0000160                                N0001580
I=0 N0000170                                N0001590
FLAG=.TRUE. N0000180                                N0001600
90 I=I+1 N0000190                                N0001610
IF (I.LE.NCON) THEN N0000200                                N0001620
IF (FLAG.AND.BUG) N0000210                                N0001630
& WRITE (6,1000) ' NODE DOF'S TO BE CONDENSED OUT' N0000220                                N0001640
FLAG=.FALSE. N0000230                                N0001650
95 FLAG=.FALSE. N0000240                                N0001660
DO 100 J=1,NNODE N0000250                                N0001670
PT=.FALSE. N0000260                                N0001680
DO 99 II=1,6 N0000270                                N0001690
IF (JFM(II).NE.0.AND. .NOT.BTSET(JTFLG(J),II-1))THEN N0000280                                N0001700
JTFLG(J)=IBSET(JTFLG(J),II+5) N0000290                                N0001710
JFM(II)=JFM(II) N0000300                                N0001720
PT=.TRUE. N0000310                                N0001730
ELSE N0000320                                N0001740
JFM(II)=0 N0000330                                N0001750
ENDIF N0000340                                N0001760
CONTINUE N0000350                                N0001770
IF (PT.AND.BUG) WRITE (6,80) ID(J),JFM N0000360                                N0001780
ELSE N0000370                                N0001790
J=IQUICK(NODE,ID,NNODE) N0000380                                N0001800
IF (I.EQ.0) THEN N0000390                                N0001810
IF (BUG) WRITE (6,110) NODE N0000400                                N0001820
ELSE N0000410                                N0001830
DO 101 II=1,6 N0000420                                N0001840
IF (JFM(II).NE.0.AND. .NOT.BTSET(JTFLG(J),II-1))THEN N0000430                                N0001850
JTFLG(J)=IBSET(JTFLG(J),II+5) N0000440                                N0001860
JFM(II)=JFM(II) N0000450                                N0001870
ELSE N0000460                                N0001880
JFM(II)=0 N0000470                                N0001890
ENDIF N0000480                                N0001900
CONTINUE N0000490                                N0001910
IF (PT.AND.BUG) WRITE (6,80) ID(J),JFM N0000500                                N0001920
ENDIF N0000510                                N0001930
IF (IGEN.GT.0) THEN N0000520                                N0001940
NODE=NODE+NODINC N0000530                                N0001950
IGEN=IGEN-1 N0000540                                N0001960
GO TO 95 N0000550                                N0001970
ENDIF N0000560                                N0001980
ENDIF N0000570                                N0001990
GO TO 90 N0000580                                N0002000
ENDIF N0000590                                N0002010
110 FORMAT (' NODE#',I5,' NOT FOUND, DOF ARE NOT CONDENSED...') N0000600                                N0002020
CALL PBIT(NNODE,JTFLG) N0000610                                N0002030
C----- INPUT CONSTRAINTS... N0000620                                N0002040
I=0 N0000630                                N0002050
FLAG=.TRUE. N0000640                                N0002060
400 I=I+1 N0000650                                N0002070
IF (I.LE.NCONST) THEN N0000660                                N0002080
IF (FLAG) WRITE (6,1000) ' NODE CONSTRAINTS ' N0000670                                N0002090
READ (5,*) ITYPE,MASTER,NODE,IGEN,NODINC N0000680                                N0002100
J=IQUICK(NODE,ID,NNODE) N0000690                                N0002110
IF (J.EQ.0) THEN N0000700                                N0002120
WRITE (6,420) NODE N0000710                                N0002130
ELSE N0000720                                N0002140
IF (ITYPE.EQ.0) THEN N0000730                                N0002150
TYPE='3D-RIGID BODY' N0000740                                N0002160
DO 415 K=1,6 N0000750                                N0002170
IF (.NOT.BTSET(JTFLG(J),K-1)) THEN N0000760                                N0002180
JTFLG(J)=IBSET(JTFLG(J),K+1) N0000770                                N0002190
MSTRJT(J,K)=IQUICK(MASTER,ID,NNODE) N0000780                                N0002200
ELSE N0000790                                N0002210
WRITE(6,430) NODE,K N0000800                                N0002220
ENDIF N0000810                                N0002230
CONTINUE N0000820                                N0002240
ELSE IF (ITYPE.EQ.1) THEN N0000830                                N0002250
TYPE='X1-PLANE' N0000840                                N0002260
K=1 N0000850                                N0002270
IF (.NOT.BTSET(JTFLG(J),K-1)) THEN N0000860                                N0002280
JTFLG(J)=IBSET(JTFLG(J),K+1) N0000870                                N0002290
MSTRJT(J,K)=IQUICK(MASTER,ID,NNODE) N0000880                                N0002300
ELSE N0000890                                N0002310
WRITE(6,430) NODE,K N0000900                                N0002320
ENDIF N0000910                                N0002330
K=2 N0000920                                N0002340
IF (.NOT.BTSET(JTFLG(J),K-1)) THEN N0000930                                N0002350
JTFLG(J)=IBSET(JTFLG(J),K+1) N0000940                                N0002360
MSTRJT(J,K)=IQUICK(MASTER,ID,NNODE) N0000950                                N0002370
ELSE N0000960                                N0002380
WRITE(6,430) NODE,K N0000970                                N0002390
ENDIF N0000980                                N0002400
K=6 N0000990                                N0002410
IF (.NOT.BTSET(JTFLG(J),K-1)) THEN N0001000                                N0002420
JTFLG(J)=IBSET(JTFLG(J),K+1) N0001010                                N0002430
MSTRJT(J,K)=IQUICK(MASTER,ID,NNODE) N0001020                                N0002440
ELSE N0001030                                N0002450
WRITE(6,430) NODE,K N0001040                                N0002460
ENDIF N0001050                                N0002470
ELSE N0001060                                N0002480
WRITE(6,440) ITYPE,MASTER,NODE N0001070                                N0002490
ENDIF N0001080                                N0002500
WRITE (6,450) TYPE,MASTER,NODE N0001090                                N0002510
ENDIF N0001100                                N0002520
WRITE (6,450) TYPE,MASTER,NODE N0001110                                N0002530
ENDIF N0001120                                N0002540
IF (IGEN.GT.0) THEN N0001130                                N0002550
NODE=NODE+NODINC N0001140                                N0002560
IGEN=IGEN-1 N0001150                                N0002570
GO TO 410 N0001160                                N0002580
ENDIF N0001170                                N0002590
GO TO 400 N0001180                                N0002600
ENDIF N0001190                                N0002610
420 FORMAT (' NODE#',I5,' IS NOT FOUND, CONSTRAINT IS IGNORED') N0001200                                N0002620
430 FORMAT (' NODE#',I5,' IS RESTRAINED, CONSTRAINT',I2, N0001210                                N0002630
& ' IS IGNORED...') N0001220                                N0002640
440 FORMAT (' INVALID CONSTRAINT TYPE',I3, N0001230                                N0002650
& ' MASTER',I6,' SLAVE',I6) N0001240                                N0002660
450 FORMAT ('X',A,' CONSTRAINT', N0001250                                N0002670
& ' MASTER',I6,' SLAVE',I6) N0001260                                N0002680
C      IF (BUG) WRITE (6,1000) ' JOINT RESTRAINED CODES...' N0001270                                N0002690
IF (BUG) CALL PBIT(NNODE,JTFLG,ID) N0001280                                N0002700
C----- DETERMINE THE DEGREES OF FREEDOM... N0001290                                N0002710
C----- ZERO CONSTRAINT MATRIX WHICH WILL HOLD THE MOMENT N0001300                                N0002720
C----- TRANSFORMATION FOR CONSTRAINED DOF N0001310                                N0002730
DO 499 I=1,NNODE N0001320                                N0002740
DO 500 IP=1,6 N0001330                                N0002750
JTF=JTFLG(IP) N0001340                                N0002760
IF (BTSET(JT,J+5) .AND. .NOT.BTSET(JT,J+11)) THEN N0001350                                N0002770
DO 499 I=1,NNODE N0001360                                N0002780
DO 499 J=1,6 N0001370                                N0002790
NDOF=0 N0001380                                N0002800
ENDIF N0001390                                N0002810
CONTINUE N0001400                                N0002820
IF (BUG) WRITE (6,80) ID(J),JFM N0001410                                N0002830
ENDIF N0001420                                N0002840
IF (IGEN.GT.0) THEN N0001430                                N0002850
NODE=NODE+NODINC N0001440                                N0002860
IGEN=IGEN-1 N0001450                                N0002870

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      NDOF=NDOF+1
      IDOF(I,J)=NDOF
      NCON=NDOF
500 CONTINUE
      NCON=NDOF
C-----UNCONSTRAINED AND UNRESTRAINED DEGREES OF FREEDOM
      DO 510 I=1,NNODE
      DO 510 J=1,6
      NDOF=NDOF+1
      IF (.NOT.(BTST(JTFLG(I),J)-1).OR. BTST(JTFLG(I),J+5).OR.
      & BTST(JTFLG(I),J+11)) ) THEN
      IF (IDOF(I,J).EQ. 0) THEN
      NDOF=NDOF+1
      IDOF(I,J)=NDOF
510 CONTINUE
      NFREE=NDOF-NCON
C-----RESTRAINED DEGREES OF FREEDOM
      DO 530 I=1,NNODE
      DO 530 J=1,6
      IF (BTST(JTFLG(I),J)-1).AND. .NOT.BTST(JTFLG(I),J+7)
      & .AND. .NOT.BTST(JTFLG(I),J+11)) THEN
      NDOF=NDOF+1
      IDOF(I,J)=NDOF
530 CONTINUE
C-----RESTRAINED DEGREES OF FREEDOM
      DO 535 I=1,NNODE
      DO 535 J=1,6
      IF (BTST(JTFLG(I),J+7).AND. .NOT.BTST(JTFLG(I),J+11)) THEN
      NDOF=NDOF+1
      IDOF(I,J)=NDOF
535 CONTINUE
      NREST=NDOF-NFREE-NCON
C-----CONSTRAINED DEGREES OF FREEDOM
      I = NODE ID OF SLAVE NODE
      II = NODE ID OF MASTER NODE
      DO 520 I=1,NNODE
      DO 520 J=1,6
      ... CHECK DOF FOR CONSTRAINT ...
      IF (BTST(JTFLG(I),J+11)) THEN
      ... DETERMINE MASTER NODE AND DOF # ...
      II=I
      NDOF=ID(II)
      I=MASTER(II,J)
      IF (II.EQ.0) THEN
      WRITE (6,525) NODE, ID(I),J
      IDOF(I,J)=0
      GO TO 520
      ENDIF
      IF (BTST(JTFLG(II),J+11)) GO TO 515
      IDOF(I,J)=IDOF(II,J)
C-----CHECK FOR DIFFERENT JOINT COORDINATE SYSTEMS...
      IF (JTCS(I).NE. JTCS(II)) THEN
      WRITE (6,411) ID(I),ID(II)
      FORMAT(' CONFLICTING COSINE ID NUMBERS, MASTER=',
      & ' SLAVE=',I6,' CORRECT INPUT & RETURN...')
      ENDIF
C-----CALCULATE THE CONSTRAINT MATRIX...
      IF (J.EQ.4) THEN
      XG=COORD(I,1)-COORD(II,1)
      YG=COORD(I,2)-COORD(II,2)
      ZG=COORD(I,3)-COORD(II,3)
      N = JTCS(I)
      XMS=COSINE(1,N)*YG+COSINE(1,2)*YG+COSINE(1,3)*N*YG
      YMS=COSINE(2,N)*YG+COSINE(2,2)*YG+COSINE(2,3)*N*YG
      ZMS=COSINE(3,N)*YG+COSINE(3,2)*YG+COSINE(3,3)*N*YG
      ENDIF
      IF (J.EQ.4) THEN
      CONST(I,1)=-ZMS
      CONST(I,2)= YMS
      ELSE IF (J.EQ.5) THEN
      CONST(I,3)= ZMS
      CONST(I,4)=-YMS
      ELSE IF (J.EQ.6) THEN
      CONST(I,5)= YMS
      CONST(I,6)= XMS
      ENDIF
520 CONTINUE
525 FORMAT (' NODE#',I6,' NOT FOUND WHILE ASSIGNING CONSTRAINED',
      & ' DOF TO NODE#',I6,' DOF#',I2,' --- DOF IS DELETED ')
C-----PRINT OUT ORDERED NODE INFO, AND DEGREES OF FREEDOM
      WRITE (6,1000) 'NODE COORDINATES AND DEGREES OF FREEDOM'
      WRITE (6,600) NDOF,NCON,NFREE,NREST
600 FORMAT
      & 4X,'TOTAL NUMBER OF DEGREES OF FREEDOM.....',I6,/,
      & 4X,'NUMBER OF DEGREES OF FREEDOM CONDENSED OUT.....',I6,/,
      & 4X,'NUMBER OF FREE DEGREES OF FREEDOM.....',I6,/,
      & 4X,'NUMBER OF RESTRAINED DEGREES OF FREEDOM.....',I6,/,
      & 4X,' NODE COS#',4X,'X-COORD',8X,'Y-COORD',8X,'Z-COORD',4X,
      & 4X,'FX',6X,'FY',6X,'FZ',6X,'MX',6X,'MY',6X,'MZ')
      DO 610 I=1,NNODE
      IF (BTST(JTFLG(I),J)-1) THEN
      CX(J)='-R'
      ELSE IF (BTST(JTFLG(I),J+11)) THEN
      CX(J)='-C'
      ELSE
      CX(J)=' '
      ENDIF
605 CONTINUE
610 WRITE(6,620) ID(I),JTCS(I),(COORD(I,J),J=1,3),
      & (IDOF(I,K),K=1,6)
620 FORMAT (4X,I6,1P,15,3G15.5,OP,6(16,A2))
      WRITE(6,625)
625 FORMAT('R',NOTE: R = RESTRAINED DEGREE OF FREEDOM,/,
      & 'C = CONSTRAINED DEGREE OF FREEDOM')
C-----PRINT OUT CONSTRAINT MATRIX
      IF (FLAG) THEN
      FLAG=.TRUE.
      DO 540 I=1,NNODE
      TEST=.FALSE.
      DO 536 J=1,6
      TEST=TEST.OR. BTST(JTFLG(I),J+11)
      IF (TEST) THEN
      IF (FLAG) WRITE (6,1000) 'NODE CONSTRAINT MATRIX'
      FLAG=.FALSE.
      WRITE(6,550) ID(I),(CONST(I,J),J=1,6)
      ENDIF
540 CONTINUE
550 FORMAT(4X,I6,1P,
      & ' T12',G12.5,' T13',G12.5,
      & ' T21',G12.5,' T23',G12.5,
      & ' T31',G12.5,' T32',G12.5)
      ENDIF
C-----PRINT OUT DIRECTION COSINES
      WRITE (6,1000) 'DIRECTION COSINES ...'
      DO 560 ICOS=1,NCOS
      WRITE (6,56) ICOS,' VX',(COSINE(1,J,ICOS),J=1,3),

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IF (IOPT.NE.4) WRITE (6,321) L,STEPID
IF (IOPT.EQ.4) WRITE (6,421)
ENDIF
IF (IOPT.EQ.3) THEN
DO 190 I=1,6
190 SUM(I)=0
ENDIF
C
IF (.NOT.(NCOS.EQ.1 .AND. COSINE(1,1,1).EQ.1 .AND.
COSINE(2,2,1).EQ.1 .AND. COSINE(3,3,1).EQ.1)
& .AND. IOPT.EQ.4) GO TO 279
C
DO 200 I=1,NNODE
C
C ..... GET REACTNS IN JOINT COORDINATE SYSTEM
FLAG=.FALSE.
DO 209 J=1,6
I3=IDOF(I,J)
IF (I3.LE.NF .OR. BTEST(JTFLG(I),J+1)) THEN
F(J)=0
ELSE
F(J)=FORCE(IJ,L)
FLAG=.TRUE.
ENDIF
209 CONTINUE
IF (.NOT.FLAG) GO TO 200
C ..... TRANSFER REACTNS TO GLOBAL COORDINATE SYSTEM
DO 229 I=1,3
I2=I+3
G(I1)=G(I)
G(I2)=0
DO 219 J1=1,3
J2=J1+3
G(I1)+G(I1)+COSINE(J1,11,JTICOS(I))*F(J1)
219 G(I2)+G(I2)+COSINE(J1,11,JTICOS(I))*F(J2)
IF (PRINT) THEN
WRITE (CD(I1),210) G(I1)
WRITE (CD(I2),210) G(I2)
ENDIF
229 CONTINUE
C ..... SUM REACTIONS .....
IF (IOPT.EQ.3) THEN
DO 240 I1=1,6
240 SUM(I1)=SUM(I1)+G(I1)
SUM(4)=SUM(4)+G(2)*COORD(1,3)+G(3)*COORD(1,2)
SUM(5)=SUM(5)+G(1)*COORD(1,3)+G(3)*COORD(1,1)
SUM(6)=SUM(6)+G(1)*COORD(1,2)+G(2)*COORD(1,1)
ENDIF
C ..... PRINT GLOBAL REACTIONS .....
NREACT=NREACT+1
IF (PRINT) WRITE (6,220) ID(I),(CD(K),K=1,6)
200 CONTINUE
IF (IOPT.EQ.3 .AND. PRINT) WRITE (6,230) SUM
279 IF (IOPT.EQ.4 .AND. PRINT) WRITE (6,231) SUM
C ..... PRINT LOCAL JOINT COORDINATE SYSTEMS REACTIONS .....
IF (.NOT. PRINT) GO TO 300
IF (NCOS.EQ.1 .AND. COSINE(1,1,1).EQ.1 .AND. COSINE(2,2,1).EQ.1
& .AND. COSINE(3,3,1).EQ.1) GO TO 300
IF (HEAD.AND. NREACT.GT.20) WRITE (6,320) TITLE(1),TITLE(2)
IF (IOPT.NE.4) WRITE (6,322) L,STEPID
IF (IOPT.EQ.4) WRITE (6,422)
DO 290 I=1,NNODE
C ..... GET REACTNS IN JOINT COORDINATE SYSTEM
FLAG=.FALSE.
DO 280 J=1,6
I3=IDOF(I,J)
IF (I3.LE.NF .OR. BTEST(JTFLG(I),J+1)) THEN
CD(J)=I
ELSE
WRITE (CD(J),210) FORCE(IJ,L)
FLAG=.TRUE.
ENDIF
280 CONTINUE
IF (.NOT.FLAG) GO TO 290
C ..... PRINT REACTNS IN LOCAL COORDINATE SYSTEM .....
WRITE (6,221) ID(I),(CD(K),K=1,6),JTICOS(I)
290 CONTINUE
300 CONTINUE
IF (IOPT.EQ.4) WRITE (6,231) SUM
300 CONTINUE
C
210 FORMAT (1P,G15.7)
220 FORMAT (5X,I5,6A)
221 FORMAT (5X,I5,6A,I10)
230 FORMAT (' ',90('-'),'')
SUMMATION(6,615.7)
231 FORMAT (' MAX OF ALL',99(' '),)
& GCS SUMM.,6015.7)
&
320 FORMAT ('1 STRUCTURE.....',A,
& SOLUTION.....',A)
321 FORMAT ('// GCS RESTRAINT REACTIONS, LOADING #',15,5X,A//
& *****//
& 6X,'NODE',7X,'FX',13X,'FY',13X,'FZ',13X,'MX',13X,'MY',13X,'MZ'//)
322 FORMAT ('// JCS RESTRAINT REACTIONS, LOADING #',15,5X,A//
& *****//
& 6X,'NODE',7X,'FX',13X,'FY',13X,'FZ',13X,'MX',13X,'MY',13X,'MZ'//
& 6X,'COSINE #')
421 FORMAT ('// MAXIMUM GCS RESTRAINT REACTIONS',
& 5X,'NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY//
& *****//
& 6X,'NODE',7X,'FX',13X,'FY',13X,'FZ',13X,'MX',13X,'MY',13X,'MZ'//)
422 FORMAT ('// MAXIMUM JCS RESTRAINT REACTIONS',
& 5X,'NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY//
& *****//
& 6X,'NODE',7X,'FX',13X,'FY',13X,'FZ',13X,'MX',13X,'MY',13X,'MZ'//
& 6X,'COSINE #')
C
ELSE
WRITE (6,*)'INVALID OPTION IN SUBROUTINE REA_CTN, IOPT=,IOPT
ENDIF
RETURN
END

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REAO1190 C EX = MEMBER END ECCENTRICITY IN THE LOCAL X AXIS. ROT00210
REAO1200 C EY = MEMBER END ECCENTRICITY IN THE LOCAL Y AXIS. ROT00220
REAO1210 C EZ = MEMBER END ECCENTRICITY IN THE LOCAL Z AXIS. ROT00230
REAO1220 C ----- OUTPUT VARIABLES----- ROT00240
REAO1230 C VX = UNIT VECTOR, X-DIRECTION OF ELEMENT COORDINATE SYS ROT00250
REAO1240 C VY = UNIT VECTOR, X-DIRECTION OF ELEMENT COORDINATE SYS ROT00260
REAO1250 C VZ = UNIT VECTOR, X-DIRECTION OF ELEMENT COORDINATE SYS ROT00270
REAO1260 C CT = ROTATED COORDINATE TRANSFORMATION ROT00280
REAO1270 C CTXZ = CONSTRAINT AND MEMBER END DEPTH TRANSFORMATION ROT00290
REAO1280 C
REAO1290 C
REAO1300 C
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REAO1600 C
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ROT02990
ROT03000

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C..... J = THE COLUMN OF THE MEMBER STIFFNESS MATRIX SKT00250 SUMX = SUMX + ADBL SMA00240
C..... IJ = THE ADDRESS OF THE MEMBER STIFFNESS TERM I,J SKT00260 SUMXSQ = SUMXSQ + ADBL**2 SMA00250
C..... LMJ = THE ROW OF THE GLOBAL STIFFNESS MATRIX CORRESPONDING SKT00270 IF (YMX.LT.A(I)) THEN SMA00260
C..... TO ROW I OF THE MEMBER STIFFNESS MATRIX SKT00280 YMX=A(I) SMA00270
C..... LMJ = THE COLUMN OF THE GLOBAL STIFFNESS MATRIX CORRESPONDING SKT00290 T1=T SMA00280
C..... TO COLUMN J OF THE MEMBER STIFFNESS MATRIX SKT00300 ELSE IF (YMX.GT.A(I)) THEN SMA00290
C..... MDIAG(JJ) = THE JJ'TH ELEMENT OF THE GLOBAL SKYLINE MATRIX. SKT00310 YMX=A(I) SMA00300
C..... AT THIS POINT THE GLOBAL SKYLINE MATRIX CONTAINS THE SKT00320 T2=T SMA00310
C..... LOWEST ROW NUMBER THAT CONTAINS A NON-ZERO STIFFNESS SKT00330 T2=DT SMA00320
C..... TERM. SKT00340 T2=DT SMA00330
C SKT00350 40 CONTINUE SMA00340
C DO 250 I=1,NDOP SKT00360 AVE=SUMX/N SMA00350
C LMI=I SKT00370 IF ((N*SUMXSQ-SUMX**2).GT.0) SMA00360
C IF (LMI.EQ.0) GO TO 250 SKT00380 IF (STDEV=SQRT((N*SUMXSQ-SUMX**2)/(N-1))) SMA00370
C DO 240 J=1,NDOP SKT00390 RMS=SQRT(SUMXSQ/N) SMA00380
C LMJ=LM(I) SKT00400 RETURN SMA00390
C IF (LMI.LE.0) GO TO 240 SKT00410 END SMA00400
C I=MDI(J)+J-1 SKT00420 C----- SOL00010
C IF (SB(IJ).EQ.0 .AND. JPJAG .EQ. 0 ) GO TO 240 SKT00430 C DEBUB UNIT(6),SUBCHK,SUBTRACE,INIT SOL00020
C IF (SB(IJ).EQ.0 ) GO TO 240 SKT00440 C END DEBUB SOL00030
C IF (LMI.LE.LMJ) THEN SKT00450 SUBROUTINE SOLN(OPTION,PLACE,CPU,TCPU,REQCPU) SOL00040
C MDIAG(LMJ) = MIN(MDIAG(LMJ),LMI) SKT00460 LOGICAL BTEST SOL00050
C ELSE SKT00470 CHARACTER*80 NAME,OPTION SOL00060
C MDIAG(LMI)=MIN(MDIAG(LMI),LMJ) SKT00480 CHARACTER*6 PLACE SOL00070
C ENDIF SKT00490 INCLUDE (ZCOMN) SOL00080
C CONTINUE SKT00500 STEPID= ' SOL00090
C CONTINUE SKT00510 C----- SOL00100
C----- SKT00520 C----- SOL00110
C ELSE IF (.NOT.(BTEST(IBUG,3) THEN SKT00530 C DETERMINE SOLUTION NO. == SOL00120
C----- CALCULATE THE INDICES OF THE MAIN DIAGONAL TERMS... SKT00540 C----- SOL00130
C IF ( BTEST(IBUG,12) ) THEN SKT00550 CALL GETINT(OPTION,'SOL',NSOLN,0.,TRUE.,.FALSE.) SOL00140
C WRITE (6,*) 'AFTER COLUMN HEIGHTS' SKT00560 C----- SOL00150
C WRITE (6,320) (I,MDIAG(I),I=1,NDOP) SKT00570 C CHOOSE THE SOLUTION == SOL00160
C ENDIF SKT00580 C----- SOL00170
C MDIAG(1)=1 SKT00590 IF (NSOLN.EQ.1) THEN SOL00180
C ICOLHT=1 SKT00600 CALL SOL01 SKT00200
C DO 310 IROW=2,(NDOP+1) SKT00620 ELSE IF (NSOLN.EQ.2) THEN SOL00210
C ILOCN=ICOLHT + MDIAG(IROW-1) SKT00630 NLOAD=1 SOL00220
C ICOLMT=IROW - (MDIAG(IROW)-1) SKT00640 MAXELD=0 SOL00230
C MDIAG(IROW)=ILOCN SKT00650 CALL SOL02(PLACE,CPU,TCPU,REQCPU) SOL00240
C IF ( BTEST(IBUG,12) ) THEN SKT00660 ELSE IF (NSOLN.EQ.3) THEN SOL00250
C WRITE (6,*) 'ADDRESS OF MAIN DIAGONAL ELEMENTS' SKT00670 NLOAD=1 SOL00260
C WRITE (6,320) (I,MDIAG(I),I=1,NDOP) SKT00680 MAXELD=0 SOL00270
C FORNAG (' MDIAG ',I3,' ',I5) SKT00690 CALL SOL03 SOL00280
C ENDIF SKT00700 ELSE IF (NSOLN.EQ.4) THEN SOL00290
C 321 FORMAT (2I6) SKT00710 NLOAD=1 SOL00300
C IF (.NOT.(BTEST(IBUG,12) .OR. BTEST(IBUG,9) ) ) RETURN SKT00720 CALL SOL04 SOL00310
C PRINT A MAP OF THE STIFFNESS ARRAY SKT00730 ELSE IF (NSOLN.EQ.5) THEN SOL00320
C NL= MDIAG(NDOP+1)-MDIAG(1) +1 SKT00750 MAXELD=1 SOL00330
C NP=NDOP/100 SKT00760 MAXELD=0 SOL00340
C NR=MD(NDOP,100) SKT00770 ELSE SKT00780 CALL SOL05 SOL00350
C IF (NR.GT.0) NP=NP+1 SKT00780 WRITE (6,*) 'INVALID SOLN#',NSOLN SOL00370
C DO 270 IP=1,NP SKT00790 STOP 'INVALID SOLUTION NUMBER' SOL00380
C JC1=(IP-1)*100 SKT00800 ENDIF SOL00390
C JC2=MIN(IP*100,NDOP) SKT00810 30 RETURN SOL00400
C J2=100 SKT00820 END SOL00410
C IF (IP.EQ.NP) J2=NR SKT00830 C#PROCESS DUMP OPT(0) GGSTMT IREF MAP SOL00010
C WRITE (6,290) TITLE(1),TITLE(2),NAME,NL,(I,I=JC1,JC2,10) SKT00840 C----- SOL00020
C DO 270 I=1,NDOP SKT00850 C DEBUB UNIT(6),SUBCHK,SUBTRACE,INIT SOL00040
C PT=FALSE SKT00860 C----- SOL00050
C DO 260 JJ=1,J2 SKT00870 SUBROUTINE SOL01 SOL00060
C JJ=JJ+(IP-1)*100 SKT00880 LOGICAL PRINT,DISPFLG,HEAD,AXIAL,BTEST SOL00070
C IJ=MDIAG(JJ)+J-1 SKT00890 CHARACTER*80 NAME SOL00080
C IF (I.LT.MDIAG(J+1) .AND. I.LT.J) THEN SKT00910 DIMENSION RINPUT(100) SOL00090
C CX(JJ)='X' SKT00920 INCLUDE (ZCOMN) SOL00100
C PT=.TRUE. SKT00930 C READ (5,*) NLOAD,MAXELD SOL00120
C ELSE IF (I.EQ.J) THEN SKT00940 C WRITE (6,1) TITLE(1),TITLE(2),NLOAD SOL00140
C CX(JJ)='D' SKT00950 1 FORMAT ('1 STRUCTURE...1',A// SOL00150
C PT=.TRUE. SKT00960 ' & SOLUTION...1',A// SOL00160
C ELSE CX(JJ)=' ' SKT00970 ' & SKYLINE OF THE 'A,' MATRIX '// SOL00170
C ENDIF SKT00990 ' & THE MATRIX REQUIRES',I6,' STORAGE LOCATIONS', SOL00180
C 260 CONTINUE SKT10000 ' & /2X,12(I4,'.....')' SOL00190
C 270 IF (PT) WRITE (6,295) I,(CX(J),J=1,J2) SKT10100 ' & /X,12(I4,'.....')' SOL00200
C 295 FORMAT (16,132A1) SKT10110 ' & /X,12(I4,'.....')' SOL00210
C ENDIF SKT10120 C IF (N3(IKGD+3).EQ.2) WRITE (6,2) SOL00220
C RETURN SKT10130 2 FORMAT ('X,' GEOMETRIC STIFFNESS IS NOT INCLUDED, ', SOL00230
C END SKT10140 ' & ' USE KGDATA(3)=1 TO INCLUDE GEOMETRIC STIFFNESS.') SOL00240
C----- INITIALISE STORAGE FOR STIFFNESS, LOAD AND DISPL... ----- SOL00250
C ELSTIC=.TRUE. SKT10150 SOL00260
C----- INITIALISE STORAGE FOR STIFFNESS, LOAD AND DISPL... ----- SOL00270
C----- SOL00280
C 2(I2LOAD) = LOCATION OF THE STIFFNESS MATRIX SOL00290
C 2(I2LOAD) = LOCATION OF THE LOAD MATRIX SOL00300
C 2(I2LOAD) = LOCATION OF THE DISPLACEMENT MATRIX SOL00310
C IF (I2LOAD.EQ.0) THEN SKT10020 IF (I2LOAD.EQ.0) THEN SOL00320
C I2 = I2 SKT10030 I2 = I2 SOL00330
C I2 = I2+NDOP*NLOAD SKT10040 I2 = I2+NDOP*NLOAD SOL00340
C ENDIF SKT10050 ENDIF SOL00350
C IF (I2DISP.EQ.0) THEN SKT10060 IF (I2DISP.EQ.0) THEN SOL00360
C I2DISP = I2 SKT10070 I2DISP = I2 SOL00370
C I2 = I2+NDOP*NLOAD SKT10080 I2 = I2+NDOP*NLOAD SOL00380
C ENDIF SKT10090 ENDIF SOL00390
C IF (I2VEL.EQ.0) THEN SKT10100 IF (I2VEL.EQ.0) THEN SOL00400
C I2VEL = I2 SKT10110 I2VEL = I2 SOL00410
C I2 = I2+NDOP SKT10120 I2 = I2+NDOP SOL00420
C ENDIF SKT10130 ENDIF SOL00430
C DO 3 I=1,NDOP SKT10140 DO 3 I=1,NDOP SOL00440
C 2(I+I2VEL-1) = 0 SKT10150 2(I+I2VEL-1) = 0 SOL00450
C IF (I2VEL.EQ.0) THEN SKT10160 IF (I2VEL.EQ.0) THEN SOL00460
C I2VEL = I2 SKT10170 I2VEL = I2 SOL00470
C I2 = I2+NDOP SKT10180 I2 = I2+NDOP SOL00480
C ENDIF SKT10190 ENDIF SOL00490
C DO 4 I=1,NDOP SKT10200 DO 4 I=1,NDOP SOL00500
C 2(I+I2VEL-1) = 0 SKT10210 2(I+I2VEL-1) = 0 SOL00510
C IF (I2SUB.EQ.0) THEN SKT10220 IF (I2SUB.EQ.0) THEN SOL00520
C I2SUB = I2 SKT10230 I2SUB = I2 SOL00530
C I2 = I2+NDOP SKT10240 I2 = I2+NDOP SOL00540
C DO 5 I=1,NDOP SKT10250 DO 5 I=1,NDOP SOL00550
C 2(I+I2SUB-1) = 0 SKT10260 2(I+I2SUB-1) = 0 SOL00560
C ENDIF SKT10270 ENDIF SOL00570
C----- SET UP CONSTANTS ----- SKT10280 SOL00580
C L1=1 SKT10290 L1=1 SOL00590
C L2=NCOND +NFRBE SKT10310 L2=NCOND +NFRBE SOL00610
C L3=NCOND +NFRBE+1 SKT10320 L3=NCOND +NFRBE+1 SOL00620
C L4=NDOP SKT10330 L4=NDOP SOL00630
C IF (MAXELD.GT.0) THEN SKT10340 IF (MAXELD.GT.0) THEN SOL00640
C I2IELD=I2 SKT10350 I2IELD=I2 SOL00650
C I2I2 = MAXELD*5 SKT10360 I2I2 = MAXELD*5 SOL00660
C I2ELD=I2 SKT10370 I2ELD=I2 SOL00670
C I2I2I2 = MAXELD*12 SKT10380 I2I2I2 = MAXELD*12 SOL00680
C ENDIF SKT10390 ENDIF SOL00690
C CALL SOL01A SKT10400 CALL SOL01A SOL00720
C (L1,L2,L3,L4,IMD,NLOAD,MAXELD,NELD,NNODE,NCOS,NELMT,ABORT, SKT10410 (L1,L2,L3,L4,IMD,NLOAD,MAXELD,NELD,NNODE,NCOS,NELMT,ABORT, SOL00730
C 2(I2STIF), 2(I2LOAD), 2(I2DISP), 2(I2VEL), 2(I2DVEL), SKT10420 2(I2STIF), 2(I2LOAD), 2(I2DISP), 2(I2VEL), 2(I2DVEL), SOL00740
C 2(I2SUB),N2(I2IELD), 2(I2ELD), 2(I2I2),N2(I2MD), SKT10430 2(I2SUB),N2(I2IELD), 2(I2ELD), 2(I2I2),N2(I2MD), SOL00750
C N2(I2ID),N2(I2IDOF), 2(I2ICNET),N2(I2JFLG), 2(I2ICORD), SKT10440 N2(I2ID),N2(I2IDOF), 2(I2ICNET),N2(I2JFLG), 2(I2ICORD), SOL00770
C 2(I2ICOS),N2(I2JCCS),SUMRCT) SKT10450 2(I2ICOS),N2(I2JCCS),SUMRCT) SOL00780
C RETURN SKT10460 RETURN SOL00790
C END SKT10470 END SOL00800
C----- SOL00010
C DEBUB UNIT(6),SUBCHK,SUBTRACE,INIT SKT10040 SOL00020
C END DEBUB SKT10050 SOL00030
C SUBROUTINE SMAX (N,DT,A,YMX,YMT,T1,T2,NLEN,AVE,STDEV,RMS) SKT10060 SUBROUTINE SMAX (N,DT,A,YMX,YMT,T1,T2,NLEN,AVE,STDEV,RMS) SOL00040
C LOGICAL BTEST SKT10070 LOGICAL BTEST SOL00050
C REAL*8 T ,SUMX,SUMSQ ,ADBL SKT10080 REAL*8 T ,SUMX,SUMSQ ,ADBL SOL00060
C AVE=0 SKT10090 AVE=0 SOL00070
C RMS=0 SKT10100 RMS=0 SOL00080
C STDEV=0 SKT10110 STDEV=0 SOL00090
C NLEN=N SKT10120 NLEN=N SOL00100
C T1=DT SKT10130 T1=DT SOL00110
C T2=DT SKT10140 T2=DT SOL00120
C IF (N.LE.1) RETURN SKT10150 IF (N.LE.1) RETURN SOL00130
C SUMX=A(1) SKT10160 SUMX=A(1) SOL00140
C ADBL=A(1) SKT10170 ADBL=A(1) SOL00150
C SUMXSQ=ADBL**2 SKT10180 SUMXSQ=ADBL**2 SOL00160
C DO 40 I=2,N SKT10190 DO 40 I=2,N SOL00170
C ADBL=A(I) SKT10200 ADBL=A(I) SOL00180

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SUBROUTINE SOL01A
  & (L1,L2,L3,L4,IMD,NLOAD,MAXELD,NELD,NNODE,NCOS,NELMT,ABORT,
  & I3DISP,I3LOAD,MSTOR,IBUG,MAXNOD,TITLE,STEPID,ESE,
  & STIF,LOAD,DISP,VEL,DVEL,
  & PUB,TELD,ELD,WORK,MD,
  & ID,IDOF,CONST,JTFLG,COORD,
  & COSINE,JTCOS,SUMRCT)
COMMON /ISPC/ ISPL,SGD(3000,2),REACT(6)
CHARACTER(*) TITLE(2),STEPID
LOGICAL PRINT,DSPLG,HEAD,AXIAL,BTEST,BUG,ABORT,BUG3
CHARACTER#0 NAME
DIMENSION RINPUT(100),SUMRCT(6)
REAL LOAD
DIMENSION STIF(IMD),LOAD(L4,NLOAD),DISP(L4,NLOAD),VEL(L4),DVEL(L4)
DIMENSION PUB(L4),IELNO(MAXELD,3),EID(MAXELD,12),WORK(1),MD(L4+1)
DIMENSION COORD(MAXNOD,3),ID(MAXNOD),
  & IDOF(MAXNOD,6),COSINE(3,3,NCOS),CONST(MAXNOD,6),
  & JTFLG(MAXNOD),JTCOS(MAXNOD)
  & BUG-BTEST(1BUG,5)
  & BUG3-BTEST(1BUG,3)
  ISPL=1
C
C-----
C= INPUT LOADING DATA ==
C-----
C
C----- ZERO LOAD AND DISPLACEMENT MATRIX -----
DO 6 I=1,L4
DO 6 J=1,NLOAD
LOAD(I,J)=0
DISP(I,J)=0
6
C
C----- INPUT JOINT LOADS -----
JOINT LOADS AND SUPPORT DISPLACEMENTS ARE STORED IN THE
DISPLACEMENT MATRIX
CALL JOIDSP(BUG3,MAXNOD,L4,NNODE,NLOAD,L2,
  & ID,IDOF,CONST,DISP,
  & DSPLG,JTFLG,TITLE,.FALSE.)
C
C----- INPUT ELEMENT LOADS -----
ELEMENT LOADS ARE STORED IN THE LOAD MATRIX
IF (MAXELD.GT.0) THEN
CALL ELELOA(BUG3,L3,NELMT,MAXELD,NELD,L4,NLOAD,
  & LOAD,TELD,ELD,I3DISP,I3LOAD,NAME,TITLE,.FALSE.)
C
...ADD ELEMENT LOADS ON FREE JOINTS TO THE JOINT LOADS ON FREE
...JOINTS, STORE IN THE DISPLACEMENT MATRIX
...SAVE ELEMENT LOADS ON ALL JOINTS IN LOAD MATRIX, USED LATER
...FOR CALCULATING THE REACTIONS.
DO 10 I=1,L2
DO 10 J=1,NLOAD
DISP(I,J)=DISP(I,J)+LOAD(I,J)
10
C
...SWAP THE SIGN OF THE FEF AT REACTIONS
DO 15 I=L3,L4
DO 15 J=1,NLOAD
LOAD(I,J)=-LOAD(I,J)
DISP(I,J)=-DISP(I,J)
15
ENDIF
C
DO 19 I=1,L4
SGD(I,1)=DISP(I,1)
19
C= FORMULATE STIFFNESS ==
C-----
CALL FORM(1)
C
C= MODIFY LOADS FOR RESTRAINT DISPLACEMENTS ==
C-----
IF (DSPLG) THEN
IOPT=1
CALL REACTN(IOPT,BUG,NNODE,L1,L2,L3,L4,LOAD,DISP,
  & MD,STIF,IMD,L4,NLOAD,1,0,
  & MAXNOD,IDOF,COORD,ID,TITLE,STEPID,.FALSE.,
  & NCOS,COSINE,JTCOS,JTFLG,SUMRCT)
ENDIF
C
C= SOLVE FOR FREE DISPL ==
C-----
IF (BTEST(1BUG,5)) THEN
CALL LMATRX(STIF,MD,L4,IMD,'GLOBAL STIFFNESS DUMP=7')
CALL WMATRX(DISP,L4,NLOAD,'LOAD MATRIX DUMP=7')
ENDIF
C
IOPT=1
CALL MSOLL(IOPT,BTEST(1BUG,6),L2,IMD,L4,NLOAD,L1,L2,
  & MD,STIF,DISP,WORK,
  & .FALSE.,WORK,WORK,L4,
  & .FALSE.,WORK,WORK,L4,ABORT)
IF (ABORT) RETURN
C
IOPT=3
CALL MSOLL(IOPT,BTEST(1BUG,6),L2,IMD,L4,NLOAD,L1,L2,
  & MD,STIF,DISP,WORK,
  & .FALSE.,WORK,WORK,L4,
  & .FALSE.,WORK,WORK,L4,ABORT)
IF (ABORT) RETURN
C
IF (BTEST(1BUG,5)) THEN
CALL WMATRX(DISP,L4,NLOAD,'DISPLACEMENT DUMP=7')
C
DO 20 I=1,L4
SGD(I,2)=DISP(I,1)
20
C
C= SOLVE FOR REACTIONS ==
C-----
IOPT=2
CALL REACTN(IOPT,BUG,NNODE,L1,L2,L3,L4,LOAD,DISP,
  & MD,STIF,IMD,L4,NLOAD,1,0,
  & MAXNOD,IDOF,COORD,ID,TITLE,STEPID,.FALSE.,
  & NCOS,COSINE,JTCOS,JTFLG,SUMRCT)
IF (BTEST(1BUG,3)) THEN
CALL WMATRX(LOAD,L4,NLOAD,'FINAL LOADS AND REACTIONS')
C
C= PRINT GLOBAL DISPLACEMENTS ==
C-----
CALL JOIDSP(1,MAXNOD,L4,NNODE,NLOAD,0,L2,ID,
  & IDOF,CONST,DISP,
  & TITLE,'DISPLACEMENTS',STEPID,.TRUE.,
  & NCOS,COSINE,JTCOS,JTFLG)
C
C= PRINT GLOBAL REACTIONS ==
C-----
IOPT=3
CALL REACTN(IOPT,.TRUE.,NNODE,L1,L2,L3,L4,LOAD,DISP,
  & MD,STIF,IMD,L4,NLOAD,1,0,
  & MAXNOD,IDOF,COORD,ID,TITLE,STEPID,.TRUE.,
  & NCOS,COSINE,JTCOS,JTFLG,SUMRCT)
DO 115 I=1,6
REACT(I)=SUMRCT(I)
115
C-----
C= CALCULATE AND PRINT MEMBER FORCES ==
C-----
IOPT=4
LSTYP=0
PRINT=.TRUE.
HEAD=.TRUE.
DO 300 IELNO=1,NELMT
300 CALL ELELIB
  & (IOPT,LSTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,
  & DUCFAG,
  & IELNO,IELDOF,KGDOP,PGEOM,RINPUT,AXIAL,I3DISP,I3LOAD,MSTOR)
C
C= ADD LAST LOAD CASE TO TOTAL MEMBER LOADS ==
C-----
C THIS SETS THE TOTAL LOAD IN THE ELEMENT SUBROUTINE EQUAL TO THE
SUM OF THE TOTAL LOAD AND THE LAST LOAD CASE.
C TWO MAJOR BENEFITS ARE DERIVED FROM THIS:
C 1) TOTAL ELEMENT LOADS ARE USED AS INITIAL VALUES FOR BUCKLING AND
DYNAMIC LOADINGS.
C 2) TOTAL ELEMENT LOADS ARE USED IN NONLINEAR ANALYSIS
C
IOPT=5
LSTYP=0
PRINT=.FALSE.
HEAD=.FALSE.
DO 310 IELNO=1,NELMT
310 CALL ELELIB
  & (IOPT,LSTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,
  & DUCFAG,
  & IELNO,IELDOF,KGDOP,PGEOM,RINPUT,AXIAL,I3DISP,I3LOAD,MSTOR)
C
C= OBTAIN TOTAL STRAIN ENERGY ==
C-----
ESE=0
DO 1312 IELNO=1,NELMT
CALL ELELIB
  & (IOPT,LSTYP,.FALSE.,IREL,.FALSE.,NAME,ESE,EPSE,DAMAGE,
  & DUCFAG,
  & IELNO,IELDOF,KGDOP,PGEOM,RINPUT,AXIAL,I3DISP,I3LOAD,MSTOR)
1312 ESE=ESE+ESE
ESE=ESE/4
WRITE(6,*)
1313 FORMAT('X','THE STATICS STRAIN ENERGY OF SYSTEM =',G12.4/)
WRITE(6,*) '-----'
C
IF (NLOAD.EQ.1) RETURN
C
C-----
WRITES (6,1000) TITLE(1),TITLE(2)
1000 FORMAT ('1 STRUCTURE.....',A,/,
  & ' SOLUTION.....',A,/,
  & '1X',MAXIMUM VALUES FOR ALL LOAD CASES '/',
  & '1X',)
C
C= GET AND PRINT MAXIMUM DISPL ==
C-----
DO 1010 I=1,L4
WORK(I,1)=DISP(I,1)
DO 1010 J=2,NLOAD
IF (ABS(DISP(I,J)).GT.ABS(WORK(I,1))) WORK(I,1)=DISP(I,J)
1010 CONTINUE
CALL JOIDSP(2,MAXNOD,L4,NNODE,1,0,L2,ID,
  & IDOF,CONST,WORK,
  & TITLE,'MAXIMUM DISPLACEMENTS',STEPID,.FALSE.,
  & NCOS,COSINE,JTCOS,JTFLG)
C
C= GET AND PRINT MAXIMUM REACTIONS ==
C-----
WRITE (6,1000) TITLE(1),TITLE(2)
DO 1020 I=1,L4
WORK(I)=LOAD(I,1)
DO 1020 J=2,NLOAD
IF (ABS(LOAD(I,J)).GT.ABS(WORK(I))) WORK(I)=LOAD(I,J)
1020 CONTINUE
DO 1030 I=1,6
1030 SUMRCT(I)=0
CALL REACTN(4,.TRUE.,NNODE,L1,L2,L3,L4,WORK,DISP,
  & MD,STIF,IMD,L4,1,0,
  & MAXNOD,IDOF,COORD,ID,TITLE,STEPID,.FALSE.,
  & NCOS,COSINE,JTCOS,JTFLG,SUMRCT)
C
C= PRINT MAXIMUM MEMBER FORCES ==
C-----
WRITE (6,1000) TITLE(1),TITLE(2)
LSTYP=0
PRINT=.TRUE.
HEAD=.FALSE.
DO 1310 IELNO=1,NELMT
1310 CALL ELELIB
  & (IOPT,LSTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,
  & DUCFAG,
  & IELNO,IELDOF,KGDOP,PGEOM,RINPUT,AXIAL,I3DISP,I3LOAD,MSTOR)
C
RETURN
C
220 FORMAT (5X,I5.1P,6G15.6)
200 FORMAT ('1 GLOBAL',A,/,
  & '1X','NODE',7X,'DX',13X,'DY',13X,'DZ',13X,'RX',13X,'RY',13X,'RZ',/)
END
C
SUBROUTINE SOL02(PLACES,CPU,TCPU,REQCPU)
LOGICAL PRINT,DSPLG,HEAD,REPEAT,SLMMASS,AXIAL,TEST,NEWKDT,BTEST
DIMENSION RINPUT(100)
CHARACTER#1 NAME
CHARACTER#6 PLACE
CHARACTER#20 INTEG
INCLUDE (COMMON)
C
C= PRINT HEADER ==
C-----
WRITE (6,10) TITLE(1),TITLE(2)
10 FORMAT ('1 STRUCTURE.....',A,/,
  & ' SOLUTION.....',A,/)
C
C= INPUT LOADING DATA ==
C-----
READ (5,*) INTEX,THETA,ELSTIC,UNBAL
READ (5,*) IPRT,IWRITES,MAXACC,SLMMASS
READ (5,*) TO,DT,TP,GRAY
IPRINT=MAX(IPRINT,1)
C
IF (TEST('INTEX','LINEAR'..FALSE.)) THEN
INTEGO=1
C
C-----

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DLOAD2(I)=DLOAD(I)+FUB(I)
WRITE(50,*)DLOAD(1),FUB(1),DLOAD2(1)
195 FUB(I)=0.0
DO 196 I=L5,L6
DLOAD2(I)=FUB(I)
C****
C**** OMIT UNBALANCED LOAD FROM REACTIONS...
C**** UNBALANCED LOADS ADDED TO THE FRAME ARE RESISTED BY
C**** 1) INERTIAL FORCES
C**** 2) DAMPING FORCES
C**** 3) MEMBER FORCES
C**** SUBTRACTING UNBALANCED FORCES FROM REACTIONS DUE TO MEMBER FORCES
C**** NEGLECTS THE REACTIONS DUE TO INERTIAL AND DAMPING FORCES.
C**** DLOAD(I)=FUB(I)
C****
196 FUB(I)=0.0
ELSE
DO 197 I=L1,L6
197 DLOAD2(I)=DLOAD(I)
ENDIF
C
C-----
C= FORMULATE STIFFNESS ---
C-----
C FORMULATE THE STIFFNESS IF THE FLAG NEWK IS TRUE, THIS FLAG IS
INITIALIZED ABOVE, AND SET WHEN THE ELEMENT FORCES ARE CALC.
C THE STIFFNESS IS ALSO FORMULATED IF KG IS TO BE CALCULATED
C AND CONDENSATION IS TO BE PERFORMED, (THE STIFFNESS IS NEEDED
FOR THE CONDENSATION PROCEDURE...)
REPEAT=.TRUE.
KNAME=
100 IF (NEWK) THEN
I=1
CALL FORM(I)
C-----
CALL LMATRX(STIFF,MD,L4,IMD,'GLOBAL STIFFNESS MATRIX')
IF (BUG5) THEN
CALL LMATRX(STIFF,MD,NDOF,IMD,'GLOBAL STIFFNESS DUMP*')
ENDIF
C
C-----
C= FORM GEOMETRIC STIFFNESS ---
C-----
C FORMULATE GEOMETRIC STIFFNESS IF THE FLAG NEWK IS TRUE, THIS
FLAG INITIALIZED ABOVE.
IF KGDATA(1)=1, THE GEOMETRIC STIFFNESS IS FORMED ONCE, AND
MULTIPLIED BY A SCALAR TO ACCOUNT FOR CHANGES
IN THE TOTAL VERTICAL LOAD. THUS THE GEOMETRIC
STIFFNESS ONLY NEEDS TO BE FORMED ONCE.
IF KGDATA(1)=2, THE GEOMETRIC STIFFNESS IS BASED ON THE AXIAL
FORCE IN THE ELEMENTS, AND IS RECALCULATED EACH
TIME THE AXIAL FORCE CHANGES.
IF KGDATA(3)=1, THE GEOMETRIC STIFFNESS IS FORMED WITH THE
STIFFNESS, THUS A SEPARATE CALL TO FORM IS
NOT NEEDED.
IF KGDATA(3)=2, A SEPARATE GEOMETRIC STIFFNESS MATRIX IS FORMED
IF (NEWKG) THEN
C
C-----
C FORMULATE SEPARATE GEOMETRIC STIFFNESS MATRIX
IF (KGFORM.EQ.2) THEN
F2 =0
I=2
CALL FORM(I)
KCOND2=KCOND
IF (BUG5) THEN
CALL LMATRX(KG,MDKG,NDOF,IMDKG,'KG MATRIX DUMP*')
ENDIF
ELSE
KCOND2=.FALSE.
ENDIF
C
ENDIF
C
C-----
C= CONDENSE OUT FREE DISPL ---
C-----
C... IF MCOND IS TRUE THE MASS MATRIX IS ALSO REDUCED BY GUYAN REDUCTION
C... IF KCOND2 IS TRUE THEN KG IS ALSO REDUCED
IF (MCOND.GT.0 .AND. NEWK) THEN
CALL MSOLL(1,BUG5,L4,IMD,NDOF,1,L1,L2,
& MD ,STIFF ,DLOAD2,WORK,
& MCND2,MASS ,MDMS ,IMDMS,
& KCOND2,KG ,MDKG ,IMDKG,ABORT)
IF (ABORT) GO TO 5000
MCND2=.FALSE.
KCOND2=.FALSE.
ELSE IF (MCOND.GT.0) THEN
REDUCE THE DYNAMIC LOAD MATRIX IF OLD STIFFNESS IS USED...
CALL MSOLL(2,BUG5,L4,IMD,NDOF,1,L1,L2,
& MD ,STIFF ,DLOAD2,WORK,
& MCND2,MASS ,MDMS ,IMDMS,
& KCOND2,KG ,MDKG ,IMDKG,ABORT)
IF (ABORT) GO TO 5000
ENDIF
C
C-----
C= DETERMINE IF A NEW DYNAMIC STIFFNESS NEEDS TO BE CALC ---
C-----
IF (NEWK .OR. NEWKG .OR. (KGLoad.EQ.1 .AND. ACCEL.EQ.ACCEL2)) THEN
NEWKDT=.TRUE.
ACCOL=ACCEL2
ELSE
NEWKDT=.FALSE.
ENDIF
C
C-----
WRITE (6,*) '----- DDISP ----- BEFORE LINACC '
WRITE (6,*) (DDISP(I),I=1,14)
C-----
C= SOLVE DYNAMIC EQUATION FOR INCREMENTAL DISPL ---
C-----
I2=2
IF (INTSGO.EQ.1) THEN
CALL LINACC(I2,BUG4,DT,TO,TF,T,1,CO,NEWKDT,
& L1,L2,L3,L4,L5,L6,IMDMS,IMD,IMDS,IMDKG,NDOF,NFREE,
& KGLoad,KGFORM,
& ACC ,VEL ,DISP ,LOAD
& DACC ,DVEL ,DDISP ,DLOAD2,WORK,
& MASS ,MDMS ,DAMP ,STIFF ,MD
& KG ,MDKG ,F2 ,GRAV ,
& SBAR ,MDS ,AN ,BN ,PBAR
& FDAMP,FMASS,C1,C2,C3,C4,C5,C6,C7,ALPHA,BETA,C10,C11,ABORT)
ELSE IF (INTSGO.EQ.2) THEN
CALL AVEACC(I2,BUG4,DT,TO,TF,T,THETA,NEWKDT,
& L1,L2,L3,L4,L5,L6,IMDMS,IMD,IMDS,IMDKG,NDOF,NFREE,
& KGLoad,KGFORM,
& ACC ,VEL ,DISP ,LOAD
& DACC ,DVEL ,DDISP ,DLOAD2,WORK,
& MASS ,MDMS ,DAMP ,STIFF ,MD
& KG ,MDKG ,F2 ,GRAV ,
& SBAR ,MDS ,AN ,BN ,PBAR
& FDAMP,FMASS,C1,C2,C3,C4,C5,C6,C7,ALPHA,BETA,C10,C11,ABORT)
ELSE IF (INTSGO.EQ.3) THEN
CALL WILSON(I2,BUG4,DT,TO,TF,T,THETA,NEWKDT,
& L1,L2,L3,L4,L5,L6,IMDMS,IMD,IMDS,IMDKG,NDOF,NFREE,
& KGLoad,KGFORM,
& ACC ,VEL ,DISP ,LOAD
& DACC ,DVEL ,DDISP ,DLOAD2,WORK,
& MASS ,MDMS ,DAMP ,STIFF ,MD
& KG ,MDKG ,F2 ,GRAV ,
& SBAR ,MDS ,AN ,BN ,PBAR
& FDAMP,FMASS,C1,C2,C3,C4,C5,C6,C7,ALPHA,BETA,C10,C11,ABORT)

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SOL02580 ENDIF
SOL02590 IF (ABORT) GO TO 5000
SOL02600 C
SOL02610 C-----
SOL02620 C= BACK SUBSTITUTION TO SOLVE FOR NON-DYNAMIC DOP ---
SOL02630 C-----
SOL02640 C DISP_C = T * DISP_F
SOL02650 DISP_C = INV(K_CC) * (-K_CF)*DISP(F)
SOL02660 C SUBRTN REA_CTN (IOPT=1)
SOL02670 C PREFORMS THE MULTIPLICATION DISP_C=(-K_CF)*DISP_F
SOL02680 C SUBRTN MSOLL (IOPT=3)
SOL02690 C PREFORMS THE BACK SUBSTITUTION TO EQUIVALENT TO
SOL02700 DISP_C=INV(K_CC)*DISP_C
SOL02710 C
SOL02720 ONE=1.00
SOL02730 IF (NCOND.GT.0) THEN
SOL02740 I=3
SOL02750 I=1
C-----
SOL02760 C----- DISPLACEMENTS -----
SOL02770 IF (BUG5) WRITE(6,*) '----- CALC DISPL'
SOL02780 WRITE (6,*) '----- DDISP -----'
SOL02790 WRITE (6,*) (DDISP(I),I=1,14)
SOL02800 CALL REACTN(I1,BUG5,NDOF,L1,L2,L3,L4,DDISP,DDISP,
& MD ,STIFF ,MD(NDOF),NDOF,1,ONE,
& MAXNOD,IDOF,COORD ,ID,TITLE,STEPID ,FALSE.,
& NCOS,COSINE ,JTCOS ,JTLG ,DSUMRC)
SOL02810 & DDISP ,WORK,
SOL02820 & .FALSE.,MASS ,MDMS ,1,
SOL02830 & .FALSE.,KG ,MDKG ,1,ABORT)
SOL02840 IF (ABORT) GO TO 5000
SOL02850 C-----
SOL02860 C----- VELOCITIES -----
SOL02870 IF (BUG5) WRITE(6,*) '----- CALC VELOC'
SOL02880 CALL REACTN(I1,BUG5,NDOF,L1,L2,L3,L4,DVEL,DVEL,
& MD ,STIFF ,MD(NDOF),NDOF,1,ONE,
& MAXNOD,IDOF,COORD ,ID,TITLE,STEPID ,FALSE.,
& NCOS,COSINE ,JTCOS ,JTLG ,DSUMRC)
SOL02890 CALL MSOLL(I3,BUG5,L4,IMD,NDOF,I1,L1,L2,MD ,STIFF,
& DVEL ,WORK,
& .FALSE.,MASS ,MDMS ,1,
& .FALSE.,KG ,MDKG ,1,ABORT)
SOL02900 IF (ABORT) GO TO 5000
SOL02910 C-----
SOL02920 C----- ACCELERATION -----
SOL02930 IF (BUG5) WRITE(6,*) '----- CALC ACCEL'
SOL02940 CALL REACTN(I1,BUG5,NDOF,L1,L2,L3,L4,DACC,DACC,
& MD ,STIFF ,MD(NDOF),NDOF,1,ONE,
& MAXNOD,IDOF,COORD ,ID,TITLE,STEPID ,FALSE.,
& NCOS,COSINE ,JTCOS ,JTLG ,DSUMRC)
SOL02950 CALL MSOLL(I3,BUG5,L4,IMD,NDOF,I1,L1,L2,MD ,STIFF,
& DACC ,WORK,
& .FALSE.,MASS ,MDMS ,1,
& .FALSE.,KG ,MDKG ,1,ABORT)
SOL02960 IF (ABORT) GO TO 5000
SOL02970 ENDIF
SOL02980 C
SOL02990 C-----
SOL03000 C= CALCULATE INCREMENTAL MEMBER FORCES ---
SOL03010 C-----
SOL03020 C ELELIB CALLS THE INDIVIDUAL ELEMENT LIBRARIES, EACH OF WHICH DETERMINES
SOL03030 1) DID THE STIFFNESS CHANGE? IF SO, NEWK=.TRUE.
SOL03040 2) SHOULD THE INCREMENTAL DISPLACEMENT BE RECALCULATED?
SOL03050 IF SO, KNAME=KNAME+ (A VALUE DEPENDING ON ELEMENT TYPE)
SOL03060 IOPR=4
SOL03070 LSTYP=0
SOL03080 PRINT=.FALSE.
SOL03090 HEAD=.FALSE.
SOL03100 NEWK=.FALSE.
SOL03110 NELMT=NELEMT
SOL03120 DO 150 I=ELNO,1,NELMT
150 CALL ELELIB
& (IOPT,LSTYP,PRINT,I,REL,HEAD,NAME,SESE,EPSE,DAMAGE,DUCFAG,
& IELNO,IELDOF,KGDOF,POEOM,RINPUT,AXIAL,I2DDSP,I2DLOA,HSTOR)
C
C-----
C= ZERO STIFFNESS CHANGE FLAGS IF ELASTIC ---
SOL03300 IF (ELASTIC) THEN
SOL03310 NEWK=.FALSE.
SOL03320 KNAME=0
SOL03330 ENDIF
C
C-----
C= DETERMINE IF KG IS TO BE UPDATED BASED ON ---
SOL03400 C= CHANGING ELEMENT AXIAL FORCE ---
SOL03410 IF (KGLoad.EQ.2) THEN
SOL03420 NEWK=.TRUE.
SOL03430 ELSE
SOL03440 NEWK=.FALSE.
SOL03450 ENDIF
C
C-----
C= REPEAT STEP IF NECESSARY ---
SOL03500 C IF AN ELEMENT STIFFNESS CHANGE DICTATED THAT THE STEP BE REANALYZED,
SOL03510 C REFORMULATE THE STIFFNESS AND REANALYZE
SOL03520 THE VARIABLE REPEAT IS SET UP TO SUPPRESS THE REANALYZING OF A STEP
SOL03530 CURRENTLY, EACH STEP MAY ONLY BE REANALYZED ONCE, THIS PREVENTS
SOL03540 AN ENDLESS LOOP.
SOL03550 IF (KNAME.NE.0 .AND. REPEAT) THEN
SOL03560 REPEAT=.FALSE.
SOL03570 GO TO 100
SOL03580 ENDIF
C
C-----
C= SOLVE FOR REACTIONS ---
SOL03600 C REACTIONS ARE DUE TO INTERNAL MEMBER FORCES
SOL03610 IF (BUG5) WRITE(6,*) '----- CALC REACT=K*X'
SOL03620 CALL REACTN(3,BUG5,NDOF,L1,L4,L5,L6,DLOAD ,DDISP,
& MD ,STIFF ,MD(NDOF),NDOF,1,1,0,
& MAXNOD,IDOF,COORD ,ID,TITLE,STEPID ,FALSE.,
& NCOS,COSINE ,JTCOS ,JTLG ,DSUMRC)
C
C-----
SOL03700 C----- GET SUMMATION OF REACTIONS
SOL03710 CALL REACTN(3,BUG5,NDOF,L1,L4,L5,L6,DLOAD ,DDISP,
& MD ,STIFF ,MD(NDOF),NDOF,1,1,0,
& MAXNOD,IDOF,COORD ,ID,TITLE,STEPID ,FALSE.,
& NCOS,COSINE ,JTCOS ,JTLG ,DSUMRC)
C
C-----
SOL03800 C= SOLVE FOR INCREMENTAL GEOMETRIC STIFFNESS FORCE, DFG ---
SOL03810 IF (KGFORM.EQ.2) THEN
SOL03820 IF (BUG5) WRITE(6,*) '----- INCREMENTAL FORCE DUE TO KG '
SOL03830 CALL TMRPL(BUG5,.TRUE.,IMD,L4,L3,L4,L3,L4,
& MDKG,KG,DFG,DDISP)
SOL03840 C
SOL03850 C SCALE FORCE TO INCLUDE GROUND ACCELERATION ...
SOL03860 DO 159 I=L3,L4

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SOL04000
SOL04010
SOL04020
SOL04030
SOL04040
SOL04050
SOL04060
SOL04070
SOL04080
SOL04090
SOL04100
SOL04110
SOL04120
SOL04130
SOL04140
SOL04150
SOL04160
SOL04170
SOL04180
SOL04190
SOL04200
SOL04210
SOL04220
SOL04230
SOL04240
SOL04250
SOL04260
SOL04270
SOL04280
SOL04290
SOL04300
SOL04310
SOL04320
SOL04330
SOL04340
SOL04350
SOL04360
SOL04370
SOL04380
SOL04390
SOL04400
SOL04410
SOL04420
SOL04430
SOL04440
SOL04450
SOL04460
SOL04470
SOL04480
SOL04490
SOL04500
SOL04510
SOL04520
SOL04530
SOL04540
SOL04550
SOL04560
SOL04570
SOL04580
SOL04590
SOL04600
SOL04610
SOL04620
SOL04630
SOL04640
SOL04650
SOL04660
SOL04670
SOL04680
SOL04690
SOL04700
SOL04710
SOL04720
SOL04730
SOL04740
SOL04750
SOL04760
SOL04770
SOL04780
SOL04790
SOL04800
SOL04810
SOL04820
SOL04830
SOL04840
SOL04850
SOL04860
SOL04870
SOL04880
SOL04890
SOL04900
SOL04910
SOL04920
SOL04930
SOL04940
SOL04950
SOL04960
SOL04970
SOL04980
SOL04990
SOL05000
SOL05010
SOL05020
SOL05030
SOL05040
SOL05050
SOL05060
SOL05070
SOL05080
SOL05090
SOL05100
SOL05110
SOL05120
SOL05130
SOL05140
SOL05150
SOL05160
SOL05170
SOL05180
SOL05190
SOL05200
SOL05210
SOL05220
SOL05230
SOL05240
SOL05250
SOL05260
SOL05270
SOL05280
SOL05290
SOL05300
SOL05310
SOL05320
SOL05330
SOL05340
SOL05350
SOL05360
SOL05370
SOL05380
SOL05390
SOL05400
SOL05410

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159 DFG(I) = (1+FS)*DFG(I)
ENDIF
C
C----- SOLVE FOR INCREMENTAL DAMPING FORCE, DFDAMP -----
C
IF (BETA.NE.0) THEN
  INCREMENTAL DAMPING FORCE DUE TO STIFFNESS ...
  IF (BUG5) WRITE(6,*)
    & '----- DAMPING FORCE/REACT DUE TO STIFFNESS'
    & CALL TEMPL(BUG5,.TRUE.,IMD,L6,L3,L4,L3,L4,
    & MD,STIFF,DFDAMP,DVEL)
    & CALL TEMPL(BUG5,.TRUE.,IMD,L6,L5,L6,L1,L4,
    & MD,STIFF,DFDAMP,DVEL)
  INCREMENTAL DAMPING FORCE DUE TO KG ...
  IF (KGFORM.EQ.2) THEN
    IF (BUG5) WRITE(6,*)
      & '----- DAMPING FORCE/REACT DUE TO KG'
      & CALL TEMPL(BUG5,.TRUE.,IMD,L6,L3,L4,L3,L4,
      & MD,KG,WORK,DVEL)
      & CALL TEMPL(BUG5,.TRUE.,IMD,L6,L5,L6,L1,L4,
      & MD,KG,WORK,DVEL)
      & DO 158 I=L3,L6
      & DFDAMP(I)=DFDAMP(I)-WORK(I)
      & ENDDIF
  C----- CALC SUM OF STIFFNESS PROPORTIONAL DAMPING REACTIONS
  CALL REACTN(3,BUG5,NNODE,L1,L4,L5,L6,DFDAMP,DVEL,
  & MDMS,MASS,MDMS(NDOF),NDOF,1,1,00,
  & MAINOD,IDOF,COORD,ID,TITLE,STEPID,.FALSE.,
  & NCOS,COSINE,JT COS,JTFLG,DSUMPD)
  C----- MULTIPLY BY BETA
  DO 160 I=1,6
  & DSUMPD(I)=DSUMPD(I)*BETA
  160 IF (BUG5) WRITE(6,*) 'I,DSUMPD(I),BETA',I,DSUMPD(I),BETA
  & DO 161 I=L3,L4
  & DFDAMP(I)=DFDAMP(I)*BETA
  161 ENDDIF
  C
  IF (ALPHA.NE.0) THEN
    INCREMENTAL DAMPING FORCE DUE TO MASS ...
    IF (BUG5) WRITE(6,*) '----- DAMPING FORCE DUE TO MASS'
    & DO 162 I=L1,L6
    & WORK(I)=0.00
    & CALL TEMPL(BUG5,.FALSE.,IMD,L4,L3,L4,L3,L4,
    & MDMS,MASS,WORK,DVEL)
  C----- CALC SUM OF PD
  CALL REACTN(3,BUG5,NNODE,L1,L2,L3,L4,WORK,DVEL,
  & MDMS,MASS,MDMS(NDOF),NDOF,1,1,00,
  & MAINOD,IDOF,COORD,ID,TITLE,STEPID,.FALSE.,
  & NCOS,COSINE,JT COS,JTFLG,TSUM)
  C----- MULTIPLY BY ALPHA ADD TO PREVIOUS SUM ...
  IF (BETA.NE.0) THEN
    DO 165 I=L3,L4
    & DFDAMP(I)=DFDAMP(I)+ALPHA*WORK(I)
    & DO 167 I=1,6
    & DSUMPD(I)=DSUMPD(I)+TSUM(I)*ALPHA
    165 ELSE
    & DO 168 I=L3,L4
    & ALPHA*WORK(I)
    & DFDAMP(I)=
    & DO 166 I=1,6
    & DSUMPD(I)=TSUM(I)*ALPHA
    166 ENDDIF
    IF (BUG5) WRITE(6,*) 'DSUMPD',DSUMPD
  ENDDIF
  IF (BUG5) THEN
    & DO 169 I=L3,L6
    & WRITE(6,*) 'I',I,'DFDAMP',DFDAMP(I),'FDAMP',FDAMP(I)
    169 ENDDIF
  C
  C= CALC AND ENERGIES AND DUCTILITIES ---
  C-----
  WRITE=MOD(ISTEP,IPRMT).EQ.0
  HEAD=WRITE
  C
  IF (HEAD) WRITE(6,250) TITLE(1),TITLE(2),',',ISTEP,STEPID
  IF (SLDMS) THEN
    ENERGY FORMULATION WITH SPECIAL LOADING MASS ---
    C-----
    BASED ON APPLIED LOADING
    CALL ENERGY
    & (L1,L2,L3,L4,L5,L6,NDOF,MAINOD,NCOS,BUG11,.FALSE.,
    & IMD,IMDMS,IMDKG,MD,MDMS,MDRG,.TRUE.,WRITE,ABORT,
    & STIFF,KG,MASS,
    & E1E,ESE,PSE,EKE,EDD,
    & ALPHA,BETA,KGFORM,NELMT,NNODE,DT,SUMBS,
    & SUMRC,DSUMRC,SUMPD,DSUMPD,DFDAMP,DFDAMP,F2,
    & BERO,GV,GD,BERO,GVT,GDT,WORK,WORK(L6+1),PG,DPG,
    & DACC,DVEL,DDISP,ACC,VEL,DISP,LOAD,DLOAD,
    & ID, IDOF, CNST, JTFLG, COORD,
    & COSINE, JT COS)
  ELSE
    ENERGY FORMULATION WITH/O SPECIAL LOADING MASS ---
    C-----
    BASED ON GROUND MOTION
    CALL ENERGY
    & (L1,L2,L3,L4,L5,L6,NDOF,MAINOD,NCOS,BUG11,.FALSE.,
    & IMD,IMDMS,IMDKG,MD,MDMS,MDRG,.TRUE.,WRITE,ABORT,
    & STIFF,KG,MASS,
    & E1E,ESE,PSE,EKE,EDD,
    & ALPHA,BETA,KGFORM,NELMT,NNODE,DT,SUMBS,
    & SUMRC,DSUMRC,SUMPD,DSUMPD,DFDAMP,DFDAMP,F2,
    & GA,GV,GD,GAT,GVT,GDT,WORK,WORK(L6+1),PG,DPG,
    & DACC,DVEL,DDISP,ACC,VEL,DISP,BERO,BERO,
    & ID, IDOF, CNST, JTFLG, COORD,
    & COSINE, JT COS)
  ENDDIF
  E1EM=AMAX1(ENGL(E1E),E1EM)
  E2EM=AMAX1(ENGL(E2E),E2EM)
  P2EM=AMAX1(ENGL(P2E),P2EM)
  E3EM=AMAX1(ENGL(E3E),E3EM)
  E4EM=AMAX1(ENGL(E4E),E4EM)
  E5EM=AMAX1(ENGL(E5E),E5EM)
  IF (ABORT) GO TO 5000
  C
  C= GET TOTAL GLOBAL LOAD, DISPL, VELOC AND ACCEL ---
  C-----
  IF (BUG4) WRITE(6,320)
  DO 310 I=1,NDOF
  & ACC(I) = DACC(I) + ACC(I)
  & VEL(I) = DVEL(I) + VEL(I)
  & DISP(I) = DDISP(I) + DISP(I)
  C
  LOAD(I) = DLOAD(I) + LOAD(I)
  TFDAMP(I) = DFDAMP(I) + TFDAMP(I)
  FG(I) = DFG(I) + FG(I)
  IF (ABS(ACC(I)).GT.ABS(ACCMAX(I))) ACCMAX(I)=ACC(I)
  IF (ABS(VEL(I)).GT.ABS(VELMAX(I))) VELMAX(I)=VEL(I)
  IF (ABS(DISP(I)).GT.ABS(DSPMAX(I))) DSPMAX(I)=DISP(I)
  IF (ABS(LOAD(I)).GT.ABS(RCTMAX(I))) RCTMAX(I)=LOAD(I)
  IF (BUG4)
  & WRITE(6,330) I,DDISP(I),DVEL(I),DACC(I),DLOAD(I),
  & DACC(I)=0.00
  & DVEL(I)=0.00
  & DDISP(I)=0.00
  & DFDAMP(I)=0.00
  310 DLOAD(I) = 0.00
  310 DLOAD(I) = 0.00
  DO 309 I=1,6
  & SUMRC(I) = DSUMRC(I) + SUMRC(I)
  & SUMPD(I) = DSUMPD(I) + SUMPD(I)
  & SUMRCOR(SQRT(SUMRC(1)**2+SUMRC(2)**2+SUMRC(3)**2))
  & SUMRCOR(SQRT(SUMRC(4)**2+SUMRC(5)**2+SUMRC(6)**2))
  & SUMRCOR(MAX(SUMRCOR,SUMRCOR))
  & SUMRCOR(MAX(SUMRCOR,SUMRCOR))
  320 FORMAT(' DOP',6X,'DD',12X,'DV',12X,'DA',12X,'DP',
  & 12X,'D',12X,'V',12X,'A',12X,'P')
  330 FORMAT(1X,I6,1P,8G14.6)
  C
  C-----
  C= PRINT TOTAL GLOBAL DISPLACEMENTS, VELOCITIES, ACCELERATIONS ---
  C-----
  IF (WRITE) THEN
    & CALL JOIDSP(1,MAINOD,NDOF,NNODE,1,L3,L4,ID,
    & IDOF,CNST,DISP,
    & TITLE,'DISPLACEMENTS',STEPID,.TRUE.,
    & NCOS,COSINE,JT COS,JTFLG)
    & CALL JOIDSP(1,MAINOD,NDOF,NNODE,1,L3,L4,ID,
    & IDOF,CNST,VEL,
    & TITLE,'VELOCITIES',STEPID,.TRUE.,
    & NCOS,COSINE,JT COS,JTFLG)
    & CALL JOIDSP(1,MAINOD,NDOF,NNODE,1,L3,L4,ID,
    & IDOF,CNST,ACC,
    & TITLE,'ACCELERATIONS',STEPID,.TRUE.,
    & NCOS,COSINE,JT COS,JTFLG)
  ENDDIF
  C
  C-----
  C= PRINT TOTAL GLOBAL REACTIONS ---
  C-----
  IOPT=3
  CALL REACTN(IOPT,WRITE,NNODE,L1,L4,L5,L6,LOAD,ACC,
  & MDMS,MASS,MDMS(NDOF),NDOF,1,ONE,
  & MAINOD,IDOF,COORD,ID,TITLE,STEPID,.TRUE.,
  & NCOS,COSINE,JT COS,JTFLG,SUMRC)
  IF (WRITE) WRITE(6,313) SUMFOR,SUMDOM
  313 FORMAT(/4X,'RESULTANT OF REACTIONS, FORCE'=.1P,G12.4,
  & 'MOMENT'=.12G12.4)
  DO 311 I=1,6
  & *NDOF(I)
  IF (ABS(SUMRC(I)).GT.ABS(RCTMAX(J))) RCTMAX(J)=SUMRC(I)
  311 CONTINUE
  C
  C-----
  C= CALCULATE AND PRINT TOTAL MEMBER FORCES ---
  C-----
  IOPT=0
  LSTTYP=0
  IF (WRITE) THEN
    PRINT=.TRUE.
    HEAD=.TRUE.
  ELSE
    PRINT=.FALSE.
    HEAD=.FALSE.
  ENDDIF
  DO 300 IELNO=1,NELMT
  & CALL ELEMFB
  & (IOPT,LSTTYP,PRINT,I,REL,HEAD,NAME,ESE,EPSE,DAMAGE,
  & DUCFAG,
  & IELNO,I,ELDOP,KGDOF,PGECM,RINPUT,AXIAL,I3DDSP,I3DLOA,MSTOR)
  C
  C-----
  C= CALCULATE AND PRINT SYSTEM DUCTILITY BASE
  C-----
  SYSTEM DUCTILITY IS BASED ON THAT THE FIRST ELEMENT REACH TO
  C
  IF (.NOT.ELSTIC) THEN
    IOPT=11
    LSTTYP=0
    IF (.NOT.DUCLOG) THEN
      PRINT=.FALSE.
      HEAD=.FALSE.
    DO 3120 IELNO=1,NELMT
    & CALL ELEMFB
    & (IOPT,LSTTYP,PRINT,I,REL,HEAD,NAME,ESE,EPSE,DAMAGE,
    & IELNO,I,ELDOP,KGDOF,PGECM,RINPUT,AXIAL,I3DDSP,I3DLOA,MSTOR)
    & IF (DUCFAG,GE,1) THEN
    & WRITE(6,3140) IELNO
    & CALL JOIDSP(1,MAINOD,NDOF,NNODE,1,L3,L4,ID,
    & IDOF,CNST,DISP,
    & TITLE,'DISPLACEMENTS',STEPID,.FALSE.,
    & NCOS,COSINE,JT COS,JTFLG)
    & DUCLOG=.TRUE.
    GO TO 3130
  ENDDIF
  3120 CONTINUE
  ENDDIF
  3140 FORMAT(' SYSTEM DISPLACEMENT ( FIRST ELEMENT REACH TO CRITICAL',
  & ' LOAD' /
  & '-----' /
  & ' ELEMENT ',I5,' FIRST REACH TO CRITICAL LOAD **')
  C
  C-----
  C= WRITE DATA TO OUTPUT FILES ---
  C-----
  C= CALCULATE AND PRINT DUCTILITY AND EXCURSION RATIOS ---
  C-----
  IOPT=11
  LSTTYP=0
  IF (WRITE) THEN
    PRINT=.TRUE.
    HEAD=.TRUE.
  ELSE
    PRINT=.FALSE.
    HEAD=.FALSE.
  ENDDIF
  312 CALL ELEMFB
  & (IOPT,LSTTYP,PRINT,I,REL,HEAD,NAME,ESE,EPSE,DAMAGE,
  & IELNO,I,ELDOP,KGDOF,PGECM,RINPUT,AXIAL,I3DDSP,I3DLOA,MSTOR)
  C
  C-----
  C= WRITE DATA TO OUTPUT FILES ---
  C-----
  IF (IWRITE.GT.0) THEN
    IF (MOD(ISTEP,IWRITE).EQ.0) THEN
      IOPT=2
      CALL DMPDAT(IOPT,IWRITE,TO,DT)
    ENDDIF
  C
  C-----
  C= CHECK ELAPSED CPU TIME ON THE UMRBX OR CORNELLF SYSTEM ---
  C-----
  DO 390 I=1,NDOF
  & WRITE(6,*) 'DISP(I,') GREATER THAN 300. PROGRAM STOP'
  GO TO 5000
  ENDDIF
  390 CONTINUE
  IF (PLACE.EQ.'UMRVM') THEN
    CALL TIMEIT(CPU)
    TCPU=CPU-TCPU
    ITLCPU=RBQCPCU
    CPUREN=RBQCPCU-TCPU
    CPUELA=ITLCPU-CPUREN

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C
C-----
C= FORMULATE STIFFNESS ----
C-----
I1=1
CALL FORM(11)
IF (BTEST(BUG,5)) CALL LMATRIX(STIFF,MD,NDOF,IMD,
& 'GLOBAL STIFFNESS MATRIX')
C
C-----
C= FORM GEOMETRIC STIFFNESS ---
C-----
C FORMULATE THE GEOMETRIC STIFFNESS IF THE FLAG NEWK IS TRUE, THIS
FLAG INITIALIZED ABOVE.
IF KGDATA(1)=1, THE GEOMETRIC STIFFNESS IS FORMED ONCE, AND
MULTIPLIED BY A SCALAR TO ACCOUNT FOR CHANGES
IN THE TOTAL VERTICAL LOAD. THUS THE GEOMETRIC
STIFFNESS ONLY NEEDS TO BE FORMED ONCE.
IF KGDATA(1)=2, THE GEOMETRIC STIFFNESS IS BASED ON THE AXIAL
FORCE IN THE ELEMENTS, AND IS RECALCULATED EACH
TIME THE AXIAL FORCE CHANGES.
IF KGDATA(3)=1, THE GEOMETRIC STIFFNESS IS FORMED WITH THE
STIFFNESS, THUS A SEPARATE CALL TO FORM IS
NOT NEEDED.
IF KGDATA(3)=2, A SEPARATE GEOMETRIC STIFFNESS MATRIX IS FORMED
C
C FORMULATE SEPARATE GEOMETRIC STIFFNESS MATRIX
IF (IOPT.EQ.2) THEN
KFORM=2
KGDATA=0
I2=2
CALL FORM(12)
IF (BTEST(BUG,5)) CALL LMATRIX(KG,MDKG,NDOF,IMDKG,
& 'GLOBAL GEOMETRIC STIFFNESS MATRIX')
ELSE
IF (BTEST(BUG,5)) CALL LMATRIX(MASS,MDMS,NDOF,IMDMS,
& 'GLOBAL MASS MATRIX')
ENDIF
C
C-----
C= CONDENSE OUT NON-FREE DOF ---
C-----
C... IF MCOND IS TRUE THE MASS MATRIX IS ALSO REDUCED BY GUYAN REDUCTIONS
C... IF KCOND IS TRUE THEN KG IS ALSO REDUCED
IF (MCOND.GT.0) THEN
I1=1
I1A=1
IF (IOPT.EQ.2) THEN
MCOND1=-.FALSE.
KCOND1=KCOND
ELSE IF (IOPT.EQ.1) THEN
MCOND1=-.FALSE.
KCOND1=MCOND
ENDIF
SET UP DUMMY ZERO LOAD VECTOR
DO 28 I=1,NDOF
TMP(I)=0
28 CALL MSOLL(1,BTEST(BUG,6),L4,IMD,NDOF,1,L1,L2,
& MD,STIFF,TMP,WORK,
& MCOND1,MASS,MDMS,IMDMS,
& KCOND1,KG,MDKG,IMDKG,ABORT)
IF (ABORT) RETURN
ENDIF
C
C-----
C= TRANSFER STIFFNESS INTO A MATRIX ---
C-----
C NOTE: THIS ROUTINE IS USED TO SOLVE FOR EIGENVALUES,
C THIS ROUTINE USES A 'FULL SYMMETRIC MATRIX'.
C
C NOTE: THE STORAGE METHOD USED FOR STIFFNESS, MASS AND
GEOMETRIC STIFFNESS ARE DIFFERENT.
C
QUIT=.FALSE.
DO 600 J=L3,L4,+1
DO 600 I=J,L3,-1
I=MD(J)+J-1
IF(IJ.LT.MD(J+1).AND.IJ.GE.MD(J)) THEN
A(I-L3+1,J-L3+1)=STIFF(I,J)
A(J-L3+1,I-L3+1)=A(I-L3+1,J-L3+1)
ELSE IF(IJ.GE.MD(J+1)) THEN
A(I-L3+1,J-L3+1)=0
A(J-L3+1,I-L3+1)=0
ENDIF
C
IF (BTEST(BUG,5)) WRITE (6,29) I,J,A(I,J)
29 FORMAT (' I ',I5, ' J ',J5, ' A(I,J)',1P,G15.5)
C
C-----
CHECK FOR ZERO'S ON MAIN DIAGONAL
IF (I.EQ.J .AND. A(I-L3+1,J-L3+1).LE.0) THEN
QUIT=.TRUE.
WRITE (6,32) I,A(I-L3+1,J-L3+1)
ENDIF
C
600 CONTINUE
32 FORMAT (5X,'DOF #',I5, ' IS .LE. 0 IN THE STIFFNESS MATRIX...',
& 5X,'K(I,I)=' ,1P,G12.4, ' SOLN IS ABORTED')
C
C-----
C= TRANSFER MASS OR GEOMETRIC STIFFNESS INTO B MATRIX ---
C-----
IF (IOPT.EQ.1) THEN
DO 700 J=L3,L4,+1
DO 700 I=J,L3,-1
IJ=MDMS(J)+J-1
IF(IJ.LT.MDMS(J+1).AND.IJ.GE.MDMS(J)) THEN
B(I-L3+1,J-L3+1)=MMS(I,J)
B(J-L3+1,I-L3+1)=B(I-L3+1,J-L3+1)
ELSE IF(IJ.GE.MDMS(J+1)) THEN
B(I-L3+1,J-L3+1)=0
B(J-L3+1,I-L3+1)=0
ENDIF
IF (BTEST(BUG,5)) WRITE (6,39) I,J,B(I,J)
39 FORMAT (' I ',I5, ' J ',J5, ' B(I,J)',1P,G15.5)
C
C-----
CHECK FOR ZERO'S ON MAIN DIAGONAL
IF (I.EQ.J .AND. B(I-L3+1,J-L3+1).LE.0) THEN
QUIT=.TRUE.
WRITE (6,42) I,B(I-L3+1,J-L3+1)
ENDIF
C
700 CONTINUE
42 FORMAT (5X,'DOF #',I5, ' IS .LE. 0 IN THE MASS MATRIX...',
& 5X,'M(I,I)=' ,1P,G12.4, ' SOLN IS ABORTED')
ELSE
DO 800 J=L3,L4,+1
DO 800 I=J,L3,-1
IJ=MDKG(J)+J-1
IF(IJ.LT.MDKG(J+1).AND.IJ.GE.MDKG(J)) THEN
B(I-L3+1,J-L3+1)=KG(I,J)
B(J-L3+1,I-L3+1)=B(I-L3+1,J-L3+1)
ELSE IF(IJ.GE.MDKG(J+1)) THEN
B(I-L3+1,J-L3+1)=0
B(J-L3+1,I-L3+1)=0
ENDIF
IF (BTEST(BUG,5)) WRITE (6,49) I,J,B(I,J)
49 FORMAT (' I ',I5, ' J ',J5, ' B(I,J)',1P,G15.5)
C
C-----
CHECK FOR ZERO'S ON MAIN DIAGONAL
IF (I.EQ.J .AND. B(I-L3+1,J-L3+1).LE.0) THEN
QUIT=.TRUE.

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SOL00620 WRITE (6,42) I,B(I-L3+1,J-L3+1) SOL02040
SOL00640 ENDIF SOL02050
SOL00650 C SOL02060
SOL00660 C 800 CONTINUE SOL02070
SOL00670 52 FORMAT (5X,'DOF #',I5, SOL02080
SOL00680 & ' IS .LE. 0 IN THE GEOMETRIC STIFFNESS MATRIX...', SOL02090
SOL00690 & 5X,'KG(I,I)=' ,1P,G12.4, ' SOLN IS ABORTED') SOL02110
SOL00700 C ENDIF SOL02120
SOL00710 C----- RETURN IF A MAIN DIAGONAL IS LE ZERO SOL02130
SOL00720 C IF (QUIT) RETURN SOL02140
SOL00730 C SOL02150
SOL00740 C----- SOL02160
SOL00750 C= SOLVE FOR EIGENVALUES AND EIGENVECTORS --- SOL02170
SOL00760 C----- SOL02180
SOL00770 C= INSL SU BRROUTINE DGVCSF ----- SOL02190
SOL00780 C EIGENVALUES AND EIGENVECTORS OF A*X-LAMBDA*B*X=0 SOL02210
SOL00790 C A = STIFFNESS REAL, FULL SYMMETRIC MATRIX SOL02220
SOL00800 C B = MASS OR GEOMETRIC STIFFNESS - REAL, FULL SYMMETRIC MATRIX SOL02230
SOL00810 C N = NUMBER OF DOF OF A AND B SOL02240
SOL00820 C PI = PERFORMANCE INDEX SOL02250
SOL00830 C PI=NK(1), PI<1 - ROUTINE PERFORMED WELL SOL02260
SOL00840 C 100<PI<100 - ROUTINE PERFORMED SATISFACTORILY SOL02270
SOL00850 C 100<PI - ROUTINE PERFORMED POORLY SOL02280
SOL00860 C EVAL= EIGENVALUES (LAMBDA), N BY 1 SOL02290
SOL00870 C EVEC= EIGENVECTORS, N BY N SOL02300
SOL00880 C LDA = LEADING DIMENSION OF MATRIX A EXACTLY AS SPECIFIED IN THE SOL02310
SOL00890 C DIMENSION STATEMENT IN THE CALLING PROGRAM. SOL02320
SOL00900 C LDA = LEADING DIMENSION OF MATRIX B EXACTLY AS SPECIFIED IN THE SOL02330
SOL00910 C DIMENSION STATEMENT IN THE CALLING PROGRAM. SOL02340
SOL00920 C LDEVEC= LEADING DIMENSION OF MATRIX EVEC EXACTLY AS SPECIFIED IN THE SOL02350
SOL00930 C DIMENSION STATEMENT IN THE CALLING PROGRAM. SOL02360
SOL00940 C----- SOL02370
SOL00950 C----- SOL02380
SOL00960 C LDEVEC=NFREE SOL02390
SOL00970 C CALL MKIN(10000) SOL02400
SOL00980 C CALL DGVCSF(NFREE,A,NFREE,B,NFREE,EVAL,EVEC,LDEVEC) SOL02410
SOL00990 C PI = DGVCSF(NFREE,NFREE,A,NFREE,B,NFREE,EVAL,EVEC,LDEVEC) SOL02420
SOL01000 C WORK(1)=PI SOL02430
SOL01010 C CALL DGVCSF(NFREE,A,NFREE,B,NFREE,EVAL,EVEC,LDEVEC,A,NK2,WORK) SOL02440
SOL01020 C WRITE (6,* ) ' PERFORMANCE INDEX',WORK(1) SOL02450
SOL01030 C----- SOL02460
SOL01040 C= NORMALIZE MODE SHAPE TO MAX VALUE OF ONE --- SOL02470
SOL01050 C----- SOL02480
SOL01060 C NOTE: THE EIGENVECTOR IN INSL DGVCSF HAS BEEN NORMALIZED SOL02490
SOL01070 C SOL02500
SOL01080 C= PRINT EIGENVALUES AND EIGENVECTORS --- SOL02510
SOL01090 C----- SOL02520
SOL01100 C 100 IEND=MIN(IST+6,IPRT) SOL02530
SOL01110 C WRITE (6,110) (I ,I=IST,IEND) SOL02540
SOL01120 IF (IOPT.EQ.2) THEN SOL02550
SOL01130 WRITE (6,120) (EVAL(I),I=IST,IEND) SOL02560
SOL01140 ELSE IF (IOPT.EQ.1) THEN SOL02570
SOL01150 DO 125 I=IST,IEND SOL02580
SOL01160 125 EVAL(I)=SQRT( EVAL(I) ) SOL02590
SOL01170 WRITE (6,126) (EVAL(I),I=IST,IEND) SOL02600
SOL01180 DO 127 I=IST,IEND SOL02610
SOL01190 127 EVAL(I)=EVAL(I) / 6.2831853 SOL02620
SOL01200 WRITE (6,128) (EVAL(I),I=IST,IEND) SOL02630
SOL01210 DO 129 I=IST,IEND SOL02640
SOL01220 129 IF (EVAL(I).EQ.0) EVAL(I)=1.E-15 SOL02650
SOL01230 WRITE (6,130) (EVAL(I),I=IST,IEND) SOL02660
SOL01240 ENDIF SOL02670
SOL01250 WRITE (6,135) MODE#,7(I10,5X) SOL02680
SOL01260 110 FORMAT(' MODE#:',7(I10,5X)) SOL02690
SOL01270 120 FORMAT(' EIGENVALUE#',1P,7(G14.5,1X) ) SOL02700
SOL01280 126 FORMAT(' FREQ (RAD/SEC)',1P,7(G14.5,1X) ) SOL02710
SOL01290 128 FORMAT(' FREQUENCY (HZ)',1P,7(G14.5,1X) ) SOL02720
SOL01300 130 FORMAT(' PERIOD (SEC)',1P,7(G14.5,1X) ) SOL02730
SOL01310 135 FORMAT(' EIGENVECTORS') SOL02740
SOL01320 C----- EIGENVECTORS SOL02750
SOL01330 DO 140 I=1,NFREE SOL02760
SOL01340 II=(NCOND) SOL02770
SOL01350 140 WRITE (6,150) II,(EVEC(I,J),J=IST,IEND) SOL02780
SOL01360 150 FORMAT(' DOF#',I6,')',1P,7(G14.5,1X) ) SOL02790
SOL01370 C IF (IEND.LT.IPRT) THEN SOL02800
SOL01380 WRITE (6,10) TITLE(1),TITLE(2),OPTION,IPRT SOL02810
SOL01390 IST=IEND+1 SOL02820
SOL01400 GO TO 100 SOL02830
SOL01410 ENDIF SOL02840
SOL01420 C----- SOL02850
SOL01430 C= CALC EIGEN VECTORS FOR NONDYNAMIC DOF --- SOL02860
SOL01440 C----- SOL02870
SOL01450 C----- TRANSFER EIGENVECTORS TO TEMP STORAGE SOL02880
SOL01460 C----- SOL02890
SOL01470 DO 500 IPREQ=1,IPRT2 SOL02900
SOL01480 C----- TRANSFER EIGENVECTORS TO TEMP STORAGE SOL02910
SOL01490 DO 200 J=L1,L2 SOL02920
SOL01500 200 WORK(J)=0 SOL02930
SOL01510 DO 210 I=L3,L4 SOL02940
SOL01520 210 WORK(I)=EVEC(J-L2,IPREQ) SOL02950
SOL01530 DO 220 J=L5,L6 SOL02960
SOL01540 220 WORK(J)=0 SOL02970
SOL01550 C----- SOL02980
SOL01560 C----- SOL02990
SOL01570 C----- SOL03000
SOL01580 C----- SOL03010
SOL01590 C----- SOL03020
SOL01600 C----- SOL03030
SOL01610 C----- SOL03040
SOL01620 200 WORK(J)=0 SOL03050
SOL01630 DO 210 I=L3,L4 SOL03060
SOL01640 210 WORK(I)=EVEC(J-L2,IPREQ) SOL03070
SOL01650 DO 220 J=L5,L6 SOL03080
SOL01660 220 WORK(J)=0 SOL03090
SOL01670 C----- CALC EIGENVECTORS FOR CONDENSED OUT DOF SOL03100
SOL01680 CALL REACTN(11,BTEST(BUG,6),NNODE,L1,L2,L3,L4,TMP,WORK, SOL03110
SOL01690 & MD,STIFF,IMD,NDOF,1,1,0, SOL03120
SOL01700 & MAXNOD,IDOF,COORD,ID,TITLE,STEPID,.FALSE., SOL03130
SOL01710 & CALL MSOLL(3,BTEST(BUG,6),L4,IMD,NDOF,11,L1,L2, SOL03140
SOL01720 & MD,STIFF,TMP,WORK, SOL03150
SOL01730 & .FALSE.,MASS,MDMS,1, SOL03160
SOL01740 & IF (ABORT) RETURN SOL03170
SOL01750 C----- SOL03180
SOL01760 C----- SOL03190
SOL01770 C----- NORMALISE MODE SHAPE TO MAX VALUE OF ONE SOL03200
SOL01780 C----- SOL03210
SOL01790 AMIN=0 SOL03220
SOL01800 AMAX=0 SOL03230
SOL01810 DO 230 I=1,L6 SOL03240
SOL01820 230 AMIN=MIN(AMIN,WORK(I)) SOL03250
SOL01830 AMAX=MAX(AMAX,WORK(I)) SOL03260
SOL01840 IF (-AMIN.GT.AMAX) AMAX=AMIN SOL03270
SOL01850 SOL03280
SOL01860 SOL03290
SOL01870 DO 240 I=1,L6 SOL03300
SOL01880 240 WORK(I)=WORK(I)/AMAX SOL03310
SOL01890 C----- PRINT EIGENVALUE AT EACH NODE SOL03320
SOL01900 IF (IOPT.EQ.2) WRITE (STEPID,250) IPREQ,EVAL(IPREQ) SOL03330
SOL01910 IF (EVAL(IPREQ).EQ.0) EVAL(IPREQ)=1.E-15 SOL03340
SOL01920 FREQ=1./EVAL(IPREQ) SOL03350
SOL01930 IF (IOPT.EQ.1) WRITE (STEPID,260) IPREQ,FREQ SOL03360
SOL01940 C----- SOL03370
SOL01950 CALL JOIDSP(1,MAXNOD,NDOF,NNODE,1,L3,L4,1D, SOL03380
SOL01960 & ID,ICSP,WORK, SOL03390
SOL01970 & TITLE,'EIGENVECTORS',STEPID,.TRUE., SOL03400
SOL01980 & NCOS,COSINE,JTCS,JTFUG) SOL03410
SOL01990 250 FORMAT ('MODE #',I3, ' EIGENVALU#',1P,G15.6) SOL03420
SOL02000 260 FORMAT ('MODE #',I3, ' FREQUNCT#',1P,G15.6) SOL03430
SOL02010 C----- PRINT EIGENVALUE TO DISK FOR PLOTTING SOL03440
SOL02020 SOL03450
SOL02030
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4 STIFF , MASS , KG , ELD , UBCLOAD ,
4 MD , MDMS , MDKG , IELD , DDISP ,
4 ID , IDOP , CNST , JTFPLG , COORD ,
4 COSINE , JTCOS , EIE, ESE, PSE, EKE, EDD,
4 FG , DFG , ) EIE, ESE, PSE, EKE, EDD,
C
DOUBLE PRECISION EIE, ESE, PSE, EDD, EKE
COMMON /IDSTEP/ISTEP
CHARACTER*(*) TITLE(2), STEPID
LOGICAL PRINT, DSPFLG, HEAD, REPEAT, AXIAL, TEST, NEWKOT, BTEST, ABORT
LOGICAL NEWK, NEWK, MCND, KGCND, UNBAL, MCND2, KGCND2, WRITE
LOGICAL BUG3, BUG5, BUG6, BUG11
CHARACTER*80 NAME
CHARACTER*20 INTEG
REAL LOAD, MASS, MASSSS, KG, MCOS
INTEG IDAMP, PMASS
DIMENSION SUMRC(6), DSUMRC(6), SUMFD(6), DSUMFD(6), SUMRCM(6), SUMUB(6)
DIMENSION DSPMAX(L6), RCTMAX(L6), DRLOA(L6), RLOAD(L6)
DIMENSION RINPUT(100)
DIMENSION STIFF(IMD), MASS(IMDMS), KG(IMDKG)
DIMENSION MD(L6+1), MDMS(L6+1), MDKG(L6+1)
DIMENSION LOAD(L6), DISP(L6), VEL(L6), Q(L6), PG(L6), DFG(L6)
DIMENSION DLOAD(L6), DDISP(L6), DVEL(L6), R(L6)
DIMENSION FUB(L6), UBCLOAD(L6), UBDISP(L6)
DIMENSION WORK(216)
DIMENSION COORD(MAXNOD, 3), ID(MAXNOD),
4 IDOP(MAXNOD, 6), COSINE(3, 3, NCOS), CNST(MAXNOD, 6),
4 JTFPLG(MAXNOD), JTCOS(MAXNOD)
DIMENSION IELD(1), ELD(1)
DIMENSION CA(3), GV(3), GD(3)
DIMENSION GAT(3), GVT(3), GDT(3)
LOGICAL NEWFAC
CHARACTER*10 STEP, STEP2
C
SUMUB(1)=0
SUMUB(2)=0
SUMUB(3)=0
SUMUB(4)=0
SUMUB(5)=0
SUMUB(6)=0
SUMFD=0
SUMRC=0
SUMF=0
SUMM=0
SUMMM=0
C-----
BUG3 = BTEST(BUG, 3)
BUG5 = BTEST(BUG, 5)
BUG6 = BTEST(BUG, 6)
BUG11 = BTEST(BUG, 11)
C-----
C= INITIALIZE ENERGY CONSTANTS ==
C-----
EIE=0
ESE=0
PSE=0
EKE=0
EDD=0
C
C= SET GEOMETRIC STIFFNESS FLAGS ==
C-----
NEWK=.TRUE.
IF (KGLOAD.EQ.0 .OR. KGTYPE.EQ.0 .OR. KGFORM.EQ.0) NEWK=.FALSE.
C
C= SET STIFFNESS FLAGS ==
C-----
NEWK = .TRUE.
KSAME= 0
C
C= INPUT LOADING DATA ==
C-----
MATRICES Q AND R ARE ZEROED IN THE CALLING PGK.
C
C----- INPUT JOINT LOADS -----
JOINT LOADS AND SUPPORT DISPLACEMENTS ARE STORED IN Q
CALL JOILOA(BUG3, MAXNOD, NDOF, NLOAD, L4,
4 ID, IDOP, CNST, 2, DSPFLG, JTFPLG, TITLE, .FALSE.)
C
C----- INPUT ELEMENT LOADS -----
ELEMENT LOADS ARE STORED IN R
IF (MAXELD.GT.0)
4 CALL ELRLOA(BUG3, L5, NEMT, MAXELD, NELD, NDOF, NLOAD,
4 R, IELD, ELD, IDISP, IZLOAD, NAME, TITLE, .FALSE.)
C
C= WRITE INITIAL DATA TO OUTPUT FILE ==
C-----
IF (IWRITE.GT.0) THEN
IOPT=2
CALL DMPDAT(IOPT, IWRITE, TO, DT)
ENDIF
C
C-----
C= LOOP FOR EACH TIME STEP , ISTEP=CURRENT STEP # -----
C... ISTEP = STEP NUMBER
C
C= FACTOR= CURRENT TARGET LOAD = FACTOR*(INPUT LOAD)
C
C= FACTL = PREVIOUS TARGET LOAD
C
C= FACT2 = NEXT TARGET LOAD
C
C= DF = LOAD STEP
C
C= DFL = PREVIOUS LOAD STEP
C
C= DFN = NEXT LOAD STEP
C
C= PMAX = MAXIMUM ALLOWABLE INCREMENTAL LOAD / STEP
C
C= DMAX = MAXIMUM ALLOWABLE INCREMENTAL DISPLACEMENT / STEP
C
ISTEP=0
FACTL=0
DF=0
READ(5, *) STEP2, FACT2, PMAX2, DMAX2, NUMB2
1000 IF (STEP2 .EQ. STEP) GO TO 5000
IF (.TEST(STEP, 'END', .FALSE.)) GO TO 5000
FACTOR=FACT2
PMAX = PMAX2
DMAX = DMAX2
NUMB = NUMB2
NUMSDP=NUMB2
READ(5, *, END=1010) STEP2, FACT2, PMAX2, DMAX2, NUMB2
GO TO 1020
1010 STEP2='END'
FACTL=FACTOR
1020 IF (.TEST(STEP, 'END', .FALSE.)) GO TO 5000
IF (FACTOR.EQ.FACTL) GO TO 1000
1100 ISTEP=ISTEP+1
WRITE (6,90) STEP, ISTEP, FACTOR
IF (BUG3.OR.BUG5.OR.BUG6.OR.BUG11) THEN
WRITE (6,91) STEP, ISTEP, FACTOR
WRITE (6, *) 'NEWK1', 'NEWK', 'NEWK2', 'NEWK3'
ENDIF
90 FORMAT (A, ' STEP', I5, ' FACTOR', I5, G11.4)
91 FORMAT ('1', A, ' STEP', I5, ' FACTOR', I5, G11.4)
C
C-----
C= CALC INCREMENTAL LOADS ==
C-----
C 1) ADD ELEMENT LOADS, AND JOINT LOADS ON FREE JOINTS
C 2) SAVE IN DLOA FOR CALCULATING UNBALANCED FORCES FOR THE NEXT STEP
C 3) PUT THE NEGATIVE ELEMENT LOAD IN DLOA FOR RESTRAINED JOINTS, THIS
C IS USED TO CALCULATE REACTIONS.
C 4) ZERO THE UNBALANCED LOAD VECTOR
C IS USED TO CALCULATE REACTIONS.
C
DF=DF
DFN=FACTOR*FACTL
DFN=FACT2-FACTOR
IF (PMAX.EQ.0 .AND. DMAX.EQ.0 .AND. NUMB.GT.0) DF=DF/NUMB
IF (DF.EQ.0) GO TO 1000
C
DO 106 I=L1, L4
DLOAD(I)=Q(I)+R(I)*DF
C
... SWAP THE SIGN OF THE FEB AT REACTIONS
DO 110 I=L5, L6
DLOAD(I)=-R(I)*DF
110 DDISP(I)=Q(I)*DF
C
C-----
C= FORMULATE STIFFNESS ==
C-----
C FORMULATE THE STIFFNESS IF THE FLAG NEWK IS TRUE, THIS FLAG IS
INITIALIZED ABOVE, AND SET WHEN THE ELEMENT FORCES ARE CALC.
THE STIFFNESS IS ALSO FORMULATED IF KG IS TO BE FORMED
REPEAT=.TRUE.
KSAME=0
100 IF (NEWK.OR.NEWK2) THEN
11=1
CALL FORM(11)
IF (KGLOAD.EQ.1) NEWK=.FALSE.
ENDIF
C
C= MODIFY LOADS FOR RESTRAINT DISPLACEMENTS ==
C-----
IF (DSPFLG) THEN
C----- ZERO INCREMENTAL REACTION LOAD VECTOR -----
DO 113 I=L1, L4
DRLOA(I)=0
113 CALL REACTN(1, BUG3, NNODE, L1, L4, L5, L6, DRLOA, DDISP,
4 MD, STIFF, IMD, NDOF, 1, 1, 0,
4 MAXNOD, IDOP, COORD, ID, TITLE, STEPID, .FALSE.,
4 NCOS, COSINE, JTCOS, JTFPLG, SUMRC)
C
C----- ADD INCREMENTAL REACTION LOADS TO OTHER LOADS -----
DO 114 I=L1, L4
114 DLOAD(I) = DLOAD(I) + DRLOA(I)
ENDIF
C
C= SAVE IN LOADS IN DDFP FOR CALCULATING FREE DISPL FOR THIS STEP.
DO 115 I=L1, L4
DDISP(I) = DLOAD(I)
115
C
C= SOLVE FOR FREE DISPL ==
C-----
IF (BUG5) THEN
IF (NEWK .OR. NEWK2)
4 CALL LMATRX(STIFF, MD, NDOF, IMD, 'GLOBAL STIFFNESS DUMP=7')
4 CALL WMATRX(DLOAD, NDOF, NLOAD, 'LOAD MATRIX DUMP=7')
ENDIF
C
IF (NEWK .OR. NEWK2) THEN
CALL MSOLL(1, .FALSE., L4, IMD, NDOF, NLOAD, L1, L4,
4 MD, STIFF, DDISP, WORK, .FALSE., MASS, MDMS, 1, .FALSE., KG, MDKG, 1,
4 ABORT)
IF (ABORT) GO TO 5000
ELSE
IF CALL MSOLL(2, .FALSE., L4, IMD, NDOF, NLOAD, L1, L4,
4 MD, STIFF, DDISP, WORK, .FALSE., MASS, MDMS, 1, .FALSE., KG, MDKG, 1,
4 ABORT) GO TO 5000
ENDIF
CALL MSOLL(3, .FALSE., L4, IMD, NDOF, NLOAD, L1, L4,
4 MD, STIFF, DDISP, WORK, .FALSE., MASS, MDMS, 1, .FALSE., KG, MDKG, 1,
4 ABORT)
IF (ABORT) GO TO 5000
C
IF (BUG5)
4 CALL WMATRX(DDISP, NDOF, NLOAD, 'DISPLACEMENT DUMP=7')
C
C= SCALE SOLN SUCH THAT PM LE PMAX, DM LE DMAX ==
C-----
DM=0
PM=0
DO 130 I=L3, L4
PM=MAX(DLOAD(I), -DLOAD(I), PM)
DM=MAX(DDISP(I), -DDISP(I), DM)
130 IF ((DM.GT.DMAX .AND. DMAX.NE.0) .OR.
4 (PM.GT.PMAX .AND. PMAX.NE.0)) THEN
F1=0
F2=0
IF (DMAX.NE.0) F1=DM/DMAX
IF (PMAX.NE.0) F2=PM/PMAX
F=MAX(F1, F2)
DF=DF/F
NEWFAC=.FALSE.
DO 140 I=L1, L4
DLOAD(I)=DLOAD(I)/F
DDISP(I)=DDISP(I)/F
140 DDFN=1
ELSE IF (DMAX.EQ.0 .AND. DMAX.EQ.0 .AND. NUMB.GT.1) THEN
NEWFAC=.FALSE.
NUMB=NUMB-1
ELSE
NEWFAC=.TRUE.
DDFN=DF*DFN
ENDIF
C
AFAC=FACTL*DF
C
C= SOLVE FOR DISPL DUE TO UNBALANCED FORCES ==
C-----
C..... USE FIRST STIFFNESS TO CALCULATE UNBALANCED FORCES.
IF THE STEP IS REPEATED, DO NOT USE THE NEW STIFFNESS.
IF (UNBAL.AND.REPEAT) THEN
DO 104 I=L1, L4
UBLOAD(I) = -FUB(I)
DDISP(I) = -FUB(I)
104 DO 105 I=L5, L6
UBLOAD(I) = -FUB(I)
105 UBDISP(I) = 0.00
IF (BUG5)
4 CALL WMATRX(UBDISP, NDOF, NLOAD, 'UNBALANCED LOAD MATRIX')
4 CALL MSOLL(2, .FALSE., L4, IMD, NDOF, NLOAD, L1, L4,
4 MD, STIFF, UBDISP, WORK, .FALSE., MASS, MDMS, 1, .FALSE., KG, MDKG, 1,
4 ABORT)
IF (ABORT) GO TO 5000
CALL MSOLL(3, .FALSE., L4, IMD, NDOF, NLOAD, L1, L4,
4 MD, STIFF, UBDISP, WORK, .FALSE., MASS, MDMS, 1, .FALSE., KG, MDKG, 1,
4 ABORT)
IF (ABORT) GO TO 5000
IF (BUG5)
4 CALL WMATRX(UBDISP, NDOF, NLOAD, 'UNBALANCED DISPLACEMENT')
C

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CALL DMPDAT(10PT,IWRITS,TO,DT)
ENDIF
C
C-----PRINT MAXIMUM DISPLACEMENTS-----
C PRINT MAXIMUM DISPLACEMENTS ==
C-----PRINT MAXIMUM GLOBAL REACTIONS-----
C PRINT MAXIMUM GLOBAL REACTIONS ==
WRITE(6,250) TITLE(1),TITLE(2),0,'MAXIMUM REACTIONS'
CALL JOIDSP(2,MAXNOD,NDOF,NMODE,1,L5,L6,10,
& ND,STIFF,IND,NDOF,1,1,0,
& MAXNOD,INDOF,COORD,1D,TITLE,'MAXIMUM VALUES',.FALSE.,
& NCOS,COSINE,JTCOS,JTFLG)
5111 FORMAT (/4X,'MAXIMUM RESULTANT OF REACTIONS, FORCE='',IP,G12.4,
& MOMENT='',G12.4)
C
C-----PRINT MAXIMUM MEMBER FORCES-----
C PRINT MAXIMUM MEMBER FORCES ==
LSTTYP=0
HEAD=.FALSE.
PRINT=.TRUE.
WRITE(6,250) TITLE(1),TITLE(2),0,'PEAK ELEMENT FORCES'
5300 CALL ELBEB
& (12) LSTTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,
& DUCFAG,
& IELNO,IELDOF,KGDF,PGEOM,RINPOT,AXIAL,I3DDSP,I3DLOA,MSTOR)
C
C-----PRINT DUCTILITIES AND EXCURSION RATIOS-----
C PRINT DUCTILITIES AND EXCURSION RATIOS ==
LSTTYP=0
HEAD=.FALSE.
PRINT=.TRUE.
WRITE(6,250) TITLE(1),TITLE(2),0,
& 'PEAK DUCTILITIES AND EXCURSION RATIOS'
5310 CALL ELBEB
& (11) LSTTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,
& DUCFAG,
& IELNO,IELDOF,KGDF,PGEOM,RINPOT,AXIAL,I3DDSP,I3DLOA,MSTOR)
C
C-----PRINT DAMAGE INDICES-----
C PRINT DAMAGE INDICES ==
LSTTYP=0
HEAD=.FALSE.
PRINT=.TRUE.
WRITE(6,250) TITLE(1),TITLE(2),0,
& 'DAMAGE INDEX'
DAMAGE=0
DO 5320 IELNO=1,NELMT
CALL ELBEB
& (14) LSTTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,EDAM,
& DUCFAG,
& IELNO,IELDOF,KGDF,PGEOM,RINPOT,AXIAL,I3DDSP,I3DLOA,MSTOR)
5320 DAMAGE=DAMAGE+DAM
IF( (ESE+PSE).EQ.0 ) RETURN
DAMAGE=DAMAGE/(ESE+PSE)
WRITE(6,5330) DAMAGE
5330 FORMAT (/10X,'STRUCTURE DAMAGE INDEX='',IP,G15.5)
RETURN
C
END
CPROCESS SDUMP OPT(0) GOSTMT XREF MAP
C-----
C DEBUG UNIT(6),SUBCHK,SUBTRACE,INIT
C ENDBUG
C-----
SUBROUTINE SOL5A
LOGICAL PRINT,HEAD,BTEST,TEST,QUIT
COMMON /RSA/ ICOM,RSAP(12)
REAL ICOMN
DIMENSION RINPOT(100)
CHARACTER*20 ICOM
INCLUDE (ZCOMN)
C
C-----PRINT HEADER-----
C PRINT HEADER
C-----
WRITE(6,10)TITLE(1),TITLE(2)
10 FORMAT('1 STRUCTURE.....',A,/,
& ' SOLUTION.....',A,/)
& DO 555 I=1,20
555 WRITE(6,*)
WRITE(6,557)
557 FORMAT(4X,'** RESPONSE SPECTRUM ANALYSIS **',/,
& 40X,' NOTE: MODAL COMBINATION APPROACH CAN BE SRSS ',
& 'OR CQC')
C
C-----INPUT DATA-----
C INPUT DATA ==
C-----
READ(5,*)ICOM,NMODE,DAMP,NA,NAP,IPRT
READ(5,*)NSTEP,MEIG
C NOTE: NSTEP = 1 : CALCULATE EIGENPAIRS, STORE EIGENPAIRS IN DISK MEIG
C NSTEP = 2 : READ EIGENPAIR FROM 'MEIG' AND DO SPECTRUM ANALYSIS
C NSTEP = 3 : CALCULATE EIGENPAIRS AND PERFORM SPECTRUM ANALYSIS,
C EIGENPAIRS ARE NOT STORED IN UNIT MEIG
C
WRITE(6,10)TITLE(1),TITLE(2)
IF( TEST(ICOM,'SRSS'.) THEN
ICOMN=1
WRITE(6,11)'SRSS MODAL COMBINATION METHOD'
ELSE IF( TEST(ICOM,'CQC'.) .FALSE.) THEN
ICOMN=2
WRITE(6,11)'CQC MODAL COMBINATION METHOD'
ELSE
WRITE(6,*)'INVALID MODAL COMBINATION METHOD'
WRITE(6,11) ICOM,
RETURN
ENDIF
C
WRITE(6,12)NMODE,DAMP,NA,NAP
11 FORMAT('1 SOLUTION #5, ',A,/,
& ' ',A,/)
12 FORMAT('1X, ' NUMBER OF MODES CONSIDERED .....',OP,14,/,
& '1X, ' DAMPING RATIO FOR ALL MODES .....',OP,G12.3,/,
& '1X, ' NO. OF SEISMIC COMPONENTS CONSIDERED .....',OP,14,/,
& '1X, ' NO. OF INPUT POINTS FOR SPECTRUM .....',OP,14)
C
IF(NSTEP.EQ.1) THEN
WRITE(6,13)NSTEP,MEIG
13 FORMAT('1X, ' EIGEN-SOLUTION IS STORED IN DISK UNIT .....',OP,14,/,
& '1X, ' EIGEN-SOLUTION IS STORED IN DISK UNIT .....',OP,14)
ELSE IF(NSTEP.EQ.2) THEN
WRITE(6,14)NSTEP,MEIG
14 FORMAT('1X, ' NSTEP .....',OP,14,/,
& '1X, ' EIGEN-SOLUTION IS READ FROM DISK UNIT .....',OP,14)
ENDIF

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SOL05828 C
SOL05830 C
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COMMON/NSA/ICOMN,RSAP(12)
REAL ICOMN
C-- COMMON BLOCK FOR IMSL SUBROUTINE DGVCSF
C
COMMON/WORKSP/RWKSP
C
REAL RWKSP(100000)
LOGICAL TEST,QUIT,BTEST,MCOND,KGCOND,MCONDI,KCONDI,ABORT,PRINT
LOGICAL HEAD,AXIAL
REAL LOAD, MASS, KG, KGDATA
DIMENSION STIFF(LMD),MASS(LMOMS),KG(LMDKG),RINPUT(100)
DIMENSION A(NFREE,NFREE),B(NFREE,NFREE)
DIMENSION MD(LS+1),MOMS(LS+1),MDKG(LS+1),DISP(L6),TMP(L6)
DIMENSION EVAL(NFREE),EVEC(NFREE,NFREE),SUMRCT(6)
DIMENSION COORD(MAXNOD,3),ID(MAXNOD),
& IDOF(MAXNOD,6),COSINE(3,3,NCOS),CNST(MAXNOD,6),
& CTFC(MAXNOD),JTCOS(MAXNOD)
DIMENSION G(6),D(6),TMP2(NFREE),EFP(NFREE,NA),CQCM2(NFREE,NMODE)
DIMENSION V1(3),V2(3),V3(3)
DIMENSION RA(NMODE,3),LOAD(L6)
DIMENSION RD(NMODE,3),DLOAD(L6,NA)
DIMENSION PLU(NFREE,3),CMC(NMODE,NMODE)
DIMENSION GLM(NMODE),ELF(NMODE,3),WORK(L4),CQCM(L4,NMODE)
DIMENSION EFG(NELMT,12,NMODE+NA),RCT(6,NMODE+NA),RSNP(2,NAP)
DIMENSION WK2(80,80)
C NOTE: WK2 DIMENSION SHOULD BE GREATER THAN NFREE*NFREE
C
CHARACTER*1 NAME
CHARACTER*40 OPTION
CHARACTER*(*) TITLE(2),STEPID
C
C-----
C FORMULATE STIFFNESS ----
C-----
I1=1
CALL FORM(I1)
IF (BTEST(1BUG,5)) CALL LMATRIX(STIFF,MD,NDOF,LMD,
& 'GLOBAL STIFFNESS MATRIX')
C
C-----
C CONDENSE OUT NON-FREE DOP ---
C-----
C... IF MCONDI IS TRUE THE MASS MATRIX IS ALSO REDUCED BY GUYAN REDUCTION
IF (MCONDI.GT.0) THEN
I1=1
I1A=1
KCONDI=.FALSE.
MCONDI=MCOND
C SET UP ZERO LOAD VECTOR
DO 28 I=1,NDOF
TMP(I)=0
28 CALL MSOLL(I,BTEST(1BUG,6),L4,LMD,NDOF,1,1,1,L2,
& MD,STIFF,TMP,WORK,
& MCONDI,MASS,MOMS,LMOMS,
& KCONDI,KG,MDKG,IMDKG,ABORT)
IF (ABORT) RETURN
ENDIF
C
C-----
C TRANSFER STIFFNESS INTO A MATRIX ---
C-----
C NOTE: IMSL ROUTINE DGVCSF IS USED TO SOLVE FOR EIGENVALUES,
C THIS ROUTINE USES A 'FULL SYMMETRIC MATRIX'.
C
QUIT=.FALSE.
DO 600 J=L3,L4,+1
DO 600 I=J,L3,-1
I=MD(J)+J-1
IF(IJ.LT. MD(J+1).AND. IJ.GE. MD(J)) THEN
A(I-L3+1,J-L3+1)=STIFF(IJ)
A(J-L3+1,I-L3+1)=A(I-L3+1,J-L3+1)
ELSE IF (IJ.GE. MD(J+1)) THEN
A(I-L3+1,J-L3+1)=0
A(J-L3+1,I-L3+1)=0
ENDIF
C
IF (BTEST(1BUG,5)) WRITE (6,29) I,J,A(I-L3+1,J-L3+1)
29 FORMAT (' I ',I5, ' J ',I5, ' A(I-L3+1,J-L3+1)',1P,G15.5)
C
C-----
C CHECK FOR ZERO'S ON MAIN DIAGONAL
IF (I.EQ.J.AND. A(I-L3+1,J-L3+1).LE.0) THEN
QUIT=.TRUE.
WRITE (6,32) I,A(I-L3+1,J-L3+1)
ENDIF
C
600 CONTINUE
32 FORMAT ('SK, 'DOP #',I5, ' IS .LE. 0 IN THE STIFFNESS MATRIX...',
& 'SK, 'K(I,I)',1P,G12.4, ' SOLN IS ABORTED')
C
C-----
C TRANSFER MASS INTO B MATRIX ---
C-----
DO 700 J=L3,L4,+1
DO 700 I=J,L3,-1
I=MD(J)+J-1
I3=MD(J)+J-1
IF (IJ.LT. MD(J+1).AND. IJ.GE. MD(J)) THEN
B(I-L3+1,J-L3+1)=MOMS(IJ)
B(J-L3+1,I-L3+1)=B(I-L3+1,J-L3+1)
ELSE IF (IJ.GE. MD(J+1)) THEN
B(I-L3+1,J-L3+1)=0
B(J-L3+1,I-L3+1)=0
ENDIF
C
IF (BTEST(1BUG,5)) WRITE (6,39) I,J,B(I-L3+1,J-L3+1)
39 FORMAT (' I ',I5, ' J ',I5, ' B(I-L3+1,J-L3+1)',1P,G15.5)
C
C-----
C CHECK FOR ZERO'S ON MAIN DIAGONAL
IF (I.EQ.J.AND. B(I-L3+1,J-L3+1).LE.0) THEN
QUIT=.TRUE.
WRITE (6,42) I,B(I-L3+1,J-L3+1)
ENDIF
C
700 CONTINUE
42 FORMAT ('SK, 'DOP #',I5, ' IS .LE. 0 IN THE MASS MATRIX...',
& 'SK, 'M(I,I)',1P,G12.4, ' SOLN IS ABORTED')
C
C-----
C RETURN IF A MAIN DIAGONAL IS LE ZERO
IF (QUIT) RETURN
C
C-----
C SOLVE FOR EIGENVALUES AND EIGENVECTORS ---
C-----
C-----
IMSL SUBROUTINE DGVCSF
C
EIGENVALUES AND EIGENVECTORS OF A*X=LAMBDA*B*X=0
C
A = STIFFNESS - REAL, FULL SYMMETRIC MATRIX
C
B = MASS - REAL, FULL SYMMETRIC MATRIX
C
N = NUMBER OF DOP OF A AND B
C
PI = PERFORMANCE INDEX
PI=MK(1), I<PI - ROUTINE PERFORMED WELL
I<PI<100 - ROUTINE PERFORMED SATISFACTORILY
100<PI - ROUTINE PERFORMED POORLY
C
EVAL= EIGENVALUES (LAMBDA), N BY 1
C
EVEC= EIGENVECTORS, N BY N
C
LDR= LEADING DIMENSION OF MATRIX A EXACTLY AS SPECIFIED IN THE
DIMENSION STATEMENT IN THE CALLING PROGRAM.
C
LDB= LEADING DIMENSION OF MATRIX B EXACTLY AS SPECIFIED IN THE
DIMENSION STATEMENT IN THE CALLING PROGRAM.
C
LDEVEC= LEADING DIMENSION OF MATRIX EVEC EXACTLY AS SPECIFIED IN THE
DIMENSION STATEMENT IN THE CALLING PROGRAM.
C
C-----
IF(NSTEP.EQ.1.OR.NSTEP.EQ.3) THEN
LDEVEC=NFREE
CALL IWKIN(100000)
C
SOL00230 C CALL DGVCSF(NFREE,A,NFREE,B,NFREE,EVAL,LDEVEC)
SOL00240 CALL DGVCSF(NFREE,A,NFREE,B,NFREE,EVAL,LDEVEC,A,WK2,WORK)
SOL00250 C
SOL00260 C-----
SOL00270 C= NORMALIZE MODE SHAPE TO MAX VALUE OF ONE ---
SOL00280 C-----
SOL00290 C NOTE: THE EIGENVECTOR IN IMSL DGVCSF HAS BEEN NORMALIZED
SOL00300 C
SOL00310 C-----WRITE EIGENVALUES AND EIGENVECTORS TO DISK MEIG
SOL00320 IF (NSTEP.EQ.1.OR.NSTEP.EQ.3) THEN
SOL00330 WRITE(MEIG,175)(EVAL(I),I=1,IPRT)
SOL00340 DO 176 I=1,NFREE
SOL00350 176 WRITE(MEIG,175)(EVEC(I,J),J=1,IPRT)
SOL00360 175 FORMAT(5(G14.5,1X))
SOL00370 ENDDIF
SOL00380 C
SOL00390 ELSE IF(NSTEP.EQ.2) THEN
SOL00400 EVAL(I)=SQRT(EVAL(I),I=1,IPRT)
SOL00410 DO 178 I=1,NFREE
SOL00420 178 READ(MBIG,175)(EVEC(I,J),J=1,IPRT)
SOL00430 ENDDIF
SOL00440 C
SOL00450 C-----
SOL00460 C= PRINT EIGENVALUES AND EIGENVECTORS ---
SOL00470 C-----
SOL00480 I1=1
SOL00490 100 IEND=MIN(I1+6,IPRT)
SOL00500 C
SOL00510 WRITE (6,110) (I, I=I1,IEND)
SOL00520 DO 125 I=I1,IEND
SOL00530 125 EVAL(I)=SQRT(EVAL(I))
SOL00540 WRITE (6,126) (EVAL(I),I=I1,IEND)
SOL00550 DO 127 I=I1,IEND
SOL00560 127 EVAL(I)=EVAL(I)/6.2831853
SOL00570 WRITE (6,128) (EVAL(I),I=I1,IEND)
SOL00580 DO 129 I=I1,IEND
SOL00590 129 IF (EVAL(I).EQ.0) EVAL(I)=1.E-15
SOL00600 EVAL(I)=1.00/EVAL(I)
SOL00610 129 WRITE (6,130) (EVAL(I),I=I1,IEND)
SOL00620 WRITE (6,135)
SOL00630 110 FORMAT(' MODE',7(I10,5X))
SOL00640 120 FORMAT(' EIGENVALUE',1P,7(G14.5,1X))
SOL00650 126 FORMAT(' FREQ (RAD/SEC)',1P,7(G14.5,1X))
SOL00660 128 FORMAT(' PERIOD (SEC)',1P,7(G14.5,1X))
SOL00670 130 FORMAT(' PERIOD (SEC)',1P,7(G14.5,1X))
SOL00680 135 FORMAT(' EIGENVECTORS')
SOL00690 C
SOL00700 C----- EIGENVECTORS
SOL00710 DO 140 I=1,NFREE
SOL00720 I1=I+1
SOL00730 140 WRITE (6,150) I1,(EVEC(I,J),J=I1,IEND)
SOL00740 150 FORMAT(' DOP ',I6, ' ',1P,7(G14.5,1X))
SOL00750 C
SOL00760 IF (IEND.LT.IPRT) THEN
SOL00770 WRITE (6,10) TITLE(1),TITLE(2)
SOL00780 I1=IEND+1
SOL00790 GO TO 100
SOL00800 ENDDIF
SOL00810 10 FORMAT('1 STRUCTURE... ',A,
& ' SOLUTION... ',A,/)
SOL00820 C
SOL00830 IF(NSTEP.EQ.1) THEN
SOL00840 ABORT=.TRUE.
SOL00850 RETURN
SOL00860 ENDDIF
SOL00870 C
SOL00880 C-----ZERO GLOBAL BASE ACCELERATION AND DISPLACEMENT SPECTRUM VALUES
SOL00890 DO 771 I=1,NMODE
SOL00900 RA(I,1)=0
SOL00910 RA(I,2)=0
SOL00920 RA(I,3)=0
SOL00930 RA(I,4)=0
SOL00940 RA(I,5)=0
SOL00950 RA(I,6)=0
SOL00960 RA(I,7)=0
SOL00970 RA(I,8)=0
SOL00980 771 RA(I,3)=0
SOL00990 C-----
SOL01000 C== INPUT NUMBER OF SEISMIC COMPONENTS, SCALING FACTOR, PRINT CONTROL =
SOL01010 C-----
SOL01020 READ(5,*)ASCAL,OPTION,PRINT
SOL01030 C
SOL01040 C-----INPUT DIRECTION VECTORS V1 AND V2
SOL01050 READ(5,*)V1,V2
SOL01060 C
SOL01070 C-----CALCULATE V3 = V1 X V2
SOL01080 CALL CROSS(V3,V1,V2,0)
SOL01090 C
SOL01100 C-----CALCULATE V2 = V3 X V1, AND NORMALIZE ALL VECTORS...
SOL01110 CALL CROSS(V2,V3,V1,1)
SOL01120 C-----
SOL01130 C== INPUT RESPONSE SPECTRUM VALUES ==
SOL01140 C-----
SOL01150 WRITE(6,10)TITLE(1),TITLE(2)
SOL01160 DO 30 I=1,NA
SOL01170 READ (5,*) IDIR
SOL01180 C
SOL01190 IF (IDIR.GE.1.AND.IDIR.LE.3) THEN
SOL01200 IF(TEST(OPTION,'ACC',.FALSE.)) THEN
SOL01210 107 WRITE (6,107) I
SOL01220 107 FORMAT ('//IX, ** ACCELERATION SPECTRUM VALUES I',
& ' INPUTTING TRANSLATIONAL #',I2,/)
SOL01230 ELSE IF(TEST(OPTION,'DISP',.FALSE.)) THEN
SOL01240 1070 WRITE (6,1070) I
SOL01250 1070 FORMAT ('//IX, ** DISPLACEMENT SPECTRUM VALUES I',
& ' INPUTTING TRANSLATIONAL #',I2,/)
SOL01260 ELSE
SOL01270 WRITE (6,*) 'INVALID DIRECTION',IDIR, ' SET 0 < IDIR < 4'
SOL01280 GO TO 30
SOL01290 ENDDIF
SOL01300 C
SOL01310 IF (IDIR.EQ.1) THEN
SOL01320 CX=V1(1)
SOL01330 CV=V1(2)
SOL01340 CS=V1(3)
SOL01350 WRITE (6,92) V1
SOL01360 ELSE IF (IDIR.EQ.2) THEN
SOL01370 CX=V2(1)
SOL01380 CV=V2(2)
SOL01390 CS=V2(3)
SOL01400 WRITE (6,92) V2
SOL01410 ELSE IF (IDIR.EQ.3) THEN
SOL01420 CX=V3(1)
SOL01430 CV=V3(2)
SOL01440 CS=V3(3)
SOL01450 WRITE (6,92) V3
SOL01460 ENDDIF
SOL01470 C
SOL01480 DO 909 I=1,NAP
SOL01490 909 READ(5,*)RSNP(1,I),RSNP(2,I)
SOL01500 WRITE(6,640)
SOL01510 640 FORMAT(30X,'PERIOD ',10X,'SPECTRUM VALUE',/
& 30X,/)
SOL01520 DO 908 I=1,NAP
SOL01530 908 WRITE(6,907)RSNP(1,I),RSNP(2,I)
SOL01540 907 FORMAT(30X,G14.6,10X,G14.6)
SOL01550 C
SOL01560 DO 95 I=1,NMODE
SOL01570 95 TI=EVAL(I)
SOL01580 CALL INTPO(TI,TMP(I),NAP,RSNP)
SOL01590 C
SOL01600 DO 95 I=1,NMODE
SOL01610 95 CONTINUE
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C
IF (TEST(OPTION, 'ACC', .FALSE.)) THEN
DO 20 J=1, NMODE
ACC=TMP(J)*ASCALB
DIS=TMP(J)*ASCALB/((6.2831853/EVAL(J))**2)
RA(J,1)=RA(J,1)+ACC*CX
RA(J,2)=RA(J,2)+ACC*CY
RA(J,3)=RA(J,3)+ACC*CX
RD(J,1)=RD(J,1)+DIS*CX
RD(J,2)=RD(J,2)+DIS*CY
RD(J,3)=RD(J,3)+DIS*CX
20 CONTINUE
ELSE IF (TEST(OPTION, 'DISP', .FALSE.)) THEN
DO 22 J=1, NMODE
ACC=TMP(J)*ASCALB/((6.2831853/EVAL(J))**2)
RA(J,1)=RA(J,1)+ACC*CX
RA(J,2)=RA(J,2)+ACC*CY
RA(J,3)=RA(J,3)+ACC*CX
RD(J,1)=RD(J,1)+DIS*CX
RD(J,2)=RD(J,2)+DIS*CY
RD(J,3)=RD(J,3)+DIS*CX
22 CONTINUE
ENDIF
C
30 CONTINUE
IF (PRINT) THEN
WRITE (6,31) 'X'
WRITE (6,93) (RA(J,1), J=1, NMODE)
WRITE (6,31) 'Y'
WRITE (6,93) (RA(J,2), J=1, NMODE)
WRITE (6,31) 'Z'
WRITE (6,93) (RA(J,3), J=1, NMODE)
31 FORMAT('X, GLOBAL BASE ACCE. -SPECTRUM VALUES IN THE ', A,
' DIRECTION ', /)
WRITE (6,319) 'X'
WRITE (6,93) (RD(J,1), J=1, NMODE)
WRITE (6,319) 'Y'
WRITE (6,93) (RD(J,2), J=1, NMODE)
WRITE (6,319) 'Z'
WRITE (6,93) (RD(J,3), J=1, NMODE)
319 FORMAT('X, GLOBAL BASE DISP. -SPECTRUM VALUES IN THE ', A,
' DIRECTION ', /)
ENDIF
C
-----
C== CALCULATE INFLUENCE COEFFICIENT VECTORS ==
-----
DO 440 J=1, NDOF
DO 440 J=1,3
440 FLU(I,J)=0
C
DO 451 K=1,3
C
C..... OMIT ALL DOF THAT ARE CONSTRAINED TO A MASTER DOF...
DO 554 NODE=1, NMODE
ICOB=JTCOB(NODE)
DO 650 JDIR=1,3
IF (.NOT. BTEST(JTFLG(NODE), JDIR+1)) THEN
II=IDOF(NODE, JDIR)
JJ=I-NCOND
IF (JJ.GE.1 .AND. JJ.LE. NFREE) THEN
FLU(JJ,K)=COSINE(JDIR,K,ICOB)
ENDIF
650 CONTINUE
554 CONTINUE
451 CONTINUE
C
IF (BTEST(IBUG,5)) CALL WMATRX(FLU, NFREE, 3, 'INFLUENCE COEF. MATRIX')
C
92 FORMAT('X, DIRECTION OF COSINE',
' 4X, SS, F9.5, ' I ', SP, F9.5, ' J ', F9.5, ' K ', SS, /)
93 FORMAT('X, I.P., I0012.4')
94 FORMAT('X, INPUT RESPONSE SPECTRUM VALUES', /)
929 FORMAT('X, DIRECTION OF DISPLACEMENT',
' 4X, SS, F9.5, ' I ', SP, F9.5, ' J ', F9.5, ' K ', SS)
C
-----
C== CALCULATE CROSS-MODAL COEFFICIENTS FOR CQC APPROACH ==
-----
IF (ICOMN.EQ.2) THEN
DO 809 I=1, NMODE
CMC(I,1)
DO 809 J=1, NMODE
WI=EWAL(I)
WJ=EWAL(J)
WJI=W/WJ
COEF1=(1-WJI)**2*(1+WJI)*WJI**1.5
COEF2=(1-WJI**2)**2+4*(DAMP**2)*WJI*(1-WJI)**2
CMC(I,J)=COEF1/COEF2
CMC(J,I)=CMC(I,J)
809 CONTINUE
WRITE (6,10) TITLE(1), TITLE(2)
CALL WMATRX(CMC, NMODE, NMODE, 'CROSS-MODAL COP. MATRIX')
ENDIF
C
C== CALCULATE GENERALIZED MASS, GM=EVEC'*B'EVEC ==
-----
DO 450 I=1, NMODE
GM(I)=0
DO 460 II=1, NFREE
DISP(II)=0
DO 470 JJ=1, NFREE
IF (B(II, JJ).EQ.0) GO TO 470
DISP(II)=DISP(II)+B(II, JJ)*EVEC(JJ,I)
470 CONTINUE
460 CONTINUE
DO 480 KK=1, NFREE
GM(I)=GM(I)+EVEC(KK,I)*DISP(KK)
480 CONTINUE
450 CONTINUE
IF (BTEST(IBUG,5))
CALL WMATRX(GM, NMODE, 1, 'GENERALIZED MASS MATRIX')
C
C== CALCULATE EFFECTIVE-LOAD FACTOR FOR EACH MODE, ELF=EVEC'*B'FLU ==
-----
DO 455 I=1, NMODE
DO 457 K=1,3
ELF(I,K)=0
DO 465 II=1, NFREE
DISP(II)=0
DO 475 JJ=1, NFREE
IF (B(II, JJ).EQ.0) GO TO 475
DISP(II)=DISP(II)+B(II, JJ)*FLU(JJ,K)
475 CONTINUE
465 CONTINUE
DO 485 KK=1, NFREE
ELF(I,K)=ELF(I,K)+EVEC(KK,I)*DISP(KK)
485 CONTINUE
455 CONTINUE
455 CONTINUE
IF (BTEST(IBUG,5))
CALL WMATRX(ELF, NMODE, 3, 'EFFECTIVE-LOAD MATRIX')
C
C== CALCULATE EFFECTIVE MASS FOR EACH MODE IN EACH INDIVIDUAL ==
C SEISMIC INPUT DIRECTION
-----
C
C----- READ EFFECTIVE MASS RATIO LIMIT IN X, Y, Z DIRECTION
C NOTE: IF NO. OF MODE BASED ON THE LIMIT IS LESS THAN NMODE.

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SOL04480
C
NO. OF MODES BASED ON THIS LIMIT IS USED.
C
READ(5,*) XLIM, FLIM, ZLIM
C
WRITE(6,10) TITLE(1), TITLE(2)
WRITE(6,48)
487 FORMAT('X, EFFECTIVE MASS IN GCS' TRANSLATIONAL DIRECTIONS', /)
' X, '-----' /)
C
WRITE(6,987) 'X', XLIM
WRITE(6,987) 'Y', FLIM
WRITE(6,987) 'Z', ZLIM
C
C.... CALCULATE TOTAL STRUCTURAL TRANSLATIONAL MASS
TOTX=0
TOTY=0
TOTZ=0
DO 705 J=1, LJ, L4
I0=NMODE/J
DO 706 I=1, NMODE
IF (.NOT. BTEST(JTFLG(I), I2)) THEN
NDOFX=IDOF(I,1)
IF (J.EQ. NDOFX) TOTX=TOTX+MASS(IJ)
ENDIF
IF (.NOT. BTEST(JTFLG(I), I3)) THEN
NDOFY=IDOF(I,2)
IF (J.EQ. NDOFY) TOTY=TOTY+MASS(IJ)
ENDIF
IF (.NOT. BTEST(JTFLG(I), I4)) THEN
NDOFZ=IDOF(I,3)
IF (J.EQ. NDOFZ) TOTZ=TOTZ+MASS(IJ)
ENDIF
706 CONTINUE
705 CONTINUE
WRITE(6,388) 'X', TOTX
WRITE(6,388) 'Y', TOTY
WRITE(6,388) 'Z', TOTZ
388 FORMAT('X, NOTE: TOTAL STRUCTURAL MASS IN ', A, ' DIRECTION IS ',
' 6', OP, G15.6)
WRITE(6,388)
386 FORMAT('X, ' MODE ' GCS X-DIRECTION(N) ',
' GCS Y-DIRECTION(N) ',
' GCS Z-DIRECTION(N) ')
987 FORMAT('X, EFFECTIVE MASS LIMIT IN GCS', A,
' DIRECTION IS ', G15.6)
C
TXLIM=0
TYLIM=0
TZLIM=0
MODEX=0
MODEY=0
MODEZ=0
FXME=0
FYME=0
FZME=0
DO 701 I=1, NMODE
IF (TOTX.EQ.0) GO TO 707
FXME=(ELF(I,1))**2/GM(I)/TOTX
TXLIM=TXLIM+FXME
IF (TXLIM.NE.0 .AND. MODEX.EQ.0) THEN
MODEX=I
ENDIF
707 IF (TOTY.EQ.0) GO TO 708
FYME=(ELF(I,2))**2/GM(I)/TOTY
TYLIM=TYLIM+FYME
IF (TYLIM.NE.0 .AND. MODEY.EQ.0) THEN
MODEY=I
ENDIF
708 IF (TOTZ.EQ.0) GO TO 709
FZME=(ELF(I,3))**2/GM(I)/TOTZ
TZLIM=TZLIM+FZME
IF (TZLIM.NE.0 .AND. MODEZ.EQ.0) THEN
MODEZ=I
ENDIF
709 WRITE(6,389) I, FXME, FYME, FZME
389 FORMAT('X, I3, SX, J(G15.4, 6X)')
701 CONTINUE
C
IMD=MAX(MODEX, MODEY, MODEZ)
IF (IMD.NE.0 .AND. IMD.LT. NMODE) THEN
NMODE=IMD
WRITE(6,*) '** BASED ON EFFECTIVE MASS LIMIT, FINAL NO. OF ',
' MODES USED IS ', NMODE, ' **'
ELSE
NMODE=NMODE
ENDIF
C
C== CALCULATE MAXIMUM DISPLACEMENT RESPONSES ==
-----
DO 560 NN=1, NA
DO 540 I=1, L6
TMP(I)=0
DO 541 I=1, NFREE
TMP2(I)=0
DO 550 J=1, NMODE
DO 551 I=1, NFREE
DISP(I)=0
559 CONTINUE
DO 570 II=1, NFREE
DISP(II)=EVEC(II,J)*ELF(J,NN)*RD(J,NN)/GM(J)+DISP(II)
570 CONTINUE
C
IF (ICOMN.EQ.2) THEN
DO 615 I=1, NFREE
CQCM(I+NCOND, J)=DISP(I)
ENDIF
C
IF (BTEST(IBUG,5)) THEN
WRITE(6,*) 'MODAL DISPLACEMENTS FOR MODE ', J
DO 9999 I=1, NFREE
WRITE(6,*) 'DISP(' I, ' ) = ', DISP(I)
9999 ENDF
C
IF (ICOMN.EQ.1) THEN
DO 580 KK=1, NFREE
TMP(KK+NCOND)=DISP(KK)**2+TMP(KK+NCOND)
580 CONTINUE
ENDIF
C
DO 500 LL=1, NFREE
LOAD(I)=LOAD(I)+DISP(LL)
DO 501 LL=L1, L2
DISP(LL)=0
DO 502 LL=L3, L4
DISP(LL)=LOAD(I)-LOAD(I)-L2, L1)
DO 503 LL=L5, L6
DISP(LL)=0
C
C== CALCULATE EQUIVALENT EARTHQUAKE FORCES ==
-----
LOAD(I)=0
DO 735 I=1, NFREE
LOAD(I+NCOND)=0
735 CONTINUE
IF (BTEST(IBUG,5)) THEN

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WRITE(6,10)TITLE(1),TITLE(2)
WRITE(6,605)J,NN
605 FORMAT(1X,'TOTAL EQUIVALENT EARTHQUAKE FORCES FOR MODE ',I3,
& ' IN THE SEISMIC COMPONENT # ',I2,/)
WRITE(6,913)
WRITE(6,212) (I+NCOND,LOAD(I+NCOND),I=1,NFREE)
ENDIF
C
IF( ICOMN .EQ. 1 ) THEN
DO 581 KK=1,NFRBS
581 TMP2(KK)=LOAD(KK+NCOND)**2 +TMP2(KK)
ELSE IF( ICOMN .EQ. 2 ) THEN
DO 816 I=1,NFRBS
816 CQCM2(I,J)=LOAD(I+NCOND)
ENDIF
C
C== CALC RESPONSE FOR CONDENSED OUT DOF ==
C-----
I=1
CALL REACTN(I1,BTEST( IBUG,6 ),NNODE,L1,L2,L3,L4,DISP,DISP,
& MD,STIFF,IMD,NDOP,1,1,0,
& MAXNOD,IDOF,COORD,ID,TITLE,STEPID,.FALSE.,
& NCOS,COSINE,JTCOS,JTFLG,SUMRCT)
CALL MSOLL(3,BTEST( IBUG,6 ),L4,IMD,NDOP,I1,L1,L2,
& MD,STIFF,DISP,WORK,
& .FALSE.,MASS,MOMS,1,
& .FALSE.,KG ,MOKG,1,ABORT)
IF (ABORT) RETURN
C
IF( ICOMN .EQ. 1 ) THEN
DO 675 I=L1,L2
675 TMP(I)=DISP(I)**2 + TMP(I)
ELSE IF( ICOMN .EQ. 2 ) THEN
DO 676 I=L1,L2
676 CQC(I,J)=DISP(I)
ENDIF
C
C-----
C= PRINT GLOBAL EQUIVALENT EARTHQUAKE FORCE ==
C-----
C
CALL JOIDSP(1,MAXNOD,L6,NNODE,1,L3,L4,LD,
& IDOF,CNST,LOAD,
& TITLE,'TOTAL EQUIVALENT EARTHQUAKE FORCES',STEPID,.TRUE.,
& NCOS,COSINE,JTCOS,JTFLG)
C
C= CALCULATE MEMBER FORCES ==
C-----
IOPT=4
LSTTYP=0
PRINT=BTEST( IBUG,5)
HEAD=.FALSE.
DO 358 IELNO=1,NELMT
CALL ELELIB
& (IOPT,LSTTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,
& DUCFAG,
& IELNO,IELDOF,KGDF,PGEOM,RINPUT,AXIAL,I2DISP,I3LOAD,MSTOR)
DO 359 I=1,12
359 ELEF( IELNO,I,J)=RSAF(I)
358 CONTINUE
C
C-----
C= SOLVE FOR REACTIONS ==
C-----
CALL REACTN(2,BTEST( IBUG,5 ),NNODE,L1,L4,L5,L6,LOAD,DISP,
& MD,STIFF,IMD,NDOP,1,1,0,
& MAXNOD,IDOF,COORD,ID,TITLE,STEPID,.FALSE.,
& NCOS,COSINE,JTCOS,JTFLG,SUMRCT)
C
C-----
C= SOLVE FOR GLOBAL REACTIONS ==
C-----
DO 366 I=1,6
366 SUMRCT(I)=0
CALL REACTN(3,BTEST( IBUG,5 ),NNODE,L1,L4,L5,L6,LOAD,DISP,
& MD,STIFF,IMD,NDOP,1,1,0,
& MAXNOD,IDOF,COORD,ID,TITLE,STEPID,.FALSE.,
& NCOS,COSINE,JTCOS,JTFLG,SUMRCT)
DO 360 I=1,6
360 RCT(I,J)=SUMRCT(I)
C
IF( BTEST( IBUG,5 ) ) THEN
WRITE(6,10)TITLE(1),TITLE(2)
WRITE(6,607)J,NN
607 FORMAT(1X,'REACTION FOR MODE ',I3,' IN SEISMIC COMP # ',I2,/)
WRITE(6,323)SUMRCT
ENDIF
C
550 CONTINUE
C
IF( ICOMN .EQ. 1 ) THEN
DO 590 JJ=1,L4
590 TMP(JJ)=SQRT(TMP(JJ))
DO 547 J=1,NFRBS
547 TMP2(JJ)=SQRT(TMP2(JJ))
DO 591 I=1,NELMT
DO 591 I1=1,12
DO 592 J=1,NMODE
592 ELEF( I,II,NMODE+NN)=ELEF( I,II,NMODE+NN)+ELEF( I,II,J)**2
ELEF( I,II,NMODE+NN)=SQRT( ELEF( I,II,NMODE+NN ) )
591 CONTINUE
DO 593 I=1,6
DO 594 J=1,NMODE
594 RCT( I,NMODE+NN)=RCT( I,NMODE+NN)+RCT( I,J)**2
RCT( I,NMODE+NN)=SQRT( RCT( I,NMODE+NN ) )
593 CONTINUE
ELSE IF( ICOMN .EQ. 2 ) THEN
DO 826 L=1,L4
SUM=0
DO 828 J=1,NMODE
DO 828 JJ=1,NMODE
828 SUM=SUM+CQCM(L,J)*CMC(J,JJ)*CQCM(L,JJ)
SUM=ABS(SUM)
TMP(L)=SQRT(SUM)
CONTINUE
DO 837 L=1,NFREE
SUM=0
DO 838 J=1,NMODE
DO 838 JJ=1,NMODE
838 SUM=SUM+CQCM2(L,J)*CMC(J,JJ)*CQCM2(L,JJ)
SUM=ABS(SUM)
TMP2(L)=SQRT(SUM)
CONTINUE
DO 595 I=1,NELMT
DO 595 I1=1,12
SUM=0
DO 596 J=1,NMODE
DO 596 JJ=1,NMODE
596 SUM=SUM+ELEF( I,II,J)*CMC(J,JJ)*ELEF( I,II,JJ)
SUM=ABS(SUM)
ELEF( I,II,NMODE+NN)=SQRT(SUM)
595 CONTINUE
DO 597 I=1,6
DO 598 J=1,NMODE

```



```

MATTYP= 2
ELSE IF (TEST(TYPE,'BEND1',.FALSE.)) THEN
MATTYP= 3
ELSE IF (TEST(TYPE,'SHEAR1',.FALSE.)) THEN
MATTYP= 4
ELSE IF (TEST(TYPE,'RCMBERG',.FALSE.)) THEN
MATTYP= 5
ELSE IF (TEST(TYPE,'TAKEDA',.FALSE.)) THEN
MATTYP= 6
ELSE IF (TEST(TYPE,'ELSP5',.FALSE.)) THEN
MATTYP= 7
ELSE IF (TEST(TYPE,'BILINER',.FALSE.)) THEN
MATTYP= 7
ELSE IF (TEST(TYPE,'BRAC',.FALSE.)) THEN
MATTYP= 8
ELSE IF (TEST(TYPE,'LONG_OJG',.FALSE.)) THEN
MATTYP= 9
ELSE IF (TEST(TYPE,'IA_BILN',.FALSE.)) THEN
MATTYP= 10
ELSE IF (TEST(TYPE,'SHORT_OJG',.FALSE.)) THEN
MATTYP= 11
ELSE IF (TEST(TYPE,'STABILITY',.FALSE.)) THEN
MATTYP= 12
ELSE IF (TEST(TYPE,'LOCBL',.FALSE.)) THEN
MATTYP= 13
ELSE IF (TEST(TYPE,'SHEAR2',.FALSE.)) THEN
MATTYP= 14
C
C..... TO ADD ADDITIONAL MATERIALS, COPY THE NEXT TWO LINES
ELSE IF (TEST(TYPE,'1',.FALSE.)) THEN
MATTYP= 12
C..... CHANGE THE FOLLOWING VALUES
C..... (1) YOUR MATERIAL NAME
C..... (2) MATERIAL ID NUMBER = XX
C..... ADD CALL MATXX TO SUBPROGRAM MATLIB
C..... ADD SUBPROGRAM MATXX
ELSE IF (TEST(TYPE,'END',.FALSE.)) THEN
NMAT=NMAT+1
N(I2MAT)=NMAT
GO TO 120
ELSE
WRITE (6,*) 'INVALID MATERIAL TYPE: ',HEAD
GO TO 110
ENDIF
C
C.....RESERVE STORAGE
N(I2MAT+MAT)=I2
I2=N(I2MAT)+200
C.....CALL MATERIAL LIBRARY TO INITIALIZE MATERIAL DATA
CALL MATLIB
& IOPT,LETTYP,PT,IREL,HEAD,NAME,ESE,EPSE,DUCT,EXCR,
& P,PE,DL,VL,LELEM,LMAT,
& MAT,MATTYP,SE,-1,DAMAGE)
C.....RESERVE STORAGE
I2=I2+LMAT+1
C
IF (MAT .LT. NMAT) GO TO 110
120 CONTINUE
C-----
C= INPUT GEOMETRIC STIFFNESS DATA =
C-----
& (I2KGD+0) = VERTICAL GROUND ACCELERATION
& N(I2KGD+1) = TYPE OF AXIAL LOAD USED
& 0, GEOMETRIC STIFFNESS IS NOT CONSIDERED
& 1, LOAD= F(INPUT)*(1.0 + ACCEL)
& 2, PREVIOUS LOAD STEP'S AXIAL LOAD IS USED
& 3, SOLUTION OF SOLUTION
& 0, GEOMETRIC STIFFNESS IS NOT CONSIDERED
& 1, LUMPED FORMULATION
& 2, CONSISTENT FORMULATION
& N(I2KGD+3) = 0, GEOMETRIC STIFFNESS IS NOT CONSIDERED
& 1, KG IS SUBTRACTED FROM ELEMENT STIFFNESS
& 2, SEPARATE GLOBAL KG IS FORMED
& LOGICAL FLAG, TRUE IF KG IS TO BE CONDENSED
I2KGD=I2
I2=I2+4
READ (5,*) (N(I2KGD+1),I=1,3),KCOND
WRITE(6,*) 'KGDATA', (N(I2KGD+1),I=1,3)
I2=I2KGD
IF (N(I+1).EQ.0 .OR. N(I+2).EQ.0 .OR. N(I+3).EQ.0) THEN
N(I+1)=0
N(I+2)=0
N(I+3)=0
ELSE
WRITE (6,150) TITLE(1),TITLE(2)
IF (N(I+1).EQ.1) WRITE (6,160)
& 'KGDATA(1)= 1, LOAD= F(INPUT)*(1.0+ACCEL)',
& F(INPUT) IS POSITIVE IN COMPRESSION',
& ACCEL IS POSITIVE GLOBAL X-AXIS ACCELERATION'
& IF (N(I+1).EQ.2) WRITE (6,160)
& 'KGDATA(1)= 2, PREVIOUS LOAD STEP'S AXIAL LOAD IS USED'
C
IF (N(I+2).EQ.1) WRITE (6,160)
& 'KGDATA(2)= 1, LUMPED FORMULATION'
& IF (N(I+2).EQ.2) WRITE (6,160)
& 'KGDATA(2)= 2, CONSISTENT FORMULATION'
C
IF (N(I+3).EQ.1) WRITE (6,160)
& IF (N(I+3).EQ.2) WRITE (6,160)
& 'KGDATA(3)= 2, SEPARATE GLOBAL KG IS FORMED'
C
WRITE (6,160)
IF (KCOND) WRITE (6,160)
& '- THE GEOMETRIC STIFFNESS MATRIX IS CONDENSED //'
& WITH THE STRUCTURAL STIFFNESS MATRIX.'
& IF (.NOT. KCOND) WRITE (6,160)
& '- THE GEOMETRIC STIFFNESS MATRIX IS NOT CONDENSED //'
& WITH THE STRUCTURAL STIFFNESS MATRIX.'
ENDIF
150 FORMAT ('1 STRUCTURE.....',A,
& SOLUTION.....',A,
& 5X,' GEOMETRIC STIFFNESS DATA '//
& 5X,'-----')
160 FORMAT(5X,A)
C-----
C= SET UP STORAGE FOR GEOMETRIC STIFFNESS DATA =
C-----
C-----
C----- GLOBAL STIFFNESS STORAGE -----
N(I2MDKG) = INDICES OF THE MAIN DIAGONAL TERMS
I2MDKG=I2
IF (N(I2MDKG+3).EQ.2) THEN
I2MDKG=I2
I2=I2+NDOF+1
CALL SKTLIN(0,1,NDOF,BUG,TITLE,N(I2MDKG),
& KGDOP,N(I2MDKG),I2MDKG),MD,'GLOBAL GEOMETRIC STIFFNESS')
ELSE
DUMMY LOCATION FOR KG TO AVOID ADDRESSING EXCEPTIONS
KG DATA IS NOT STORED IN THESE LOCATIONS...
I2MDG=I2KGD
I2KG=I2KGD
I2KGD=I2KGD
ENDIF
C
STRO1030 C
STRO1040 C
STRO1050 C= INPUT ELEMENT DATA =
STRO1060 C-----
STRO1070 C READ (5,*) NELMT
STRO1080 C----- ELEMENT STORAGE -----
STRO1090 C N(I2ELE)= # OF MATERIAL PROPERTIES (NELMT)
STRO1100 C N(I2ELE+1)= ABSOLUTE ADDRESS OF ELEMENT #1
STRO1110 C N(I2ELE+2)= ABSOLUTE ADDRESS OF ELEMENT #2
STRO1120 C N(I2ELE+3)= ABSOLUTE ADDRESS OF ELEMENT #3
STRO1130 C
STRO1140 C
STRO1150 C
STRO1160 C N(I2IHM+NELMT)= ABSOLUTE ADDRESS FOR ELEM # NELMT
STRO1170 C I2(I2ELE+1)= MATERIAL DATA FOR ELEMENT #1
STRO1180 C
STRO1190 C
STRO1200 C
STRO1210 C I2(I2ELE+2)= MATERIAL DATA FOR ELEMENT #1
STRO1220 C
STRO1230 C
STRO1240 C
STRO1250 C
STRO1260 C IOPT=1
STRO1270 C I2ELE=I2
STRO1280 C I2=I2+NELMT+1
STRO1290 C N(I2ELE)=NELMT
STRO1300 C FIRST=.TRUE.
STRO1310 C HEAD=.TRUE.
STRO1320 C IGEN=0
STRO1330 C IELNO=0
STRO1340 C LETYP=0
C----- LOOP FOR EACH ELEMENT -----
210 IELNO=IELNO+1
IF (IGEN.LE.0) THEN
READ(10,*) TYPE
IF (TEST(TYPE,'3D-BEAM',.FALSE.)) THEN
NINPUT= 10
NIN2 = 437
NELTYP= 1
ELSE IF (TEST(TYPE,'SPRING',.FALSE.)) THEN
NINPUT= 13
NIN2 = 133
NELTYP= 2
ELSE IF (TEST(TYPE,'SHEAR WALL',.FALSE.)) THEN
NINPUT= 9
NIN2 = 769
NELTYP= 3
ELSE IF (TEST(TYPE,'BRACE',.FALSE.)) THEN
NINPUT= 10
NIN2 = 133
NELTYP= 8
ELSE IF (TEST(TYPE,'E3DBEAM',.FALSE.)) THEN
NINPUT= 14
NIN2 = 614
NELTYP= 9
ELSE IF (TEST(TYPE,'STABILITY',.FALSE.)) THEN
NINPUT= 8
NIN2 = 618
NELTYP= 12
ELSE IF (TEST(TYPE,'SHEAR_OPEN',.FALSE.)) THEN
NINPUT= 9
NIN2 = 775
NELTYP= 14
C..... TO ADD ADDITIONAL ELEMENTS, COPY THE NEXT FOUR LINES
ELSE IF (TEST(TYPE,'1',.FALSE.)) THEN
NINPUT= (2)
NIN2 = (3)
NELTYP= (4)
C..... CHANGE THE FOLLOWING VALUES
C..... (1) YOUR ELEMENT NAME
C..... (2) # OF VALUES TO BE INPUT
C..... (3) # OF STORAGE LOCATIONS REQD FOR ELEMENT
C..... (4) ELEMENT ID NUMBER = XX
C..... ADD CALL ELEMX TO SUBPROGRAM ELE_LIB
C..... ADD SUBPROGRAM ELEMX
ELSE IF (TEST(TYPE,'END',.FALSE.)) THEN
NELMT=IELNO-1
N(I2ELE)=NELMT
GO TO 300
ELSE
WRITE (6,*) 'INVALID ELEMENT TYPE: ',HEAD
GO TO 210
ENDIF
BACKSPACE (5)
READ (5,*) TYPE,NAME, (RINPUT(I),I=1,NINPUT),IGEN
IF (IGEN.GT.0) READ (5,*) (GINPUT(I),I=1,NINPUT)
ELSE
IGEN=IGEN-1
NAME=GENERATED'
DO 220 I=1,NINPUT
RINPUT(I)=RINPUT(I)+GINPUT(I)
220 ENDF
C WRITE (6,*) 'BEFORE STORAGE IS RESERVED IN STRUCT'
CDBG WRITE(6,111) (I,N(I),I,I)=68,91)
111 FORMAT ('0P,') 2MATRX('16,')I15,1P,G20,10)
C.....RESERVE STORAGE
I2=I2+NIN2+MSTOR
I2WK=I2
C.....STORE ELEMENT TYPE
N(I2C)=NELTYP
C.....CALL ELEMENT LIBRARY TO DETERMINE THE STORAGE FOR MATL DATA
IOPT=9
HEAD=.FALSE.
LETTYP=0
CALL ELEMIB
& IOPT,LETTYP,.FALSE.,IREL,HEAD2,NAME,ESE,EPSE,DAMAGE,
& DUCTAG,
& IELNO,I2ELEM,KGDOP,PGEOM,RINPUT,AXIAL,I2DISP,I2LOAD,MSTOR)
C.....RESERVE STORAGE
I2=I2+NIN2+MSTOR
I2WK=I2
C.....DU_MP ELEMENT DATA IF DEBU_GING
IF (TEST(IGEN,8)) THEN
WRITE (6,*) 'I2LM,I2LKM,I2KRE,I2KRE'
JJ=1
DO 7100 I=C,(I2WK-1-IREL)
JJ=JJ+1
7100 IF (N(I).NE.0) WRITE (6,7010) IELNO,JJ,I,N(I),I(I)
ENDF
C.....CALL ELEMENT LIBRARY TO INITIALIZE ELEMENT DATA AND RETURN
INFO ON ELEMENT DOP...
IOPT=1
CALL ELEMIB
& IOPT,LETTYP,.TRUE.,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,
& DUCTAG,
& IELNO,I2ELEM,KGDOP,PGEOM,RINPUT,AXIAL,I2DISP,I2LOAD,MSTOR)
C.....RELEASE UNUSED STORAGE .....
STRO2450
STRO2460
STRO2470
STRO2480
STRO2490
STRO2500
STRO2510
STRO2520
STRO2530
STRO2540
STRO2550
STRO2560
STRO2570
STRO2580
STRO2590
STRO2600
STRO2610
STRO2620
STRO2630
STRO2640
STRO2650
STRO2660
STRO2670
STRO2680
STRO2690
STRO2700
STRO2710
STRO2720
STRO2730
STRO2740
STRO2750
STRO2760
STRO2770
STRO2780
STRO2790
STRO2800
STRO2810
STRO2820
STRO2830
STRO2840
STRO2850
STRO2860
STRO2870
STRO2880
STRO2890
STRO2900
STRO2910
STRO2920
STRO2930
STRO2940
STRO2950
STRO2960
STRO2970
STRO2980
STRO2990
STRO3000
STRO3010
STRO3020
STRO3030
STRO3040
STRO3050
STRO3060
STRO3070
STRO3080
STRO3090
STRO3100
STRO3110
STRO3120
STRO3130
STRO3140
STRO3150
STRO3160
STRO3170
STRO3180
STRO3190
STRO3200
STRO3210
STRO3220
STRO3230
STRO3240
STRO3250
STRO3260
STRO3270
STRO3280
STRO3290
STRO3300
STRO3310
STRO3320
STRO3330
STRO3340
STRO3350
STRO3360
STRO3370
STRO3380
STRO3390
STRO3400
STRO3410
STRO3420
STRO3430
STRO3440
STRO3450
STRO3460
STRO3470
STRO3480
STRO3490
STRO3500
STRO3510
STRO3520
STRO3530
STRO3540
STRO3550
STRO3560
STRO3570
STRO3580
STRO3590
STRO3600
STRO3610
STRO3620
STRO3630
STRO3640
STRO3650
STRO3660
STRO3670
STRO3680
STRO3690
STRO3700
STRO3710
STRO3720
STRO3730
STRO3740
STRO3750
STRO3760
STRO3770
STRO3780
STRO3790
STRO3800
STRO3810
STRO3820
STRO3830
STRO3840
STRO3850
STRO3860

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```

13=I3-IREL
C .....DU,MP ELEMENT DATA IF DEBU_GING
IF (BTTEST(1BUG,8)) THEN
  WRITE (6,*) 'I2LM','I2LM', 'I2K8', 'I2K8
  J2=1
  DO 7000 I=IC,(I2MK-1-IREL)
  J2=J2+1
7000 IF (N3(I),NE.0) WRITE (6,7010) IELNO,JJ,I,N2(I),Z(I)
7010 FORMAT (' IELNO:',I3,' #',I6,' Z(',I6,',)',I10,1P,G18.8)
ENDIF
C ----- RESERVE SPACE IN THE SKYLINE FOR THE STIFFNESS
JFLAG=0
IF (TEST(TYPE,'STABILITY',FALSE)) JFLAG=1
CALL SKYLN(JFLAG,2,NDOP,IBUG,TITLE,N2(I2MD))
  IELDOF,N2(I2LM),Z(I2KE),MD,'GLOBAL STIFFNESS')
C ----- RESERVE SPACE IN THE SKYLINE FOR THE GEOMETRIC STIFFNESS
IF (N2(I2KGT+3)-EQ.1 .AND. AXIAL) THEN
  CALL SKYLN(0.2,NDOP,IBUG,TITLE,N2(I2MD))
  & KGDOF,N2(I2LMKG),Z(I2KEKG),MD,'GLOBAL GEOMETRIC STIFFNESS')
ELSE IF (N2(I2KGT+3)-EQ.2 .AND. AXIAL) THEN
  CALL SKYLN(0.2,NDOP,IBUG,TITLE,N2(I2MDKG))
  & KGDOF,N2(I2LMKG),Z(I2KEKG),MD,'GLOBAL GEOMETRIC STIFFNESS')
ENDIF
WRITE (6,*) 'AFTER SKYLN IN STRUCT'
CDBG WRITE(6,111) (I,N2(I),Z(I),I=68,91)
C
IF (IELNO.LT.NELMT) GO TO 210
300 CONTINUE
C
C -----
C= MODIFY GEOMETRIC STIFFNESS MATRIX FOR CONDENSATION ***
C THE CONDENSATION PROCESS MAY EXPAND THE GEOMETRIC STIFFNESS MATRIX'S
C SKYLINE. THE MAXIMUM SKYLINE OF BOTH THE GEOMETRIC STIFFNESS AND
C THE STIFFNESS MATRIX IS USED.
IF (N2(I2KGT+3)-EQ.2 .AND. KCOND .AND. NCOND.GT.0) THEN
  DO 310 I=0,NDOP
  J=I
  MDKG1=N2(I2MDKG+I)
  N2(I2MDKG+I)=MAX( N2(I2MDKG+I),N2(I2MD+I) )
  N2(I2MDKG+I)=MIN( N2(I2MDKG+I),N2(I2MD+I) )
310 CONTINUE
ENDIF
C
C SET UP GLOBAL STIFFNESS STORAGE **
N2(I2MD) = INDICES OF THE MAIN DIAGONAL TERMS
Z(I2STIF) = GLOBAL STIFFNESS MATRIX
CALL SKYLN(0.3,NDOP,IBUG,TITLE,N2(I2MD))
  & IELDOF,N2(I2LM),Z(I2KE),MD,'GLOBAL STIFFNESS')
I2STIF=II
I2 = I2+N2(I2MD+NDOP) +1
C -----
C= SET UP STORAGE FOR GEOMETRIC STIFFNESS DATA ***
N2(I2MDKG) = INDICES OF THE MAIN DIAGONAL TERMS
Z(I2KKG) = GLOBAL GEOMETRIC STIFFNESS MATRIX
IF (N2(I2KGT+3)-EQ.2) THEN
  CALL SKYLN(0.3,NDOP,IBUG,TITLE,N2(I2MDKG))
  & KGDOF,N2(I2LMKG),Z(I2KEKG),MD,'GLOBAL GEOMETRIC STIFFNESS')
  I2KKG = I2
  I2 = I2+N2(I2MDKG+NDOP)
ENDIF
C
C INPUT MASS MATRIX *****
C NMASS = NUMBER OF NODAL MASSES TO BE INPUT
FMASS = 0. NO MASS MATRIX
1. MASS MATRIX DUE TO NODE MASSES ONLY
MCOND = LOGICAL FLAG, TRUE IF MASS IS TO BE CONDENSED...
READ (5,*) INMASS,FMASS,MCOND
CALL MASSIN(INMASS,FMASS,MCOND,BUG,IBUG,TITLE,MAXI,MAXDS,
  & MAXNOD,NCOE,NCOE,I2,I2,I2,I2,I2,I2,I2,I2,I2,I2,I2,I2,
  & I2MDMS,I2MASE,I2LM,I2KE,I2MDAT,
  & NDOP,NMASE,I2COS,I2IDOP,I2CNST,I2JPLG,I2JCOE,'GLOBAL MASS',
  & NCOND,NFRSE,I2MD)
IF (INMASE.GT.0) THEN
  WRITE (6,160)
  IF (MCOND) THEN
    WRITE (6,160) ' - THE MASS MATRIX IS //'
    & ' CONDENSED WITH THE STRUCTURAL STIFFNESS MATRIX.'
  ELSE
    WRITE (6,160) ' - THE MASS MATRIX IS NOT //'
    & ' CONDENSED WITH THE STRUCTURAL STIFFNESS MATRIX.'
  ENDIF
ELSE
  WRITE (6,499)
499 FORMAT ('/SX, ZERO MASS MATRIX '//
  & 'SX,-----')
ENDIF
C -----
C INPUT DAMPING DATA ***
NDAMP = NUMBER OF NODAL DAMPING COEF TO BE INPUT =0
FDAMP = 0. NO DAMPING MATRIX
1. ALPHA AND BETA INPUT
NDAMP=0
FDAMP=1
READ (5,*) NDAMP,FDAMP
IF (FDAMP.EQ.1) THEN
  I2DAMP=I2
  I2=I2+2
  READ (5,*) ALPHA,BETA
  Z(I2DAMP) =ALPHA
  Z(I2DAMP+1) =BETA
  WRITE(6,500) ALPHA,BETA
ENDIF
500 FORMAT ('/SX, PROPORTIONAL DAMPING COEFFICIENTS//
  & 'SX,ALPHA',1P,G13.5,' BETA',1P,G13.5)
C
C ZERO LOAD AND DISPL INDICES ***
I2LOAD=0
I2DISP=0
I2VEL =0
I2ACC =0
I2DLOA=0
I2DOSP=0
I2DVEL=0
I2DACC=0
RETURN
END
C -----
DEBUG UNIT(6),SUBCHK,SUBTRACE

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END DEBUG
LOGICAL FUNCTION TEST(OPTION,TARGET,DEFLT)
CHARACTER*(*) OPTION,TARGET
LOGICAL DEFLT,BTEST
J= INDEX(OPTION,TARGET)
IF (DEFLT) THEN
  IF (J.LT.2) THEN
    TEST=.TRUE.
  ELSE IF (OPTION((J-2):(J-1)).NE.'NO') THEN
    TEST=.TRUE.
  ELSE
    TEST=.FALSE.
ENDIF
ELSE
  IF (J.EQ.0) THEN
    TEST=.FALSE.
  ELSE IF (J.LT.2) THEN
    TEST=.TRUE.
  ELSE IF (OPTION((J-2):(J-1)).EQ.'NO') THEN
    TEST=.FALSE.
  ELSE
    TEST=.TRUE.
ENDIF
RETURN
END
C -----
DEBUG UNIT(6),SUBCHK,SUBTRACE,INIT
C END DEBUG
C -----
C TRIANGULAR MATRIX MULTIPLICATION ***
C MATRIX A IS AN UPPER TRIANGULAR MATRIX, WITH MAIN DIAGONAL ADDRESS
C MATRIX MD, X IS A VECTOR, P=AX
C THE MULTIPLICATION IS CARRIED OUT FROM ROW I1 TO I2 AND FROM
C COLUMN J1 TO J2.
SUBROUTINE UTMPLY(PRINT,ZERO,IND,N,IR1,IR2,JC1,JC2,MD,A,P,X)
LOGICAL BTEST,PRINT,ZERO
IJJ(I,J)=MD(J)+J-I
IF (PRINT) THEN
  WRITE (6,500) 'TRIANGULAR MULTIPLICATION',
  & 'IR1,IR2,JC1,JC2,N
  & CALL WMATRX(A,MD,N,MD(N+1),'MATRIX A ')
  & CALL WMATRX(X,N,1,'MATRIX X')
ENDIF
P(J)=0
IF (ZERO) THEN
  DO 320 J=IR1,IR2
  P(J)=0
ENDIF
C -----
OLD METHOD OF MULTIPLICATION, TOO SLOW
DO 140 I=IR1,IR2
DO 140 J=JC1,JC2
IF (I.LE.K) THEN
  IK=MD(K)+K-I
  MDK=MD(K+I)
ELSE
  IK=MD(I)+I-K
  MDK=MD(I+1)
ENDIF
P(I)=P(I)+A(IK)*X(K)
140 CONTINUE
IF (PRINT) CALL WMATRX(P,N,1,'RESULT = A*X')
C -----
MULTIPLY TERMS ABOVE AND INCLUDING THE MAIN DIAGONAL
DO 20 J=JC1,JC2
  IRC=J-MD(J+1)+MD(J)+1
  MDJ=MD(J)+J
  I1=MAX(IRC,IR1)
  I2=MIN(J,IR2)
  IF (I2.GE.I1) THEN
    DO 10 I=I1,I2
      P(I)=P(I)+A(MDJ-I)*X(J)
    10 ENDF
  20 CONTINUE
C -----
MULTIPLY TERMS BELOW THE MAIN DIAGONAL
DO 30 I=IR1,IR2
  IRC=I-MD(I+1)+MD(I)+1
  MDI=MD(I)+1
  J1=MAX(IRC,JC1)
  J2=MIN(I-1,JC2)
  IF (J2.GE.J1) THEN
    DO 40 J=J1,J2
      P(I)=P(I)+A(MDI-J)*X(J)
    40 ENDF
  30 CONTINUE
CC IF (PRINT) CALL WMATRX(P,N,1,'RESULT = A*X')
C
500 FORMAT ('/X,A,2X, ' IR1',I5, ' IR2',I5,
  & ' JC1',I5, ' JC2',I5, ' N',I5)
IF (PRINT) THEN
  DO 515 I=IR1,IR2
  WRITE(6,510) I,X(I),P(I)
  515 WRITE (6,*)
  520 FORMAT (2X,'DOF',I6,' X(DOF)',1P,G14.5,' A*X(DOF)',G14.5)
ENDIF
RETURN
END
C-----
C=PROCESS SDUMP OPT(0) GOBMT XREF MAP
STR04900
STR04910
STR04920
STR04930
STR04940
STR04950
STR04960
STR04970
STR04980
STR04990
STR05000
STR05010
STR05020
STR05030
STR05040
STR05050
STR05060
STR05070
STR05080
STR05090
STR05100
STR05110
STR05120
STR05130
STR05140
STR05150
STR05160
STR05170
STR05180
STR05190
STR05200
STR05210
STR05220
STR05230
STR05240
STR05250
STR05260
TES00010
TES00020
C -----
DEBUG UNIT(6),SUBCHK,SUBTRACE

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```

20 CONTINUE
C----- MULTIPLY S_RED * DISP = P
DO 340 J=L3,L4
  IC1=J-(MD(J+1)-MD(J))+1
  IC2=MAX(IC1,L1)
  DO 340 I=J,IC2,-1
    IJ=I*(I+J)
    P(I)=P(I)+A(IJ)*X(J)
340 IF (PRINT) CALL WMATRX(P,N,I,'RESULT = A*X')
500 FORMAT (/2X,A/,2X,' IR1',15,' IR2',15,
  & ' JC1',15,' JC2',15,' N1',15)
  IF (PRINT) THEN
    DO 515 I=IR1,IR2
      WRITE(6,520) I,X(1),P(I)
515 WRITE(6,*)
520 FORMAT (2X,'DOF',16,' X(DOF)',1P,G14.5,' A*X=P(DOF)',G14.5)
  ENDF
  RETURN
END
C-----
C DEBUC UNIT(6),SUBCHK,SUBFRACE
C END DEBUC
C-----
REAL FUNCTION VCOS(V1,V2)
DIMENSION V1(3),V2(3)
VALUE (A1,A2,A3)=SQRT(A1**2+A2**2+A3**2)
DOT (A1,B1,C1,A2,B2,C2)=A1*A2+B1*B2+C1*C2
C-----
AV1=VALUE(V1(1),V1(2),V1(3))
AV2=VALUE(V2(1),V2(2),V2(3))
VDOT=DOT(V1(1),V1(2),V1(3),V2(1),V2(2),V2(3))
VCOS=VDOT/(AV1*AV2)
RETURN
END
C-----
V3 = V1 X V2, VECTOR CROSS PRODUCT ...
C-----
SUBROUTINE VCROSS(V3,V1,V2)
DIMENSION V1(3),V2(3),V3(3)
V3(1) = V1(2)*V2(3) - V1(3)*V2(2)
V3(2) = -(V1(1)*V2(3) - V1(3)*V2(1))
V3(3) = V1(1)*V2(2) - V1(2)*V2(1)
RETURN
END
C-----
DEBUC UNIT(6),SUBCHK,SUBFRACE
END DEBUC
C-----
SUBROUTINE WILSON(IOP,T,BUG,DT,TO,TF,T,THETA,NEWKDT,
  & L1,L2,L3,L4,L5,L6,IMDMS,IMD,IMDS,INDKG,INDOP,NFREE,
  & KLOAD,KGFORM,
  & V, D, P,
  & DA, DV, DD, DP, WORK,
  & MASS, MDMASS, DAMP, STIFF, MD,
  & KG, MDKG, ACC, GRAV,
  & S, MDG, R, PBAR,
  & PDAMP, FMASS, C1,C2,C3,C4,C5,C6,C7,ALPHA,BETA,C10,C11,ABORT)
COMMON /IDSTEP/ISTEP
INTEGER PDAMP,FMASS
REAL MASS,KG
LOGICAL BUG,NEWKDT,BREST,ABORT
DIMENSION A(NDOP),V(NDOP),D(NDOP),DA(NDOP),DV(NDOP),DD(NDOP)
DIMENSION P(NDOP),DP(NDOP),DUMMT(1),IDUMMT(1),WORK(2*NDOP)
DIMENSION PBAR(3),Q(1),Q1(3),R(1),R1(4)
DIMENSION MDMASS(NDOP+1),MASS(1MDMS)
DIMENSION MDKG(NDOP+1),KG(1MDKG)
DIMENSION MD(NDOP+1),STIFF(1MD)
DIMENSION MDS(NDOP+1),S(1MDS),DAMP(3)
C-----
GO TO (100,200,300) IOP
C-----
C INITIALIZE VALUES ---
C-----
100 CONTINUE
C1=6./(THETA*DT)**2
C2=3./(THETA*DT)
C3=6./(THETA*DT)
C4=THETA*DT/2.
C5=DT/2.
C6=DT**2/2.
C7=DT**2/6.
ALPHA=DAMP(1)
BETA=DAMP(2)
C10=1-BETA*C2
C11=C1-ALPHA*C2
C-----
C----- DEVELOP SKYLINE OF DYNAMIC STIFFNESS S
C----- LSTIFF = # OF STIFFNESS TERMS IN COLUMN J
C----- LMASS = # OF MASS TERMS IN COLUMN J
C----- LKG = # OF KG TERMS IN COLUMN J
MDS(1)=1
LKG=0
DO 110 J1=2,NFREE+1
  J=L3-1
  LSTIFF=MD(J)-MD(J-1)
  LMASS=MDMASS(J)-MDMASS(J-1)
  IF (KGFORM.EQ.2) LKG=MDKG(J)-MDKG(J-1)
  L=MIN(J1-1,MAX(LSTIFF,LMASS,LKG))
110 MDS(J1)=MDS(J1-1)+L
RETURN
C-----
C----- SOLVE FOR INCREMENTAL DISPLACEMENTS, VELOCITIES AND ACCELERATIONS --
C-----
200 CONTINUE
IF (BUG) THEN
  DO 205 I=L3,L4
    L=I-1
    D(I)=D(L)+V(I),A(I),P(I),DP(I)
205 WRITE(6,206) I,D(I),DISP',1P,G12.4,
206 FORMAT ('DOF',16,' DISP',1P,G12.4,' VEL',G12.4,
  & ' ACC',G12.4,' LOADS', G12.4,' DP',G12.4)
  ENDF
C-----
C----- FORM DYNAMIC STIFFNESS
IF (FDAMP.EQ.1) THEN
C-----
C----- SOME LOCAL VARIABLES
C-----
C J = COLUMN NUMBER FOR MASS AND STIFF
C I = ROW NUMBER FOR MASS AND STIFF
C J1 = COLUMN NUMBER FOR S
C I1 = ROW NUMBER FOR S
C I3STIF = ADDRESS OF ELEMENT I, J IN MATRIX STIFF
C IJMASS = ADDRESS OF ELEMENT I, J IN MATRIX MASS
C IJKG = ADDRESS OF ELEMENT I, J IN GEOMETRIC STIFFNESS MATRIX
C IJS = ADDRESS OF ELEMENT I, J1 IN MATRIX S
C LSTIFF = ROW # CONTAINING THE FIRST NONZERO TERM IN COLUMN J OF STIFF
C LMASS = ROW # CONTAINING THE FIRST NONZERO TERM IN COLUMN J OF MASS
C LKG = ROW # CONTAINING THE FIRST NONZERO TERM IN COLUMN J OF KG
C LS = ROW # CONTAINING THE FIRST NONZERO TERM IN COLUMN J OF S
C-----
DETERMINE THE FACTOR FOR KG
IF (KLOAD.EQ.1) THEN
  C10KG=C10*(1+ACC)
ELSE IF (KLOAD.EQ.2) THEN
  C10KG=C10
ENDF
C-----
205 J=L3,L4
  J1=J-1
  LSTIFF=J-(MD(J+1)-MD(J)-1)
  L5 = J1-(MDS(J1+1)-MDS(J1)-1)+L2
  L=MAX(L5,LSTIFF)
  DO 210 I=J,L,-1
    I1=I-L3+1
    IJS=(MDS(J1)+J1)-I1
    IJSTIF=(MD(J)+J)-I1
    S(IJS)=STIFF(IJSTIF)+C10
210 IF (L5.LT.LSTIFF) THEN
  DO 215 I=L3,L,-1
    IJS=(MDS(J1)+J1)-I1
    S(IJS)=0
215 ENDF
C-----
LMASS=J-(MDMASS(J+1)-MDMASS(J)-1)
  DO 220 I=J,L,-1
    I1=I-L3+1
    IJS=(MDS(J1)+J1)-I1
    IJMASS=(MDMASS(J)+J)-I1
    S(IJS)=MASS(IJMASS)+C11 + S(IJS)
220 IF (KGFORM.EQ.2) THEN
C-----
NOTE: THE GEOMETRIC STIFFNESS WAS DERIVED WITH COMPRESSION
BEING POSITIVE, KG HAS POSITIVE TERMS ON THE MAIN DIAGONAL
REPRESENTING COMPRESSION, THUS KG IS SUBTRACTED FROM S
  C-----
  CKG=1.00
  222 I=J,L,-1
    I1=I-L3+1
    IJS=(MDS(J1)+J1)-I1
    IJKG=(MDKG(J)+J)-I1
    S(IJS)=S(IJS) - KG(IJKG)*CKG
222 ENDF
C-----
225 CONTINUE
ENDF
C-----
CALCULATE Q AND R VECTORS
C-----
Q1 IS TEMPORARILY STORED IN DA
Q(I) AND R(I) ARE MULTIPLIED BY THE PROPER COEFFICIENTS FOR
PROPORTIONAL DAMPING
DO 230 I=L3,L4
  QI=C3*V(I) + C4*A(I)
  RI=J*V(I) + C4*A(I)
  DA(I)=QI
  R(I)=RI + ALPHA*RI
230 R(I)=BETA*RI
C-----
FORM DYNAMIC LOAD
C-----
INITIALISE PBAR=DP*THETA
DO 240 I=L3,L4
  PBAR(I)=DP(I)*THETA
240 IF (PDAMP.EQ.1) THEN
  PBAR=PBAR+MASS*Q
DO 250 J=L3,L4
  MDJ=MDMASS(J)+J
  M=J-(MDMASS(J+1)-MDMASS(J))+1
  M=MAX(M,L3)
  DO 250 I=M,J
    PBAR(I)=PBAR(I)+MASS(MDJ-I)*Q(J)
250 CONTINUE
DO 255 I=L3,L4
  MDI=MD(I)+1
  M=I-(MD(I+1)-MD(I))+1
  M=MAX(M,L3)
  DO 255 J=M,I-1
    PBAR(I)=PBAR(I)+MASS(MDI-J)*Q(J)
255 CONTINUE
C-----
PBAR=PBAR+STIFF*R
DO 260 J=L3,L4
  MDJ=MD(J)+J
  M=J-(MD(J+1)-MD(J))+1
  M=MAX(M,L3)
  DO 260 I=J,L4
    PBAR(I)=PBAR(I)+STIFF(MDJ-I)*R(J)
260 CONTINUE
DO 265 I=L3,L4
  MDI=MD(I)+1
  M=I-(MD(I+1)-MD(I))+1
  M=MAX(M,L3)
  DO 265 J=M,I-1
    PBAR(I)=PBAR(I)+STIFF(MDI-J)*R(J)
265 CONTINUE
ENDF
C-----
SOLVE FOR DISPLACEMENT AT TIME THETA*DT
C-----
NOTE: - WORK IS USED FOR TEMPORARY WORK SPACE
- PBAR CONTAINS THE INCREMENTAL DISPLACEMENT BETWEEN
TIMES T AND T+THETA*DT AFTER THE SECOND CALL TO MSOLL
CALL MSOLL(1,BUG,NFREE,IMDS,NFREE,1,1,NFREE,MDS,S,PBAR,WORK,
  & .FALSE.,DUMMT,1,DUMMT,1,
  & .FALSE.,DUMMT,1,DUMMT,1,ABORT)
IF (ABORT) RETURN
CALL MSOLL(3,BUG,NFREE,IMDS,NFREE,1,1,NFREE,MDS,S,PBAR,WORK,
  & .FALSE.,DUMMT,1,DUMMT,1,
  & .FALSE.,DUMMT,1,DUMMT,1,ABORT)
IF (ABORT) RETURN
C-----
DETERMINE ACCELERATION, VELOCITY AND DISPLACEMENT AT TIME DT
C-----
RECALL THAT Q1 WAS TEMPORARILY STORED IN DA(I)
DA = INCREMENTAL ACCELERATION BETWEEN TIMES T AND T+DT
DV = INCREMENTAL VELOCITY BETWEEN TIMES T AND T+DT
DD = INCREMENTAL DISPLACEMENT BETWEEN TIMES T AND T+DT
DO 270 I=L3,L4
  DA(I)=(C1*PBAR(I) - DA(I))/THETA
  DV(I)=DV(I) + C5*DA(I)
  DD(I)=(V(I)*DT + C6*A(I) + C7*DA(I)
270 DD(I)=V(I)*DT + C6*A(I) + C7*DA(I)
RETURN
C-----
CALC TOTAL DISPLACEMENTS, VELOCITIES AND ACCELERATIONS --
C-----
DO 310 I=1,NDOP
  A(I)=DA(I) + A(I)
  V(I)=DV(I) + V(I)
  D(I)=DD(I) + D(I)
  P(I)=DP(I) + P(I)
310 DP(I)=0.00
RETURN
END
C-----

```

```

C      DEBUG UNIT(6),SUBCHK, SUBTRACE          WMA00020
C      END DEBUG                               WMA00030
SUBROUTINE WMATRX(A,IR,IC,NAME)              WMA00040
DIMENSION A(IR,IC)                          WMA00050
CHARACTER*(*) NAME                          WMA00060
LOGICAL DMP,BTEST                            WMA00070
C
CALL GETINT(NAME,'DUMP',IDUMP,0.,.TRUE.,.FALSE.)
IF (IDUMP.NE.0) THEN
DMP=.TRUE.
ELSE
DMP=.FALSE.
ENDIF
C
WRITE (6,*) ' '
WRITE (6,*) 'PRINTING MATRIX: ',NAME
DO 50 ICG=1,IC,5
IF (ICG.GT.1) WRITE (6,*) ' '
K=MIN((ICG+5),IC)
WRITE (6,10) (J,J=ICG,K)
DO 20 I=1,IR
IF (DMP) THEN
DO 15 J=ICG,K
IF (A(I,J).NE.0) WRITE (IDUMP,*) I,J,A(I,J),NAME
ENDIF
20 WRITE (6,30) I,(A(I,J),J=ICG,K)
50 CONTINUE
RETURN
10 FORMAT(' COLUMN',T17,15,5(15X,15))
30 FORMAT(' ROW',14,6G20.6)
END
-----
C      DEBUG UNIT(6),SUBCHK, SUBTRACE          WMT00020
C      END DEBUG                               WMT00030
SUBROUTINE WMTX2(A,IR,IC,NAME,IR0M)        WMT00040
DIMENSION A(IR0M,IC)                       WMT00050
CHARACTER*(*) NAME                         WMT00060
LOGICAL DMP,BTEST                          WMT00070
C
CALL GETINT(NAME,'DUMP',IDUMP,0.,.TRUE.,.FALSE.)
IF (IDUMP.NE.0) THEN
DMP=.TRUE.
ELSE
DMP=.FALSE.
ENDIF
C
WRITE (6,*) ' '
WRITE (6,*) 'PRINTING MATRIX: ',NAME
DO 50 ICG=1,IC,5
IF (ICG.GT.1) WRITE (6,*) ' '
K=MIN((ICG+5),IC)
WRITE (6,10) (J,J=ICG,K)
DO 20 I=1,IR
IF (DMP) THEN
DO 15 J=ICG,K
IF (A(I,J).NE.0) WRITE (IDUMP,*) I,J,A(I,J),NAME
ENDIF
20 WRITE (6,30) I,(A(I,J),J=ICG,K)
50 CONTINUE
RETURN
10 FORMAT(' COLUMN',T17,15,5(15X,15))
30 FORMAT(' ROW',14,6F20.6)
END
-----
.*
.pf ;.ls normal
.fo on
.sp 1 A
.ub c .PROGRAM SEE
.fo off trunc
.sp 1 A
.ub FILE: SEE PORTMAN
.sp 1 A
.bf p4 ;.ls normal p5
-----
C      DEBUG UNIT(6),TRACE,SUBCHK,INIT, SUBTRACE
C      END DEBUG
PARAMETER (MMAX=1500)
LOGICAL PAXIS
CHARACTER*20 LABEL(3)
CHARACTER*60 TITLE
CHARACTER*42 TEMP
DIMENSION VX(3),VT(3),VZ(3),X(4),Y(4)
DIMENSION S(3,MMAX),SR(3,MMAX),KEY(MMAX),RMAX(3),RMIN(3)
DIMENSION E(1,MMAX),ER(3,MMAX)
CROSSJ(A1,B1,C1,A2,B2,C2)=B1*C2-B2*C1
CROSSK(A1,B1,C1,A2,B2,C2)=C1*A2-C2*A1
CROSSL(A1,B1,C1,A2,B2,C2)=A1*B2-A2*B1
VALUE (A1,A2,A3)=SQRT(A1**2+A2**2+A3**2)
DOT (A1,B1,C1,A2,B2,C2)=A1*A2+B1*B2+C1*C2
LABEL(1)='ROTATED X AXIS'
LABEL(2)='ROTATED Y AXIS'
LABEL(3)='ROTATED Z AXIS'
C
C----- INITIALISE THE PLOTTING DEVICE ----
CALL PLOTS(0,0,7)
C
C----- INPUT & PRINT POINTS -----
DO 10 I=1,MMAX
READ (10,*,END=15) KEY(I),(S(J,I),J=1,3),(B(J,I),J=1,3)
10 WRITE (6,20) KEY(I),(S(J,I),J=1,3),(B(J,I),J=1,3), 'INPUT POINT'
15 NUMB=I
IF (I.LE.MMAX) NUMB=I-1
20 FORMAT (1X,15,1P,6G12.4,1,A)
C
C----- LOOP FOR EACH VIEW -----
READ(5,*) NVIEW
DO 2000 IVIEW=1,NVIEW
C
C----- INPUT VIEWING DATA -----
READ (5,*) TITLE,KLEN,ILEN,PAXIS,VX,VT
C
C----- CALCULATE DIRECTION COSINES -
VZ(1)= VX(2)*VT(3)-VX(3)*VT(2)
VZ(2)=-VX(1)*VT(3)+VX(3)*VT(1)
VZ(3)= VX(1)*VT(2)-VX(2)*VT(1)
VT(1)= VZ(2)*VX(3)-VZ(3)*VX(2)
VT(2)=-VZ(1)*VX(3)+VZ(3)*VX(1)
VT(3)= VZ(1)*VX(2)-VZ(2)*VX(1)
DX=VALUE(VT(1),VT(2),VT(3))
DY=VALUE(VT(1),VT(2),VT(3))
DZ=VALUE(VZ(1),VZ(2),VZ(3))
DO 30 I=1,3
VX(I)=VX(I)/DX
VT(I)=VT(I)/DY
VZ(I)=VZ(I)/DZ
30
C
C----- PRINT DIRECTION COSINES -----
WRITE (6,*) ' VIEW NUMBER.....',IVIEW
WRITE (6,*) ' TITLE.....',TITLE
WRITE (6,*) ' X-LENGTH.....',KLEN
WRITE (6,*) ' Y-LENGTH.....',ILEN
WRITE (6,*) ' VX.....',VX
WRITE (6,*) ' VT.....',VT
WRITE (6,*) ' VZ.....',VZ
C
C----- CALCULATE & PRINT ROTATED COORDINATES ----
DO 100 J=1,NUMB
DO 40 I=1,3
SR(I,J)=0
ER(I,J)=0
DO 50 I=1,3
SR(I,J)= SR(I,J) + VX(I)*S(I,J)
ER(3,J)= ER(3,J) + VZ(I)*S(I,J)
ER(1,J)= ER(1,J) + VX(I)*E(I,J)
ER(2,J)= ER(2,J) + VT(I)*E(I,J)
IF (J.EQ.1) THEN
DO 60 I=1,3
RMAX(I)=MAX(SR(I,J),ER(I,J))
RMIN(I)=MIN(SR(I,J),ER(I,J))
ELSE
DO 70 I=1,3
RMAX(I)=MAX(SR(I,J),ER(I,J),RMAX(I))
RMIN(I)=MIN(SR(I,J),ER(I,J),RMIN(I))
70
ENDIF
100 WRITE (6,20) KEY(J),(SR(I,J),I=1,3),(ER(I,J),I=1,3),
& 'ROTATED POINT'
C
C----- PLOT POINTS, USING CALCOMP 1051 PLOTTER COMMANDS...
C----- LOOP FOR DIFFERENT VIEWS ----
DO 500 IJ=1,3
IF (IJ.EQ.1) THEN
IX=1
IY=2
ELSE IF (IJ.EQ.2) THEN
IX=2
IY=3
ELSE
IX=3
IY=1
ENDIF
C
C----- DRAW TRIM LINE -----
CALL PLOT(0.,11.,2)
CALL PLOT(0.,0.,2)
C
C----- MOVE ORIGIN TO 0.5,0.5 -----
CALL PLOT(-.75,-.5,-3)
C
C----- GET SCALE FACTORS FOR X-Y PLOT ---
X(1)=RMIN(IX)
X(2)=RMAX(IX)
Y(1)=RMIN(IY)
Y(2)=RMAX(IY)
CALL SCALE(X,KLEN,2.1)
CALL SCALE(Y,ILEN,2.1)
K3=X(3)
K4=Y(4)
T3=Y(3)
T4=Y(4)
C
C----- PLOT AXIS -----
IF (PAXIS) THEN
CALL AXIS(0.00,-.05,LABEL(IX),-20,KLEN,0.,X(3),X(4))
CALL AXIS(-.05,0.00,LABEL(IY), 20,ILEN,90.,T(3),T(4))
ENDIF
C
C----- PLOT MEMBERS -----
DO 200 J=1,NUMB
X(1)=SR(IX,J)
X(2)=ER(IX,J)
Y(1)=SR(IY,J)
Y(2)=ER(IY,J)
200 CALL LINE(X,T,2,1,0,0)
C
C----- PLOT TITLE -----
CALL SYMBOL(0,1,(ILEN+15),14,TITLE ,0.,80)
TEMP=LABEL(IX)/---//LABEL(IY)
CALL SYMBOL(0,1,(ILEN+01),.05,TEMP ,0.,42)
C
C----- MOVE ORIGIN BACK TO 0,0 AND ADVANCE PAGE ----
CALL PLOT((KLEN+.25),-.5,-3)
C
500 CONTINUE
1000 CONTINUE
2000 CONTINUE
C
C----- DRAW TRIM LINE -----
CALL PLOT(0.,11.,2)
CALL PLOT(0.,0.,2)
C
C----- CLOSE THE PLOTTING DEVICE ----
CALL PLOT(0.,0.,999)
C
STOP
END
-----
SEB00010
SEB00020
SEB00030
SEB00040
SEB00050
SEB00060
SEB00070
SEB00080
SEB00090
SEB00100
SEB00110
SEB00120
SEB00130
SEB00140
SEB00150
SEB00160
SEB00170
SEB00180
SEB00190
SEB00200
SEB00210
SEB00220
SEB00230
SEB00240
SEB00250
SEB00260
SEB00270
SEB00280
SEB00290
SEB00300
SEB00310
SEB00320
SEB00330
SEB00340
SEB00350
SEB00360
SEB00370
SEB00380
SEB00390
SEB00400
SEB00410
SEB00420
SEB00430
SEB00440
SEB00450
SEB00460
SEB00470
SEB00480
SEB00490
SEB00500
SEB00510
SEB00520
SEB00530
SEB00540
SEB00550
SEB00560
SEB00570
SEB00580
SEB00590
SEB00600
SEB00610
SEB00620
SEB00630
SEB00640
SEB00650
SEB00660
SEB00670
SEB00680
SEB00690

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