

Civil Engineering Study Structural Series 96-5



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**INRESB-3D-SUPII
Program Listing for PC**

**GENERAL PURPOSE PROGRAM FOR INELASTIC ANALYSIS
OF RC AND STEEL BUILDING SYSTEMS FOR 3D STATIC
AND DYNAMIC LOADS AND SEISMIC EXCITATIONS**

by

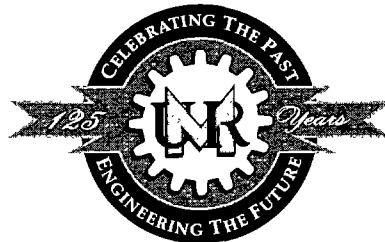
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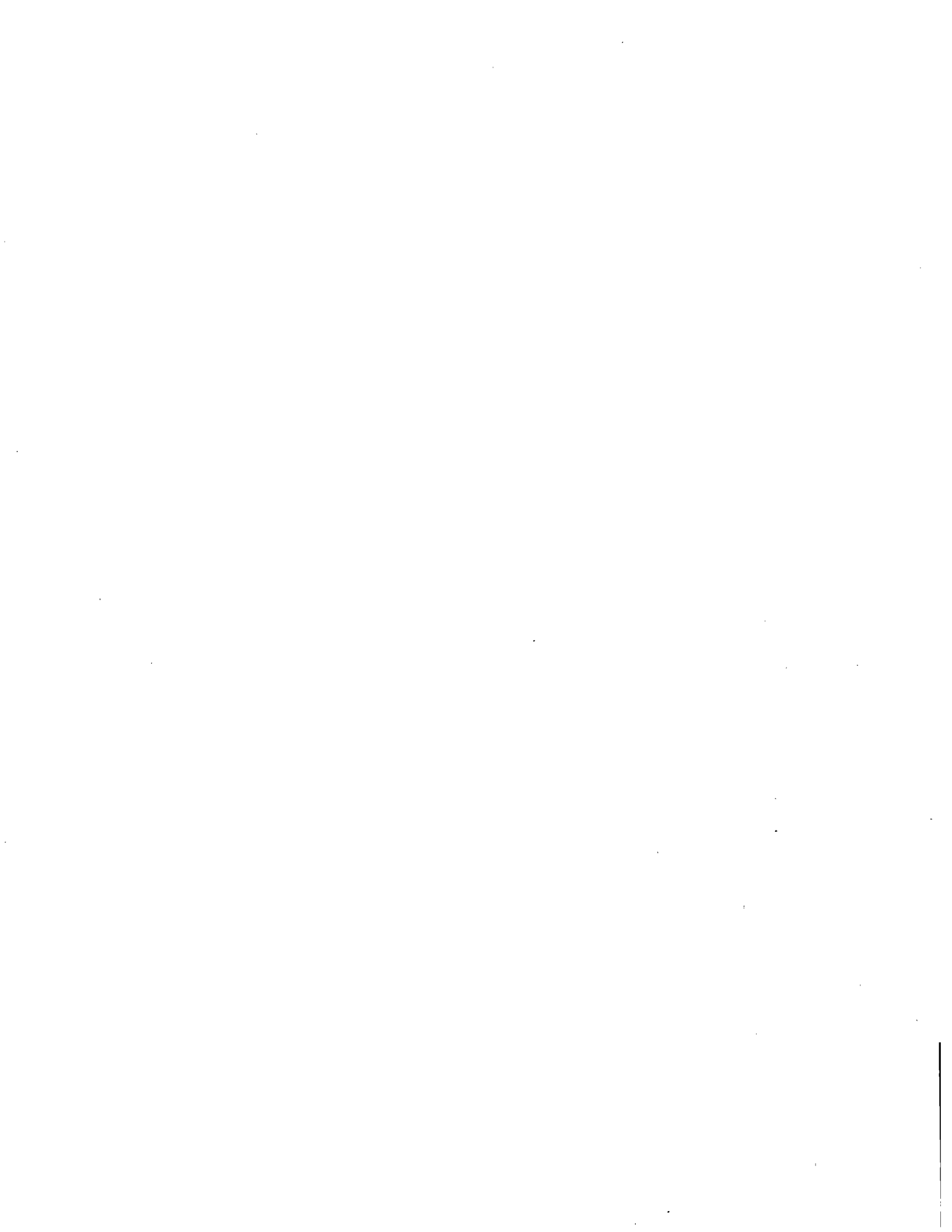
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ABSTRACT

INRESB-3D-SUPII (PC version) is a general purpose program written in FORTRAN and containing about 30,000 statements. While the PC and supercomputer versions have identical functions, PC solution capacity is limited by its memory.

INRESB-3D-SUPII is suitable for analyzing elastic and inelastic building systems subject to static loads, dynamic forces, multi-component earthquake motion, and pseudo-static cyclic loads. Also, the program is capable of calculating inelastic post-buckling behavior of steel members and systems, natural frequency, and spectral response analysis. A joint-based system is used to define the geometry of a structure. Structural members may be elastic 3D prismatic beams, nonlinear reinforced concrete shear walls, nonlinear springs, inelastic 3D-beam-column elements, finite-segment elements, and nonlinear bracing elements.

INRESB-3D-SUPII has five main functions. They are 1) elastic static analysis with multiple load cases based on control program SOL01; 2) elastic or inelastic analysis of a structure subjected to multiple ground acceleration (one-, two-, or three-dimensional seismic input) based on SOL02; 3) calculation of either natural frequencies and mode shape or buckling load and mode shape for an elastic structure based on SOL03; 4) calculation of nonlinear static cyclic response for a given loading pattern which consists of joint loads, imposed displacements or element loads for based on SOL04; and 5) calculation of maximum response for an elastic structure subject to pseudo-dynamic force obtained from the response spectrum based on SOL05.

INRESB-3D-SUPII pertains to analysis of building systems above ground level. Any connection with basements is treated as a boundary condition fixed, hinged, or with a spring.

Detail instruction of using the program is published in Civil Engineering Study Structural Series 96-3, INRESB-3D--SUPII, User's Manual.

OPERATION PROCEDURE

INRESB-3D-SUPII is written in FORTRAN and contains about 30,000 statements. In its PC version, this program comprises five independent programs: SOL01 (FEM1), SOL02 (FEM2), SOL03 (FEM3), SOL04 (FEM4), and SOL05 (FEM5). Five BAT files, LI1, LI2, LI3, LI4, and LI5, show the information needed to construct each program. These files are:

LI1.BAT

LINK FEM1+SOL01+HYST12+ELE09+STRUCT+ELELIB+MATLIB+MAEL1
+MAEL2+ELE01+MAT01+SKYLIN /ST:0X1000

LI2.BAT

LINK FEM2+SOL02+SOL02A+HYST12+ELE09+STRUCT+ELELIB+MATLIB+
MAEL1+MAEL2+ELE01+MAT01+SKYLIN /ST:0X1000

LI3.BAT

LINK FEM3+SOL03+SOL03A+HYST12+ELE09+STRUCT+ELELIB+MATLIB+
MAEL1+MAEL2+ELE01+MAT01+SKYLIN /ST:0X1000

LI4.BAT

LINK FEM4+SOL04+SOL04A+HYST12+ELE09+STRUCT+ELELIB+MATLIB+
MAEL1+MAEL2+ELE01+MAT01+SKYLIN /ST:0X1000

LI5.BAT

LINK FEM5+SOL05+SOL05A+HYST12+ELE09+STRUCT+ELELIB+MATLIB+
MAEL1+MAEL2+ELE01+MAT01+SKYLIN /ST:0X1000

ACKNOWLEDGMENTS

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```

-----CPFEM10010          WRITE(108,52) 'RELEASING MEMORY' FEM11470
PROGRAM FEM - ANALYSIS OF STRUCTURES CPFEM10020          WRITE(108,52) 'RELEASING MEMORY' FEM11480
CPFEM10030          IF (INDEX(OPTION, 'ELEMENT') .NE. 0) THEN FEM11490
CPFEM10040          WRITE(108,52) 'RESETTING ELEMENT FORCES' FEM11500
CPFEM10050          WRITE(108,52) 'RESETTING ELEMENT FORCES' FEM11510
CPFEM10060          DO 170 IELNO=1, NEMLT FEM11520
NOTE: USE IBM FORTRAN2 COMPILER OPTION AUTODBL(DBLPAD4) CPFEM10070          LETTYF=0 FEM11530
TO COMPILE THIS PROGRAM IN DOUBLE PRECISION CPFEM10080          HEAD=.FALSE. FEM11540
CPFEM10090          CPFEM10100          170 CALL ELLELIB FEM11550
NOTE: USE IBM FORTRAN2 COMPILER OPTION OPT(2) CPFEM10110          (13, LSTYP, .FALSE., IREL, HEAD, INPUT, EISE, EPSE, DAMAGE, DUCFAG, FEM11570
TO OPTIMIZE THE CODE FOR FASTER EXECUTION CPFEM10120          IELNO, IELDOF, KGDOP, PGEOM, 2, AXIAL, IZDOSP, IZDLOA, HSTCR) FEM11580
CPFEM10130          ELSE FEM11590
CHARACTER*80 OPTION, INPUT, BLANK FEM10140          WRITE(108,52) ' FEM11600
CHARACTER*8 TIME1, DATE1 FEM10150          ENDP FEM11610
CHARACTER*1 CHR(6,25) FEM10160          IZ=IREL FEM11620
LOGICAL TEST, HEAD, AXIAL FEM10170          IZACC=0 FEM11630
INCLUDE 'ZCCMN' FEM10180          IZDACC=0 FEM11640
DATA CHR('A','B','C','D','E','F','G','H','I','J','K','L','M', FEM10190          IZDOSP=0 FEM11650
'N','O','P','Q','R','S','T','U','V','W','X','Y','Z') FEM10200          IZDLOA=0 FEM11660
C FEM10210          IZDVEL=0 FEM11670
C FEM10220          IZDVEL=0 FEM11680
C-- ECHO THE INPUT FEM10230          IZDVEL=0 FEM11690
C FEM10240          IZDVEL=0 FEM11700
C FEM10250          IZDVEL=0 FEM11710
C FEM10260          RATIZ=100.*REAL(IZ)/REAL(MAXZ) FEM11720
9995 READ(105, '(A)', END=9996) INPUT FEM10270          C FEM11730
IF (INDEX(INPUT, 'NOBCHO') .NE. 0) GO TO 9996 FEM10280          C-- FEM11740
I=1 FEM10290          C-- GAVE MEMGRY FOR A RESTART -- FEM11750
L=L+1 FEM10300          C FEM11760
IF ( I.GT.55 ) THEN FEM10310          ELSE IF (INDEX(OPTION, 'SAVE') .NE. 0) THEN FEM11770
WRITE(108,9994) (KK, KK-1, 8) FEM10320          WRITE(*,52) 'SAVING DATA' FEM11780
I=0 FEM10330          IZ, RATIZ FEM11790
ENDIF FEM10340          WRITE(*,51) TITLE(1)(1:70) FEM11800
WRITE(108,9997) L, INPUT FEM10350          WRITE(*,51) TITLE(2)(1:70) FEM11810
IF (INDEX(INPUT, 'STOP') .EQ. 0) GO TO 9995 FEM10360          IZ=IREL FEM11820
9994 FORMAT ('1', 'ECHO OF INPUT DATA //', FEM10370          200 FORMAT(5X, '***** WRITING DATA TO UNIT FT09 *****') FEM11830
'1X,3X, 'LINE ', 8('...', 11, '0')) FEM10380          OPEN(UNIT=9, ACCESS='SEQUENTIAL', STATUS='UNKNOWN') FEM11840
9997 FORMAT (1X, I7, ' ', A) FEM10390          & FORM='UNFORMATTED') FEM11850
9996 REWIND(105) FEM10400          WRITE(9) TITLE FEM11860
C FEM10410          WRITE(9) IZ, IZREL FEM11870
C FEM10420          & NDOP, NSCOND, NPREE, NREST, MAXNO, NMAT, NEMT, NNODE, FEM11880
C-- INITIALIZE DATA FEM10430          & NLOAD, NSOLN, MAXEDF, MD, NCOS, MAXELD, NELD, FEM11890
C FEM10440          IZID, IZIDOF, IZCORD, IZCOS, IZCNSY, IZMAT, IZEL, FEM11900
C FEM10450          & IZJFLG, IZJCOS, FEM11910
C FEM10460          & IZKE, IZLM, IZSTIP, IZMD, IZLOAD, IZDISP, IZIELD, IZELD FEM11920
DO 210 J=1, IZ, 256 FEM10470          DO 210 J=1, IZ, 256 FEM11930
MD(I)=1 FEM10480          K=J-255 FEM11940
DO 1 I=2, (MAXEDF-1) FEM10490          K=MIN(K, IZ) FEM11950
1 MD(I)=MD(I-1)-(I-1) FEM10500          210 WRITE(9) (2(I), I=J, K) FEM11960
C FEM10510          CLOSE (UNIT=9) FEM11970
C FEM10520          RATIZ=100.*REAL(IZ)/REAL(MAXZ) FEM11980
C FEM10530          C FEM11990
C FEM10540          C-- FEM12000
C FEM10550          C-- RESTART PROGRAM -- FEM12010
C FEM10560          C FEM12020
C FEM10570          WRITE(*,52) 'RETRIVING DATA' FEM12030
C FEM10580          IZ, RATIZ FEM12040
C FEM10590          ELSE IF (INDEX(OPTION, 'RESTART') .NE. 0) THEN FEM12050
C FEM10600          WRITE(108,1001) FEM12060
C-- DEFINE THE STRUCTURE --- FEM10610          300 FORMAT(5X, '***** READING DATA FROM UNIT FT09 *****') FEM12070
C FEM10620          OPEN(UNIT=9, ACCESS='SEQUENTIAL', STATUS='OLD') FEM12080
C FEM10630          & FORM='UNFORMATTED') FEM12090
C FEM10640          READ (9) TITLE FEM12100
C FEM10650          READ (9) IZ, IZREL FEM12110
C FEM10660          & NDOP, NSCOND, NPREE, NREST, MAXNO, NMAT, NEMT, NNODE, FEM12120
C FEM10670          & NLOAD, NSOLN, MAXEDF, MD, NCOS, MAXELD, NELD, FEM12130
C FEM10680          IZID, IZIDOF, IZCORD, IZCOS, IZCNSY, IZMAT, IZEL, FEM12140
C FEM10690          & IZJFLG, IZJCOS, FEM12150
C FEM10700          & IZKE, IZLM, IZSTIP, IZMD, IZLOAD, IZDISP, IZIELD, IZELD FEM12160
DO 310 J=1, IZ, 256 FEM10710          DO 310 J=1, IZ, 256 FEM12170
MD(I)=1 FEM10720          K=J-255 FEM12180
DO 1 I=2, (MAXEDF-1) FEM10730          K=MIN(K, IZ) FEM12190
1 MD(I)=MD(I-1)-(I-1) FEM10740          310 READ (9) (2(I), I=J, K) FEM12200
C FEM10750          CLOSE (UNIT=9) FEM12210
C FEM10760          WRITE(*,52) 'DATA RETRIVED' FEM12220
C FEM10770          IZ, RATIZ FEM12230
C FEM10780          & WRITE(*,51) TITLE(1)(1:70) FEM12240
C FEM10790          & WRITE(*,51) TITLE(2)(1:70) FEM12250
C FEM10800          RATIZ=100.*REAL(IZ)/REAL(MAXZ) FEM12260
C FEM10810          C FEM12270
C-- DEFINE THE LOADING AND SOLVE --- FEM10820          C FEM12280
C FEM10830          C-- INVALID OPTION -- FEM12290
C FEM10840          ELSE IF (INDEX(OPTION, 'SOL') .NE. 0) THEN FEM12300
C FEM10850          ELSE FEM12310
C FEM10860          IF (TEST(OPTION, 'NOBCHO', .FALSE.)) GO TO 20 FEM12320
C FEM10870          IF (OPTION.EQ.BLANK) GO TO 20 FEM12330
C FEM10880          WRITE(108,1002) OPTION FEM12340
C FEM10890          WRITE(*,51) ('INVALID OPTION: '//OPTION(1:54)) FEM12350
C FEM10900          ENDP FEM12360
C FEM10910          GO TO 20 FEM12370
C FEM10920          C FEM12380
C FEM10930          45 FORMAT(' TIME ', A, ', DATE ', A) FEM12390
C FEM10940          50 FORMAT(A1, ' STRUCTURE...', A, /X, ' SOLUTION ... ', A, /) FEM12400
C FEM10950          51 FORMAT (1X, A) FEM12410
C FEM10960          52 FORMAT (1X, '...', A5, '...', IZ-', IZ', ' MEM=', P6, ' ', I) FEM12420
C FEM10970          30 FORMAT (' INVALID OPTION: ', A) FEM12430
C FEM10980          110 FORMAT (' 2MATRX('', I6, ' ', I5, I1P, G20 10, ', G20 10) FEM12440
C FEM10990          100 STOP FEM12450
C FEM11000          END FEM12460
C FEM11010          C FEM12470
C FEM11020          C-- TAKE PRESENT TIME AND DATE --- FEM12480
C FEM11030          C FEM12490
C FEM11040          SUBROUTINE TIDAE(TIME1, DATE1) FEM12500
C FEM11050          INTEGER*2 IYR, IMON, IDAY, IHR, IMIN, ISEC, I100TH FEM12510
C FEM11060          CHARACTER*8 TIME1, DATE1 FEM12520
C FEM11070          CALL GETDAT(IYR, IMON, IDAY) FEM12530
C FEM11080          IYR=IYR-1900 FEM12540
C FEM11090          CALL GETTIM(IHR, IMIN, ISEC, I100TH) FEM12550
C FEM11100          WRITE(DATE1, '(12, I4, ', 12, I4, ', 12)') IMON, IDAY, IYR FEM12560
C FEM11110          WRITE(TIME1, '(12, I4, ', 12, I4, ', 12)') IHR, IMIN, ISEC FEM12570
C FEM11120          RETURN FEM12580
C FEM11130          END FEM12590
C FEM11140          C FEM12600
C FEM11150          C FEM12610
C FEM11160          C FEM12620
C FEM11170          SUBROUTINE SOLN(OPTION) FEM12630
C FEM11180          CHARACTER*80 OPTION FEM12640
C FEM11190          INCLUDE 'ZCCMN' FEM12650
C FEM11200          STEP=1 FEM12660
C FEM11210          C FEM12670
C FEM11220          C-- DETERMINE SOLUTION --- FEM12680
C FEM11230          C FEM12690
C FEM11240          CALL GETINT(OPTION, 'SOL', NSOLN, 0, .TRUE., .FALSE.) FEM12700
C FEM11250          C FEM12710
C FEM11260          C FEM12720
C FEM11270          C-- CHOOSE THE SOLUTION --- FEM12730
C FEM11280          C FEM12740
C FEM11290          IF (NSOLN.EQ.1) THEN FEM12750
C FEM11300          CALL SOL1 FEM12760
C FEM11310          C FEM12770
C FEM11320          C ELSE IF (NSOLN.EQ.2) THEN FEM12780
C FEM11330          NLOAD=1 FEM12790
C FEM11340          MAXELD=0 FEM12800
C FEM11350          CALL SOL2 FEM12810
C FEM11360          C FEM12820
C FEM11370          C ELSE IF (NSOLN.EQ.3) THEN FEM12830
C FEM11380          NLOAD=1 FEM12840
C FEM11390          MAXELD=0 FEM12850
C FEM11400          CALL SOL3 FEM12860
C FEM11410          C FEM12870
C FEM11420          C ELSE IF (NSOLN.EQ.4) THEN FEM12880
C FEM11430          NLOAD=1 FEM12890
C FEM11440          CALL SOL4 FEM12900
C FEM11450          C FEM12910
C FEM11460          C ELSE IF (NSOLN.EQ.5) THEN FEM12920
C FEM11470          NLOAD=1 FEM12930
C FEM11480          MAXELD=0 FEM12940
C FEM11490          CALL SOL5 FEM12950
C FEM11500          C FEM12960

```

```

ELSE
  WRITE(106,*) 'INVALID SOLN',NSOLN
  STOP 'INVALID SOLUTION NUMBER'
ENDIF
RETURN
END
-----
C MATRIX SOLUTION OF EQUATION AX=P, WHERE THE UPPER TRIANGULAR MATRIX
C OF THE SYMMETRIC MATRIX A IS STORED BY SKYLINES. P IS STORED AS A
C FULL MATRIX. THE MATRIX P IS REPLACED BY X AFTER BACK SUBSTITUTION
C IS COMPLETE. BOTH A AND P ARE ALIGNED.
C GUYAN OPTION: IF GUYAN IS TRUE, MASS MATRIX IS ALSO CONDENSED
-----
SUBROUTINE MSOL(IOPT,PRINT,N,IMD,LROW,NLC,L1,L2,MD,A,P,FACTOR,
  & GUYAN,MASS,MDMASS,INDMAS,KGCCND,KG,MDKG,IMDKG)
  REAL MASS(INDMAS),KGCND
  LOGICAL GUYAN,PRINT,KGCCND
  DIMENSION A(IMD),FACTOR(N),P(LROW,NLC),MD(N*1)
  DIMENSION MDMASS(N*1),MDKG(N*1)
  IJ(I,J)=MD(J)-J+1
  IJM(I,J)=MDMASS(J)-J+1
  IJK(I,J)=MDKG(J)-J+1
-----
C MATRIX STORAGE SCHEME:
C THE MATRIX A IS Banded AND SYMMETRIC. THUS ONLY THE SKYLINE ABOVE
C AND THE MAIN DIAGONAL NEEDS TO BE STORED.
C THE MATRIX A IS STORED IN A LINEAR ARRAY BY COLUMNS. THE VALUE ON
C THE MAIN DIAGONAL IS STORED IN THE LINEAR ARRAY FIRST.
C ADDRESSES OF THE LINEAR ARRAY ELEMENTS CONTAINING THE MAIN DIAGONAL
C TERMS ARE STORED IN MD WORKED WITH.
C EXAMPLE: LET B BE THE 5X5 FULL SYMMETRIC MATRIX BELOW...
  1 1 3 12
  B = 2 5 11 NOTE: B(1,3),B(1,4) AND B(2,4) ARE ZERO.
      4 7 10
  SYMM: 6 9 MD = 1, 2, 4, 6, 8, 13
          8
  THUS, B(3,3)=A( MD(3) ) = A(4)
  B(1,3)=A( MD(3)+J-1 ) IF J=1 AND MD(J)-J-1=MD(J-1)
  B(1,4)=A( MD(4)+J-1 ) IF J=1 AND MD(J)-J-1=MD(J-1)
  MATRIX P IS NOT SYMMETRIC, AND IS STORED AS A FULL MATRIX
-----
C VARIABLE TABLE
C IOPT - OPTION NUMBER
C N - NUMBER OF DEGREES OF FREEDOM OF A MATRIX
C NLC - NUMBER OF LOAD CASES FOR THE P MATRIX
C L1 - FIRST ROW TO BE WORKED WITH
C L2 - LAST ROW TO BE WORKED WITH
C MD - ADDRESS OF THE MAIN DIAGONAL TERMS OF A
C P - LOAD MATRIX ON INPUT, SOLN (X) OF AX=P ON COMPLETION OF
C IOPT=4
C GUYAN - GUYAN REDUCTION FLAG FOR MASS MATRIX
C MASS - MASS MATRIX
C MDMASS - MASS MATRIX ADDRESSES OF MAIN DIAGONAL ELEMENTS
C KGCND - FLAG FOR CONDENSING OUT KG
C KG - GEOMETRIC STIFFNESS MATRIX
C MDKG - KG MATRIX ADDRESSES OF MAIN DIAGONAL ELEMENTS
C L - ROW NUMBER FOR ELIMINATION
C J - COLUMN NUMBER
-----
L1=MAX(L1,1)
L2=MIN(L2,N)
GO TO (10,110,210,310) IOPT
10 CONTINUE
  IOPT=1, REDUCE A AND P
  GAUSSIAN ELIMINATION IS USED TO REDUCE THE A AND P MATRICES FROM
  FROM ROW L1 TO ROW L2. IF L1 IS NOT 1, IT IS ASSUMED THAT
  A AND P ARE ALREADY REDUCED FROM 1 TO L1.
  IF (PRINT) THEN
    WRITE(108,500) 'IOPT=1, REDUCE STIFFNESS AND LOADS',L1,L2,N
    CALL LMATRX(A,MD,N,IMD,'INPUT STIFFNESS')
    CALL WHMATRX(P,N,NLC,'INPUT LOADS')
    IF (GUYAN) CALL LMATRX(MASS,MDMASS,N,INDMAS,'INPUT MASS')
    IF (KGCND) CALL LMATRX(KG,MDKG,N,IMDKG,'INPUT KG')
  ENDIF
  DO 100 J=L1,L2
    IF (L.EQ.N) GO TO 100
  C TEST FOR ZERO PIVOT
  IF (A(MD(J),EQ.0) THEN
    WRITE(108,2000) L,MD(L),A(MD(L))
    STOP
  ENDIF
  C CALCULATE THE FACTORS EACH ROW IS MULTIPLIED BY
  DO 30 I=L1,L2
    LI=I(L-1)
    IF (LI.LT.MD(I+1)) THEN
      FACTOR(I)=A(LI)/A(MD(I))
    ELSE
      FACTOR(I)=0
    ENDIF
  CONTINUE
  30
  C REDUCE A BY COLUMNS (J) EACH ROW (I)...
  DO 40 J=L1,L2
    LJ=J(L-1)
    IF (LJ.GE.MD(J-1)) GO TO 40
    DO 35 I=L1,J
      A(IJ)=A(IJ)-FACTOR(I)*A(LJ)
    CONTINUE
  40
  C REDUCE P BY COLUMNS (J) EACH ROW (I)...
  DO 50 J=L1,NLC
    IF (P(LJ,EQ.0) GO TO 50
    DO 45 I=L1,J-1
      P(I,J)=P(I,J)-FACTOR(I)*P(L,J)
    CONTINUE
  50
  C REDUCE MASS MATRIX BY GUYAN REDUCTION
  IF (GUYAN) THEN
    M22=M22 - K21/K11 * M11 * 1/K11 * K12
    RM=MASS(MDMASS(L))
    IF (RM.NE.0) THEN
      DO 60 J=L1,N
        IF (FACTOR(J,EQ.0) GO TO 60
        I1=MAX(L1,J-1),MDMASS(J-1)
        DO 55 I=L1,J
          IJ=I(J)
          MASS(IJ)=MASS(IJ)-FACTOR(I)*RM*FACTOR(J)
        CONTINUE
      55
    ENDIF
  60
  M22=M22 - K21/K11 * M12
  DO 66 J=L1,N
    LJ=J(L-1)
    IF (LJ.GE.MDMASS(J-1)) GO TO 66
    IF (MASS(LJ,EQ.0) GO TO 66
    DO 65 I=L1,J-1
      IJ=I(J)
      MASS(IJ)=MASS(IJ)-FACTOR(I)*MASS(LJ)
    CONTINUE
  65
  CONTINUE
  66
  M22=M22 - M21 * 1/K11 * K12
  DO 70 J=L1,N
    IF (FACTOR(J,EQ.0) GO TO 70
    DO 69 I=L1,J-1
      LI=I(J)
      IF (LI.GE.MDMASS(I-1)) GO TO 69
      IJ=I(J)
      IF (IJ.LT.MDMASS(I-1))
        MASS(IJ)=MASS(IJ)-FACTOR(J)*MASS(LI)
    CONTINUE
  69
  CONTINUE
  70
  ENDIF
  C REDUCE GEOMETRIC STIFFNESS MATRIX
  IF (KGCND) THEN
    KG22=KG22 - K21/K11 * K21 * 1/K11 * K22
    RKG=KG(MDKG(L))
    IF (RKG.NE.0) THEN
      DO 80 J=L1,N
        IF (FACTOR(J,EQ.0) GO TO 80
        I1=MAX(L1,J-1),MDKG(J-1)
        DO 75 I=L1,J-1
          IJ=I(J)
          KG(IJ)=KG(IJ)-FACTOR(I)*RKG*FACTOR(J)
        CONTINUE
      75
    ENDIF
  80
  IOPT=2, REDUCE LOADS ONLY
  DO 90 J=L1,N
    IF (FACTOR(J,EQ.0) GO TO 90
    DO 89 I=L1,J-1
      LI=I(J)
      IF (LI.GE.MDKG(I-1)) GO TO 89
      IJ=I(J)
      IF (IJ.LT.MDKG(I-1))
        MDKG(IJ)=MDKG(IJ)-FACTOR(I)*KG(LI)
    CONTINUE
  89
  CONTINUE
  90
  ENDIF
  100 CONTINUE
  IF (PRINT) THEN
    CALL LMATRX(A,MD,N,IMD,'REDUCED STIFFNESS')
    CALL WHMATRX(P,N,NLC,'REDUCED LOADS')
    IF (GUYAN) CALL LMATRX(MASS,MDMASS,N,INDMAS,'REDUCED MASS')
    IF (KGCND) CALL LMATRX(KG,MDKG,N,IMDKG,'REDUCED KG')
  ENDIF
  RETURN
  C 110 CONTINUE
  IOPT=2, REDUCE P ONLY
  GAUSSIAN ELIMINATION IS USED TO REDUCE THE P MATRIX FROM ROW L1 TO ROW L2. IF L1 IS NOT 1, IT IS ASSUMED THAT A AND P ARE ALREADY REDUCED FROM 1 TO L1.
  IF (PRINT) THEN
    WRITE(108,500) 'IOPT=2, REDUCE LOADS ONLY',L1,L2,N
    CALL LMATRX(A,MD,N,IMD,'REDUCED STIFFNESS')
    CALL WHMATRX(P,N,NLC,'NEW LOAD MATRIX')
  ENDIF
  C LOOP FOR EACH ROW (L) TO REDUCE...
  DO 160 L=L1,L2
  C IF (A(MD(L),EQ.0) THEN
    WRITE(108,2000) L,MD(L),A(MD(L))
    STOP
  ENDIF
  C LOOP FOR EACH COLUMN (J)
  DO 150 J=L1,NLC
    IF (P(LJ,EQ.0) GO TO 150
    DO 120 I=L1,L2
      LI=I(L-1)
      IF (LI.LT.MD(I+1)) P(I,J)=P(I,J)-A(LI)/A(MD(L))
    CONTINUE
  120
  CONTINUE
  150
  CONTINUE
  160
  RETURN
  C 210 CONTINUE
  IOPT=3, BACK SUBSTITUTION
  BACK SUBSTITUTION IS USED TO SOLVE FOR X IN S*D=P, WHERE S IS THE REDUCED UPPER TRIANGULAR FACTOR OF THE A MATRIX, AND V IS THE REDUCED PCRM OF THE P MATRIX. THE RESULTS (X) ARE STORED IN P.
  BACK SUBSTITUTION ONLY TAKES PLACE BETWEEN ROWS L1 AND L2.
  IF (PRINT) THEN
    WRITE(108,500) 'IOPT=3, BACK SUBSTITUTION',L1,L2,N
    CALL LMATRX(A,MD,N,IMD,'REDUCED STIFFNESS')
    CALL WHMATRX(P,N,NLC,'REDUCED LOADS')
  ENDIF
  C LOOP FOR EACH ROW L
  CALCULATE X FOR EACH LOAD CASE, STORE IN P
  DO 250 J=L1,L2
    IF (A(MD(J),EQ.0) THEN
      WRITE(108,2000) J,MD(J),A(MD(J))
      STOP
    ENDIF
    DO 215 LC=1,NLC
      P(J,LC)=P(J,LC)/A(MD(J))
    IF (J.GT.1) THEN
      I=MD(J-1)
      WHILE (MD(J-1).MD(J)) -1
      DO 220 I=NI,J-1,-1
        IJ=I(J)
        DO 220 LC=1,NLC
          P(IJ,LC)=P(IJ,LC)-P(J,LC)*A(MD(J))
        CONTINUE
      220
    ENDIF
  250
  CONTINUE
  IF (PRINT) CALL WHMATRX(P,N,NLC,'DISPLACEMENT')
  RETURN
  C 310 CONTINUE
  IOPT=4, REDUCED MULTIPLICATION
  SOLVE FOR V IN S*D=P WHERE S HAS BEEN REDUCED BY GAUSSIAN ELIMINATION
  THE RESULTING LOAD IS STORED IN P
  THE DISPLACEMENTS ARE INPUT IN D
  MULTIPLICATION ONLY TAKES PLACE BETWEEN ROWS L1 AND L2
  IF (PRINT) THEN
    WRITE(108,500) 'IOPT=4, REDUCED MULTIPLICATION',L1,L2,N
    CALL LMATRX(A,MD,N,IMD,'REDUCED STIFFNESS')
    CALL WHMATRX(FACTOR,N,NLC,'DISPLACEMENT')
  ENDIF
  ONLY ONE LOAD CASE...

```



```
LC=1
C----- ZERO P -----
DO 320 J=L1,L2
  IF (A(MD(J)), EQ 6) THEN
    WRITE(108,2006) J,MD(J),A(MD(J))
  STOP
  ENDIF
  P(J,LC)=0
C----- MULTIPLY S_RED * DISP = P -----
DO 340 J=L1,N
  IC1=MD(J+1),MD(J) *-1
  IC2=MAX(IC1,L1)
  DO 340 I=J,IC2,-1
    IJ=I*(I+J)
    P(I,LC)=P(I,LC)+A(IJ)*FACTOR(I)
  IF (PRINT) CALL WMATRIX(P,N,1,'REDUCED LOAD')
C----- UNREDUCE THE LOAD MATRIX -----
DO 360 L=L2,L1,-1
C----- LOOP FOR EACH COLUMN (J) -----
DO 350 J=L1,NLC
  IF (P(L,LC), EQ 0) GO TO 350
  DO 345 I=(L-1),N
    LI=I*(L+I)
    IF (LI LT MD(I+1)) P(I,LC)=P(I,LC)+A(LI)*P(L,LC)/A(MD(L))
  CONTINUE
345 CONTINUE
350 CONTINUE
400 CONTINUE
C----- IF (PRINT) CALL WMATRIX(P,N,1,'LOAD') -----
500 FORMAT (/2X,A,2X,'L1',15,' L2',15,' N:',15)
2000 FORMAT (///1X,50(' '))
C----- FOR IN SUBROUTINE MSO1L -----
* * * * * PIVOT IS LE 0 * * * * * ROW '15', T51, * * *
* * * * * STORAGE LOCN '15', T51, * * *
* * * * * PIVOT 'IP,G15,6', T51, * * *
* * * * * SOLUTION IS ABORTED * * *
* * * * * 1X,50(' '))
C----- RETURN -----
END
C----- SUBROUTINE REACTN(IOPT,PRINT,NODE,L1,L2,L3,L4,FORCE,DISPL,
* * * * * MD,A,NX,NDOF,NLOAD,FACTOR,
* * * * * MAXNOD,IDOF,COORD,ID,TITLE,STEPID,HEAD,
* * * * * NCOS,COSINE,JTCOS,JTFLG,SUM)
* * * * * CHARACTER*(*) TITLE,STEPID
* * * * * CHARACTER*15 CD(6)
LOGICAL PRINT,HEAD,FLAG,BTEST
DIMENSION A(NM),MD(NDOF*3),IDOF(MAXNOD,6),G(6)
DIMENSION FORCE(NDOF,NLOAD),DISPL(NDOF,NLOAD)
DIMENSION ID(MAXNOD),COORD(MAXNOD,3),JTFLG(MAXNOD)
DIMENSION P(6),SUM(6),COSINE(3,3),NCOS,JTCOS(MAXNOD)
C----- IF (IOPT.EQ.1) THEN -----
C----- MODIFY LOADS FOR RESTRAINT DISPLACEMENTS -----
C----- CONSIDER THE PARTITIONED STIFFNESS WHERE X2 ARE THE RESTRAINT DISPL. -----
C----- P1: X11 X12 X13 -----
C----- P2: X21 X22 X23 -----
C----- THE MODIFIED FREE LOAD IS (P1-K12-X2) - X11 * K -----
DO 10 I=L1,L2
  DO 10 J=L1,NLOAD
    DO 10 K=L3,L4
      FORCE(I,J)=FORCE(I,J)-K(I,K)*DISPL(K,J)
  CONTINUE
  DO 20 J=L1,NLOAD
    DO 10 I=L2,L1,-1
      IK=MD(K)+K-1
      IF (IK.GE.MDK) GO TO 20
      F13=FORCE(I,J)
      AIK=A(IK)
      DKJ=DISPL(K,J)
      FORCE(I,J)=FORCE(I,J)+AIK*DISPL(K,J)
  CONTINUE
  IF (PRINT)
    CALL WMATRIX(FORCE,NDOF,NLOAD,'MODIFIED LOADS')
  ELSE IF (IOPT.EQ.2) THEN
    C----- SOLVE FOR REACTIONS -----
    C----- CONSIDER THE PARTITIONED STIFFNESS WHERE X2 ARE THE RESTRAINT DISPL. -----
    C----- P1: X11 X12 X13 -----
    C----- P2: X21 X22 X23 -----
    C----- THE REACTIONS ARE P2 - P21 - (X21*X1 + X22*X2) * FACTOR -----
    NOTE: BOTH P1 AND P2 CONTAIN FIXED END FORCES FROM MEMBER LOADS...
    C----- THUS, X21*X1 + X22*X2 IS ADDED TO P2 TO GET THE REACTIONS -----
    DO 120 I=L3,L4
      DO 120 J=L1,NLOAD
        DO 120 K=L3,L4
          FORCE(I,J)=0
        P2 CONTAINS PEP
    CONTINUE
    DO 110 I=L3,L4
      DO 110 J=L1,NLOAD
        DO 110 K=L3,L4
          KI=MD(I)+K-1
          IF (KI.GE.MD(I+1)) GO TO 120
          AKI=A(KI)
          FORCE(I,J)=FORCE(I,J)+AKI*DISPL(K,J)*FACTOR
    CONTINUE
    DO 140 I=L3,L4
      DO 140 K=L3,L4
        IF (I LE K) THEN
          IK=MD(K)+K-1
          MDK=MD(K)-1
        ELSE
          IK=MD(I)+I-K
          MDK=MD(I)+1
        ENDIF
        IF (IK.GE.MDK) GO TO 140
        AIK=A(IK)
        DO 130 J=L1,NLOAD
          FORCE(I,J)=FORCE(I,J)+AIK*DISPL(K,J)*FACTOR
    CONTINUE
    IF (PRINT)
      CALL WMATRIX(FORCE,NDOF,NLOAD,'FINAL LOADS AND REACTIONS')
    ELSE IF (IOPT.EQ.3 .OR. IOPT.EQ.4) THEN
      NF=L2
    C----- LOOP FOR EACH LOAD CASE -----
MSO12870
MSO12880
MSO12890
MSO12900
MSO12910
MSO12920
MSO12930
MSO12940
MSO12950
MSO12960
MSO12970
MSO12980
MSO12990
MSO13000
MSO13010
MSO13020
MSO13030
MSO13040
MSO13050
MSO13060
MSO13070
MSO13080
MSO13090
MSO13100
MSO13110
MSO13120
MSO13130
MSO13140
MSO13150
MSO13160
MSO13170
MSO13180
MSO13190
MSO13200
MSO13210
MSO13220
MSO13230
MSO13240
MSO13250
MSO13260
MSO13270
MSO13280
MSO13290
MSO13300
REAC0010
REAC0020
REAC0030
REAC0040
REAC0050
REAC0060
REAC0070
REAC0080
REAC0090
REAC0100
REAC0110
REAC0120
REAC0130
REAC0140
REAC0150
REAC0160
REAC0170
REAC0180
REAC0190
REAC0200
REAC0210
REAC0220
REAC0230
REAC0240
REAC0250
REAC0260
REAC0270
REAC0280
REAC0290
REAC0300
REAC0310
REAC0320
REAC0330
REAC0340
REAC0350
REAC0360
REAC0370
REAC0380
REAC0390
REAC0400
REAC0410
REAC0420
REAC0430
REAC0440
REAC0450
REAC0460
REAC0470
REAC0480
REAC0490
REAC0500
REAC0510
REAC0520
REAC0530
REAC0540
REAC0550
REAC0560
REAC0570
REAC0580
REAC0590
REAC0600
REAC0610
REAC0620
REAC0630
REAC0640
REAC0650
REAC0660
REAC0670
REAC0680
REAC0690
REAC0700
REAC0710
REAC0720
REAC0730
REAC0740
REAC0750
REAC0760
REAC0770
REAC0780
REAC0790
REAC0800
REAC0810
REAC0820
REAC0830
REAC0840
REAC0850
REAC0860
REAC0870
REAC0880
REAC0890
REAC0900
REAC0910
REAC0920
REAC0930
REAC0940
REAC0950
REAC0960
REAC0970
REAC0980
REAC0990
REAC1000
REAC1010
REAC1020
C----- DO 300 L=L1,NLOAD -----
C----- C----- PRINT GLOBAL REACTIONS -----
C----- IF (PRINT) THEN -----
C----- IF (HEAD) WRITE(108,320) TITLE(1),TITLE(2)
C----- NREACT=0
C----- IF (IOPT.NE.4) WRITE(108,321) L,STEPID
C----- IF (IOPT.EQ.4) WRITE(108,421)
ENDIF
C----- IF (IOPT.EQ.3) THEN -----
190 DO 290 I=1,6
  SUM(I)=0
  ENDF
C----- IF (.NOT.(NCOS.EQ.1 .AND. COSINE(1,1),EQ.1 .AND. COSINE(2,2),EQ.1
* * * * * EQ.1 .AND. COSINE(3,3),EQ.1) .AND. IOPT.EQ.4) GO TO 279
C----- DO 200 I=1,NNODE -----
C----- GET REACTNS IN JOINT COORDINATE SYSTEM -----
FLAG=.FALSE.
DO 209 J=1,6
  IJ=IDOF(I,J)
  IF (IJ LE MF .OR. BTEST(JTFLG(I),J-1)) THEN
    F(J)=0
  ELSE
    F(I)=FORCE(IJ,L)
    FLAG=.TRUE.
  ENDF
209 CONTINUE
  IF (.NOT.FLAG) GO TO 200
C----- TRANSFER REACTNS TO GLOBAL COORDINATE SYSTEM -----
DO 229 I1=1,3
  I2=I1+3
  G(I1)=G(I1) + COSINE(J1,I1,JTCOS(I1))*F(J1)
  G(I2)=G(I2) + COSINE(J1,I1,JTCOS(I1))*F(J2)
  IF (PRINT) THEN
    WRITE(CD(I1),210) G(I1)
    WRITE(CD(I2),210) G(I2)
  ENDF
229 CONTINUE
C----- SUM REACTIONS -----
IF (IOPT.EQ.3) THEN
  DO 240 I1=1,6
    SUM(I1)=SUM(I1) + G(I1)
  SUM(1)=SUM(1) + G(1)*COORD(I,2)
  SUM(2)=SUM(2) + G(1)*COORD(I,3)
  SUM(3)=SUM(3) + G(1)*COORD(I,1)
  SUM(4)=SUM(4) + G(1)*COORD(I,2) + G(2)*COORD(I,1)
  ENDF
C----- PRINT GLOBAL REACTIONS -----
NREACT=NREACT+1
IF (PRINT) WRITE(108,220) ID(I),CD(K),K=1,6)
200 CONTINUE
IF (IOPT.EQ.3 .AND. PRINT) WRITE(108,230) SUM
279 IF (IOPT.EQ.4 .AND. PRINT) WRITE(108,231) SUM
C----- PRINT LOCAL JOINT COORDINATE SYSTEMS REACTIONS -----
IF (.NOT.PRINT) GO TO 300
IF (NCOS.EQ.1 .AND. COSINE(1,1),EQ.1 .AND. COSINE(2,2),EQ.1
* * * * * .AND. COSINE(3,3),EQ.1) GO TO 300
IF (HEAD .AND. NREACT.GT.20) WRITE(108,320) TITLE(1),TITLE(2)
IF (IOPT.NE.4) WRITE(108,322) L,STEPID
IF (IOPT.EQ.4) WRITE(108,422)
DO 290 I=1,NNODE
C----- GET REACTNS IN JOINT COORDINATE SYSTEM -----
FLAG=.FALSE.
DO 280 J=1,6
  IJ=IDOF(I,J)
  IF (IJ LE MF .OR. BTEST(JTFLG(I),J-1)) THEN
    CD(J)=0
  ELSE
    WRITE(CD(J),210) FORCE(IJ,L)
    FLAG=.TRUE.
  ENDF
280 CONTINUE
  IF (.NOT.FLAG) GO TO 290
C----- PRINT REACTNS IN LOCAL COORDINATE SYSTEM -----
WRITE(108,221) ID(I),CD(K),K=1,6),JTCOS(I)
290 CONTINUE
300 CONTINUE
210 FORMAT (1P,G15.7)
220 FORMAT (5X,15,6A)
221 FORMAT (5X,15,6A,I10)
222 FORMAT (1X,90(' '),SIMULATION',G15.7)
223 FORMAT (1X,89(' '),GCS SUM',5G15.7)
224 FORMAT (1X,STRUCTURE',A,A, SOLUTION',A)
225 FORMAT (/// GCS RESTRAINT REACTIONS, LOADING #,15,5X,A, /
* * * * * //6X,'NODE',7X,'FX',
* * * * * 13X,'FY',13X,'FZ',13X,'MX',13X,'MY',13X,'MZ')
226 FORMAT (/// JCS RESTRAINT REACTIONS, LOADING #,15,5X,A, /
* * * * * //6X,'NODE',7X,'FX',
* * * * * 13X,'FY',13X,'FZ',13X,'MX',13X,'MY',13X,'MZ',
* * * * * 6X,' COSINE #')
421 FORMAT (/// MAXIMUM GCS RESTRAINT REACTIONS' /
* * * * * 5X,'NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY' /
* * * * * //6X,'NODE',7X,'FX',
* * * * * 13X,'FY',13X,'FZ',13X,'MX',13X,'MY',13X,'MZ')
422 FORMAT (/// MAXIMUM JCS RESTRAINT REACTIONS' /
* * * * * 5X,'NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY' /
* * * * * //6X,'NODE',7X,'FX',
* * * * * 13X,'FY',13X,'FZ',13X,'MX',13X,'MY',13X,'MZ',
* * * * * 6X,' COSINE #')
ELSE
  WRITE(108,*) 'INVALID OPTION IN SUBROUTINE REACTN, IOPT=',IOPT
  ENDF
  RETURN
  END
C----- SUBROUTINE PGM(IOPT) -----
INCLUDE 'COMMON'
C----- MATRIX STORAGE SCHEME -----
C----- THE STIFFNESS MATRIX (K) IS BANDED AND SYMMETRIC -----
C----- THUS ONLY THE SKYLINE ABOVE THE MAIN DIAGONAL NEEDS TO BE STORED -----
C----- THE MATRIX K IS STORED IN A LINEAR ARRAY BY COLUMNS -----
C----- THE VALUE ON THE MAIN DIAGONAL IS STORED IN THE LINEAR ARRAY FIRST -----
C----- RELATIVE ADDRESSES OF THE LINEAR ARRAY ELEMENTS CONTAINING THE -----
C----- MAIN DIAGONAL TERMS ARE STORED IN MD -----
C----- THE ABSOLUTE ADDRESS OF THE FIRST ELEMENT IN THE K MATRIX IS I2STIF -----
C----- THE STIFFNESS MATRIX K IS STORED IN THE 2 ARRAY, 2(I2STIF)+K(1,1) -----
C----- THE ABSOLUTE ADDRESS OF THE FIRST ELEMENT IS THE MD MATRIX IS I2MD -----
C----- THE STIFFNESS MATRIX K IS STORED IN THE N2 ARRAY, N2(I2MD)+MD(1) -----
FORM0030
FORM0040
FORM0050
FORM0060
FORM0070
FORM0080
FORM0090
FORM0100
FORM0110
FORM0120
FORM0130
FORM0140
FORM0150
FORM0160
FORM0170
FORM0180
FORM0190
FORM0200
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C THUS: IF I LE J (UPPER TRIANGULAR MATRIX) FORM0210 RETURN FORM1670
C IF I2ND=1+J (I2ND=1+J-1) FORM0220 END FORM1680
C 1J+NZ(I2ND-1+J)-J-1 RELATIVE ADDRESS IF ELEMENT (I,J) FORM0230 C-----CFEM20010
C B(I,J)-Z(I2STIP+1-1J) FORM0240 FORM0640 CFEM20020
C ELSE B(I,J)-Z(I2STIP+1-1J) FORM0250 C PROGRAM FEM - ANALYSIS OF STRUCTURES CFEM20030
C B(I,J)-0 (I,J) IS ABOVE THE SKYLINE FORM0260 C-----CFEM20040
C EXAMPLE: LET B BE THE 5X5 FULL SYMMETRIC MATRIX BELOW... FORM0270 C-----CFEM20050
C 1 1 3 4 12 13 NOTE: B(1,3),B(1,4) AND B(2,4) ARE ZERO. FORM0280 CFEM20060
C 2 2 5 7 11 11 FORM0290 C NOTE: USE IBM FORTRAN COMPILER OPTION AUTOBL(DBLPAD) CFEM20070
C 3 3 6 8 10 10 TO COMPIL THIS PROGRAM IN DOUBLE PRECISION CFEM20080
C 4 4 7 10 9 9 FORM0300 C NOTE: USE IBM FORTRAN COMPILER OPTION OPT(1) CFEM20090
C 5 5 8 9 8 8 TO OPTIMIZE THE CODE FOR FASTER EXECUTION CFEM20100
C SYMM. 6 6 9 8 8 MD= 1, 2, 4, 6, 8, 13 FORM0310 C-----CFEM20110
C SAY I2ND=121,I2STIP=200 FORM0320 C-----CFEM20120
C NZ(121)=1 2(200)=1 FORM0330 C-----CFEM20130
C NZ(122)=2 2(199)=2 FORM0340 FEM20140
C NZ(123)=4 2(198)=3 FORM0350 C CHARACTER*40 OPTION INPUT, BLANK FEM20150
C NZ(124)=6 2(197)=4 FORM0360 CHARACTER*4 TIME1,DATE1 FEM20160
C NZ(125)=8 2(196)=5 FORM0370 CHARACTER*4 CHR(0:25) FEM20170
C NZ(126)=13 2(195)=6 FORM0380 LOGICAL TEST FEM20180
C 2(194)=7 FORM0390 DATA CHR('A','B','C','D','E','F','G','H','I','J','K','L','M', FEM20190
C 2(193)=8 FORM0400 '&' 'N','O','P','Q','R','S','T','U','V','W','X','Y','Z')/ FEM20200
C 2(192)=9 FORM0410 C-----CFEM20210
C 2(191)=10 FORM0420 C INCLUDE 'ZCON' FEM20220
C 2(190)=11 FORM0430 C-----CFEM20230
C 2(189)=12 FORM0440 C C-----CFEM20240
C THUS, FOR B(3,4) I=3, J=4 FORM0450 I=55 FEM20250
C NZ(I2ND-1+J)-J-1 = NZ(121-1+4)-4-3 = 6+4-3 = 7 FORM0460 I=0 FEM20260
C B(I,J)-Z(I2STIP+1-1J) = Z(200-1+7) = Z(206) = 7 OK FORM0470 9995 READ(105, '(A)', END=9996) INPUT FEM20270
C THUS, FOR B(2,4) I=2, J=4 FORM0480 IP (INDEX(INPUT, 'MCECHO').NE.0) GO TO 9996 FEM20280
C 1J+NZ(I2ND-1+J)-J-1 = NZ(121-1+4)-4-2 = 6+4-2 = 8 FORM0490 L=1 FEM20290
C NZ(I2ND-1+J)-NZ(121-1+4)-1 = 8 FORM0500 IP ( I.GT.55 ) THEN FEM20300
C SINCE 1J GE NZ(I2ND-1+J)+1 FORM0510 WRITE(108,9994) (KX, KK-1,8) FEM20310
C B(I,J)=0 (I,J) IS ABOVE THE SKYLINE... FORM0520 I=0 FEM20320
C-----CFEM20330
C BUGS = BTEST(1,BUG,5) FORM0530 ENDIF FEM20340
C BUGS7 = BTEST(1,BUG,7) AND BUGS FORM0540 WRITE(108,9997) L,INPUT FEM20350
C LSTYP=0 FORM0550 IP (INDEX(INPUT, 'STOP') .EQ.0) GO TO 9995 FEM20360
C PRINT = .FALSE. FORM0560 9994 FORMAT (1) ECHO OF INPUT DATA // FEM20370
C HEAD = .FALSE. FORM0570 & 1X,3X,'LINE ',8('.....'),11,'0') FEM20380
C-----CFEM20390
C-----CFEM20400
C-----CFEM20410
C-----CFEM20420
C-----CFEM20430
C-----CFEM20440
C-----CFEM20450
C-----CFEM20460
C-----CFEM20470
C-----CFEM20480
C-----CFEM20490
C-----CFEM20500
C-----CFEM20510
C-----CFEM20520
C-----CFEM20530
C-----CFEM20540
C-----CFEM20550
C-----CFEM20560
C-----CFEM20570
C-----CFEM20580
C-----CFEM20590
C-----CFEM20600
C-----CFEM20610
C-----CFEM20620
C-----CFEM20630
C-----CFEM20640
C-----CFEM20650
C-----CFEM20660
C-----CFEM20670
C-----CFEM20680
C-----CFEM20690
C-----CFEM20700
C-----CFEM20710
C-----CFEM20720
C-----CFEM20730
C-----CFEM20740
C-----CFEM20750
C-----CFEM20760
C-----CFEM20770
C-----CFEM20780
C-----CFEM20790
C-----CFEM20800
C-----CFEM20810
C-----CFEM20820
C-----CFEM20830
C-----CFEM20840
C-----CFEM20850
C-----CFEM20860
C-----CFEM20870
C-----CFEM20880
C-----CFEM20890
C-----CFEM20900
C-----CFEM20910
C-----CFEM20920
C-----CFEM20930
C-----CFEM20940
C-----CFEM20950
C-----CFEM20960
C-----CFEM20970
C-----CFEM20980
C-----CFEM20990
C-----CFEM21000
C-----CFEM21010
C-----CFEM21020
C-----CFEM21030
C-----CFEM21040
C-----CFEM21050
C-----CFEM21060
C-----CFEM21070
C-----CFEM21080
C-----CFEM21090
C-----CFEM21100
C-----CFEM21110
C-----CFEM21120
C-----CFEM21130
C-----CFEM21140
C-----CFEM21150
C-----CFEM21160
C-----CFEM21170
C-----CFEM21180
C-----CFEM21190
C-----CFEM21200
C-----CFEM21210
C-----CFEM21220
C-----CFEM21230
C-----CFEM21240
C-----CFEM21250
C-----CFEM21260
C-----CFEM21270
C-----CFEM21280
C-----CFEM21290
C-----CFEM21300
C-----CFEM21310
C-----CFEM21320
C-----CFEM21330
C-----CFEM21340
C-----CFEM21350
C-----CFEM21360
C-----CFEM21370
C-----CFEM21380
C-----CFEM21390
C-----CFEM21400
C-----CFEM21410
C-----CFEM21420
C-----CFEM21430
C-----CFEM21440

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ELSE
  IF (TEST(OPTION,'NOBCHO',.FALSE.)) GO TO 20
  IF (OPTION.EQ.BLANK) GO TO 20
  WRITE(108,30) OPTION
  WRITE(*,51) 'INVALID OPTION. '//OPTION(1:54)
ENDIF
GO TO 20
C
45 FORMAT(' TIME: ',A,' DATE: ',A)
50 FORMAT(' ALL STRUCTURE ... ',A,'/X,' SOLUTION ... ',A,/)
51 FORMAT(' IX,A)
52 FORMAT(' IX, '---',A65,'---', ' I2-',17,' MEM-',F6.3,' ')
53 FORMAT(' (1) INVALID OPTION= ',A)
60 FORMAT(' ZMATRIX(1,16,1),',115,IG,20.10,',G20.10)
1000 STOP
END
C
----- TAKE PRESENT TIME AND DATE -----
C
SUBROUTINE TIDAP(TIME1,DATE1)
  INTEGER*2 IYR,IMON,IDAY,IHR,IMIN,ISEC,1100TH
  CHARACTER*8 TIME1,DATE1
  CALL GETDAT(IYR,IMON,IDAY)
  IYR=IYR-1900
  CALL GETTIM(IHR,IMIN,ISEC,1100TH)
  WRITE(DATE1,'(12,1h,12,1h,12i)') IMON,IDAY,IYR
  WRITE(TIME1,'(12,1h,12,1h,12i)') IHR,IMIN,ISEC
  RETURN
END
C
-----
C
SUBROUTINE SOLN(OPTION)
  CHARACTER*80 OPTION
$INCLUDE 'ZCOMM'
  STEP10=
C
-----
C
C DETERMINE SOLUTION NO.
C
CALL GETINT(OPTION,'SOL',NSOLN,0,'TRUE',.FALSE.)
C
-----
C
C CHOOSE THE SOLUTION --
C
IF (NSOLN.EQ.1) THEN
  CALL SOL01
ELSE IF (NSOLN.EQ.2) THEN
  IF (NSOLN.EQ.3) THEN
    NLOAD=1
    MAXELD=0
    CALL SOL02
  ELSE IF (NSOLN.EQ.3) THEN
    NLOAD=1
    MAXELD=0
    CALL SOL03
  ELSE IF (NSOLN.EQ.4) THEN
    NLOAD=1
    CALL SOL04
  ELSE IF (NSOLN.EQ.5) THEN
    NLOAD=1
    MAXELD=0
    CALL SOL05
  ELSE
    WRITE(108,*) 'INVALID SOLUTION',NSOLN
    STOP 'INVALID SOLUTION'
  ENDIF
  RETURN
END
C
-----
C
C FULL SOLUTION OF EQUATION AX=P, WHERE THE UPPER TRIANGULAR MATRIX
C OF THE SYMMETRIC MATRIX A IS STORED BY SKYLINES. P IS STORED AS A
C FULL MATRIX. THE MATRIX P IS REPLACED BY X AFTER BACK SUBSTITUTION
C IS COMPLETE. BOTH A AND P ARE ALTERED.
C
C GUYAN OPTION: IF GUYAN IS TRUE, MASS MATRIX IS ALSO CONDENSED
C
SUBROUTINE SOLN(IOPT,PRINT,N,IMD,LROW,NLC,L1,L2,MD,A,P,FACTOR,
  & GUYAN,MASS,MDMASS,INDMAS,KGCOND,KG,MDKG,INDKG)
  REAL MASS(IMD),AS(KG(IMDKG))
  LOGICAL GUYAN,PRINT,KGCOND
  DIMENSION A(IMD),FACTOR(N),P(LROW,NLC),MD(N+1)
  DIMENSION MDMASS(N+1),MDKG(N+1)
C
  IJ1(I,J)=MD(J)-J+1
  IJM(I,J)=MDMASS(J)-J+1
  IJKG(I,J)=MDKG(J)-J+1
C
MATRIX STORAGE SCHEME:
  -THE MATRIX A IS Banded and SYMMETRIC. THUS ONLY THE SKYLINE ABOVE
  AND THE MAIN DIAGONAL NEEDS TO BE STORED
  -THE MATRIX P IS STORED IN A LINEAR ARRAY BY COLUMNS. THE VALUE ON
  THE MAIN DIAGONAL IS STORED IN THE LINEAR ARRAY FIRST.
  -ADDRESSES OF THE LINEAR ARRAY ELEMENTS CONTAINING THE MAIN DIAGONAL
  TERMS ARE STORED IN MD
  -EXAMPLE: LET B BE THE SKS FULL SYMMETRIC MATRIX BELOW.
  B=
  1 3 11
  2 5 11
  4 7 10
  SYMM. 6 9 MD= 1, 2, 4, 6, 9, 13
  THUS, B(3,3)-A(MD(3)) = A(4)
  B(I,J)-A(MD(J)-J+1) IF J>I AND (MD(J)-J+1)>MD(J-1)
C
MATRIX P IS NOT SYMMETRIC, AND IS STORED AS A FULL MATRIX
C
-----
C
VARIABLE TABLE
IOPT = OPTION NUMBER
N = NUMBER OF DEGREES OF FREEDOM OF A MATRIX
NLC = NUMBER OF LOAD CASES FOR THE P MATRIX
L1 = FIRST ROW TO BE WORKED WITH
L2 = LAST ROW TO BE WORKED WITH
C MD = ADDRESS OF THE MAIN DIAGONAL TERMS OF A
P = LOAD MATRIX ON INPUT, SOLN(X) OF AX=P ON COMPLETION OF
C
IOPT=4
FACTOR = TEMPORARY STORAGE MATRIX
GUYAN = GUYAN REDUCTION FLAG FOR MASS MATRIX
MASS = MASS MATRIX
MDMASS= MASS MATRIX ADDRESSES OF MAIN DIAGONAL ELEMENTS
KGCOND= FLAG FOR CONDENSING OUT KG
KG = GEOMETRIC STIFFNESS MATRIX
MDKG = KG MATRIX ADDRESSES OF MAIN DIAGONAL ELEMENTS
L = ROW NUMBER FOR ELIMINATION
J = ROW NUMBER
J = COLUMN NUMBER
C
L1=MAX(L1,1)
L2=MIN(L2,N)
GO TO (10,110,210,310) IOPT
10 CONTINUE
IOPT=1, REDUCE A AND P
GAUSSIAN ELIMINATION IS USED TO REDUCE THE A AND P MATRICES
FROM ROW L1 TO ROW L2 IF L1 IS NOT 1, IT IS ASSUMED THAT
A AND P ARE ALREADY REDUCED FROM 1 TO L1
IF (PRINT) THEN
  WRITE(108,500) 'IOPT=1, REDUCE STIFFNESS AND LOADS',L1,L2,N
  CALL LMATRIX(A,MD,N,IMD,'INPUT STIFFNESS')
  CALL WMATRIX(P,N,NLC,'INPUT LOAD')
  IF (GUYAN) CALL LMATRIX(MASS,MDMASS,N,INDMAS,'INPUT MASS')
  IF (KGCOND) CALL LMATRIX(KG,MDKG,N,INDKG,'INPUT KG')
ENDIF
110 CONTINUE
IOPT=2, REDUCE P ONLY
GAUSSIAN ELIMINATION IS USED TO REDUCE THE P MATRIX
FROM ROW L1 TO ROW L2 IF L1 IS NOT 1, IT IS ASSUMED THAT
A AND P ARE ALREADY REDUCED FROM 1 TO L1
IF (PRINT) THEN
  WRITE(108,500) 'IOPT=2, REDUCE LOADS ONLY',L1,L2,N
  CALL LMATRIX(A,MD,N,IMD,'REDUCED STIFFNESS')
  CALL WMATRIX(P,N,NLC,'NEW LOAD MATRIX')
ENDIF
100 CONTINUE
IF (PRINT) THEN
  CALL LMATRIX(A,MD,N,IMD,'REDUCED STIFFNESS')
  CALL WMATRIX(P,N,NLC,'REDUCED LOAD')
  IF (GUYAN) CALL LMATRIX(MASS,MDMASS,N,INDMAS,'REDUCED MASS')
  IF (KGCOND) CALL LMATRIX(KG,MDKG,N,INDKG,'REDUCED KG')
  RETURN
ENDIF
110 CONTINUE
IOPT=3, REDUCE P BY COLUMNS (J) EACH ROW (I)
DO 50 J=L1,N
  IF (P(L,J).EQ.0) GO TO 50
  I1=MAX(L1+1,J)
  I2=MIN(L2,J)
  DO 55 I=I1,I2
    IJ=IJM(I,J)
    MASS(IJ)=MASS(IJ)+FACTOR(I)*P(L,J)
  CONTINUE
  ENDIF
55 CONTINUE
50 CONTINUE
IOPT=4, REDUCE MASS MATRIX BY GUYAN REDUCTION
IF (GUYAN) THEN
  M22=M22 - K21/K11 * M11 * 1/K11 * K12
  RM=MASS(MD(I))
  IF (RM.NE.0) THEN
    DO 60 J=L1,N
      IF (FACTOR(J).EQ.0) GO TO 60
      I1=MAX(L1+1,J)
      I2=MIN(L2,J)
      DO 65 I=I1,I2
        IJ=IJM(I,J)
        MASS(IJ)=MASS(IJ)+FACTOR(I)*RM*FACTOR(J)
      CONTINUE
    ENDIF
  ENDIF
  M22=M22 - K21/K11 * M12
  DO 66 J=L1,N
    L1=IJM(L1,J)
    IF (LJ.GE.MDMASS(J+1)) GO TO 66
    IF (MASS(L1).EQ.0) GO TO 66
    DO 65 I=L1+1,J
      IJ=IJM(I,J)
      IF (IJ.LT.MDMASS(J+1))
        MASS(IJ)=MASS(IJ)+FACTOR(I)*MASS(L1)
    CONTINUE
  ENDIF
  M22=M22 - M21 * 1/K11 * K12
  DO 70 J=L1,N
    IF (FACTOR(J).EQ.0) GO TO 70
    DO 69 I=L1+1,J
      L1=IJM(L1,I)
      IF (L1.GE.MDMASS(I+1)) GO TO 69
      IJ=IJM(I,J)
      IF (IJ.LT.MDMASS(J+1))
        MASS(IJ)=MASS(IJ)+FACTOR(J)*MASS(I)
    CONTINUE
  ENDIF
  KG22=KG22 - K21/K11 * KG11 * 1/K11 * KG12
  RKG=KG(MDKG(L))
  IF (RKG.NE.0) THEN
    DO 80 J=L1,N
      IF (FACTOR(J).EQ.0) GO TO 80
      I1=MAX(L1+1,J)
      I2=MIN(L2,J)
      DO 75 I=I1,I2
        IJ=IJM(I,J)
        IJKG(IJ)=IJKG(IJ)+FACTOR(I)*RKG*FACTOR(J)
      CONTINUE
    ENDIF
  ENDIF
  KG22=KG22 - K21/K11 * KG12
  DO 90 J=L1,N
    IF (FACTOR(J).EQ.0) GO TO 90
    DO 89 I=L1+1,J
      L1=IJKG(L1,I)
      IF (L1.GE.MDKG(I+1)) GO TO 89
      IJ=IJKG(I,J)
      IF (IJ.LT.MDKG(J+1))
        KG(IJ)=KG(IJ)+FACTOR(I)*KKG*FACTOR(J)
    CONTINUE
  ENDIF
  IOPT=5, LOOP FOR EACH ROW (L) TO REDUCE
  DO 160 L=L1,L2
  IF (A(MD(L)).EQ.0) THEN
    WRITE(108,2000) L,MD(L),A(MD(L))
    STOP
  ENDIF

```

```

C      LOOP FOR EACH COLUMN (J)
DO 150 J=1,NLC
  IF (P(L,J).EQ.0) GO TO 150
  DO 120 I=(L+1),N
    LI=J(L,I)
    IF (LI.LT.MD(I+1)) P(I,J)=P(I,J)+A(LI)*P(L,J)/A(MD(L))
  CONTINUE
120 CONTINUE
150 CONTINUE
160 CONTINUE
RETURN

C
210 CONTINUE
C----- IOPT=3, BACK SUBSTITUTION
BACK SUBSTITUTION IS USED TO SOLVE FOR X IN SK-V, WHERE S IS
THE REDUCE UPPER TRIANGULAR FACTOR OF THE A MATRIX, AND V
IS THE REDUCED FORM OF THE F MATRIX. THE RESULTS (X) ARE
STORED IN P.
C----- BACK SUBSTITUTION ONLY TAKES PLACE BETWEEN ROWS L1 AND L2.
IF (PRINT) THEN
  WRITE(108,500) 'IOPT=3, BACK SUBSTITUTION',L1,L2,N
  CALL LMATRX(A,MD,N,MD(N+1),'REDUCED STIFFNESS')
  CALL WHATRX(P,N,NLC,'REDUCED LOAD')
ENDIF

C----- LOOP FOR EACH ROW L
CALCULATE X FOR EACH LOAD CASE, STORE IN P
DO 250 J=L2,L1,-1
  IF (A(MD(J)).EQ.0) THEN
    WRITE(108,2000) J,MD(J),A(MD(J))
  STOP
ENDIF
DO 215 LC=1,NLC
  P(J,LC)=P(J,LC)/A(MD(J))
  IF (J.GT.1) THEN
    IJ=MD(J-1)
    NI=J-(MD(J-1)-MD(J)) + 1
    DO 220 I=NI,J-1,*
      JI=I
      DO 220 LC=1,NLC
        P(I,LC)=P(I,LC)-P(J,LC)*A(IJ)
  CONTINUE
ENDIF
250 CONTINUE
IF (PRINT) CALL WHATRX(P,N,NLC,'DISPLACEMENT')

C
310 CONTINUE
C----- IOPT=4, REDUCED MULTIPLICATION
SOLVE FOR V IN S=D-P WHERE S HAS BEEN REDUCED BY GAUSSIAN
ELIMINATION.
THE RESULTING LOAD IS STORED IN P.
THE DISPLACEMENTS ARE INPUT IN D.
MULTIPLICATION ONLY TAKES PLACE BETWEEN ROWS L1 AND L2.
IF (PRINT) THEN
  WRITE(108,500) 'IOPT=4, REDUCED MULTIPLICATION',L1,L2,N
  CALL LMATRX(A,MD,N,MD(N+1),'REDUCED STIFFNESS')
  CALL WHATRX(FACTOR,N,1,'DISPLACEMENT')
ENDIF

C----- ONLY ONE LOAD CASE ...
LC=1
... ZERO P ...
DO 320 J=L1,L2
  IF (A(MD(J)).EQ.0) THEN
    WRITE(108,2000) J,MD(J),A(MD(J))
  STOP
ENDIF
320 CONTINUE
P(J,LC)=0

C----- MULTIPLY S_RED = DISP * P
DO 340 J=L1,N
  IC1=J-(MD(J)-1)-MD(J) + 1
  IC2=MAX(IC1,L1)
  DO 340 I=J,IC2,1
    IJ=I(J,I)
    340 P(I,LC)=P(I,LC)+A(IJ)*FACTOR(J)
  IF (PRINT) CALL WHATRX(P,N,1,'REDUCED LOAD')

C----- UNREDUCE THE LOAD MATRIX ...
DO 360 L=L2,L1,1

C----- LOOP FOR EACH COLUMN (J)
DO 350 J=1,NLC
  IF (P(L,J).EQ.0) GO TO 350
  DO 345 I=(L+1),N
    LI=J(L,I)
    IF (LI.LT.MD(I+1)) P(I,J)=P(I,J)+A(LI)*P(L,J)/A(MD(L))
  CONTINUE
345 CONTINUE
360 CONTINUE
360 CONTINUE
RETURN

IF (PRINT) CALL WHATRX(P,N,1,'LOAD')
500 FORMAT (/2X,A,2X,'L1=',I5,' L2=',I5,' N=',I5)
2000 FORMAT (//1X,50(//))
4 * PIVOT IS LE 0 ..... ROW ',I5,' T51,///
4 * STORAGE LCN ',I5,' T51,///
4 * PIVOT=,IP,G15.6,T51,///
4 * SOLUTION IS ABORTED ', T51,///
4 1X,50(//))

C
RETURN
END

C----- SUBROUTINE REACTN(IOPT,PRINT,KNODE,L1,L2,L3,L4,FORCE,DISPL,
4 MD,A,KM,NDOP,NLOAD,FACTOR,
4 MAXNOD,NDOP,COORD,ID,TITLE,STEPID,HEAD,
4 CHARACTER*(*) TITLE(2),STEPID
CHARACTER*15 CD(6)
LOGICAL PRINT,HEAD,FLAG,BTEST
DIMENSION A(20,20),MD(NDOP*),IDOP(MAXNOD,6),G(6)
DIMENSION FORCE(NDOP,NLOAD),DISPL(NDOP,NLOAD)
DIMENSION ID(MAXNOD),COORD(MAXNOD,3),JTFLG(MAXNOD)
DIMENSION F(6),SUM(6),COSINE(3,3),NCOSI,JTCOS(MAXNOD)

C
IF (IOPT.EQ.1) THEN
C-----
C----- MODIFY LOADS FOR RESTRAINT DISPLACEMENTS ---
C----- CONSIDER THE PARTITIONED STIFFNESS WHERE X2 ARE THE RESTRAINT DISPL.
IP1: K11 K12 K13
IP2: K21 K22 K23
C----- THE MODIFIED FREE LOAD IS (P1-K12*X2) - K11*X1
DO 10 I=L1,L2
  DO 10 J=L1,L2
    DO 10 K=L3,L4
      FORCE(I,J)=FORCE(I,J)-K(I,K)*DISPL(K,J)
    DO 20 M=L3,L4
      MDR=MD(K-1)
      DO 20 J=1,NLOAD
        DO 10 I=L2,L1,-1
          IK=MD(K-1)
          IF (IK.GE.MDK) GO TO 20
          IF (I.KE.MDK) GO TO 140
          ENDIF
          IF (I.KE.MDK) GO TO 140
          A(IK)=A(IK)
          130 FORCE(I,J)=FORCE(I,J)+A(IK)*DISPL(K,J)*FACTOR
          140 CONTINUE
        IF (PRINT) THEN
          & CALL WHATRX(FORCE,NDOP,NLOAD,'FINAL LOADS AND REACTIONS')
        ELSE IF (IOPT.EQ.3 .OR. IOPT.EQ.4) THEN
          NP=L2
C----- LOOP FOR EACH LOAD CASE ---
DO 300 L=1,NLOAD
C----- PRINT GLOBAL REACTIONS ---
IF (PRINT) THEN
  IF (HEAD) WRITE(108,320) TITLE(1),TITLE(2)
  NREACT=0
  IF (IOPT.NE.4) WRITE(108,321) L,STEPID
  IF (IOPT.EQ.4) WRITE(108,421)
ENDIF
IF (IOPT.EQ.3) THEN
  DO 190 I=1,6
    SUM(I)=0
  ENDIF
  IF (.NOT.(NCOSI.EQ.1 .AND. COSINE(1,1).EQ.1 .AND. COSINE(2,2).EQ.1
& EQ.1 .AND. COSINE(3,3).EQ.1) .AND. IOPT.EQ.4) GO TO 279
  DO 200 I=1,KNODE
C----- GET REACTNS IN JOINT COORDINATE SYSTEM
FLAG=.FALSE.
DO 209 J=1,6
  IF (I.LE.NP .OR. BTEST(JTFLG(I),J+1)) THEN
    F(J)=0
  ELSE
    F(J)=FORCE(I,J)
    FLAG=.TRUE.
  ENDIF
  209 CONTINUE
  IF (.NOT.FLAG) GO TO 200
C----- TRANSFER REACTNS TO GLOBAL COORDINATE SYSTEM
DO 229 I1=1,3
  I2=I1+3
  G(I1)=0
  G(I2)=0
  DO 219 J1=1,3
    J2=J1+3
    G(I1)+G(I2) * COSINE(J1,I1,JTCOS(I1))*F(J2)
    G(I2)+G(I1) * COSINE(J1,I1,JTCOS(I1))*F(J2)
  IF (PRINT) THEN
    WRITE(CD(13),210) G(I1)
    WRITE(CD(12),210) G(I2)
  ENDIF
CONTINUE
C----- SUM REACTIONS .....
IF (IOPT.EQ.3) THEN
  DO 240 I1=1,6
    SUM(I1)=SUM(I1) + G(I1)
  SUM(4)=SUM(4) + G(2)*COORD(I,3) - G(3)*COORD(I,2)
  SUM(5)=SUM(5) + G(1)*COORD(I,3) - G(3)*COORD(I,1)
  SUM(6)=SUM(6) - G(1)*COORD(I,2) + G(2)*COORD(I,1)
ENDIF
C----- PRINT GLOBAL REACTIONS .....
NREACT=NREACT+1
IF (PRINT) WRITE(108,220) ID(I),CD(XI,K+1,6)
CONTINUE
IF (IOPT.EQ.3 .AND. PRINT) WRITE(108,230) SUM
279 IF (IOPT.EQ.4 .AND. PRINT) WRITE(108,231) SUM
C----- PRINT LOCAL JOINT COORDINATE SYSTEMS REACTIONS ---
IF (.NOT.PRINT) GO TO 300
IF (NCOSI.EQ.1 .AND. COSINE(1,1).EQ.1 .AND. COSINE(2,2).EQ.1
& .AND. COSINE(3,3).EQ.1) EQ.1) GO TO 300
IF (HEAD .AND. NREACT.GT.20) WRITE(108,320) TITLE(1),TITLE(2)
IF (IOPT.NE.4) WRITE(108,322) L,STEPID
IF (IOPT.EQ.4) WRITE(108,422)
DO 280 I=1,KNODE
  FLAG=.FALSE.
  DO 280 J=1,6

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```

IJ-IDOP(I,J) REAC1860 NZ(I2ZKGD+3)+NZ(I2ZKGD-3) FORM1040
IF (I,J).LE.NP GR. BTEST(JTPLG(I),J+1)) THEN REAC1870 C IP (NZ(I2ZKGD-3).EQ.1) THEN FORM1050
CD(J)=' ' REAC1880 FALSE=FALSE FORM1060
ELSE REAC1890 CALL ELEM3 FORM1070
WRITE(CD(J),210) FORCE(I,J,L) REAC1900 & (2,LSTTYP,FALSE,IREL,FALSE,NAME,ESE,EPSE,DAMAGE,DUCTAG, FORM1080
FLAG=.TRUE. REAC1910 & IELNO,IELDOP,KGDOP,PGBOM,2 ,AXIAL,I2DISP,I2LOAD,MSTCR) FORM1090
ENDIF REAC1920 C KGDOP-KGDOP FORM1100
CONTINUE IF (.NOT.FLAG) GO TO 230 REAC1940 C REAC1950 FORM1110
PRINT REACTNS IN LOCAL COORDINATE SYSTEM REAC1960 C IP (BUG57) THEN FORM1140
WRITE(108,221) ID(I),CD(K),K-1,6,JTCOS(I) REAC1970 WRITE (ELNO,15) IELNO,(NZ(I-I2LXMG-1),I-1,MIN(KGDOP,18)) FORM1150
CONTINUE REAC1980 CALL LMATRX(2(I2ZKEG),MD,KGDOP,MD(KGDOP+1),ELNO) FORM1160
300 CONTINUE REAC1990 ENDFI FORM1170
REAC2000 C IP (.NOT.AXIAL) GO TO 130 FORM1180
REAC2010 DETERMINE THE FACTOR FOR KG FORM1190
REAC2020 IF (NZ(I2ZKGD+1).EQ.1) THEN FORM1210
REAC2030 ACCELZ=2(I2ZKGD) FORM1220
REAC2040 PGBOM=PGBOM*(1+ACCELZ) FORM1230
REAC2050 ENDFI FORM1240
REAC2060 C SWAP THE SIGN ON PGBOM FOR SUBTRACTION..... FORM1250
REAC2070 PGBOM=-PGBOM FORM1260
REAC2080 C CALL FORML(2(I2STIP),NZ(I2MD),NDOP,NZ(I2MD-NDOP), FORM1280
& 2(I2ZKEG),MD,KGDOP,MD(KGDOP+1),NZ(I2LXMG),PGBOM) FORM1290
REAC2090 ENDFI FORM1300
REAC2100 C 130 CONTINUE FORM1310
REAC2110 C RETURN FORM1320
REAC2120 C ..... ASSEMBLE GEOMETRIC STIFFNESS FORM1330
REAC2130 200 CONTINUE FORM1340
REAC2140 IP (NZ(I2ZKGD+3).NE.2) RETURN FORM1350
REAC2150 1ST=I2KG FORM1360
REAC2160 I2ND=I2KG + NZ(I2MDKG-NDOP) * 1 FORM1370
REAC2170 C ..... ZERO STIFFNESS FORM1380
REAC2180 DO 210 I=1ST,I2ND FORM1390
REAC2190 2(I)=0 FORM1400
REAC2200 C ..... LOOP FOR EACH ELEMENT FORM1410
REAC2210 DO 220 IELNO=1,NELMT FORM1420
REAC2220 C ..... GET STIFFNESS OF EACH ELEMENT FORM1430
REAC2230 FALSE=FALSE FORM1440
REAC2240 CALL ELEM3 FORM1450
REAC2250 & (2,LSTTYP,FALSE,IREL,FALSE,NAME,ESE,EPSE,DAMAGE,DUCTAG, FORM1460
& IELNO,IELDOP,KGDOP,PGBOM,2 ,AXIAL,I2DISP,I2LOAD,MSTCR) FORM1470
REAC2260 C IP (.NOT.AXIAL) GO TO 230 FORM1480
REAC2270 IF (BUG57) THEN FORM1490
REAC2280 WRITE (6,*) PGBOM+PGBOM FORM1500
REAC2290 WRITE (ELNO,15) IELNO,(NZ(I-I2LXMG-1),I-1,MIN(KGDOP,18)) FORM1510
REAC2300 CALL LMATRX(2(I2ZKEG),MD,KGDOP,MD(KGDOP+1),ELNO) FORM1520
REAC2310 ENDFI FORM1530
REAC2320 C CALL FORML(2(I2KG),NZ(I2MDKG),NDOP,NZ(I2MDKG-NDOP), FORM1540
& 2(I2ZKEG),MD,KGDOP,MD(KGDOP+1),NZ(I2LXMG),PGBOM) FORM1550
REAC2330 210 CONTINUE FORM1560
REAC2340 C RETURN FORM1570
REAC2350 ENDFI FORM1580
-----CFEM30010
CFEM30020 C PROGRAM FEM - ANALYSIS OF STRUCTURES
CFEM30030
CFEM30040 C NOTE: USE IBM FORTRAN2 COMPILER OPTION AUTODBL(DBLPAD)
CFEM30050 C TO COMPILE THIS PROGRAM IN DOUBLE PRECISION
CFEM30060
CFEM30070 C NOTE: USE IBM FORTRAN2 COMPILER OPTION OPT(2)
CFEM30080 C TO OPTIMIZE THE CODE FOR FASTER EXECUTION
CFEM30090
CFEM30100
CFEM30110
CFEM30120
CFEM30130
CFEM30140 CHARACTER*80 OPTION,INPUT,BLANK
CFEM30150 CHARACTER*8 TIME,DATEI
CFEM30160 CHARACTER*1 CHR(0:25)
CFEM30170 LOGICAL TEST,READ,AXIAL
CFEM30180
CFEM30190
CFEM30200
CFEM30210
CFEM30220
CFEM30230 C--- ECHO THE INPUT.....
CFEM30240
CFEM30250 I=55
CFEM30260 READ(105,('(A)',END=9996) INPUT
CFEM30270 IF (INDEX(INPUT,'NOECHO') NE.0) GO TO 9996
CFEM30280 I=1-1
CFEM30290 L=1-1
CFEM30300 IF (I.GT.55) THEN
CFEM30310 WRITE(108,9994) (KK,XX=1,8)
CFEM30320 I=0
CFEM30330 ENDFI
CFEM30340 WRITE(108,9997) L,INPUT
CFEM30350 IF (INDEX(INPUT,'STOP') .EQ.0) GO TO 9995
CFEM30360 9994 FORMAT ('1 ECHO OF INPUT DATA //
CFEM30370 & 1X,13,'LINE',8(' ',11,'0'))
CFEM30380 9997 FORMAT (1X,17,' ',A)
CFEM30390 9996 REWIND(105)
CFEM30400
CFEM30410
CFEM30420
CFEM30430
CFEM30440
CFEM30450 TITLE(1)=' '
CFEM30460 TITLE(2)=' '
CFEM30470 BLANK=' '
CFEM30480 BUG=.FALSE.
CFEM30490 DOUBLE=.FALSE.
CFEM30500 IBUG=0
CFEM30510 MAXEDF=24
CFEM30520 MD(1)=1
CFEM30530 DO 1 I=2,(MAXEDF+1)
CFEM30540 1 MD(I)=MD(I-1)-(I-1)
CFEM30550 I2=0
CFEM30560 20 READ(105,*,END=1000) OPTION
CFEM30570 RATI2=100.*REAL(I2)/REAL(MAXZ)
CFEM30580
CFEM30590
CFEM30600 C--- DEFINE THE STRUCTURE ---
CFEM30610
CFEM30620
CFEM30630 IF (INDEX(OPTION,'STR') NE.0) THEN
CFEM30640 I2=1
CFEM30650 TITLE(1)=' '
CFEM30660 TITLE(2)=' '
CFEM30670
CFEM30680 GRAV=.3864
CFEM30690 READ(105,*,END=1000) TITLE(1)
CFEM30700 CALL TIDAE(TIMEI,DATEI)
CFEM30710 WRITE(TITLE(1)(80),45) TIMEI,DATEI
CFEM30720 RATI2=100.*REAL(I2)/REAL(MAXZ)
CFEM30730 WRITE(*,52) 'BUILDING STRUCTURAL MODEL .....
CFEM30740 I2,RATI2
CFEM30750
CFEM30760 WRITE(*,51) TITLE(1)(1:70)
CFEM30770 CALL STRUCT
CFEM30780 IREL=I2
CFEM30790 RATI2=100.*REAL(I2)/REAL(MAXZ)
CFEM30800
CFEM30810
-----
SUBTRACT GEOMETRIC STIFFNESS

```

```

C-- DEFINE THE LOADING AND SOLVE --
C
ELSE IF (INDEX(OPTION, 'SOL', NE.0) THEN
  READ(105, *, END=1000) TITLE(2)
  CALL TIDAE(TIMEI, DATEI)
  WRITE(TITLE(2)(80), 45) TIMEI, DATEI
  WRITE(*, 52) ' SOLVING STRUCTURAL MODEL ..... '
  &
  WRITE(*, 51) TITLE(2)(1:70)
  WRITE(*, 51) TITLE(2)(1:70)
  CALL SOLN(OPTION)
  RAT12=100.*REAL(12)/REAL(MAX2)
C
C-- STOP .. END OF PROGRAM ..
C
ELSE IF (INDEX(OPTION, 'STOP', NE.0) THEN
  WRITE(*, 52) ' NORMAL STOP ..... '
  &
  STOP
C
C-----
C-- READ RESULTS FROM INPUT FILE ---
C
ELSE IF (INDEX(OPTION, 'READ', NE.0) THEN
  140 CALL GETINT(OPTION, 'UNIT', IUNIT, 0, TRUE, TRUE)
  CALL GETINT(OPTION, 'INC', INC, 1, TRUE, FALSE)
  IF (IUNIT.GT.0) THEN
    WRITE(*, 52) ' READING DATA FROM FILE ..... '
    &
    IOPT=4
    CALL DMPDAT(IOPT, IWRITE, TO, DT)
    GO TO 140
  ENDIF
  RAT12=100.*REAL(12)/REAL(MAX2)
C
C-----
C-- SET BUG OPTION ---
C
ELSE IF (TEST(OPTION, 'BUG', FALSE)) THEN
  CALL GETCHR(OPTION, 'BUG', INPUT, TRUE, FALSE)
  IBUG=0
  WRITE(OPTION, 160) INPUT(1:70)
  DO 150 I=0, 23
    IF (TEST(INPUT, CHR(I), FALSE)) IBUG=IBSET(IBUG, I)
  CONTINUE
  150 FORMAT ('BUG', A)
  160 WRITE(*, 52) ' SET BUG OPTION ..... '
  &
  WRITE(*, 51) OPTION(1:70)
  WRITE(108, 51) ' SET BUG OPTION: '
  WRITE(108, 51) OPTION(1:70)
C
C-----
C-- SET BUG OPTION ---
C
ELSE IF (INDEX(OPTION, 'BUG', NE.0) THEN
  WRITE(*, 51) ('SET BUG OPTION: //OPTION(1:54) ')
  BUG=TRUE
  IF (INDEX(OPTION, 'NBUG', NE.0) BUG=FALSE
C
C-----
C-- RELEASE MEMORY FROM LAST SOLUTION ---
C
ELSE IF (INDEX(OPTION, 'RELEASE', NE.0) THEN
  WRITE(108, 52) '
  WRITE(*, 52) ' RELEASING MEMORY ..... '
  IF (INDEX(OPTION, 'ELEMENT', NE.0) THEN
    WRITE(108, 52) ' RESETTING ELEMENT FORCES ..... '
    DO 170 IELNO=1, NELMT
      LSTYPT=0
      HEAD = FALSE
      170 CALL ELEM(I,
        (13) LSTYPT, FALSE, IREL, HEAD, INPUT, EISE, EPSB, DAMAGE, DUCFAG,
        &
        IELNO, IZELDP, XGDOF, PGEOM, 2, AXIAL, IZDDSP, IZDLOA, MSTCR)
      ELSE
        WRITE(108, 52) '
        ENDIF
        IZ=IZREL
        IZACC=0
        IZDACC=0
        IZDISP=0
        IZDISP=0
        IZDLOA=0
        IZDVEL=0
        IZEFUB=0
        IZLOAD=0
        IZVEL=0
        RAT12=100.*REAL(12)/REAL(MAX2)
C
C-----
C-- SAVE MEMORY FOR A RESTART ---
C
ELSE IF (INDEX(OPTION, 'SAVE', NE.0) THEN
  WRITE(*, 52) ' SAVING DATA ..... '
  &
  WRITE(*, 51) TITLE(2)(1:70)
  WRITE(*, 51) TITLE(2)(1:70)
  WRITE(108, 200)
  200 FORMAT(5X, '***** WRITING DATA TO UNIT PT09 *****')
  OPEN(UNIT=9, ACCESS='SEQUENTIAL', STATUS='UNKNOWN',
    &
    FORM='UNFORMATTED')
  WRITE(9) TITLE
  WRITE(9) IZ, IZREL
  &
  NDOF, NCOND, NFREE, NREST, MAXNOD, NMAT, NELMT, NNODE,
  &
  NLOAD, NSOLN, MAXEFP, MD, NCOB, MAXELD, NELD,
  &
  IZID, IZIDOF, IZCORD, IZCOS, IZCNST, IZMAT, IZELE,
  &
  IZJFLG, IZJCOS,
  &
  IZKE, IZLM, IZSTIP, IZMD, IZLOAD, IZDISP, IZIELD, IZELD
  DO 210 J=1, IZ, 255
    K=J-255
    K=MIN(K, IZ)
    WRITE(9) (2(I), I=J, K)
  210 CLOSE (UNIT=9)
  RAT12=100.*REAL(12)/REAL(MAX2)
C
C-----
C-- RESTART PROGRAM ---
C
WRITE(*, 52) ' RETRIEVING DATA ..... '
  &
  ELSE IF (INDEX(OPTION, 'RESTART', NE.0) THEN
    WRITE(108, 300)
    300 FORMAT(5X, '***** READING DATA FROM UNIT PT09 *****')
    OPEN(UNIT=9, ACCESS='SEQUENTIAL', STATUS='OLD',
      &
      FORM='UNFORMATTED')
    READ (9) TITLE
    READ (9) IZ, IZREL
    &
    NDOF, NCOND, NFREE, NREST, MAXNOD, NMAT, NELMT, NNODE,
    &
    NLOAD, NSOLN, MAXEFP, MD, NCOB, MAXELD, NELD,
    &
    IZID, IZIDOF, IZCORD, IZCOS, IZCNST, IZMAT, IZELE,
    &
    IZJFLG, IZJCOS,
    &
    IZKE, IZLM, IZSTIP, IZMD, IZLOAD, IZDISP, IZIELD, IZELD
    DO 310 J=1, IZ, 255
      K=J-255
      K=MIN(K, IZ)
      310 CLOSE (UNIT=9)
      WRITE(*, 52) ' DATA RETRIEVED ..... '
      &
      WRITE(*, 51) TITLE(2)(1:70)
      WRITE(*, 51) TITLE(2)(1:70)
      RAT12=100.*REAL(12)/REAL(MAX2)
C
C-----

```



```

DO 10 I=L2,L1,-1
IK=MD(K)*K-I
IF (IK.GE.MDK) GO TO 20
PJ=FORCE(I,J)
AIX=A(I,K)
DKO=DISPL(K,J)
10 FORCE(I,J)=FORCE(I,J)+A(I,K)*DISPL(K,J)
20 CONTINUE
IF (PRINT)
CALL WHATRX(FORCE,NDOP,NLOAD,'MODIFIED LOADS')
ELSE IF (IOPT.EQ.2) THEN
C-----
C- SOLVE FOR REACTIONS ---
C CONSIDER THE PARTITIONED STIFFNESS WHERE X2 ARE THE RESTRAINT DISPL.
C-----
IP1: K11 K12 K13 K14
IP2: K21 K22 K23 K24 * FACTOR
C-----
C THE REACTIONS ARE P2 = P2 + (K21*K1 + K22*X2) * FACTOR
NOTE: BOTH P1 AND P2 CONTAIN FIXED END FORCES FROM MEMBER LOADS.
THUS, K21*K1 + K22*X2 IS ADDED TO P2 TO GET THE REACTIONS
C-----
DO 120 I=L3,L4
DO 120 J=1,NLOAD
FORCE(I,J)=0 * P2 CONTAINS PEP
DO 110 K=L1,L2
FORCE(I,J)=FORCE(I,J)+K(I,K)*DISPL(K,J)
DO 120 K=L3,L4
FORCE(I,J)=FORCE(I,J)+K(I,K)*DISPL(K,J)
C-----
DO 120 I=L3,L4
DO 120 J=1,NLOAD
DO 110 K=L2,L1,-1
KI=MD(I)-I-K
IF (KI.GE.MD(I-1)) GO TO 120
AIX=A(KI)
110 FORCE(I,J)=FORCE(I,J)+A(KI)*DISPL(K,J)*FACTOR
120 CONTINUE
C-----
DO 140 I=L1,L4
DO 140 K=L1,L4
IF (I.LE.K) THEN
IK=MD(K)-K-I
MDX=MD(K-I)
ELSE
IK=MD(I)-I-K
MDX=MD(I-1)
ENDIF
IF (IK.GE.MDX) GO TO 140
AIX=A(IK)
130 FORCE(I,J)=FORCE(I,J)+A(IK)*DISPL(K,J)*FACTOR
140 CONTINUE
IF (PRINT)
CALL WHATRX(FORCE,NDOP,NLOAD,'FINAL LOADS AND REACTIONS')
ELSE IF (IOPT.EQ.3 OR IOPT.EQ.4) THEN
IF (PRINT)
WRITE(108,320) TITLE(1),TITLE(2)
NREACT=0
IF (IOPT.EQ.4) WRITE(108,321) L,STEPID
IF (IOPT.EQ.4) WRITE(108,421)
ENDIF
IF (IOPT.EQ.3) THEN
DO 190 I=1,6
SUM(I)=0
ENDIF
IF (.NOT.(NCOS.EQ.1 AND COSINE(1,1).EQ.1 AND COSINE(2,2).EQ.1 AND COSINE(3,3).EQ.1 AND IOPT.EQ.4)) GO TO 279
DO 200 I=1,NNODE
C-----
C- GET REACTNS IN JOINT COORDINATE SYSTEM
FLAG=FALSE
DO 290 J=1,6
IJ=IDOP(I,J)
IF (IJ.LE.NF OR BTEST(JTPLG(I),J-1)) THEN
P(J)=0
ELSE
P(J)=FORCE(IJ,L)
FLAG=TRUE
ENDIF
CONTINUE
IF (.NOT.FLAG) GO TO 200
C-----
C- TRANSFER REACTNS TO GLOBAL COORDINATE SYSTEM
DO 229 I=1,3
I2=I-1
G(I1)=0
G(I2)=0
DO 219 J1=1,3
J2=J-1
G(I1)+G(I2)*COSINE(J1,1)*JTCOS(I)*P(J1)
G(I2)+G(I1)*COSINE(J2,1)*JTCOS(I)*P(J2)
IF (PRINT) THEN
WRITE(CD(I1),210) G(I1)
WRITE(CD(I2),210) G(I2)
ENDIF
CONTINUE
C-----
C- SUM REACTIONS
IF (IOPT.EQ.3) THEN
DO 240 I=1,560
SUM(I)=SUM(I)+G(I)
SUM(4)+SUM(4)+G(2)*COORD(1,3)+G(3)*COORD(1,2)
SUM(5)+SUM(5)+G(1)*COORD(1,3)+G(2)*COORD(1,1)
SUM(6)+SUM(6)+G(1)*COORD(1,2)+G(2)*COORD(1,1)
ENDIF
C-----
C- PRINT GLOBAL REACTIONS
NREACT=NREACT+1
IF (PRINT) WRITE(108,220) ID(I),CD(K),K-1,6)
CONTINUE
IF (IOPT.EQ.3 AND PRINT) WRITE(108,230) SUM
IF (IOPT.EQ.4 AND PRINT) WRITE(108,231) SUM
279 I=1,NNODE
C-----
C- PRINT LOCAL JOINT COORDINATE SYSTEMS REACTIONS ---
IF (.NOT.PRINT) GO TO 300
IF (NCOS.EQ.1 AND COSINE(1,1).EQ.1 AND COSINE(2,2).EQ.1 AND COSINE(3,3).EQ.1) GO TO 300
IF (HEAD AND NREACT.GT.20) WRITE(108,320) TITLE(1),TITLE(2)
IF (IOPT.EQ.4) WRITE(108,421)
DO 290 I=1,NNODE
REACT0790 GET REACTNS IN JOINT COORDINATE SYSTEM
REACT0790 FLAG=FALSE
REACT0790 DO 290 J=1,6
REACT0790 IJ=IDOP(I,J)
REACT0790 IF (IJ.LE.NF OR BTEST(JTPLG(I),J-1)) THEN
REACT0790 P(J)=0
REACT0790 ELSE
REACT0790 P(J)=FORCE(IJ,L)
REACT0790 FLAG=TRUE
REACT0790 ENDIF
REACT0790 CONTINUE
REACT0790 IF (.NOT.FLAG) GO TO 290
REACT0790 C-----
REACT0790 PRINT REACTNS IN LOCAL COORDINATE SYSTEM
REACT0790 WRITE(108,221) ID(I),CD(K),K-1,6),JTCOS(I)
REACT0790 290 CONTINUE
REACT0790 300 CONTINUE
REACT0790 210 FORMAT (1P,G15.7)
REACT0790 220 FORMAT (5X,15.6A)
REACT0790 221 FORMAT (5X,15.6A,I10)
REACT0790 230 FORMAT (1P,90(' '),//SUMMATION',6G15.7)
REACT0790 231 FORMAT (1P,MAX OF ALL',89(' '),//GCS SUMM',6G15.7)
REACT0790 320 FORMAT (1P,STRUCTURE',A,/,SOLUTION',A)
REACT0790 321 FORMAT (1P,GCS RESTRAINT REACTIONS,LOADING #',15,5X,A,/,
REACT0790 //6X,MODE',7X,'FX',
REACT0790 //13X,'FY',13X,'PZ',13X,'MX',13X,'MY',13X,'MZ',/)
REACT0790 322 FORMAT (1P,MAXIMUM GCS RESTRAINT REACTIONS,LOADING #',15,5X,A,/,
REACT0790 //6X,MODE',7X,'FX',
REACT0790 //13X,'FY',13X,'PZ',13X,'MX',13X,'MY',13X,'MZ',/,
REACT0790 //6X,COSINE #'/)
REACT0790 421 FORMAT (1P,MAXIMUM GCS RESTRAINT REACTIONS',
REACT0790 //5X,NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY',/
REACT0790 //6X,MODE',7X,'FX',
REACT0790 //13X,'FY',13X,'PZ',13X,'MX',13X,'MY',13X,'MZ',/)
REACT0790 422 FORMAT (1P,MAXIMUM GCS RESTRAINT REACTIONS',
REACT0790 //5X,NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY',/
REACT0790 //6X,MODE',7X,'FX',
REACT0790 //13X,'FY',13X,'PZ',13X,'MX',13X,'MY',13X,'MZ',/,
REACT0790 //6X,COSINE #'/)
REACT0790 ELSE
REACT0790 WRITE(108,*) 'INVALID OPTION IN SUBROUTINE REACTN, IOPT=',IOPT
REACT0790 ENDIF
REACT0790 RETURN
REACT0790 END
C-----
C- SUBROUTINE FORM(IOPT)
C- CHARACTER NAME=80,ELNO=130
C- LOGICAL PRINT,HEAD,FALSE,AXIAL,BTEST,BUGS,BUGS7
C- INCLUDE: ECOM
C-----
C- MATRIX STORAGE SCHEME
C- THE STIFFNESS MATRIX (K) IS Banded AND SYMMETRIC
C- THUS ONLY THE SKYLINE ABOVE THE MAIN DIAGONAL NEEDS TO BE STORED.
C- THE MATRIX K IS STORED IN A LINEAR ARRAY BY COLUMNS
C- THE VALUE OF THE MAIN DIAGONAL IS STORED IN THE LINEAR ARRAY FIRST.
C- RELATIVE ADDRESSES OF THE LINEAR ARRAY ELEMENTS CONTAINING THE
C- MAIN DIAGONAL TERMS ARE STORED IN MD.
C- THE ABSOLUTE ADDRESS OF THE FIRST ELEMENT IN THE K MATRIX IS I2STIP
C- THE STIFFNESS MATRIX K IS STORED IN THE 2 ARRAY, 2(I2STIP+K(1,1))
C- THE ABSOLUTE ADDRESS OF THE FIRST ELEMENT IN THE MD MATRIX IS I2MD.
C- THE STIFFNESS MATRIX X IS STORED IN THE N2 ARRAY, N2(I2MD)+MD(1)
C- THUS, IF I LE J (UPPER TRIANGULAR MATRIX)
C- I2=I2MD-1+I-1
C- I2=N2(I2MD-1+J)-J-1: RELATIVE ADDRESS OF ELEMENT (I,J)
C- ELSE
C- B(I,J)=0 (I,J) IS ABOVE THE SKYLINE
C-----
C- EXAMPLE: LET B BE THE 5X5 FULL SYMMETRIC MATRIX BELOW.
C- 1 3 12 12 12
C- 2 5 10 10 10 NOTE: B(1,3),B(1,4) AND B(2,4) ARE ZERO.
C- 4 7 10 10 10
C- SYMM 6 9 9 9 9 MD= 1, 2, 4, 6, 8, 13
C-----
C- SAY I2MD=121,I2STIP=200
C- N2(121)=1 2(200)=3
C- N2(122)=2 2(201)=2
C- N2(123)=4 2(202)=3
C- N2(124)=6 2(203)=4
C- N2(125)=8 2(204)=5
C- N2(126)=13 2(205)=6
C- 2(206)=7
C- 2(206)=8
C- 2(206)=9
C- 2(206)=10
C- 2(206)=11
C- 2(206)=12
C- THUS, FOR B(3,4) I=3, J=4
C- IJ=N2(I2MD-1+J)-J-1 = N2(121-1+4)-4-3 = 6-4-3 = -7
C- THUS, FOR B(2,4) I=2, J=4
C- IJ=N2(I2MD-1+J)-J-1 = N2(121-1+4)-4-2 = 6-4-2 = 0
C- SINCE IJ GE N2(I2MD-1+J)-1
C- B(I,J)=0 (I,J) IS ABOVE THE SKYLINE
C-----
C- BUGS = BTEST(BUGS,5)
C- BTEST = BTEST(BUGS,9) AND BUGS
C- LSTTYP=0
C- PRINT=FALSE
C- HEAD=FALSE
C-----
C- IOPT=1 FORM STIFFNESS MATRIX
C- IOPT=2 FORM GEOMETRIC STIFFNESS MATRIX
C-----
C- IF (BUGS) WRITE (6,*) ' IN ROUTINE FORM IOPT=',IOPT
C- 49 FORMAT (1X,A/(' I=',13, ' MDMASS(1)',11))
C- GO TO (100,200) IOPT
C-----
C- ASSEMBLE STIFFNESS
C- 100 CONTINUE
C- IEND=I2STIP
C- IEND=I2STIP+N2(I2MD+NDOP)-1
C-----
C- ZERO STIFFNESS
C- DO 110 I=1,IEND
C- 110 2(I)=0
C-----
C- LOOP FOR EACH ELEMENT
C- DO 120 IELNO=1,NELMT
C-----
C- GET STIFFNESS OF EACH ELEMENT
C- FLAG=FALSE
C- CALL ELBLE
C- (3,LSTTYP,FALSE,I,REL,FAK,NAME,ESE,EPSE,DAMAGE,DUCPAG,
C- & IELNO,IELDOP,XGDOF,PGEOM,2,AXIAL,I2DISP,I2LOAD,MYTOR)
C- IF (BUGS7) THEN
C- WRITE (ELNO,15) IELNO,(N2(I-122M-1),1-1,MIN(IELDOP,18))
C- 15 FORMAT ('ELEMENT #',16,' LM ',18)
C- CHE LMA7KX(I22K),MD,IELDOP,MD(IELDOP-1),ELNO)
C- ENDF
C- CALL FORML(2,I2STIP,N2(I2MD),NDOP,N2(I2MD+NDOP),

```



```

      6      Z(I2KX),MD,IELDOF,MD(IELDOF+1),NZ(I2LM),1.)
C----- SUBTRACT GEOMETRIC STIFFNESS
      NZ(I2KDOT+3)-NZ(I2KDOT+3)
C-----
      IF (NZ(I2KDOT+3).EQ.1) THEN
      FALSE=FALSE
      CALL ELEM1B
      4      (2,LSSTYP,FALSE,IREL,FALSE,NAME,ESE,EPSE,DAMAGE,DUCTFAG,
      4      IELNO,IELDOF,KGDOP,PGEOM,2,AXIAL,I2DISP,I2LOAD,MSTOR)
C-----
      XGDOP=XGDOP
C-----
      IF (BUGS?) THEN
      WRITE (I2NO,15) IELNO,(NZ(I-I2LMKG-1),I-1,MIN(XGDOP,18))
      CALL LMATRX(I2I2KX),MD,KGDOP,MD(KGDOP+1),IELNO)
      ENDF
C-----
      IF (.NOT. AXIAL) GO TO 120
      DETERMINE THE FACTOR FOR KG
      IF (NZ(I2KDOT+1).EQ.1) THEN
      ACCEL2=2(I2KDOT)
      PGEOM-PGEOM*(1+ACCEL2)
      ENDF
C-----
      SWAP THE SIGN ON PGEOM FOR SUBTRACTION. ...
      PGEOM=-PGEOM
C-----
      CALL FORM12(I2STIF),NZ(I2MD),NDOP,NZ(I2MD+NDOP),
      4      2(I2KX),MD,KGDOP,MD(KGDOP+1),NZ(I2LMKG),PGEOM)
      ENDF
C-----
120  CONTINUE
C-----
      RETURN
C-----
      ASSEMBLE GEOMETRIC STIFFNESS
200  CONTINUE
      IF (NZ(I2KDOT+3).NE.2) RETURN
      I2K=I2K
      IEND=I2K+NZ(I2MDKG+NDOP)-1
C-----
      ZERO STIFFNESS
      DO 210 I=1,IEND
      210  Z(I)=0
C-----
      LOOP FOR EACH ELEMENT
      DO 220 IELNO=1,NELMT
C-----
      GET STIFFNESS OF EACH ELEMENT
      FALSE=FALSE
      CALL ELEM1B
      4      (2,LSSTYP,FALSE,IREL,FALSE,NAME,ESE,EPSE,DAMAGE,DUCTFAG,
      4      IELNO,IELDOF,KGDOP,PGEOM,2,AXIAL,I2DISP,I2LOAD,MSTOR)
C-----
      IF (.NOT. AXIAL) GO TO 220
      IF (BUGS?) THEN
      WRITE (I2NO,15) 'PGEOM=',PGEOM
      WRITE (I2NO,15) IELNO,(NZ(I-I2LMKG-1),I-1,MIN(KGDOP,18))
      CALL LMATRX(I2I2KX),MD,KGDOP,MD(KGDOP+1),IELNO)
      ENDF
C-----
      CALL FORM12(I2KX),NZ(I2MDKG),NDOP,NZ(I2MDKG+NDOP),
      4      2(I2KX),MD,KGDOP,MD(KGDOP+1),NZ(I2LMKG),PGEOM)
      ENDF
220  CONTINUE
C-----
      RETURN
      END
C-----
PROGRAM FEM - ANALYSIS OF STRUCTURES
C-----
NOTE: USE IBM FORTRAN2 COMPILER OPTION AUTODBL(DBLPAD)
      TO COMPILE THIS PROGRAM IN DOUBLE PRECISION
C-----
NOTE: USE IBM FORTRAN2 COMPILER OPTION OPT(2)
      TO OPTIMIZE THE CODE FOR FASTER EXECUTION
C-----
CHARACTER*80 OPTION,INPUT,BLANK
CHARACTER*1 TIME,DATE1
CHARACTER*1 CHR(10:25)
LOGICAL TEST,HEAD,AXIAL
INCLUDE 'COMMON'
DATA CHR/'A','B','C','D','E','F','G','H','I','J','K','L','M',
      & 'N','O','P','Q','R','S','T','U','V','W','X','Y','Z'/
C-----
C-- ECHO THE INPUT -----
I=55
L=0
9995 READ (105,('(A) ',END=9996) INPUT
      IF (INDEX(INPUT,'NOCHO').NE.0) GO TO 9996
      I=I+1
      L=L+1
      IF (L.GT.55) THEN
      WRITE (108,9994) (XX,XX-1,8)
      I=0
      ENDF
      WRITE (108,9997) L,INPUT
      IF (INDEX(INPUT,'STOP').EQ.0) GO TO 9995
9994 FORMAT ('1 ECHO OF INPUT DATA //
      1X,3X,LINE ',E'(10.10) ',11,0')
9997 FORMAT (1X,17,10,1A)
9996 REWIND(105)
C-----
C-- INITIALIZE DATA -----
TITLE(1)=
TITLE(2)=
BLANK
BUG=FALSE
DOUBLE=FALSE
I2K=0
MAXEDP=24
MD(1)=1
DO 1 I=2,(MAXEDP+1)
      1 MD(I)-MD(I-1)*(I-1)
I2=0
C-----
20 READ (105,'END=1000) OPTION
      RATI2=100.*REAL(I2)/REAL(MAX2)
C-----
C-- DEFINE THE STRUCTURE -----
C-----
      IF (INDEX(OPTION,'STR').NE.0) THEN
      I2=
      TITLE(1)=
      TITLE(2)=
      GRAV=184.4
      READ (105,'END=1000) TITLE(1)
      CALL TIDAE(TIME,DATE1)
      WRITE (TITLE(1)(80),45) TIME,DATE1
      RATI2=100.*REAL(I2)/REAL(MAX2)
      WRITE (*,52) ' SOLVING STRUCTURAL MODEL .....'
      4      I2,RATI2
      WRITE (*,51) TITLE(1)(1:70)
      CALL STRUCT
      I2REL=I2

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      RATI2=100.*REAL(I2)/REAL(MAX2)
C-----
C-- READ RESUME FROM INPUT FILE -----
C-- DEFINE THE LOADING AND SOLVE --
      ELSE IF (INDEX(OPTION,'SOL').NE.0) THEN
      READ (105,'END=1000) TITLE(2)
      CALL TIDAE(TIME,DATE1)
      WRITE (*,52) ' SOLVING STRUCTURAL MODEL .....'
      4      I2,RATI2
      WRITE (*,51) TITLE(1)(1:70)
      WRITE (*,51) TITLE(2)(1:70)
      CALL SOLN(OPTION)
      RATI2=100.*REAL(I2)/REAL(MAX2)
C-----
C-- STOP ----- END OF PROGRAM -----
C-----
      ELSE IF (INDEX(OPTION,'STOP').NE.0) THEN
      WRITE (*,52) ' NORMAL STOP .....'
      4      STOP
C-----
C----- READ RESUME FROM INPUT FILE -----
      ELSE IF (INDEX(OPTION,'READ').NE.0) THEN
140  CALL GETINT(OPTION,'UNIT',IUNIT,0,TRUE,TRUE)
      CALL GETINT(OPTION,'INC',INC,1,TRUE,FALSE)
      IF (IUNIT.GT.0) THEN
      WRITE (*,52) ' READING DATA FROM FILE .....'
      4      I2,RATI2
      IOPT=4
      CALL DOPDAT(IOPT,IWRITE,TO,DT)
      GO TO 140
      ENDF
      RATI2=100.*REAL(I2)/REAL(MAX2)
C-----
C-- SET BUG OPTION -----
      ELSE IF (TEST(OPTION,'BUG',FALSE)) THEN
      CALL GETCHR(OPTION,'BUG',INPUT,TRUE,FALSE)
      IBUG=0
      WRITE (OPTION,160) INPUT(1:70)
      DO 150 I=0,23
      IF (TEST(INPUT,CHR(I),FALSE)) IBUG=IBSET(IBUG,I)
150  CONTINUE
160  FORMAT ('BUG=',A)
      WRITE (*,52) ' SET BUG OPTION .....'
      4      I2,RATI2
      WRITE (*,51) OPTION(1:70)
      WRITE (108,51) ' SET BUG OPTION .....'
      WRITE (108,51) OPTION(1:70)
C-----
C----- RELEASE MEMORY FROM LAST SOLUTION -----
      ELSE IF (INDEX(OPTION,'BUG') NE.0) THEN
      WRITE (*,51) ' SET BUG OPTION //OPTION(1:54) '
      BUG=TRUE
      IF (INDEX(OPTION,'NOBUG').NE.0) BUG=FALSE.
C-----
C----- RELEASE MEMORY FROM LAST SOLUTION -----
      ELSE IF (INDEX(OPTION,'RELEASE').NE.0) THEN
      WRITE (108,52) ' RELEASING MEMORY .....'
      WRITE (108,52) ' RELEASING MEMORY .....'
      IF (INDEX(OPTION,'ELEMENT').NE.0) THEN
      WRITE (108,52) ' RESETTING ELEMENT FORCES .....'
      DO 170 IELNO=1,NELMT
      LSSTYP=0
      HEAD=FALSE
      CALL ELEM1B
      4      (1,LSSTYP,FALSE,IREL,HEAD,INPUT,ESE,EPSE,DAMAGE,DUCTFAG,
      4      IELNO,IELDOF,XGDOP,PGEOM,2,AXIAL,I2DISP,I2LOAD,MSTOR)
      ELSE
      WRITE (108,52) ' RELEASING MEMORY .....'
      ENDF
      I2=I2REL
      I2ACC=0
      I2DACC=0
      I2DDSP=0
      I2DISP=0
      I2ELDO=0
      I2VEL=0
      I2FUB=0
      I2LOAD=0
      I2VEL=0
      RATI2=100.*REAL(I2)/REAL(MAX2)
C-----
C-- SAVE MEMORY FOR A RESTART -----
      ELSE IF (INDEX(OPTION,'SAVE').NE.0) THEN
      WRITE (*,52) ' SAVING DATA .....'
      4      I2,RATI2
      WRITE (*,51) TITLE(1)(1:70)
      WRITE (*,51) TITLE(2)(1:70)
      WRITE (108,200)
200  FORMAT (5X,***** WRITING DATA TO UNIT PT09 ***** )
      OPEN (UNIT=9,ACCESS='SEQUENTIAL',STATUS='UNKNOWN',
      4      FORM='UNFORMATTED')
      WRITE (9) TITLE
      WRITE (9) I2
      4      NDOP,NCORD,NPREX,NREST,MAXNOID,MMAT,NELMT,NNODE,
      4      NLOAD,NSOLN,MAXEDP,MD,NCOS,MAXELD,NELD,
      4      I2ID,I2IDOP,I2CCORD,I2COS,I2CNST,I2MAT,I2ELE,
      4      I2FPLG,I2JCOS,
      4      I2KE,I2LM,I2STIF,I2MD,I2LOAD,I2DISP,I2IELD,I2ELD
      DO 210 J=1,I2,256
      K=J-255
      210  K=MIN(K,I2)
      CLOSE (UNIT=9)
      RATI2=100.*REAL(I2)/REAL(MAX2)
C-----
C-- RESTART PROGRAM -----
      WRITE (*,52) ' RETRIEVING DATA .....'
      4      I2,RATI2
      ELSE IF (INDEX(OPTION,'RESTART').NE.0) THEN
      WRITE (108,300)
300  FORMAT (5X,***** READING DATA FROM UNIT PT09 ***** )
      OPEN (UNIT=9,ACCESS='SEQUENTIAL',STATUS='OLD',
      4      FORM='UNFORMATTED')
      READ (9) TITLE
      READ (9) I2,I2REL
      4      NDOP,NCORD,NPREX,NREST,MAXNOID,MMAT,NELMT,NNODE,
      4      NLOAD,NSOLN,MAXEDP,MD,NCOS,MAXELD,NELD,
      4      I2ID,I2IDOP,I2CCORD,I2COS,I2CNST,I2MAT,I2ELE,
      4      I2FPLG,I2JCOS,
      4      I2KE,I2LM,I2STIF,I2MD,I2LOAD,I2DISP,I2IELD,I2ELD
      DO 310 J=1,I2,256
      K=J-255
      310  K=MIN(K,I2)
      READ (9) (Z(I),I=J,K)
      CLOSE (UNIT=9)
      WRITE (*,52) ' DATA RETRIEVED .....'
      4      I2,RATI2
      WRITE (*,51) TITLE(1)(1:70)

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PEM40790
PEM40800
PEM40810
PEM40820
PEM40830
PEM40840
PEM40850
PEM40860
PEM40870
PEM40880
PEM40890
PEM40900
PEM40910
PEM40920
PEM40930
PEM40940
PEM40950
PEM40960
PEM40970
PEM40980
PEM40990
PEM41000
PEM41010
PEM41020
PEM41030
PEM41040
PEM41050
PEM41060
PEM41070
PEM41080
PEM41090
PEM41100
PEM41110
PEM41120
PEM41130
PEM41140
PEM41150
PEM41160
PEM41170
PEM41180
PEM41190
PEM41200
PEM41210
PEM41220
PEM41230
PEM41240
PEM41250
PEM41260
PEM41270
PEM41280
PEM41290
PEM41300
PEM41310
PEM41320
PEM41330
PEM41340
PEM41350
PEM41360
PEM41370
PEM41380
PEM41390
PEM41400
PEM41410
PEM41420
PEM41430
PEM41440
PEM41450
PEM41460
PEM41470
PEM41480
PEM41490
PEM41500
PEM41510
PEM41520
PEM41530
PEM41540
PEM41550
PEM41560
PEM41570
PEM41580
PEM41590
PEM41600
PEM41610
PEM41620
PEM41630
PEM41640
PEM41650
PEM41660
PEM41670
PEM41680
PEM41690
PEM41700
PEM41710
PEM41720
PEM41730
PEM41740
PEM41750
PEM41760
PEM41770
PEM41780
PEM41790
PEM41800
PEM41810
PEM41820
PEM41830
PEM41840
PEM41850
PEM41860
PEM41870
PEM41880
PEM41890
PEM41900
PEM41910
PEM41920
PEM41930
PEM41940
PEM41950
PEM41960
PEM41970
PEM41980
PEM41990
PEM42000
PEM42010
PEM42020
PEM42030
PEM42040
PEM42050
PEM42060
PEM42070
PEM42080
PEM42090
PEM42100
PEM42110
PEM42120
PEM42130
PEM42140
PEM42150
PEM42160
PEM42170
PEM42180
PEM42190
PEM42200
PEM42210
PEM42220
PEM42230
PEM42240

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WRITE(,51) TITLE(2)(1,70)
RAT1Z=100 *REAL(I2)/REAL(MAXZ)
C-----
C-- INVALID OPTION --
C-----
ELSE
IF (TEST(OPTION,'NOCHEO',.FALSE.)) GO TO 20
IF (OPTION.EQ.BLANK) GO TO 20
WRITE(108,30) OPTION
WRITE(,51) ('INVALID OPTION: '//OPTION(1:54))
ENDIF
GO TO 20
C
45 FORMAT(' TIME ',A,' DATE ',A)
50 FORMAT(' A1 ',STRUCTURE,' A2 ',A/1X,' SOLUTION...',A,/)
51 FORMAT(1X,A)
52 FORMAT(1X,'...A45...',1Z',17',MCM',P6.3,'M')
53 FORMAT(' INVALID OPTION ',A)
116 FORMAT(' ZMATRX(1,16,1,1),115,1P,G20 10,,G20 10)
1000 STOP
END
C-----
C----- TAKE PRESENT TIME AND DATE -----
C
SUBROUTINE TIDAE(TIME1,DATE1)
INTEGER*2 IYR,IMON,IDAY,IHR,IMIN,ISEC,1100TH
CHARACTER*4 TIME1,DATE1
CALL GETDAT(IYR,IMON,IDAY)
IYR=IYR-1900
CALL GETTIM(IHR,IMIN,ISEC,1100TH)
WRITE(DATE1,'(12,1A,12,1A,12)') IMON,IDAY,IYR
WRITE(TIME1,'(12,1A,12,1A,12)') IHR,IMIN,ISEC
RETURN
END
C-----
C
SUBROUTINE SOLN(OPTION)
CHARACTER*80 OPTION
$INCLUDE 'ZCOMGN'
$FEID=
C-----
C- DETERMINE SOLUTION NO. --
C
CALL GETINT(OPTION,'SOL',NSOLN,0,.TRUE.,.FALSE.)
C-----
C- CHOOSE THE SOLUTION --
C
IF (NSOLN.EQ.1) THEN
CALL SOL1
ELSE IF (NSOLN.EQ.2) THEN
NLOAD=0
MAXELD=0
CALL SOL2
ELSE IF (NSOLN.EQ.3) THEN
NLOAD=0
MAXELD=0
CALL SOL3
ELSE IF (NSOLN.EQ.4) THEN
IF (NSOLN.EQ.4) THEN
NLOAD=
CALL SOL4
ELSE IF (NSOLN.EQ.5) THEN
NLOAD=0
MAXELD=0
CALL SOL5
ELSE
WRITE(108,*) 'INVALID SOLN',NSOLN
STOP 'INVALID SOLUTION NUMBER'
ENDIF
RETURN
END
C-----
MATRIX SOLUTION OF EQUATION AX=P, WHERE THE UPPER TRIANGULAR MATRIX
OF THE SYMMETRIC MATRIX A IS STORED BY SKYLINE, P IS STORED AS A
FULL MATRIX THE MATRIX P IS REPLACED BY X AFTER BACK SUBSTITUTION
IS COMPLETE. BOTH A AND P ARE ALTERED
GUYAN OPTION: IF GUYAN IS TRUE, MASS MATRIX IS ALSO CONDENSED
SUBROUTINE MSOLL(IOPT,PRINT,N,IMD,LROW,NLC,L1,L2,MD,A,P,FACTOR,
& GUYAN,MASS,MDMASS,IMDMAS,KGCCOND,KG,MDKG,IMDKG)
REAL MASS(IMDMAS),KG(IMDKG)
LOGICAL GUYAN,PRINT,KGCCOND
DIMENSION A(IMD),FACTOR(N),P(LROW,NLC),MD(N*1)
DIMENSION MDMASS(N*1),IMDKG(N*1)
C
IJ(I,J)=MD(J)+J-1
IJM(I,J)=MDMASS(J)+J-1
IJK(I,J)=IMDKG(J)+J-1
C
MATRIX STORED AS SKYLINE
C
THE MATRIX A IS Banded and SYMMETRIC thus ONLY the SKYLINE ABOVE
AND the MAIN DIAGONAL NEEDS to BE STORED.
C
THE MATRIX A IS STORED IN A LINEAR ARRAY BY COLUMNS THE VALUE ON
THE MAIN DIAGONAL IS STORED IN THE LINEAR ARRAY FIRST
ADDRESSES OF THE LINEAR ARRAY ELEMENTS CONTAINING THE MAIN DIAGONAL
TERMS ARE STORED IN MD.
EXAMPLE: LET B BE THE 5X5 FULL SYMMETRIC MATRIX BELOW
1 2 3 4 5
2 5 6 7 11
3 6 7 11 11
4 7 11 11 11
5 11 11 11 11
NOTE: B(1,3),B(1,4) AND B(2,4) ARE ZERO.
B=
SYMM. 6 9 14 (4) 1, 2, 4, 6, 8, 13
C
THUS, B(1,3)+A(MD(3))=A(4)
B(1,4)+A(MD(4)+J-1) IF J=1 AND (MD(J)+J-1)<MD(J-1)
C
MATRIX P IS NOT SYMMETRIC, AND IS STORED AS A FULL MATRIX
C
VARIABLE TABLE
IOPT - OPTION NUMBER
N - NUMBER OF DEGREES OF FREEDOM OF A MATRIX
NLC - NUMBER OF LOAD CASES FOR THE P MATRIX
L1 - FIRST ROW TO BE WORKED WITH
L2 - LAST ROW TO BE WORKED WITH
MD - ADDRESS OF THE MAIN DIAGONAL TERMS OF A
P - LOAD MATRIX ON INPUT, SOLN (X) OF AX=P ON COMPLETION OF
FACTOR - TEMPORARY STORAGE MATRIX
GUYAN - GUYAN REDUCTION FLAG FOR MASS MATRIX
MASS - MASS MATRIX
MDMASS - MASS MATRIX ADDRESSES OF MAIN DIAGONAL ELEMENTS
KGCCOND - FLAG FOR CONDENSING OUT KG
KG - GEOMETRIC STIFFNESS MATRIX
MDKG - KG MATRIX ADDRESSES OF MAIN DIAGONAL ELEMENTS
L - ROW NUMBER FOR ELIMINATION
I - ROW NUMBER
J - COLUMN NUMBER
C-----
LI=MAX(L1,1)
L2=MIN(L2,N)
GO TO (10,110,210,310) IOPT
10 CONTINUE
IOPT=1, REDUCE A AND P
GAUSSIAN ELIMINATION IS USED TO REDUCE THE A AND P MATRICES
FROM ROW L1 TO ROW L2 IF L1 IS NOT 1, IT IS ASSUMED THAT
A AND P ARE ALLREADY REDUCED FROM 1 TO L1
IF (PRINT) THEN
WRITE(108,500) 'IOPT=1, REDUCE STIFFNESS AND LOADS',L1,L2,N
ENDIF
C-----
110 CONTINUE
IOPT=2, REDUCE P ONLY
GAUSSIAN ELIMINATION IS USED TO REDUCE THE P MATRIX
FROM ROW L1 TO ROW L2 IF L1 IS NOT 1, IT IS ASSUMED THAT
A AND P ARE ALLREADY REDUCED FROM 1 TO L1
IF (PRINT) THEN
WRITE(108,500) 'IOPT=2, REDUCE LOADS ONLY',L1,L2,N
CALL LMATRIX(A,MD,N,IMD,'REDUCED STIFFNESS')
CALL WMATRIX(P,N,NLC,'REDUCED STIFFNESS')
CALL LMATRIX(KG,MDKG,N,IMDKG,'REDUCED KG')
ENDIF
C-----
LOOP FOR EACH ROW (L) TO REDUCE
DO 160 L=L1,L2

```

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C
IF (A(MD(L),EQ.0) THEN
  WRITE(108,2000) L,MD(L),A(MD(L))
  STOP
  ENDP
C
LOOP FOR EACH COLUMN (J)
DO 150 J=1,NLC
IF (P(L,J),EQ.0) GO TO 150
DO 120 I=(L-1),N
  LI=J(L,I)
  IF (LI.LT.MD(I-1)) P(I,J)-P(I,J)-A(LI)*P(L,J)/A(MD(L))
120 CONTINUE
150 CONTINUE
160 RETURN
C
210 CONTINUE
IOPT=3, BACK SUBSTITUTION
BACK SUBSTITUTION IS USED TO SOLVE FOR X IN S=V, WHERE S IS
THE REDUCED UPPER TRIANGULAR FACTOR OF THE A MATRIX, AND V
IS THE REDUCED FORM OF THE P MATRIX. THE RESULTS (X) ARE
STORED IN P.
BACK SUBSTITUTION ONLY TAKES PLACE BETWEEN ROWS LI AND L2
C
IF (PRINT) THEN
  WRITE(108,500) 'IOPT=3, BACK SUBSTITUTION',LI,L2,N
  CALL LMATRIX(A,MD,N,MD(N+1),'REDUCED STIFFNESS')
  CALL WHATRX(P,N,NLC,'REDUCED LOAD')
  ENDP
C
LOOP FOR EACH ROW L
CALCULATE FOR EACH LOAD CASE, STORE IN P
DO 250 J=L2,L1,-1
IF (A(MD(J),EQ.0) THEN
  WRITE(108,2000) J,MD(J),A(MD(J))
  STOP
  ENDP
DO 215 LC=1,NLC
  P(L,LC)=P(J,LC)/A(MD(J))
  IF (J.GT.1) THEN
    IJ=MD(J-1)
    NI=J-(MD(J-1)+MD(J))+1
    DO 220 I=NI,J-1,-1
      LI=I
      DO 220 LC=1,NLC
        P(I,LC)=P(I,LC)-P(J,LC)*A(IJ)
220 CONTINUE
  ENDP
250 CONTINUE
IF (PRINT) CALL WHATRX(P,N,NLC,'DISPLACEMENT')
RETURN
C
310 CONTINUE
IOPT=4, REDUCED MULTIPLICATION
SOLVE FOR V IN S*V=V WHERE S HAS BEEN REDUCED BY GAUSSIAN
ELIMINATION.
THE RESULTING LOAD IS STORED IN P.
THE DISPLACEMENTS ARE INPUT IN D
MULTIPLICATION ONLY TAKES PLACE BETWEEN ROWS LI AND L2
C
IF (PRINT) THEN
  WRITE(108,500) 'IOPT=4, REDUCED MULTIPLICATION',LI,L2,N
  CALL LMATRIX(A,MD,N,MD(N+1),'REDUCED STIFFNESS')
  CALL WHATRX(FACTOR,N,NLC,'DISPLACEMENT')
  ENDP
C
ONLY ONE LOAD CASE...
LC=1
ZERO P
DO 320 J=L1,L2
  IF (A(MD(J),EQ.0) THEN
    WRITE(108,2000) J,MD(J),A(MD(J))
    STOP
  ENDP
320 P(J,LC)=0
C
MULTIPLY S*RED + DISP = P
DO 340 J=L1,L2
  IC1=J-(MD(J-1)+MD(J))+1
  IC2=MAX(IC1,L1)
  DO 340 I=J,IC2,-1
    LI=I
    P(I,LC)=P(I,LC)+A(IJ)*FACTOR(J)
  IF (PRINT) CALL WHATRX(P,N,NLC,'REDUCED LOAD')
C
UNREDUCE THE LOAD MATRIX...
DO 360 L=L2,L1,-1
C
LOOP FOR EACH COLUMN (J)
DO 150 J=1,NLC
IF (P(L,LC),EQ.0) GO TO 350
DO 345 I=(L-1),N
  LI=J(L,I)
  IF (LI.LT.MD(I-1)) P(I,LC)-P(I,LC)-A(LI)*P(L,LC)/A(MD(L))
345 CONTINUE
350 CONTINUE
360 CONTINUE
400 CONTINUE
C
IF (PRINT) CALL WHATRX(P,N,NLC,'LOAD')
500 FORMAT (/2X,A,2X,'L1',15,'L2',15,'N',15)
2000 FORMAT (//1X,50('**'))
ER' FOR IN SUBROUTINE MSSOLL, T51,*/
* PIVOT IS LE 0 ***** ROW 15, T51,*/
* STORAGE LCN 15, T51,*/
* PIVOT 1P,G15.6,T51,*/
* SOLUTION IS ABORTED 15, T51,*/
1X,50('**')/
C
RETURN
END
C
SUBROUTINE REACTN(IOPT,PRINT,NODE,L1,L2,L3,L4,FORCE,DISPL,
  MD,A,KM,NDOF,NLOAD,FACTOR,
  HAXNND,IDOF,COORD,ID,TITLE,STEPID,HEAD,
  NCOSS,COSINE,JTCOS,JTFLG,SUM)
CHARACTER(*) TITLE(2),STEPID
CHARACTER*15 CD(6)
LOGICAL PRINT,HEAD,FLAG,BTEST
DIMENSION A(KM,MD,NDOF+1),IDOF(MAXNOD,6),G(6)
DIMENSION FORCE(NDOF,NLOAD),DISPL(NDOF,NLOAD)
DIMENSION ID(MAXNOD),COORD(MAXNOD,3),JTFLG(MAXNOD)
DIMENSION F(6),SUM(6),COSINE(3,3),NCOSS,JTCOS(MAXNOD)
C
IF (IOPT.EQ.1) THEN
C
*****
C
MODIFY LOADS FOR RESTRAINT DISPLACEMENTS ***
C
CONSIDER THE PARTITIONED STIFFNESS WHERE X2 ARE THE RESTRAINT DISPL.
C
IP1: K11 K12 : X1
I-1: : : X2
IP2: K21 K22 : X2
C
THE MODIFIED FREE LOAD IS (P1-K12*X2) - K11*X1
C
DO 10 I=L1,L2 C FULL STORAGE
DO 10 J=L1,NLOAD C EQUIVALENT
DO 10 K=L3,L4 C
C 10 FORCE(I,J)=FORCE(I,J)-K(I,K)*DISPL(K,J) C
C
DO 20 K=L3,L4
  MDX=MD(K-1)
  DO 20 J=1,NLOAD
    DO 10 I=L2,L1,-1
      IK=MD(K)-K-1
      IF ((IK.GE.MD(K)) GO TO 20
      FKJ=FORCE(I,J)
      AIK=A(IK)
      DKJ=DISPL(K,J)
      FORCE(I,J)-FORCE(I,J)-A(IK)*AIK*DKJ
20 CONTINUE
  IF (PRINT)
    & CALL WHATRX(FORCE,NDOF,NLOAD,'MODIFIED LOADS')
  ELSE IF (IOPT.EQ.2) THEN
C
*****
C
SOLVE FOR REACTIONS ***
C
CONSIDER THE PARTITIONED STIFFNESS WHERE X2 ARE THE RESTRAINT DISPL.
C
IP1: K11 K12 : X1
I-1: : : X2
IP2: K21 K22 : X2
C
THE REACTIONS ARE P2 = P2 + (K21*X1 + K22*X2) * FACTOR
NOTE: BOTH P1 AND P2 CONTAIN FIXED END FORCES FROM MEMBER LOADS.
THUS K21*X1 + K22*X2 IS ADDED TO P2 TO GET THE REACTIONS.
C
DO 120 I=L3,L4 C FULL STORAGE
DO 120 J=1,NLOAD C MODE EQUIVALENT
DO 110 K=L3,L4 C
C 110 FORCE(I,J)=FORCE(I,J)+P2 CONTAINS PEP** C
C 110 FORCE(I,J)=FORCE(I,J)+K(I,K)*DISPL(K,J) C
C 120 FORCE(I,J)=FORCE(I,J)+K(I,K)*DISPL(K,J) C
C
DO 120 I=L3,L4
  DO 120 K=L3,L4
    IK=MD(K)-K-1
    MDK=MD(K-1)
    ELSE
      IK=MD(I)-I-K
      MDK=MD(I-1)
    ENDP
    IF ((IK.GE.MD(K)) GO TO 140
    AIK=A(IK)
    FORCE(I,J)-FORCE(I,J)+A(IK)*AIK*DISPL(K,J)*FACTOR
120 CONTINUE
C
DO 140 I=L3,L4
  DO 140 K=L3,L4
    IF ((I.LE.K) THEN
      IK=MD(K)-K-1
      MDK=MD(K-1)
    ELSE
      IK=MD(I)-I-K
      MDK=MD(I-1)
    ENDP
    IF ((IK.GE.MD(K)) GO TO 140
    AIK=A(IK)
    DO 130 J=1,NLOAD
      FORCE(I,J)=FORCE(I,J)+A(IK)*AIK*DISPL(K,J)*FACTOR
140 CONTINUE
C
IF (PRINT)
  & CALL WHATRX(FORCE,NDOF,NLOAD,'FINAL LOADS AND REACTIONS')
  NP=L2
C
LOOP FOR EACH LOAD CASE ***
DO 300 L=L1,NLOAD
C
*****
C
PRINT GLOBAL REACTIONS ***
C
IF (PRINT) THEN
  IF (HEAD) WRITE(108,320) TITLE(1),TITLE(2)
  NREACT=0
  IF (IOPT.NE.4) WRITE(108,321) L,STEPID
  IF (IOPT.EQ.4) WRITE(108,421)
  ENDP
  IF (IOPT.EQ.3) THEN
    DO 190 J=1,6
      SUM(I)=0
190 ENDP
C
IF (NOT (NCOSS.EQ.1 .AND. COSINE(1,1),EQ.1 .AND. COSINE(2,2),EQ.1
  & .AND. COSINE(3,3),EQ.1) .AND. IOPT.EQ.4) GO TO 279
C
DO 200 I=1,NNODE
C
*****
C
GET REACTNS IN JOINT COORDINATE SYSTEM
FLAG=.FALSE.
DO 209 J=1,6
  IF (I.LE.NF OR BTEST(JTFLG(I),J+11)) THEN
    F(J)=0
  ELSE
    F(J)=FORCE(IJ,L)
    FLAG=.TRUE.
  ENDP
  IF (NOT FLAG) GO TO 300
C
*****
C
TRANSFER REACTNS TO GLOBAL COORDINATE SYSTEM
DO 229 I=1,3
  G(I)=0
  G(I2)=0
  DO 215 J=1,3
    G(I1)-G(I1) + COSINE(J1,I1,JTCOS(I1))*F(J1)
    G(I2)-G(I2) + COSINE(J1,I1,JTCOS(I1))*F(J2)
  IF (PRINT) THEN
    WRITE(CD(I1),210) G(I1)
    WRITE(CD(I2),210) G(I2)
  ENDP
  IF (NOT FLAG) GO TO 300
C
*****
C
SUM REACTIONS
IF (IOPT.EQ.3) THEN
  DO 240 I=1,6
    SUM(I)=SUM(I)+G(I)
240 SUM(I)=SUM(I)+G(I)
SUM(4)=SUM(4)+G(2)*COORD(I,3)+G(3)*COORD(I,2)
SUM(5)=SUM(5)+G(1)*COORD(I,3)+G(3)*COORD(I,1)
SUM(6)=SUM(6)+G(1)*COORD(I,2)+G(2)*COORD(I,1)
  ENDP
C
*****
C
PRINT GLOBAL REACTIONS
NREACT=NREACT+1
IF (PRINT) WRITE(108,220) ID(I),CD(K),K-1,6)
200 CONTINUE
IF (IOPT.EQ.3 AND PRINT) WRITE(108,230) SUM
279 IF (IOPT.EQ.4 AND PRINT) WRITE(108,231) SUM
C
*****
C
PRINT LOCAL JOINT COORDINATE SYSTEMS REACTIONS ***
C
IF (NOT PRINT) GO TO 300
IF (NCOSS.EQ.1 .AND. COSINE(1,1),EQ.1 .AND. COSINE(2,2),EQ.1
  & .AND. COSINE(3,3),EQ.1) GO TO 300
IF (HEAD AND NREACT.GT.20) WRITE(108,326) TITLE(1),TITLE(2)
IF (IOPT.NE.4) WRITE(108,322) L,STEPID

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WRITE(*,51) TITLE(1)(1:70)
CALL STRUCT
I2REL=12
RAT12=100.*REAL(12)/REAL(MAX2)
C-----
C-- DEFINE THE LOADING AND SOLVE
C-----
ELSE IF (INDEX(OPTION,'SOL') NE.0) THEN
  READ(105,*,END=1000) TITLE(2)
  CALL TIMEA(TIME1,DATE1)
  WRITE(TITLE(2)(1:6),45) TIME1,DATE1
  WRITE(*,52) ' SOLVING STRUCTURAL MODEL'
  I2,RAT12
  WRITE(*,51) TITLE(1)(1:70)
  WRITE(*,51) TITLE(2)(1:70)
  CALL SOLN(OPTION)
  RAT12=100.*REAL(12)/REAL(MAX2)
C-----
C-- STOP... END OF PROGRAM
C-----
ELSE IF (INDEX(OPTION,'STOP') NE.0) THEN
  WRITE(*,52) ' NORMAL STOP'
  I2,RAT12
  STOP
C-----
C-- READ RESULTS FROM INPUT FILE
C-----
ELSE IF (INDEX(OPTION,'READ') NE.0) THEN
  140 CALL GETINT(OPTION,'UNIT',IUNIT,C.TRUE.,TRUE.)
  CALL GETINT(OPTION,'INC',INC,I.TRUE.,FALSE.)
  IF (IUNIT.GT.0) THEN
    WRITE(*,52) ' READING DATA FROM FILE'
    I2,RAT12
    IOPT=4
    CALL DMPDAT(IOPT,IMWRITE,TO,DT)
    GO TO 140
  ENDIF
  RAT12=100.*REAL(12)/REAL(MAX2)
C-----
C-- SET BUG OPTION
C-----
ELSE IF (TEST(OPTION,'BUG',FALSE)) THEN
  CALL GETCHR(OPTION,'BUG',INPUT,TRUE.,FALSE)
  IBUG=0
  WRITE(OPTION,160) INPUT(1:70)
  DO 150 I=0,23
    IF (TEST(INPUT,CHR(I),.FALSE.)) IBUG=IBSET(IBUG,I)
  CONTINUE
  150 PFORMAT('BUG-',A)
  WRITE(*,52) ' SET BUG OPTION'
  I2,RAT12
  WRITE(*,51) OPTION(1:70)
  WRITE(108,51) ' SET BUG OPTION'
  WRITE(108,51) OPTION(1:70)
C-----
C-- SET BUG OPTION
C-----
ELSE IF (INDEX(OPTION,'BUG') NE.0) THEN
  WRITE(*,51) ('SET BUG OPTION. '//OPTION(1:54))
  BUG=.TRUE
  IF (INDEX(OPTION,'NBUG') NE.0) BUG=.FALSE.
C-----
C-- RELEASE MEMORY FROM LAST SOLUTION
C-----
ELSE IF (INDEX(OPTION,'RELEASE') NE.0) THEN
  WRITE(108,52) ' RELEASING MEMORY'
  WRITE(108,52) ' RELEASING MEMORY'
  IF (INDEX(OPTION,'ELEMENT') NE.0) THEN
    WRITE(108,52) ' RESETTING ELEMENT FORCES'
    DO 170 IELNO=1,NELMT
      LETTV=0
      HEAD=.FALSE
      CALL ELELIB
      170 (1) LETTV=.FALSE.,IREL,HEAD,INPUT,EISE,EPSE,DAMAGE,DUCFAG,
      IELNO,TELODP,KGDDP,POBCH,2,ATIAL,I2DDSP,I2DLQA,MSTR)
    ELSE
      WRITE(108,52) ' RELEASING MEMORY'
      ENDIF
      I2=I2REL
      I2ACC=0
      I2DACC=0
      I2DDSP=0
      I2DDEL=0
      I2DLQA=0
      I2DVEL=0
      I2DFUB=0
      I2LQAD=0
      I2VEL=0
      RAT12=100.*REAL(12)/REAL(MAX2)
C-----
C-- SAVE MEMORY FOR A RESTART
C-----
ELSE IF (INDEX(OPTION,'SAVE') NE.0) THEN
  WRITE(*,52) ' SAVING DATA'
  I2,RAT12
  WRITE(*,51) TITLE(1)(1:70)
  WRITE(*,51) TITLE(2)(1:70)
  WRITE(108,200)
  200 FFORMAT(5X,'***** WRITING DATA TO UNIT PT09 *****')
  OPEN(UNIT=9,ACCESS='SEQUENTIAL',STATUS='UNKNOWN',
    FORM='UNFORMATTED')
  WRITE(9) TITLE
  WRITE(9) I2,I2REL
  4 NDOF,NCOND,NPREP,NREST,MAXNOF,NMAT,NELMT,NNODE,
  4 NLOAD,NNSCN,MAXEFP,MD,NCOS,MAXELD,NELD,
  4 I2ID,I2IDOF,I2CCOR,I2COS,I2CNST,I2MAT,I2ELE,
  4 I2JPLG,I2JCOS,
  4 I2KE,I2LM,I2STIP,I2MD,I2LOAD,I2DISP,I2IELD,I2ELD
  DO 210 J=1,I2,256
    X=J-255
    K=MIN(K,I2)
  210 CLOSE(UNIT=9)
  RAT12=100.*REAL(12)/REAL(MAX2)
C-----
C-- RESTART PROGRAM
C-----
WRITE(*,52) ' RETRIEVING DATA'
  I2,RAT12
  ELSE IF (INDEX(OPTION,'RESTART') NE.0) THEN
    WRITE(108,300)
    300 FFORMAT(5X,'***** READING DATA FROM UNIT PT09 *****')
    OPEN(UNIT=9,ACCESS='SEQUENTIAL',STATUS='OLD',
      FORM='UNFORMATTED')
    READ(9) TITLE
    READ(9) I2,I2REL
    4 NDOF,NCOND,NPREP,NREST,MAXNOF,NMAT,NELMT,NNODE,
    4 NLOAD,NNSCN,MAXEFP,MD,NCOS,MAXELD,NELD,
    4 I2ID,I2IDOF,I2CCOR,I2COS,I2CNST,I2MAT,I2ELE,
    4 I2JPLG,I2JCOS,
    4 I2KE,I2LM,I2STIP,I2MD,I2LOAD,I2DISP,I2IELD,I2ELD
    DO 310 J=1,I2,256
      X=J-255
      K=MIN(K,I2)
    310 CLOSE(UNIT=9)

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MSOL0690 MSOL0700 MSOL0710 MSOL0720 MSOL0730 MSOL0740 MSOL0750 MSOL0760 MSOL0770 MSOL0780 MSOL0790 MSOL0800 MSOL0810 MSOL0820 MSOL0830 MSOL0840 MSOL0850 MSOL0860 MSOL0870 MSOL0880 MSOL0890 MSOL0900 MSOL0910 MSOL0920 MSOL0930 MSOL0940 MSOL0950 MSOL0960 MSOL0970 MSOL0980 MSOL0990 MSOL1000 MSOL1010 MSOL1020 MSOL1030 MSOL1040 MSOL1050 MSOL1060 MSOL1070 MSOL1080 MSOL1090 MSOL1100 MSOL1110 MSOL1120 MSOL1130 MSOL1140 MSOL1150 MSOL1160 MSOL1170 MSOL1180 MSOL1190 MSOL1200 MSOL1210 MSOL1220 MSOL1230 MSOL1240 MSOL1250 MSOL1260 MSOL1270 MSOL1280 MSOL1290 MSOL1300 MSOL1310 MSOL1320 MSOL1330 MSOL1340 MSOL1350 MSOL1360 MSOL1370 MSOL1380 MSOL1390 MSOL1400 MSOL1410 MSOL1420 MSOL1430 MSOL1440 MSOL1450 MSOL1460 MSOL1470 MSOL1480 MSOL1490 MSOL1500 MSOL1510 MSOL1520 MSOL1530 MSOL1540 MSOL1550 MSOL1560 MSOL1570 MSOL1580 MSOL1590 MSOL1600 MSOL1610 MSOL1620 MSOL1630 MSOL1640 MSOL1650 MSOL1660 MSOL1670 MSOL1680 MSOL1690 MSOL1700 MSOL1710 MSOL1720 MSOL1730 MSOL1740 MSOL1750 MSOL1760 MSOL1770 MSOL1780 MSOL1790 MSOL1800 MSOL1810 MSOL1820 MSOL1830 MSOL1840 MSOL1850 MSOL1860 MSOL1870 MSOL1880 MSOL1890 MSOL1900 MSOL1910 MSOL1920 MSOL1930 MSOL1940 MSOL1950 MSOL1960 MSOL1970 MSOL1980 MSOL1990 MSOL2000 MSOL2010 MSOL2020 MSOL2030 MSOL2040 MSOL2050 MSOL2060 MSOL2070 MSOL2080 MSOL2090 MSOL2100 MSOL2110 MSOL2120 MSOL2130 MSOL2140 MSOL2150 MSOL2160 MSOL2170 MSOL2180 MSOL2190 MSOL2200 MSOL2210 MSOL2220 MSOL2230 MSOL2240 MSOL2250 MSOL2260 MSOL2270 MSOL2280 MSOL2290 MSOL2300 MSOL2310 MSOL2320 MSOL2330 MSOL2340 MSOL2350 MSOL2360 MSOL2370 MSOL2380 MSOL2390 MSOL2400 MSOL2410 MSOL2420 MSOL2430 MSOL2440 MSOL2450 MSOL2460 MSOL2470 MSOL2480 MSOL2490 MSOL2500 MSOL2510 MSOL2520 MSOL2530 MSOL2540 MSOL2550 MSOL2560 MSOL2570 MSOL2580 MSOL2590 MSOL2600 MSOL2610 MSOL2620 MSOL2630 MSOL2640 MSOL2650 MSOL2660 MSOL2670 MSOL2680 MSOL2690 MSOL2700 MSOL2710 MSOL2720 MSOL2730 MSOL2740 MSOL2750 MSOL2760 MSOL2770 MSOL2780 MSOL2790 MSOL2800 MSOL2810 MSOL2820 MSOL2830 MSOL2840 MSOL2850 MSOL2860 MSOL2870 MSOL2880 MSOL2890 MSOL2900 MSOL2910 MSOL2920 MSOL2930 MSOL2940 MSOL2950 MSOL2960 MSOL2970 MSOL2980 MSOL2990 MSOL3000 MSOL3010 MSOL3020 MSOL3030 MSOL3040 MSOL3050 MSOL3060 MSOL3070 MSOL3080 MSOL3090 MSOL3100 MSOL3110 MSOL3120 MSOL3130 MSOL3140 MSOL3150 MSOL3160 MSOL3170 MSOL3180 MSOL3190 MSOL3200 MSOL3210 MSOL3220 MSOL3230 MSOL3240 MSOL3250 MSOL3260 MSOL3270 MSOL3280 MSOL3290 MSOL3300 MSOL3310 MSOL3320 MSOL3330 MSOL3340 MSOL3350 MSOL3360 MSOL3370 MSOL3380 MSOL3390 MSOL3400 MSOL3410 MSOL3420 MSOL3430 MSOL3440 MSOL3450 MSOL3460 MSOL3470 MSOL3480 MSOL3490 MSOL3500 MSOL3510 MSOL3520 MSOL3530 MSOL3540 MSOL3550 MSOL3560 MSOL3570 MSOL3580 MSOL3590 MSOL3600 MSOL3610 MSOL3620 MSOL3630 MSOL3640 MSOL3650 MSOL3660 MSOL3670 MSOL3680 MSOL3690 MSOL3700 MSOL3710 MSOL3720 MSOL3730 MSOL3740 MSOL3750 MSOL3760 MSOL3770 MSOL3780 MSOL3790 MSOL3800 MSOL3810 MSOL3820 MSOL3830 MSOL3840 MSOL3850 MSOL3860 MSOL3870 MSOL3880 MSOL3890 MSOL3900 MSOL3910 MSOL3920 MSOL3930 MSOL3940 MSOL3950 MSOL3960 MSOL3970 MSOL3980 MSOL3990 MSOL4000

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C DO 10 K=L3,L4 REAC0310
C 10 FORCE(I,J)+FORCE(I,J)+K(I,K)*DISPL(X,J) C REAC0320
C ..... C REAC0330
DO 20 K=L3,L4 REAC0340
MDX-MD(K+1) REAC0350
DO 20 J=1,NLOAD REAC0360
DO 10 I=L2,L1,-1 REAC0370
IK-MD(K)*K-I REAC0380
IF (IK GE MDK) GO TO 20 REAC0390
F(J)+FORCE(I,J) REAC0400
ATX-A(IK) REAC0410
DKJ-DISPL(X,J) REAC0420
10 FORCE(I,J)+FORCE(I,J)+A(IK)*DISPL(X,J) REAC0430
20 CONTINUE REAC0440
IF (PRINT) REAC0450
& CALL WMATRX(FORCE,NDOP,NLOAD,'MODIFIED LOADS') REAC0460
C REAC0470
ELSE IF (IOPT.EQ.2) THEN REAC0480
C REAC0490
C ..... C REAC0500
C SOLVE FOR REACTIONS --- C REAC0510
C ..... C REAC0520
C CONSIDER THE PARTITIONED STIFFNESS WHERE X2 ARE THE RESTRAINT DISPL. C REAC0530
C ..... C REAC0540
C ..... C REAC0550
C ..... C REAC0560
C ..... C REAC0570
C ..... C REAC0580
C ..... C REAC0590
C ..... C REAC0600
C ..... C REAC0610
C ..... C REAC0620
C ..... C REAC0630
C ..... C REAC0640
C ..... C REAC0650
C ..... C REAC0660
C ..... C REAC0670
C ..... C REAC0680
C ..... C REAC0690
C ..... C REAC0700
C ..... C REAC0710
C ..... C REAC0720
C ..... C REAC0730
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C ..... C REAC0780
C ..... C REAC0790
C ..... C REAC0800
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C ..... C REAC0820
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C ..... C REAC0880
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C ..... C REAC0900
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C ..... C REAC0930
C ..... C REAC0940
C ..... C REAC0950
C ..... C REAC0960
C ..... C REAC0970
C ..... C REAC0980
C ..... C REAC0990
C ..... C REAC1000
C ..... C REAC1010
C ..... C REAC1020
C ..... C REAC1030
C ..... C REAC1040
C ..... C REAC1050
C ..... C REAC1060
C ..... C REAC1070
C ..... C REAC1080
C ..... C REAC1090
C ..... C REAC1100
C ..... C REAC1110
C ..... C REAC1120
C ..... C REAC1130
C ..... C REAC1140
C ..... C REAC1150
C ..... C REAC1160
C ..... C REAC1170
C ..... C REAC1180
C ..... C REAC1190
C ..... C REAC1200
C ..... C REAC1210
C ..... C REAC1220
C ..... C REAC1230
C ..... C REAC1240
C ..... C REAC1250
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C ..... C REAC1360
C ..... C REAC1370
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C ..... C REAC1460
C ..... C REAC1470
C ..... C REAC1480
C ..... C REAC1490
C ..... C REAC1500
C ..... C REAC1510
C ..... C REAC1520
C ..... C REAC1530
C ..... C REAC1540
C ..... C REAC1550
C ..... C REAC1560
C ..... C REAC1570
C ..... C REAC1580
C ..... C REAC1590
C ..... C REAC1600
C ..... C REAC1610
C ..... C REAC1620
C ..... C REAC1630
C ..... C REAC1640
C ..... C REAC1650
C ..... C REAC1660
C ..... C REAC1670
C ..... C REAC1680
C ..... C REAC1690
C ..... C REAC1700
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C ..... C REAC1720
C ..... C REAC1730
C ..... C REAC1740
C ..... C REAC1750
C ..... C REAC1760

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IP (HEAD,AND, NREACT,GT,20) WRITE(108,320) TITLE(1),TITLE(2) REAC1770
IF (IOPT.NE.4) WRITE(108,322) L,STEPID REAC1780
IF (IOPT.EQ.4) WRITE(108,422) REAC1790
DO 290 I=1,NNODE REAC1800
C ..... C REAC1810
C ..... C REAC1820
C ..... C REAC1830
C ..... C REAC1840
C ..... C REAC1850
C ..... C REAC1860
C ..... C REAC1870
C ..... C REAC1880
C ..... C REAC1890
C ..... C REAC1900
C ..... C REAC1910
C ..... C REAC1920
C ..... C REAC1930
C ..... C REAC1940
C ..... C REAC1950
C ..... C REAC1960
C ..... C REAC1970
C ..... C REAC1980
C ..... C REAC1990
C ..... C REAC2000
C ..... C REAC2010
C ..... C REAC2020
C ..... C REAC2030
C ..... C REAC2040
C ..... C REAC2050
C ..... C REAC2060
C ..... C REAC2070
C ..... C REAC2080
C ..... C REAC2090
C ..... C REAC2100
C ..... C REAC2110
C ..... C REAC2120
C ..... C REAC2130
C ..... C REAC2140
C ..... C REAC2150
C ..... C REAC2160
C ..... C REAC2170
C ..... C REAC2180
C ..... C REAC2190
C ..... C REAC2200
C ..... C REAC2210
C ..... C REAC2220
C ..... C REAC2230
C ..... C REAC2240
C ..... C REAC2250
C ..... C REAC2260
C ..... C REAC2270
C ..... C REAC2280
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C ..... C REAC2700
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C ..... C REAC2760
C ..... C REAC2770
C ..... C REAC2780
C ..... C REAC2790
C ..... C REAC2800
C ..... C REAC2810
C ..... C REAC2820
C ..... C REAC2830
C ..... C REAC2840
C ..... C REAC2850
C ..... C REAC2860
C ..... C REAC2870
C ..... C REAC2880
C ..... C REAC2890
C ..... C REAC2900
C ..... C REAC2910
C ..... C REAC2920
C ..... C REAC2930
C ..... C REAC2940
C ..... C REAC2950
C ..... C REAC2960
C ..... C REAC2970
C ..... C REAC2980
C ..... C REAC2990
C ..... C REAC3000

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```
WRITE (IELNO,15) IELNO,(N2(I-12LM-1),I-1,MIN(IELODF,18))
FORMAT ('ELEMENT #',16,' LM',18)
CALL LMATRX(2(IZKEX),MD,IELODF,MD(IELODF-1),ELNO)
ENDIF
C
CALL FORML(2(IZSTIP),N2(IZMD),KDOF,N2(IZMD-NDOP),
2(IZKE),MD,IELODF,MD(IELODF-1),N2(IZLM),1.1)
C
SUBTRACT GEOMETRIC STIFFNESS
N2(IZKGD-3)-N2(IZKGD-3)
C
IF (N2(IZKGD-3).EQ.1) THEN
FALSE=.FALSE
CALL ELELIB
(2,LSSTYP,FALSE,IREL,FALSE,NAME,ESE,EPSE,DAMAGE,DUCTAG,
IELNO,IELODF,KGDOP,PGBOM,2,AXIAL,IZDISP,IZLOAD,HSTOR)
KGDOP-KGDOP
C
IF (BUG57) THEN
WRITE (IELNO,15) IELNO,(N2(I-12LM-1),I-1,MIN(KGDOP,18))
CALL LMATRX(2(IZKEX),MD,KGDOP,MD(KGDOP-1),ELNO)
ENDIF
C
IF (.NOT. AXIAL) GO TO 120
DETERMINE THE FACTOR FOR XG
IF (N2(IZKGD-1).EQ.1) THEN
ACCEL=2(IZKGD)
PGBOM-PGBOM*(1+ACCEL)
ENDIF
SWAP THE SIGN ON PGBOM FOR SUBTRACTION
PGBOM=-PGBOM
C
CALL FORML(2(IZSTIP),N2(IZMD),KDOF,N2(IZMD-NDOP),
2(IZKEX),MD,KGDOP,MD(KGDOP-1),N2(IZLM),PGBOM)
ENDIF
120 CONTINUE
C
RETURN
C
ASSEMBLE GEOMETRIC STIFFNESS
200 CONTINUE
IF (N2(IZKGD-3) NE 2) RETURN
1ST=IZKG
IEND=IZKG+N2(IZMD-KGDOP)-1
C
ZERO STIFFNESS
DO 210 I=1ST,IEND
2(1)=0
C
LOOP FOR EACH ELEMENT
DO 220 IELNO=1,NELMT
C
GET STIFFNESS OF EACH ELEMENT
FALSE=.FALSE
CALL ELELIB
(2,LSSTYP,FALSE,IREL,FALSE,NAME,ESE,EPSE,DAMAGE,DUCTAG,
IELNO,IELODF,KGDOP,PGBOM,2,AXIAL,IZDISP,IZLOAD,HSTOR)
IF (.NOT. AXIAL) GO TO 230
IF (BUG57) THEN
WRITE (IELNO,15) IELNO,(N2(I-12LM-1),I-1,MIN(KGDOP,18))
CALL LMATRX(2(IZKEX),MD,KGDOP,MD(KGDOP-1),ELNO)
ENDIF
CALL FORML(2(IZKEX),N2(IZMD),KDOF,N2(IZMD-KGDOP),
2(IZKEX),MD,KGDOP,MD(KGDOP-1),N2(IZLM),PGBOM)
220 CONTINUE
RETURN
END
SUBROUTINE ELER (A-H,O-Z)
COMMON /RSA/ICORN,RSAP(12)
REAL K12,K13,K14,K15,K16,K17,K18,K19,K20,K21,LENGTH,ICORN
INTEGER REL,RELT
LOGICAL FIRST,PRINT,PENLG,FALSE,AXIAL,BTEST
CHARACTER NAME*8,IREL*6
DIMENSION IDOP(MAOND,6),COORD(MAOND,6),COSINE(1,3,3),MCOB)
DIMENSION CONST(MAOND,6),ID(MAOND),JTCOS(MAOND)
DIMENSION ASAT(12,12),BAT(12,12),STIFF(155),KGDATA(3)
DIMENSION R(12,12),S(12,12),LM(12),LMT(12),LAKG(12),PMAJ(144)
DIMENSION CTS(3,3),CTE(3,3),VX(3),VY(3),F(3),FUB(NDOP)
DIMENSION BS(3,3),BE(3,3),SE(15),EMDAT(2),TM(14)
DIMENSION R(12),DISP(LNDOP,NLAD),P(12),RT(12),PT(12)
DIMENSION FORC(LNDOP,NLAD),IELO(5,MAXEL),REL(12,MAXELD)
DIMENSION RINP(100),PEM(12,NLAD),GFEN(12),DUCT(3),EXCR(6)
C
VARIABLES
GLOBAL VARIABLES
C IOPT = 1, INITIALIZE BEAM COLUMN ELEMENT
C = 2, CALCULATE GEOMETRIC STIFFNESS
C = 3, FORM STIFFNESS
C = 4, CALCULATE INCREMENTAL FORCES
C = 5, CALCULATE TOTAL FORCES AND ENERGIES
C JUNIT = OUTPUT FILE UNIT # FOR PRINTING OUTPUT
C FIRST = FLAG, FIRST=.TRUE., PRINT HEADERS
C PRINT = FLAG, PRINT=.TRUE., PRINT DATA
C KDOF = # OF GLOBAL DOP
C NLAD = # OF LOAD COMBINATIONS
C MAOND = # ROW DIMENSION OF NODE ARRAY
C NNODE = # OF NODES
C ID = ARRAY OF EXTERNAL NODE NUMBERS
C IDOP = ARRAY OF DEGREES OF FREEDOM
C COORD = ARRAY OF COORDINATES
C COSINE = ARRAY OF DIRECTION COSINES OF NODES
C CONST = ARRAY OF CONSTRAINT TRANSFORMATIONS
C DISP = GLOBAL DISPLACEMENT MATRIX
C IZMAT = LOCATION OF MATERIAL DATA IN GLOBAL STORAGE VECTOR
C NAME = ELEMENT NAME
C IELNO = ELEMENT NUMBER
C RINP = INPUT DATA
C STIFF = OUTPUT ELEMENT STIFFNESS (UPPER TRIANGULAR FORM)
C LM = OUTPUT LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C ELEMENT VARIABLES
C TM(2,14),D
C = INDIVIDUAL TERMS IN THE ELEMENT STIFFNESS (S) MATRIX
C PKG = TM(1) INPUT LOAD FOR FORMING GEOMETRIC STIFFNESS MATRIX
C R = INCREMENTAL DISPLACEMENTS
C RT = TOTAL DISPLACEMENTS
C F = INCREMENTAL FORCES
C FT = TOTAL FORCES
C LM = LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C CTS = LOCAL TO GLOBAL ROTATION MATRIX START
C CTE = LOCAL TO GLOBAL ROTATION MATRIX END
C BS = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX START/EL00720
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C BE = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX END
C STIFF = ELEMENT STIFFNESS
C IELTYP = ELEMENT TYPE = 16
C NDEI = EXTERNAL JOINT NUMBER, END I
C NDEJ = EXTERNAL JOINT NUMBER, END J
C JOINTI = INTERNAL JOINT NUMBER, END I
C JOINTJ = INTERNAL JOINT NUMBER, END J
C = TEMPORARY VARIABLE
C VX = VECTOR DEFINING THE ELEMENT X AXIS
C VY = VECTOR DEFINING THE ELEMENT Y AXIS
C COSLOC = LOCAL COSINE MATRIX
C A = GLOBAL TO LOCAL FORCE TRANSFORMATION MATRIX
C S = ELEMENT STIFFNESS AT LOCAL COORD, AT ENDS OF FLEXIBLE PART
C SAT = PRODUCT OF S*A
C ASAT = PRODUCT OF A*SAT
C
CHOOSE OPTION
IF (IOPT.LT 1 OR IOPT.GT 14) THEN
WRITE (108,*) 'INVALID OPTION IN ELE-01, IOPT=',IOPT
STOP
ENDIF
GO TO (100,200,300,400,500,600,700,800,900,1000,1100,
1200,1300,1400) IOPT
C
100 CONTINUE
ZERO STRAIN ENERGY
EMDAT(1) = 0
EMDAT(2) = 0
C
INTERPRETATE INPUT DATA
IELTYP = 1
MAT = RINPUT(1)
NDEI = RINPUT(2)
NDEJ = RINPUT(3)
VY(1) = RINPUT(4)
VY(2) = RINPUT(5)
VY(3) = RINPUT(6)
XS = RINPUT(7)
XE = RINPUT(8)
TM(1) = RINPUT(9)
RELT = RINPUT(10)
C
GET INTERNAL NODE # ASSOCIATED WITH EXTERNAL NODE #'S
JOINTI=IOUCX(NDEI,1D,NODE)
JOINTJ=IOUCX(NDEJ,1D,NODE)
C
PRINT DATA FOR PLOTTING
IF (BTEST(18UG,2)) WRITE (7,10) NDEJ,NDEI,(COORD(JOINTJ,1)
,I-1,3),(COORD(JOINTI,3),I-1,3)
10 FORMAT (15,1X,15,1P,6G12.4)
C
GET GLOBAL TO LOCAL TRANSFORMATION MATRIX
C
DEFINE VECTOR X, LENGTH
VX(1)=COORD(JOINTI,1)-COORD(JOINTI,1)
VX(2)=COORD(JOINTI,2)-COORD(JOINTI,2)
VX(3)=COORD(JOINTI,3)-COORD(JOINTI,3)
LENGTH=SQRT(VX(1)**2+VX(2)**2+VX(3)**2)*XS-XE
IF (LENGTH.LE 0) THEN
WRITE(108,10) IELNO,JOINTI,JOINTJ,LENGTH
101 FORMAT(1X,20(' '),ER',RGR',LENGTH IS LE 0',/
4 5X,'IELNO=',15,5X,'JOINTI=',15,5X,'JOINTJ=',15,
4 5X,'LENGTH',1P,6I5.6/
4 5X,'REVISE INPUT.....')
LENGTH=1.0
ENDIF
C
GET LOCAL TO GLOBAL TRANSFORMATION MATRICES CT AND B
ICB=JTCOS(JOINTJ)
CALL ROTXYZ(VX,VY,COSINE(1,1,ICS),CTS,XS,0.,0.,BS,
CONST(JOINTI,1),MAOND)
CALL ROTXYZ(VY,VY,COSINE(1,1,ICB),CTE,XE,0.,0.,BE,
CONST(JOINTJ,1),MAOND)
C
GET GLOBAL CONSTRAINT MATRIX BS, BE
BS(1,1)=CONST(JOINTI,1)*CTS(2,1)+CONST(JOINTI,2)*CTS(3,1)+BS(1,1)
BS(1,2)=CONST(JOINTI,1)*CTS(2,2)+CONST(JOINTI,2)*CTS(3,2)+BS(1,2)
BS(1,3)=CONST(JOINTI,1)*CTS(2,3)+CONST(JOINTI,2)*CTS(3,3)+BS(1,3)
BS(2,1)=CONST(JOINTI,3)*CTS(1,1)+CONST(JOINTI,4)*CTS(2,1)+BS(2,1)
BS(2,2)=CONST(JOINTI,3)*CTS(1,2)+CONST(JOINTI,4)*CTS(2,2)+BS(2,2)
BS(2,3)=CONST(JOINTI,3)*CTS(1,3)+CONST(JOINTI,4)*CTS(2,3)+BS(2,3)
BS(3,1)=CONST(JOINTI,5)*CTS(1,1)+CONST(JOINTI,6)*CTS(2,1)+BS(3,1)
BS(3,2)=CONST(JOINTI,5)*CTS(1,2)+CONST(JOINTI,6)*CTS(2,2)+BS(3,2)
BS(3,3)=CONST(JOINTI,5)*CTS(1,3)+CONST(JOINTI,6)*CTS(2,3)+BS(3,3)
BE(1,1)=CONST(JOINTJ,1)*CTE(2,1)+CONST(JOINTJ,2)*CTE(3,1)+BE(1,1)
BE(1,2)=CONST(JOINTJ,1)*CTE(2,2)+CONST(JOINTJ,2)*CTE(3,2)+BE(1,2)
BE(1,3)=CONST(JOINTJ,1)*CTE(2,3)+CONST(JOINTJ,2)*CTE(3,3)+BE(1,3)
BE(2,1)=CONST(JOINTJ,3)*CTE(1,1)+CONST(JOINTJ,4)*CTE(2,1)+BE(2,1)
BE(2,2)=CONST(JOINTJ,3)*CTE(1,2)+CONST(JOINTJ,4)*CTE(2,2)+BE(2,2)
BE(2,3)=CONST(JOINTJ,3)*CTE(1,3)+CONST(JOINTJ,4)*CTE(2,3)+BE(2,3)
BE(3,1)=CONST(JOINTJ,5)*CTE(1,1)+CONST(JOINTJ,6)*CTE(2,1)+BE(3,1)
BE(3,2)=CONST(JOINTJ,5)*CTE(1,2)+CONST(JOINTJ,6)*CTE(2,2)+BE(3,2)
BE(3,3)=CONST(JOINTJ,5)*CTE(1,3)+CONST(JOINTJ,6)*CTE(2,3)+BE(3,3)
C
ZERO MATRICES
DO 40 I=1,12
F(I)=0
LPTYP=0
R(I)=0
RT(I)=0
DO 40 J=1,12
PMAJ(I,1)=12-J-0
S(I,J)=0
A(I,J)=0
C
ASSEMBLE TRANSFORMATION MATRIX A
DO 50 I=1,3
DO 50 J=1,3
A(I+J-1,3)-CTS(I,J)
A(I+J-1,3)-CTS(I,J)
A(I+J-1,3)-CTS(I,J)
A(I+J-1,3)-CTS(I,J)
A(I+J-1,3)-CTS(I,J)
A(I+J-1,3)-CTS(I,J)
50 A(I+J-1,3)=BE(I,J)
C
GET MATERIAL PROPERTIES
FALSE=.FALSE
CALL MATLIB(2,LSSTYP,FALSE,ESE,EPSE,DUCT,EXCR,
PT,F,R,0.,0.,LELM,LMAT,MAT,MATTF,SE,IELNO,DAMAGE)
IF (MATTF.NE.1) THEN
WRITE(108,190) IELNO,MATTF,NDEI,NDEJ
FORMAT(' ELEMENT ',16,' INCOMPATIBLE MATERIAL PROP.#',15,
190 START JOINT=',15,' END JOINT=',15,/
4 'THE ELEMENT IS REJECTED AND ER',FOR CHECKING CONTINUES')
GO TO 191
ENDIF
C
EAX =SE(1)
GAY =SE(2)
GAZ =SE(3)
EIX =SE(4)
EIY =SE(5)
EIZ =SE(6)
C
LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
LOCAL TO GLOBAL ROTATION MATRIX START
LOCAL TO GLOBAL ROTATION MATRIX END
RELT IS AN INPUT MEMBER RELEASE CODE
ELE00730
ELE00740
ELE00750
ELE00760
ELE00770
ELE00780
ELE00790
ELE00800
ELE00810
ELE00820
ELE00830
ELE00840
ELE00850
ELE00860
ELE00870
ELE00880
ELE00890
ELE00900
ELE00910
ELE00920
ELE00930
ELE00940
ELE00950
ELE00960
ELE00970
ELE00980
ELE00990
ELE01000
ELE01010
ELE01020
ELE01030
ELE01040
ELE01050
ELE01060
ELE01070
ELE01080
ELE01090
ELE01100
ELE01110
ELE01120
ELE01130
ELE01140
ELE01150
ELE01160
ELE01170
ELE01180
ELE01190
ELE01200
ELE01210
ELE01220
ELE01230
ELE01240
ELE01250
ELE01260
ELE01270
ELE01280
ELE01290
ELE01300
ELE01310
ELE01320
ELE01330
ELE01340
ELE01350
ELE01360
ELE01370
ELE01380
ELE01390
ELE01400
ELE01410
ELE01420
ELE01430
ELE01440
ELE01450
ELE01460
ELE01470
ELE01480
ELE01490
ELE01500
ELE01510
ELE01520
ELE01530
ELE01540
ELE01550
ELE01560
ELE01570
ELE01580
ELE01590
ELE01600
ELE01610
ELE01620
ELE01630
ELE01640
ELE01650
ELE01660
ELE01670
ELE01680
ELE01690
ELE01700
ELE01710
ELE01720
ELE01730
ELE01740
ELE01750
ELE01760
ELE01770
ELE01780
ELE01790
ELE01800
ELE01810
ELE01820
ELE01830
ELE01840
ELE01850
ELE01860
ELE01870
ELE01880
ELE01890
ELE01900
ELE01910
ELE01920
ELE01930
ELE01940
ELE01950
ELE01960
ELE01970
ELE01980
ELE01990
ELE02000
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C REL IS THE INTERNAL MEMBER RELEASE CODE ELE02190 D =KIJY*(EIZ/LENGTH) ELE03650
C BIT 6 AXIAL TRUE = AXIAL DEFORMATION CONSIDERED ELE02200 TM( 9 ) = (TM( 4 )+2*TM( 6 )+TM( 5 ))/(LENGTH**2) ELE03660
C BIT 5 MX AT END I ELE02210 TM(10) = (TM( 4 ) - TM( 6 ))/(LENGTH ) ELE03670
C BIT 4 MY AT END I ELE02220 TM(11) = (TM( 6 ) - TM( 5 ))/(LENGTH ) ELE03680
C BIT 3 MZ AT END I ELE02230 TM(12) = (TM(7)+2*D+TM( 8 ))/(LENGTH**2) ELE03690
C BIT 2 MX AT END J ELE02240 TM(13) = (TM(7) - D )/(LENGTH ) ELE03700
C BIT 1 MY AT END J ELE02250 TM(14) = ( D+TM( 8 ))/(LENGTH ) ELE03710
C BIT 0 MZ AT END J ELE02260 S( 1, 1 ) = TM( 2 ) ELE03720
C ELE02270 S( 1, 7 ) = TM( 2 ) ELE03730
C ELE02280 S( 2, 2 ) = TM( 9 ) ELE03740
C ELE02290 S( 2, 6 ) = TM(10) ELE03750
C ELE02300 S( 2, 8 ) = TM( 9 ) ELE03760
C ELE02310 S( 2,12 ) = TM(11) ELE03770
C ELE02320 S( 3, 3 ) = TM(12) ELE03780
C ELE02330 S( 3, 5 ) = TM(13) ELE03790
C ELE02340 S( 3, 9 ) = TM(12) ELE03800
C ELE02350 S( 3,11 ) = TM(14) ELE03810
C ELE02360 S( 4, 4 ) = TM( 3 ) * KT ELE03820
C ELE02370 S( 4,10 ) = TM( 3 ) ELE03830
C ELE02380 S( 5, 5 ) = TM(7) ELE03840
C ELE02390 S( 5, 9 ) = TM(13) ELE03850
C ELE02400 S( 5,11 ) = D ELE03860
C ELE02410 S( 6, 6 ) = TM( 4 ) ELE03870
C ELE02420 S( 6, 8 ) = TM(10) ELE03880
C ELE02430 S( 6,12 ) = TM( 6 ) ELE03890
C ELE02440 S( 7, 7 ) = TM( 2 ) ELE03900
C ELE02450 S( 8, 8 ) = TM( 9 ) ELE03910
C ELE02460 S( 8,12 ) = TM(11) ELE03920
C ELE02470 S( 9, 9 ) = TM(12) ELE03930
C ELE02480 S( 9,11 ) = TM(14) ELE03940
C ELE02490 S(10,10) = TM( 3 ) * KTJ ELE03950
C ELE02500 S(11,11) = TM( 5 ) ELE03960
C ELE02510 S(12,12) = TM( 1 ) ELE03970
C ELE02520 DO 55 I=1,12 ELE03980
C ELE02530 DO 55 J=I+1,12 ELE03990
C ELE02540 S( J, I ) = S( I, J ) ELE04000
C ELE02550 ELE04010
C ELE02560 C----- CALCULATE SAT ELE04020
C ELE02570 CALL MULTM(3,12,SAT,S,A) ELE04030
C ELE02580 ELE04040
C ELE02590 C----- CALCULATE ASAT ELE04050
C ELE02600 CALL MULTM(6,12,ASAT,A,SAT) ELE04060
C ELE02610 ELE04070
C ELE02620 C----- DEGREE OF FREEDOM ELE04080
C ELE02630 C REMOVE DOP'S THAT ARE CONSTRAINED TO THE SAME DOP ELE04090
C ELE02640 DO 75 I=1,6 ELE04100
C ELE02650 IF ( IDOP(JOINT,I) NE IDOP(JOINT,I) ) THEN ELE04110
C ELE02660 LMT(I)=IDOP(JOINT,I) ELE04120
C ELE02670 LMT(I)=6-IDOP(JOINT,I) ELE04130
C ELE02680 ELSE ELE04140
C ELE02690 LMT(I)=0 ELE04150
C ELE02700 LMT(I)=6 ELE04160
C ELE02710 ELE04170
C ELE02720 75 CONTINUE ELE04180
C ELE02730 ELE04190
C ELE02740 C----- CONVERT TO HALF STORAGE MODE BY COLUMNS ELE04200
C ELE02750 ELE04210
C ELE02760 1 3 6 10 15 21 28 36 45 55 66 78 NUMBER INDICATES ELE04220
C ELE02770 2 5 9 14 20 27 35 44 54 65 77 LOCATION OF DATA ELE04230
C ELE02780 4 8 13 19 26 34 43 52 64 76 IN ARRAY STIFF ELE04240
C ELE02790 7 12 18 25 33 42 52 63 75 ELE04250
C ELE02800 11 17 24 32 41 51 62 74 ELE04260
C ELE02810 16 23 31 40 50 61 73 ELE04270
C ELE02820 22 30 39 49 60 72 ELE04280
C ELE02830 29 38 48 59 71 ELE04290
C ELE02840 37 47 58 70 ELE04300
C ELE02850 46 57 69 ELE04310
C ELE02860 56 68 ELE04320
C ELE02870 67 ELE04330
C ELE02880 ELE04340
C ELE02890 ELE04350
C ELE02900 L=0 ELE04360
C ELE02910 IELDOP=0 ELE04370
C ELE02920 DO 90 J=1,12 ELE04380
C ELE02930 IF (LMT(I).EQ.0 .OR. ASAT(I,I).LE.0) GO TO 90 ELE04390
C ELE02940 IELDOP=IELDOP+1 ELE04400
C ELE02950 LMT(IELDOP)=LMT(I) ELE04410
C ELE02960 DO 80 J=1,12 ELE04420
C ELE02970 IF (LMT(J).EQ.0 .OR. ASAT(J,J).LE.0) GO TO 80 ELE04430
C ELE02980 L=L+1 ELE04440
C ELE02990 STIFF(L)=ASAT(I,J) ELE04450
C ELE03000 80 CONTINUE ELE04460
C ELE03010 90 CONTINUE ELE04470
C ELE03020 C----- ASSEMBLE GEOMETRIC STIFFNESS MATRIX FOR A UNIT LOAD ELE04480
C ELE03030 ELE04490
C ELE03040 LKG=L ELE04500
C ELE03050 IF ( TM(1).EQ.0 .AND. XGDATA(1).NE.2 ) THEN ELE04510
C ELE03060 AXIAL=FALSE ELE04520
C ELE03070 REL=IBCL8(REL,6) ELE04530
C ELE03080 GO TO 180 ELE04540
C ELE03090 ELE04550
C ELE03100 ELE04560
C ELE03110 C----- DETERMINE IF AXIAL DEFORMATION IS CONSIDERED ELE04570
C ELE03120 BIT #6 OF REL IS TRUE IF AXIAL DEFORMATION IS CONSIDERED ELE04580
C ELE03130 DO 999 I=1,3 ELE04590
C ELE03140 CE=C*(I,1) ELE04600
C ELE03150 CE=C*(E,I,1) ELE04610
C ELE03160 IF (ABS(CS).LE..0001 .AND. ABS(CE).LE..0001) GO TO 999 ELE04620
C ELE03170 IS=IDOP(JOINT,I) ELE04630
C ELE03180 IS=IDOP(JOINT,I) ELE04640
C ELE03190 IF (IS.EQ.IE) GO TO 999 ELE04650
C ELE03200 REL=IBSET(REL,6) ELE04660
C ELE03210 999 CONTINUE ELE04670
C ELE03220 ELE04680
C ELE03230 C----- AXIAL-BTEST(REL,6) ELE04690
C ELE03240 IF (KGDATA(1).NE.0 .AND. XGDATA(2).NE.0 .AND. BTEST(REL,6)) THEN ELE04700
C ELE03250 ELE04710
C ELE03260 C----- DETERMINE THE AXIAL LOAD TO BE USED FOR KG.. ELE04720
C ELE03270 IF (KGDATA(1).EQ.1) THEN ELE04730
C ELE03280 PGBOW = TM(1) ELE04740
C ELE03290 ELSE IF (KGDATA(1).EQ.2) THEN ELE04750
C ELE03300 PGBOW = 0.5*(P(1)-P(7)) ELE04760
C ELE03310 ELE04770
C ELE03320 ELE04780
C ELE03330 C----- ASSEMBLE LOCAL GEOMETRIC STIFFNESS MATRIX FOR A UNIT LOAD ELE04790
C ELE03340 ERO LOCAL KG MATRIX ELE04800
C ELE03350 DO 998 I=1,12 ELE04810
C ELE03360 DO 998 J=1,12 ELE04820
C ELE03370 S(I,J)=0 ELE04830
C ELE03380 ELE04840
C ELE03390 C----- LIMPED MASS FORM OF GEOMETRIC STIFFNESS ELE04850
C ELE03400 IF (KGDATA(2).EQ.1) THEN ELE04860
C ELE03410 F1=1/LENGTH ELE04870
C ELE03420 S( 2, 2 ) = F1 ELE04880
C ELE03430 S( 2, 8 ) = -F1 ELE04890
C ELE03440 S( 8, 2 ) = -F1 ELE04900
C ELE03450 S( 8, 8 ) = F1 ELE04910
C ELE03460 S( 3, 3 ) = F1 ELE04920
C ELE03470 S( 3, 9 ) = -F1 ELE04930
C ELE03480 S( 9, 3 ) = -F1 ELE04940
C ELE03490 S( 9, 9 ) = F1 ELE04950
C ELE03500 ELE04960
C ELE03510 C----- CONSISTENT MASS FORM OF GEOMETRIC STIFFNESS ELE04970
C ELE03520 ELSE IF (KGDATA(2).EQ.2) THEN ELE04980
C ELE03530 S( 2, 2 ) = F2 ELE04990
C ELE03540 S( 2, 6 ) = D2 ELE05000
C ELE03550 S( 2, 8 ) = -F2 ELE05010
C ELE03560 S( 2,12 ) = E2 ELE05020
C ELE03570 S( 6, 2 ) = D2 ELE05030
C ELE03580 S( 6, 6 ) = A2 ELE05040
C ELE03590 S( 6, 8 ) = -D2 ELE05050
C ELE03600 S( 6,12 ) = C2 ELE05060
C ELE03610 S( 8, 2 ) = F2 ELE05070
C ELE03620 S( 8, 6 ) = -D2 ELE05080
C ELE03630 S( 8, 8 ) = F2 ELE05090
C ELE03640 S( 8,12 ) = -E2 ELE05100

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S(12, 2) = E2
S(12, 6) = -C2
S(12, 8) = -E2
S(12, 12) = B2
S(3, 3) = PY
S(3, 5) = -DY
S(3, 9) = -FY
S(11, 1) = -EY
S(5, 3) = -DY
S(5, 5) = AY
S(5, 9) = -FY
S(5, 11) = -CY
S(9, 3) = -DY
S(9, 5) = DY
S(9, 9) = -FY
S(9, 11) = -EY
S(11, 3) = -EY
S(11, 5) = -CY
S(11, 9) = -EY
S(11, 11) = -EY
ENDIF
C
C ***** CALCULATE SAT
CALL MULTM(3, 12, SAT, S, A)
C
C ***** CALCULATE ASAT
CALL MULTM(0, 12, ASAT, A, SAT)
C
C ***** CONVERT TO HALF STORAGE MODE BY COLAPSE
KGDOP=0
DO 1090 I=1, 12
IF (LMT(I), EQ, 0 .OR. ASAT(I, I), LE, 0) GO TO 1090
KGDOP=KGDOP+1
LARG(KGDOP)=LMT(I)
DO 1080 J=1, 12
IF (LMT(J), EQ, 0 .OR. ASAT(J, J), LE, 0) GO TO 1080
L=L+1
STIFF(L)=ASAT(I, J)
CONTINUE
1080 CONTINUE
1090 CONTINUE
ENDIF
C
C ***** RELEASE UNUSED STORAGE
150 IREL=156-L
C
C ***** PRINT COLUMN DATA
191 IF (FIRST) WRITE(108, 12)
191 WRITE(108, 193) NAME, IELNO, MAT, NODEI, NODEJ, IREL, LENGTH,
& VY(1), VY(2), VY(3), XS, XE, TM(1)
C
192 FORMAT(/ ' ELEMENT 08, 3D BEAM ELEMENT' //
& T2, ' # MATL START END, REL CD, 3X, LENGTH', 3X, 11(' '),
& ' Y-AXIS ', 12(' '), ' START DIST END DIST PKG')
193 FORMAT(1X, A15, 416, 2X, A, 1P, G12.4, 0P,
& ' P9.5, ' 1, SP, P9.5, ' J', P9.5, ' K', 1P, SS, G12.4)
C
C ***** DETERMINE GEOMETRIC STIFF
200 CONTINUE
C ***** 3D IS DETERMINED FOR A UNIT LOAD WITH IOPT=1,
C ***** THE ACTUAL LOAD (PQGM) IS DETERMINED HERE
C ***** KG IS MULTIPLIED BY PQGM WHEN THE TOTAL STIFFNESS IS ASSEMBLED
C ***** NEGATIVE PQGM IS COMPRESSION ...
AXIAL=STEF(REL, 6)
C
C ***** DETERMINE THE AXIAL LOAD TO BE USED FOR KG ...
IF (AXIAL) THEN
IF (KGDATA(1), EQ, 1) THEN
PQGM = TM(1)
ELSE IF (KGDATA(1), EQ, 2) THEN
PQGM = 0.50*(P(1)-P(7))
ENDIF
C
C ***** RETURN
C ***** DETERMINE STIFFNESS
300 CONTINUE
C ***** FORCE-FRAME DISPLACEMENT MATRIX
C ***** THE STIFFNESS IS STORED IN STIFF, THE MAPPING MATRIX IN LM
C ***** THESE HAVE THE GLOBAL ADDRESSES 13XZ AND IZLM ...
C ***** RETURN
C ***** DETERMINE INCREMENTAL FORCES
400 CONTINUE
IF (PRINT AND FIRST) WRITE(108, 491) STEPID
C ***** CALL BML0A TO DETERMINE THE FIXED END FORCES
IF (MAXELD, GT, 0) THEN
CALL BML0A(IELANO, FEMPLG, LENGTH, REL, MAXELD, NELD, NLOAD,
& FEM, IELD, ELD)
ELSE
FEMPLG = -FALSE
ENDIF
C ***** LOOP FOR EACH LOAD
DO 430 I=1, NLOAD
C ***** TRANSFORM FRAME DISPL INTO LOCAL DISPL
DO 430 J=1, 3
R(1)=0
R(1-3)=0
R(1-6)=0
R(1-9)=0
DO 430 K=1, 3
D1=DISPL(IDOOF(JOINTI, J), LOAD)
D2=DISPL(IDOOF(JOINTJ, J-3), LOAD)
D3=DISPL(IDOOF(JOINTJ, J), LOAD)
D4=DISPL(IDOOF(JOINTJ, J+3), LOAD)
R(1)=R(1)+CTS(J, I)*D1+BS(J, I)*D2
R(1-3)=R(1-3)+CTS(J, I)*D3
R(1-6)=R(1-6)+CTS(J, I)*D4
R(1-9)=R(1-9)+CTS(J, I)*D4
430 CONTINUE
C ***** INCREMENTAL FORCES
F(1)=TM(2)*(R(1)-R(7))
F(2)=TM(9)*(R(2)-R(8))+TM(10)*(R(6)-TM(11)*R(12))
F(3)=TM(12)*(R(3)-R(9))+TM(13)*(R(5)-TM(14)*R(11))
F(4)=TM(3)*(R(4)-R(10))
F(5)=TM(11)*(R(3)-R(9))+TM(7)*(R(5)+D+R(11))
F(6)=TM(10)*(R(2)-R(8))+TM(4)*(R(6)-TM(6)*R(12))
F(7)=TM(2)*(R(1)+R(7))
F(8)=TM(9)*(R(2)-R(8))+TM(10)*(R(6)-TM(11)*R(12))
F(9)=TM(12)*(R(3)-R(9))+TM(13)*(R(5)+TM(14)*R(11))
F(10)=TM(3)*(R(4)-R(10))
F(11)=TM(14)*(R(3)-R(9))+D*(R(5)-TM(8)*R(11))
F(12)=TM(11)*(R(2)-R(8))+TM(6)*(R(6)-TM(5)*R(12))
C ***** SUBTRACT OF FIXED END FORCES
IF (FEMPLG) THEN
DO 420 I=1, 12
F(I)=F(I)-FEM(I, LOAD)
ENDIF
C ***** DETERMINE MAXIMUM VALUES FOR MULTIPLE LOAD CASES
IF (NLOAD, GT, 1) THEN
DO 425 I=1, 12
IF (ABS(F(I)), GT, ABS(PMAX(I*12-12, I))) THEN
DO 426 J=1, 11
PMAK(I*12-12, J)=F(I, J)
ENDIF
425 CONTINUE
ENDIF
C ***** PRINT FORCES
IF (BTST(1BUG, 0) AND PRINT) WRITE(108, 492) IELNO, LOAD,
& NODEI, (R(J), J=1, 6), NODEJ, (R(J), J=7, 12)

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C RETURN
C ..... DETERMINE THE TOTAL STRAIN ENERGY
1000 CONTINUE
ESES=0
EPSE=0
C
DO 1010 I=1,12
1010 ESE=ESE + (PT(I)*P(I))*(R(I)-RT(I))/2.
ENGDAT(1)=ESE
ENGDAT(2)=0.
C
RETURN
C ..... DUCTILITIES AND EXCURSION RATIOS
NO D & E RATIOS FOR ELASTIC ELEMENT
1100 RETURN
C ..... PRINT MAXIMUM ELEMENT LOADS
1200 CONTINUE
LOAD2=0
IF (FIRST) WRITE(108,494) ' COLUMN ( LOAD ) REPRESENTS THE '
IF (FIRST) WRITE(108,491) ' MAXIMUM VALUES FOR ALL STEPS '
// NOTE: MAXIMUM VALUES WITH THE OTHER DOPS FORCES '
// ARE PRINT OUT AT THE SAME TIME '
DO 309 I=0,11
LOAD2=I+1
IF (PHAX(I)*12+1) EQ. 0 ) GO TO 309
WRITE(108,492) IELNO,LOAD2,NOE1,(PHAX(I)*12+J),J=1,6)
NOEJ,(PHAX(I)*12+J),J=7,12)
309 CONTINUE
RETURN
C ..... RESET ELEMENT FORCES
1300 CONTINUE
ZERO MATRICES
DO 1340 I=1,12
PHAX(I)=12+11-0
F (I)=0
PT(I)=0
R (I)=0
RT(I)=0
1340 WRITE(108,1330) IELNO
1330 FORMAT(' 3D BEAM..... ELEMENT #',16,
& ' INTERNAL FORCES AND HYSTERESIS MODELS ARE RESET TO ZERO')
FALSE=FALSE
LSTYP=
CALL MATLIB(2,LSTYP,FALSE,ESE,EPSE,DUCT,ENCR,
& PT,P,RT,R,0,0,LELEN,LMAT,MAT,MATYP,SE,IELNO,DAMAGE)
C
RETURN
C ..... DAMAGE INDEX
1400 DAMAGE = 0.
RETURN
END
C
SUBROUTINE ELE03
& ILOPT, & FIRST, & PRINT, & ID, & IDOP,
& COORD, & COSINE, & JTCOS, & CONST, & DISPL,
& VELOC, & PUB, & IREL, & NAME, & IELNO,
& RINPUT, & EPSE, & EPSE, & DAMAGE, & DUCPAG,
& IELTYP, & IELDOP, & NOEJ, & NOEJ, & JOINTJ,
& JOINTJ, & R, & RT, & P, & PT,
& V, & VT, & ISE, & H, & LENGTH,
& MAT, & A, & LM, & KTYPE, & PHAX,
& SE, & STIFF )
C
IMPLICIT REAL(A-H,O-Z)
COMMON /BSA/ICOM,RSAP(12)
REAL ICOM,LENGTH
LOGICAL FIRST,PRINT,FALSE,BTEST
CHARACTER*80 NAME
CHARACTER*8 TYPE(6)
INCLUDE 'ZCOMN3'
DIMENSION IDOP(MAXNO,6),COORD(MAXNO,6),COSINE(3,3,NCOS)
DIMENSION CONST(MAXNO,6),ID(MAXNO),JTCOS(MAXNO),SAT(2,12)
DIMENSION STIFF(7,8),A(12,2),S(2,2),LM(12),PHAX(2)
DIMENSION CTS(3,3),CTE(3,3),VX(1),VY(1)
DIMENSION BS(3,3),BE(3,3),SE(ISE)
DIMENSION DISPL(NDOF,NLOAD),PUB(NDOF),VELOC(NDOF)
DIMENSION RINPUT(100),NCR(6),ENCR(6)
DATA TYPE/' AXIAL', ' SHEAR-Y', ' SHEAR-Z',
& ' TORSION', ' BENDING-Y', ' BENDING-Z'//
C
VARIABLES:
C .....
C ILOPT - 1, INITIALIZE BEAM COLUMN ELEMENT
C 2, CALCULATE GEOMETRIC STIFFNESS
C 3, FORM STIFFNESS
C 4, CALCULATE INCREMENTAL FORCES
C 5, CALCULATE TOTAL FORCES AND ENERGIES
C IUNIT - OUTPUT FILE UNIT # FOR PRINTING OUTPUT
C FIRST - FLAG, FIRST=TRUE, PRINT HEADERS
C PRINT - FLAG, PRINT=TRUE, PRINT DATA
C NDOF - # OF GLOBAL DOP
C NLOAD - # OF LOAD COMBINATIONS
C MAXNO - # ROW DIMENSION OF NDOE ARRAY
C NNODE - # OF NDOE
C ID - ARRAY OF EXTERNAL NODE NUMBERS
C IDOP - ARRAY OF DEGREES OF FREEDOM
C COORD - ARRAY OF COORDINATES
C COSINE - ARRAY OF DIRECTION COSINES OF NODES
C CONST - ARRAY OF CONSTRAINT TRANSFORMATIONS
C DISPL - GLOBAL DISPLACEMENT MATRIX
C Z - REAL GLOBAL STORAG VECTOR
C NE - INTERSE GLOBAL STORAG VECTOR
C IZMAT - LOCATION OF MATERIAL DATA IN GLOBAL STORAGE VECTOR
C NAME - ELEMENT NAME
C IELNO - ELEMENT NUMBER
C RINPUT - INPUT DATA
C STIFF - OUTPUT ELEMENT STIFFNESS (UPPER TRIANGULAR FORM)
C LM - OUTPUT LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C ..... ELEMENT VARIABLES
C N,D,A1,A2, S22, S33, S212, S33, S35, S311, D
C ..... INDIVIDUAL TERMS IN THE ELEMENT STIFFNESS (S) MATRIX
C PKG - INPUT LOAD FOR FORMING GEOMETRIC STIFFNESS MATRIX...
C R - INCREMENTAL DISPLACEMENTS
C RT - TOTAL DISPLACEMENTS
C F - INCREMENTAL FORCES
C FT - TOTAL FORCES
C LM - LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C LCTS - LOCAL TO GLOBAL ROTATION MATRIX START
C CTE - LOCAL TO GLOBAL ROTATION MATRIX END
C BS - LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX START
C BE - LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX END
C STIFF - ELEMENT STIFFNESS
C IELTYP - ELEMENT TYPE = 10
C NOE1 - EXTERNAL JOINT NUMBER, END 1
C NOE2 - EXTERNAL JOINT NUMBER, END 2
C JOINT1 - INTERNAL JOINT NUMBER, END 1
C JOINT2 - INTERNAL JOINT NUMBER, END 2
C ..... TEMPORARY VARIABLES

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LSTYP=0
CALL MATLIB(2,LSTYP,FALSE,ESE,EPSE,DUCT,EXCR,
  FT,F,RT,R,0,0,LELEM,LMAT,MAT,MATYP,SE,IELNO,DAMAGE)
C
C ***** M IS THE SPRING STIFFNESS
M = SE(1)
HH= SE(1)/LENGTH
S(1,1)= HH
S(1,2)= -HH
S(2,1)= -HH
S(2,2)= HH
C
C ***** CALCULATE SAT
DO 60 I=1,2
  DO 60 J=1,I,IEDOF
  SAT(I,J)=0
DO 60 K=1,2
  SAT(I,J)=SAT(I,J)+S(1,K)*A(J,K)
60
C ***** CALCULATE ASAT AND CONVERT TO HALF STORAGE MODE BY COLUMNS
      1 3 6 10 15 21 28 36 45 55 66 78 NUMBER INDICATES
      2 5 9 14 20 27 35 44 54 65 77 LOCATION OF DATA
      4 8 13 19 26 34 43 53 64 76 IN ARRAY STIFF
      7 12 18 25 33 42 52 63 75
      11 17 24 32 41 51 62 74
      16 23 31 40 50 61 73
      22 30 39 49 60 72
      29 38 48 59 71
      37 47 58 70
      46 57 69
      56 68
      67
L=0
DO 70 J=1,IEDOF
  DO 70 I=J,1,-1
  L=L-1
  STIFF(L)=0
  DO 70 K=1,2
  STIFF(L)=STIFF(L)+A(I,K)*SAT(K,J)
70
C ***** RELEASE UNUSED STORAGE
190 IREL=78 L
C
C ***** PRINT COLUMN DATA
IF (FIRST) WRITE(108,192)
191 WRITE(108,193) NAME,IELNO,MAT,MODEI,MODEJ,TYPE(KTYPE),LENGTH,
  VY(1),VY(2),VY(3),XS,XZ
C
C ***** 192 FORMAT (' ELEMENT NO. ONE (LOCAL) DOP SPRING ELEMENT '//
4 T22,'# MATL START END',2X,'TYPE',6X,'LENGTH',3X,
4 11,'-',1,' Y AXIS',12,'-',1,' START DIST END DIST')
193 FCMAT(1X,A15,416,2X,A9,3X,1P,G12.4,CP,P9.5,' I',
4 SP,P9.5,' J',P9.5,' X',1P,SS,3G12.4)
RETURN
C ***** DETERMINE GEOMETRIC STIFF
200 RETURN
C ***** THE GEOMETRIC STIFFNESS MATRIX IS NOT AVAILABLE FOR THIS ELEMENT
C ***** DETERMINE STIFFNESS
300 CONTINUE
C ***** M CONTAINS THE SPRING STIFFNESS, WHICH WAS DETERMINED IN THE
LAST CALL TO MATLIB
IF M=SE(1) THEN THE STIFFNESS HAS NOT CHANGED, RETURN
IF (N.EQ.SE(1)) RETURN
M = SE(1)
HH= SE(1)/LENGTH
S(1,1)= HH
S(1,2)= -HH
S(2,1)= -HH
S(2,2)= HH
C ***** CALCULATE SAT
DO 360 I=1,2
  DO 360 J=1,I,IEDOF
  SAT(I,J)=0
DO 360 K=1,2
  SAT(I,J)=SAT(I,J)+S(1,K)*A(J,K)
360
C ***** CALCULATE ASAT, STORE IN UPPER TRIANGULAR MATRIX
L=0
DO 370 J=1,IEDOF
  DO 370 I=J,1,-1
  L=L-1
  STIFF(L)=0
  DO 370 K=1,2
  STIFF(L)=STIFF(L)+A(I,K)*SAT(K,J)
370
C ***** DETERMINE INCREMENTAL STIFFNESS
400 CONTINUE
IF (PRINT.AND.FIRST) WRITE(108,491) STEPID
C ***** LOOP FOR EACH LOAD
DO 430 LOAD=1,NLOAD
C ***** TRANSFORM FRAME DISPL INTO LOCAL DISPL
C ***** LOCAL DISPL ARE AT THE END OF THE SPRING, THUS
C ***** POSITIVE IS TENSION AND NEGATIVE IS COMPRESSION
V=0
DO 410 I=1,IEDOF
  J=LM(I)
  A1= A(I,1)
  A2= A(I,2)
  DISP= DISPL(J,LOAD)
  VEL= VELOC(J)
  R= R + (A(I,2) A(I,1)) * DISPL(J,LOAD)
  V= V + (A(I,2) A(I,1)) * VELOC(J)
410
C ***** INCREMENTAL FORCES
P= H*R/LENGTH
C ***** CHECK STIFFNESS AND CALCULATE UNBALANCED FORCES
THIS IS PREPARED ONLY FOR INELASTIC ANALYSIS
P=PT-P
IF (.NOT.ELSTIC) THEN
  SET UP TEMPORARY TOTAL LOADS, DISPL'S AND VELOC
  PL=PT
  D=(RT-R)/LENGTH
  DL=RT/LENGTH
  VC=VT-V
  VL=V
  TRANSFER PERMANENT HYSTERESIS DATA TO TEMPORARY STORAGE.
  ISE2=ISE/2
  DO 415 I=1,ISE2
  SE(I)=ISE2-SE(I)
  CALL HWT MODEL TO GET NEW STIFFNESS AND
  THE TOTAL FORCE P AT DISPL D
  CALL MATLIB(3,LSTYP,FALSE,ESE,EPSE,DUCT,EXCR,P,PL,D,DL,
  VC,VL,LELEM,LMAT,MAT,MATYP,SE(I)SE2-1,IELNO,DAMAGE)
  ISE2=ISE-1
  ENDF
C ***** SAVE PEAK VALUES FOR MULTIPLE LOAD CASES
IF (NLOAD.GT.1 .AND. ABS(P).GT.ABS(PMAX(1))) THEN
  PMAX(1)=P
  PMAX(2)=R
  ENDF
C ***** PRINT DATA
HH= H/LENGTH
IF (PRINT) WRITE(108,492) IELNO,LOAD,TYPE(KTYPE),MODEI,MODEJ,
  F,R,HH
430 CONTINUE
C ***** THIS IS FOR SOLOSA USE ONLY
IF (ICOMN.EQ.1 .OR. ICOMN.EQ.2) THEN
  RSAP(1)=P
  ENDF
C ***** CALCULATE THE UNBALANCED MEMBER FORCE ON A JOINT.
UBAL= P-FT-P
P= P
FT= FT
DO 440 I=1,IEDOF
  A1= A(I,1)
  A2= A(I,2)
  PUB(J)= PUB(J) + (A(I,1) A(I,2)) * UBAL
440 CONTINUE
C ***** 491 FORMAT (' SPRING FORCES...',5X,A/
4 ' ELEMENT LOAD',7I7,' TYPE',MODEI,' MODEJ',
4 ' FORCE',DISPLACEMENT,STIFFNESS')
492 FORMAT (1I0,15,2X,AS,2I6,1P,3G15.6)
C ***** ADJUST P FOR UNBALANCED LOAD P
P=P-PT
RETURN
C ***** DETERMINE TOTAL FORCES, ENERGIES ECT
500 CONTINUE
IF (ICOMN.NE.1 .AND. ICOMN.NE.2) THEN
  IF (PRINT.AND.FIRST) WRITE(108,491) STEPID
C ***** TOTAL FORCES, DISPLACEMENTS AND VELOCITIES
PT=PT-P
RT=RT-R
VT=VT-V
IF (ABS(PT).GT.ABS(PMAX(1))) THEN
  PMAX(1)=PT
  PMAX(2)=RT
  ENDF
C ***** PRINT DATA
HH= H/LENGTH
IF (PRINT) WRITE(108,492) IELNO,TYPE(KTYPE),MODEI,MODEJ,
  F,PT,RT,HH
ELSE IF (ICOMN.EQ.1 .OR. ICOMN.EQ.2) THEN
  PT=RSAP(1)
  IF (PRINT.AND.FIRST) WRITE(108,497) STEPID
  IF (PRINT) WRITE(108,493) IELNO,TYPE(KTYPE),MODEI,MODEJ,PT
  ENDF
C ***** 497 FORMAT (' SPRING FORCES...',5X,A/
4 ' ELEMENT LOAD',7I7,' TYPE',MODEI,' MODEJ',
4 ' FORCE')
C ***** TRANSFER TEMPORARY HYSTERESIS DATA TO PERMANENT STORAGE...
ISE2=ISE/2
IF (.NOT.ELSTIC) THEN
  DO 510 I=1,ISE2
  SE(I)=SE(I)-SE(2-ISE2)
510
  ENDF
RETURN
C ***** DETERMINE FIXED END FORCES, AND ADD TO FORCE
600 RETURN
C ***** FIXED END FORCES ARE NOT AVAILABLE FOR THE SPRING
C ***** WRITE MEMBER FORCES TO OUTPUT FILE
700 CONTINUE
CALL MATLIB(4,LSTYP,FALSE,ESE,EPSE,DUCT,EXCR,
  FT,F,RT,R,0,0,LELEM,LMAT,MAT,MATYP,SE,IELNO,DAMAGE)
C ***** WRITE (IUNIT,710) MODEI,MODEJ,KTYPE,FT,RT,H,ESE,EPSE,
( DUCT(1),EXCR(1),I,-1,3)
710 FORMAT (3I6,1P,5G12.4/6G12.4)
C ***** RETURN
C ***** READ AND PRINT MEMBER FORCES FROM OUTPUT FILE
800 CONTINUE
BACKSPACE(IUNIT)
READ (IUNIT,' ') ITYPE,IELNO,IWRITE,TO,DT
ISTEP=IWRITE
C ***** 810 READ (IUNIT,815,END=830) MODEI,MODEJ,KTYPE,FT,RT,H,ESE,PSE,
(WORK(1),I,-1,6)
815 FORMAT (3I6,5G12.4/6G12.4)
ISTEP=ISTEP+IWRITE
T=TO-DT*ISTEP
IF (ISTEP.EQ.0) WRITE(108,805) MODEI,MODEJ,TYPE(KTYPE)
IF (MOD(ISTEP,INC).EQ.0) WRITE(108,820) ISTEP,T,FT,RT,
H,ESE,PSE
GO TO 810
C ***** 805 FORMAT (' SPRING FORCES...',/
4 5X,MODEI,' 16,5X,MODEJ',' 16,5X,'SPRING TYPE',A,/,/
4 4X,'STEP TIME',/
4 ' FORCE',DISPLACEMENT,STIFFNESS',/
4 ' ESE',PSE')
820 FORMAT (1I0,1P,6I5.5,6G13.5)
C ***** 830 WRITE(108,840) (WORK(I),I=1,6)
840 FORMAT ('/T10',MAXIMUM DUCTILITIES AND EXCURSION RATIOS'/
4 T10,/,/
4 T15,'DISPLACEMENT DEFINATION: U1-',1P,G12.4,5X,'E1-',G12.4/
4 T15,'ENERGY DEFINATION #1: U2-',1P,G12.4,5X,'E2-',G12.4/
4 T15,'ENERGY DEFINATION #2: U3-',1P,G12.4,5X,'E3-',G12.4/
RETURN
C ***** DETERMINE THE LENGTH OF THE MATERIAL DATA
900 CONTINUE
MHST=1
ISE=0
MAT = RINPUT(1)
CALL MATLIB(0,LSTYP,FALSE,ESE,EPSE,DUCT,EXCR,
  FT,F,RT,R,0,0,LELEM,LMAT,MAT,MATYP,SE(I)SE1,IELNO,DAMAGE)
ISE=ISE+LELEM
C ***** DOUBLE THE STORAGE, TO ALLOW FOR TEMPORARY VALUES.
ISE2=ISE*2
RETURN
C ***** DETERMINE THE STRAIN ENERGY
1000 CONTINUE
C ***** DUCTILITIES AND EXCURSION RATIO'S
1100 CONTINUE
OPTION 10 AND 11 HAVE THE SAME CODE FOR THIS ELEMENT.
THE ENERGY WAS CALCULATED WITH THE LAST CALL FOR INCREMENTAL FORCE
THE TOTAL FORCES HAVE NOT BEEN CALLED YET, CALL FOR THE ENERGIES
IN THE MATRIX BE BEGINNING AT ADDRESS ISE21.
ISE21=ISE/2-1
CALL MATLIB(4,LSTYP,FALSE,ESE,EPSE,DUCT,EXCR,
  FT,F,RT,R,0,0,LELEM,LMAT,MAT,MATYP,SE(I)SE21,IELNO,DAMAGE)
IF ( DUCFAG.EQ.0 ) THEN
  IF (ABS(DUCT(1)) LT 1) DUCFAG=0

```

IF (ABS(DUCT(1)) .GE. 1) DUCFAC=ABS(DUCT(1))
ENDIF
C
IP (ELSTIC) EBE=(PT-F)*R*(RT-R)*0.50
IP (PRINT AND FIRST) WRITE(108,1191)
IP (PRINT) WRITE(108,1192) IELNO,DUCT(1),EXCR(1),I-1,3)
1191 FORMAT (/,' SPRING DUCTILITIES AND EXCURSION RATIOS.....,5X/
& ' ELEMENT',I2,' : DUCTILITY DEFINITION #1 :',
& ' : DUCTILITY DEFINITION #2 :',
& ' : DUCTILITY DEFINITION #3 :',/, I2,
& ' DUCTILITY EXCURSION ',
& ' DUCTILITY EXCURSION ',
& ' DUCTILITY EXCURSION ')
1192 FORMAT (110,1P,6G15.6)
C
RETURN
C----- MAXIMUM FORCES
1200 CONTINUE
IP(FIRST) WRITE(108,491) 'MAXIMUM LOAD AND DISPL AT MAXIMUM LOAD'
& ' // NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY'
WRITE(108,492) IELNO,G,TYPE(KTYPE),NODI,NODEI,PHAX(1),PHAX(2)
RETURN
C----- RESET INTERNAL FORCES
1300 CONTINUE
C----- ZERO MATRICES
F = 0
FT=0
R = 0
RT=0
V = 0
VT=0
PHAX(1)=0
PHAX(2)=0
DO 1310 I=1,ISE
SE(I)=0
1310 WRITE(108,1330) IELNO
1330 FORMAT(' SPRING #', I, ' ELEMENT #', I5,
& ' INTERNAL FORCES AND HYPERSTRESS MODELS ARE RESET TO ZERO')
& CALL MATLIB(2,LSHTYP,FALSE,EBE,EPSE,DUCT,EXCR,
& PT,F,RT,R,0.,0.,LELEN,LMAT,MAT,MATVP,SE,IELNO,DAMAGE)
RETURN
C----- DAMAGE INDEX
1400 IF (ELSTIC) RETURN
C
PHAX1=PHAX(1)
PHAX2=PHAX(2)
CALL MATLIB(5,LSHTYP,FALSE,EBE,EPSE,DUCT,EXCR,
& PHAX1,PHAX2,0.,0.,0.,LELEN,LMAT,MAT,MATVP,SE,IELNO,DAMAGE)
IP (PRINT AND FIRST) WRITE(108,1410)
1410 FORMAT (/,' ELEMENT',I2,' DAMAGE INDEX F MAX ',
& D MAX EBE NODI
& IP (PRINT) WRITE(108,1420) IELNO,DAMAGE,PHAX1,PHAX2,EBE,EPSE
1420 FORMAT (110,1P,6G15.6)
C
DAMAGE=DAMAGE*(EBE+EPSE)
RETURN
END
C----- SUBROUTINE ELE12
& (IOPT, FIRST, PRINT, IREL, NAME,
& ID, IDOP, COORD, COSINE, JTCOS,
& CONST, DISPL, RINPUT, PUB, EBE,
& EPSE, DAMAGE, IZLOOP, R,
& RT, PT, LM, CTS,
& CTE, BS, NODI, NODEI,
& JOINTI, JOINTJ, REL, LENGTH,
& ISE, ENGDAT, PHAX, S, A,
& MAT, SE, STIFF)
C
IMPLICIT REAL(A-H,O-Z)
COMMON /ISA/ICOM,RSAP(12)
REAL ICOM,LENGTH
INTEGER REL
LOGICAL FIRST,PRINT,FALSE,BTEST
CHARACTER*80 NAME
\$INCLUDE 'COMMON'
DIMENSION IDOP(MAXNOD,6),COORD(MAXNOD,6),COSINE(3,3,NCOS)
DIMENSION CONST(MAXNOD,6), ID(MAXNOD), JTCOS(MAXNOD)
DIMENSION ASAT(12,12),SAT(12,12),STIFF(78)
DIMENSION A(12,12), S(12,12),LM(12),PHAX(144)
DIMENSION CTS(3, 3),CTE(3, 3),VX(12),VY(3)
DIMENSION BE(3, 3),BE(3, 3),SE(ISE),ENGDAT(2)
DIMENSION N(3),DISP(NODP,NLGD),FT(12),PT(12),PUB(NODP)
DIMENSION RINPUT(100),DUCT(3),EXCR(6),P(12),D(12),DL(12)
C-----
C----- GLOBAL VARIABLES
C IOPT = 1, INITIALIZE BEAM COLUMN ELEMENT
C = 2, CALCULATE GEOMETRIC STIFFNESS
C = 3, FORM STIFFNESS
C = 4, CALCULATE INCREMENTAL FORCES
C = 5, CALCULATE TOTAL FORCES AND ENERGIES
C IUNIT = OUTPUT FILE UNIT # FOR PRINTING OUTPUT
C FIRST = FLAG, FIRST= TRUE, PRINT HEADERS
C PRINT = FLAG, PRINT= TRUE, PRINT DATA
C NODP = # OF GLOBAL DOP
C NLGD = # OF LOAD COMBINATIONS
C MAXNOD = # ROW DIMENSION OF NODE ARRAY
C NNODE = # OF NODES
C ID = ARRAY OF EXTERNAL NODE NUMBERS
C IDOP = ARRAY OF DEGREES OF FREEDOM
C COORD = ARRAY OF COORDINATES
C COSINE = ARRAY OF DIRECTION COSINES OF NODES
C CONST = ARRAY OF CONSTRAINT TRANSFORMATIONS
C DISPL = GLOBAL DISPLACEMENT MATRIX
C IZMAT = LOCATION OF MATERIAL DATA IN GLOBAL STORAGE VECTOR
C NAME = ELEMENT NAME
C IELNO = ELEMENT NUMBER
C RINPUT = INPUT DATA
C STIFF = OUTPUT ELEMENT STIFFNESS (UPPER TRIANGULAR FORM)
C LM = OUTPUT LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C----- ELEMENT VARIABLES
C H,D,AT,AO,C,BI,BJ,S2,S6,S12,S33,S35,S311,D
C = INDIVIDUAL TERMS IN THE ELEMENT STIFFNESS (8) MATRIX
C R = INCREMENTAL DISPLACEMENTS
C RT = TOTAL DISPLACEMENTS
C F = INCREMENTAL FORCES
C FT = TOTAL FORCES
C LM = LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C CTS = LOCAL TO GLOBAL ROTATION MATRIX START
C CTE = LOCAL TO GLOBAL ROTATION MATRIX END
C BS = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX START
C BE = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX END
C STIFF = ELEMENT STIFFNESS
C IELTYP = ELEMENT TYPE
C NODEI = EXTERNAL JOINT NUMBER, END I
C NODEJ = EXTERNAL JOINT NUMBER, END J
C JOINTI = INTERNAL JOINT NUMBER, END I
C JOINTJ = INTERNAL JOINT NUMBER, END J
C----- TEMPORARY VARIABLES
C VX = VECTOR DEFINING THE ELEMENT X AXIS
C VY = VECTOR DEFINING THE ELEMENT Y AXIS
C A = GLOBAL TO LOCAL FORCE TRANSFORMATION MATRIX
C S = ELEMENT STIFFNESS AT LOCAL COORD, AT ENDS OF FLEXIBLE PART
C SAT = PRODUCT OF S*A
C ASAT = PRODUCT OF A*SAT
C----- CHOOSE OPTION
IF (IOPT.LT.1) GR.IOPT=GT.14) THEN
WRITE(108,*) 'INVALID OPTION IN ELE-09, IOPT=',IOPT
STOP
ENDIF
C
GO TO (100,200,300,400,500,600,700,800,900,1000,
& 1100,1200,1300,1400) IOPT
C
100 CONTINUE
C----- ZERO STRAIN ENERGY
ENGDAT(1) = 0
ENGDAT(2) = 0
C----- INTERPERTATE INPUT DATA
IELTYP = 12
MAT = RINPUT(1)
NODEI = RINPUT(2)
NODEJ = RINPUT(3)
VY(1) = RINPUT(4)
VY(2) = RINPUT(5)
VY(3) = RINPUT(6)
XS = RINPUT(7)
XE = RINPUT(8)
C----- GET INTERNAL NODE # ASSOCIATED WITH EXTERNAL NODE #'S
JOINTI=IQUICK(NODEI, ID,NNODE)
JOINTJ=JQUICK(NODEJ, ID,NNODE)
C----- PRINT DATA FOR PLOTTING
IP (BTEST(18UG,2)) THEN
WRITE(7,10) NODEI, NODEJ, (COORD(JOINTI, I), I=1,3),
& (COORD(JOINTJ, J), J=1,3)
10 FORMAT(15,1X,15,1P,6G12.4)
ENDIF
C-----
C GET GLOBAL TO LOCAL TRANSFORMATION MATRIX
C----- DEFINE VECTORS X, LENGTH
VX(1)=COORD(JOINTI,1)-COORD(JOINTJ,1)
VX(2)=COORD(JOINTI,2)-COORD(JOINTJ,2)
VX(3)=COORD(JOINTI,3)-COORD(JOINTJ,3)
LENGTH=SQRT(VX(1)**2+VX(2)**2+VX(3)**2)-XS-XE
SE(79)=LENGTH
C
IF (LENGTH.LE.0) THEN
WRITE(108,101) IELNO,JOINTI,JOINTJ,LENGTH
101 FORMAT(1X,30,' ','SE',/,'RC',LENGTH IS LE 0',
& 5X, 'IELNO=',15,5X, 'JOINTI=',15,5X, 'JOINTJ=',15,
& 5X, 'LENGTH',1P,6G15.6/
& 5X, 'REVERSE INPUT',
& 'LENGTH=-1.0'
ENDIF
C----- GET LOCAL TO GLOBAL TRANSFORMATION MATRICES CT AND B
ICS=JTCOS(JOINTI)
ICE=JTCOS(JOINTJ)
CALL ROTXYZ(VX,VY,COSINE(1,1,ICS),CTS, XS,0.,0.,BS,
& CALL ROTXYZ(VY,VY,COSINE(1,1,ICE),CTE, XE,0.,0.,BE,
& CONST(JOINTJ,1),MAJNOD)
C----- ZERO MATRICES
DO 40 I=1,12
PHAX(I)=0
F(I)=0
R(I)=0
RT(I)=0
DO 40 J=1,12
S(I,J)=0
40 A(I,J)=0
C----- ASSEMBLE TRANSFORMATION MATRIX A
DO 50 I=1,3
DO 50 J=1,3
A(I, J)=CTS(I,J)
A(I+3, J+3)=CTB(I,J)
A(I+6, J+6)=CTE(I,J)
A(I+9, J+6)=CTE(I, J)
A(I+3, J)=BS(I,J)
A(I+9, J+6)=BE(I,J)
50 A(I+9, J+6)=BE(I,J)
C----- GET MATERIAL PROPERTIES
FALSE=FALSE
LSHTYP=0
& CALL MATLIB(2,LSHTYP,FALSE,EBE,EPSE,DUCT,EXCR,PT,F,RT,R,
& 0.,0.,LELEN,LMAT,MAT,MATVP,SE,IELNO,DAMAGE)
IP (MATTVP.NE.12) THEN
WRITE(108,190) IELNO,MATTVP,NODEI,NODEJ
190 FORMAT (' ELEMENT',I6,' INCOMPATIBLE MATERIAL PROP. #',
& 15,' START JOINT',15,' END JOINT',15,/,
& ' THE ELEMENT IS REJECTED AND ERROR CHECKING CONTINUES')
GO TO 191
ENDIF
C-----
EBE=0
EPSE=0
C----- NO END RELEASE FOR THIS ELEMENT
REL=0
C----- SET STIFFNESS COEFFICIENTS...
C----- ASSEMBLE LOCAL STIFFNESS MATRIX
K=0
DO 77 I=1,12
DO 77 J=1,12
K=K+1
IF (ABS(SE(K)) .LT. 1.E-6) SE(K)=0
S(I,J)=SE(K)
S(J,I)=S(I,J)
77 CONTINUE
C----- CALCULATE SAT
CALL MULTM(2,12,SAT,S,A)
C----- CALCULATE ASAT
CALL MULTM(0,12,ASAT,A,SAT)
C----- DECREASES OF FREEDOM
REMOVE DOP'S THAT ARE CONSTRAINED TO THE SAME DOP...
DO 75 I=1,6
IF (IDOP(JOINTI,I).NE.IDOP(JOINTJ,I)) THEN
LMT(I)=IDOP(JOINTI,I)
ELSE
LMT(I)=0
LMT(I+6)=0
75 CONTINUE
ENDIF
C----- CONVERT TO HALF STORAGE MODE BY COLUMNS
L=0
IEBOP=0
DO 90 I=1,12
IF (LMT(I).EQ.0) GO TO 90

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IELDOP=IELDOP-1
LM(IELDOP)-LMT(I)
DO 80 J=1,1
  IF (LMT(J).EQ.0) GO TO 80
  L=L-1
  STIFF(L)-ASAT(I,J)
  CONTINUE
80 CONTINUE
90 CONTINUE
C----- RELEASE UNUSED STORAGE
180 IREL=78-L
C
C PRINT COLUMN DATA
IF (FIRST) WRITE(108,192)
191 WRITE(108,193) NAME,IELNO,MAT,MODEI,MODEJ,LENGTH,
  & VY(1),VY(2),VY(3),XS,XE
C
192 FORMAT(' ELEMENT 12. STABILITY ELEMENT'//
  & T22,'M MATL START END',3X,'LENGTH',3X,
  & 11(' '),Y-AXIS
  & 12(' '),START DIST END DIST ')
193 FORMAT(1X,A15,316,1X,26,2X,1P,G10.4,0P,
  & F9.5,' I',SP,F9.5,' J',F9.5,' K',1P,SS,2G12.4)
RETURN
C----- DETERMINE GEOMETRIC STIFF -----
200 CONTINUE
C----- GEOMETRIC STIFFNESS IS INCLUDED IN THE ELEMENT STIFFNESS.
C NO NEED TO CALCULATE
RETURN
C----- DETERMINE STIFFNESS -----
300 CONTINUE
C----- STIFFNESS CHANGES AT EACH STEP FOR STABILITY ELEMENT
C----- LOCAL STIFFNESS MATRIX
K=0
DO 87 I=1,12
  DO 87 J=1,12
    K=K+1
    IF (ABS(SE(K)) .LT. 1.E-6) SE(K)=0
    S(I,J)=SE(K)
    S(J,I)=S(I,J)
87 CONTINUE
C----- CALCULATE SAT
CALL MULTR(3,12,SAT,S,A)
C----- CALCULATE ASAT
CALL MULTR(0,12,ASAT,A,SAT)
C----- DEGREES OF FREEDOM
REMOVE DOP'S THAT ARE CONSTRAINED TO THE SAME DOP...
DO 12 I=1,6
  IF (IDOP(JOINTI,I) .NE. IDOP(JOINTJ,I)) THEN
    LMT(I)=IDOP(JOINTI,I)
    LMT(J)=IDOP(JOINTJ,I)
  ELSE
    LMT(I)=0
    LMT(J)=0
12 CONTINUE
C----- CONVERT TO HALF STORAGE MODE BY COLUMNS
L=0
IELDOP=0
DO 92 I=1,12
  IF (LMT(I).EQ.0) GO TO 92
  IELDOP=IELDOP+1
  LM(IELDOP)=LMT(I)
  DO 92 J=1,12
    IF (LMT(J).EQ.0) GO TO 82
    L=L+1
    STIFF(L)-ASAT(I,J)
    CONTINUE
82 CONTINUE
92 CONTINUE
RETURN
C----- DETERMINE INCREMENTAL FORCES -----
400 CONTINUE
IF (PRINT.AND.FIRST) WRITE(108,491) STEPID
C----- LOOP FOR EACH LOAD
DO 430 LOAD=1,NLOAD
C----- TRANSFORM FRAME DISPL INTO LOCAL DISPL
DO 410 I=1,3
  R(I)=0.
  R(I+3)=0.
  R(I+6)=0.
  R(I+9)=0.
  DO 410 J=1,3
    D1=DISPL(IDOP(JOINTI,J),LOAD)
    D2=DISPL(IDOP(JOINTI,J+3),LOAD)
    D3=DISPL(IDOP(JOINTI,J+6),LOAD)
    D4=DISPL(IDOP(JOINTI,J+9),LOAD)
    R(I)=R(I)+CTS(J,I)*D1+BS(I,1)*D2
    R(I+3)=R(I+3)+CTS(J,I)*D3+BS(I,2)*D4
    R(I+6)=R(I+6)+CTS(J,I)*D3+BS(I,3)*D4
    R(I+9)=R(I+9)+CTS(J,I)*D4
410 CONTINUE
C----- INCREMENTAL FORCES
DO 435 I=1,12
  P(I)=0
  DO 435 K=1,12
    P(I)=P(I)+S(I,K)*R(K)
435 CONTINUE
C----- DETERMINE MAXIMUM VALUES FOR MULTIPLE LOAD CASES
IF (NLOAD.GT.1) THEN
  DO 425 I=1,12
    IF (ABS(P(I)).GT.ABS(PMAX(I*12-I))) THEN
      DO 426 J=0,11
        PMAX(I*12-I+J)=P(I)
426 CONTINUE
425 CONTINUE
ENDIF
C----- PRINT FORCES AND DEFORMATIONS
SP=SE(84)
FLP=SE(86)
IF (PRINT) WRITE(108,493) IELNO,LOAD,MODEI,(R(J),J=1,6),
  & NODEJ,(R(J),J=7,12),FLP
IF (PRINT) WRITE(108,492) IELNO,LOAD,MODEI,(P(J),J=1,6),
  & NODEJ,(P(J),J=7,12),SP
430 CONTINUE
C----- THIS IS FOR SOLOSA USE ONLY
IF (ICOMN.EQ.1 .OR. ICOMN.EQ.2) THEN
  DO 431 I=1,12
    R8AF(I)=P(I)
431 CONTINUE
ENDIF
CALL MATLIB TO GET STIFFNESS
THIS IS PERFORMED ONLY FOR INELASTIC ANALYSIS
IF (.NOT. ELASTIC) THEN
  DO 93 I=1,12
    P(I)=PT(I)-P(I)
    D(I)=RT(I)-R(I)
    FL(I)=PT(I)
93 CONTINUE
DL(I)=RT(I)
CONTINUE
522 TRANSFER PERMANENT HYSTERESIS DATA TO TEMPORARY STORAGE
ISE2=ISE2/2
DC 522 J=1,ISE2
  SE(I)=ISE2-SE(I)
C CALL MATLIB(LSTTYP,FALSE,ESE,EPSE,DUCT,ENCR,P,PL,D,DL,
  & 0.,0.,LELEM,LMAT,MAT,MATTYP,SE(ISE2-1),IELNO,DAMAGE)
NEWX=.TRUE.
ENDIF
C----- CALCULATE THE UNBALANCED MEMBER FORCES ON THE JOINT
THE EXACT MEMBER FORCES ARE DETERMINED FROM HYSTERESIS MODEL
BASED ON DISPLACEMENT D, BECAUSE NO HYSTERESIS MODEL FOR MAT12,
THE MEMBER PUB ARE NOT CALCULATED IN THIS ELEMENT
DO 440 I=1,3
  J1=IDOP(JOINTI,I)
  J2=IDOP(JOINTI,I+3)
  J3=IDOP(JOINTI,I+6)
  DO 440 J=1,3
    PUB(I1)=PUB(I1)+CTS(I,J)*D
    PUB(I2)=PUB(I2)+BS(I,J)*D
    PUB(I3)=PUB(I3)+CTS(I,J)*D
    PUB(J1)=PUB(J1)+CTE(I,J)*D
    PUB(J2)=PUB(J2)+CTE(I,J)*D
440 CONTINUE
491 FORMAT(' STABILITY ELEMENT FORCES...S/R,X/A//
  & ELEMENT LOAD NODE',7X,'AXIAL',11X,'FY',13X,'FZ',
  & 11X,'TORSION',10X,'MY',13X,'M2 P.LP,SP ')
492 FORMAT ('/110,15,2X,'FORCE',16,1P,6G15.6,
  & /15X,2X,'FORCE',16, 6G15.6,1X,G10.3)
493 FORMAT ('/110,15,2X,'DISP',16,1P,6G15.6,
  & /15X,2X,'DISP',16, 6G15.6,1X,G10.3)
494 FORMAT (7X,A)
RETURN
C----- DETERMINE TOTAL FORCES -----
500 CONTINUE
IF (PRINT.AND.FIRST) WRITE(108,491) STEPID
IF (ICOMN.NE.1 .AND. ICOMN.NE.2) THEN
C----- TOTAL FORCES AND DISPLACEMENTS
DO 510 I=1,12
  PT(I)=PT(I)+P(I)
  RT(I)=RT(I)+R(I)
510 CONTINUE
DO 512 I=1,12
  IF (ABS(PT(I)).GT.ABS(PMAX(I*12-I))) THEN
    DO 511 J=0,11
      PMAX(I*12-I+J)=PT(I)
511 CONTINUE
512 CONTINUE
513 PT(I)=R8AF(I)
ENDIF
LOAD=1
ISE2=ISE2/2
SP=SE(ISE2+84)
FLP=SE(ISE2+86)
IF (PRINT) WRITE(108,493) IELNO,LOAD,MODEI,(R(J),J=1,6),
  & NODEJ,(R(J),J=7,12),FLP
IF (PRINT) WRITE(108,492) IELNO,LOAD,MODEI,(P(J),J=1,6),
  & NODEJ,(P(J),J=7,12),SP
C----- TRANSFER TEMPORARY HYSTERESIS DATA TO PERMANENT STORAGE
ISE2=ISE2/2
IF (.NOT. ELASTIC) THEN
  DO 415 I=1,ISE2
    SE(I)=SE(I)+ISE2
415 CONTINUE
ENDIF
RETURN
C----- DETERMINE FIXED END FORCES, AND ADD TO FORCE-----
600 CONTINUE
C----- FIXED END FORCES ARE NOT AVAILABLE FOR STABILITY ELEMENT
RETURN
C----- WRITE MEMBER FORCES TO OUTPUT FILE -----
700 CONTINUE
SP=SE(84)
FLP=SE(86)
WRITE (IUNIT,710) NODEI,(PT(J),J=1,6),NODEJ,(PT(J),J=7,12),SP
WRITE (IUNIT,710) NODEI,(RT(J),J=1,6),NODEJ,(RT(J),J=7,12),FLP
710 FORMAT (16,1P,6G12.5,16,1P,6G12.5,1X,G10.3)
RETURN
C----- READ AND PRINT MEMBER FORCES FROM OUTPUT FILE -----
800 CONTINUE
BACKSPACE(IUNIT)
READ (IUNIT,' ') ITYPE,IELNO,IWRITE,TO,DT
WRITE(108,805)
ISTEP=IWRITE
810 READ(IUNIT,815,END=830)
  & NODEI,(PT(J),J=1,6),NODEJ,(PT(J),J=7,12),SP
  & NODEI,(RT(J),J=1,6),NODEJ,(RT(J),J=7,12),FLP
815 FORMAT (16,6G12.5,16,6G12.5,1X,G10.3)
ISTEP=ISTEP+IWRITE
7-TO=DT-ISTEP
IF (MOD(ISTEP,INCI.EQ.5) THEN
  WRITE(108,820) ISTEP,T,MODEI,(PT(J),J=1,6),
    & NODEJ,(PT(J),J=7,12),SP
  WRITE(108,920) (RT(J),J=1,12),FLP
  ENDF
  GO TO 810
805 FORMAT (' STABILITY ELEMENT FORCES.../
  & STEP TIME NODE',7X,'AXIAL',11X,'FY',13X,'FZ',
  & 11X,'TORSION',10X,'MY',13X,'M2 P.LP,SP ')
820 FORMAT ('/110,1P,G15.6,1P,1P,6G15.6,25X,16,6G15.6,1X,G10.3)
830 FORMAT ('25X,'DISP',1P,6G15.6,31X,6G15.6,1X,G10.3)
830 RETURN
C----- DETERMINE THE LENGTH OF THE MATERIAL DATA
ISE=0
MAT = RINPUT(1)
CALL MATLIB(LSTTYP,FALSE,ESE,EPSE,DUCT,ENCR,PT,P,R,T,R,
  & 0.,0.,LELEM,LMAT,MAT,MATTYP,SE,IELNO,DAMAGE)
ISE=ISE+LELEM
C----- DOUBLE THE STORAGE, TO ALLOW FOR TEMPORARY VALUES
ISE=ISE*2
RETURN
C----- DETERMINE THE TOTAL STRAIN ENERGY
1000 CONTINUE
ESEE=0
EPSEE=0
C /* ENERGY FORMULATION FOR ELE12 ELEMENT IS NOT AVAILABLE */

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DO 1010 I=1,12
ESE=SE*(FT(I)+P(I))*(R(I)+RT(I))/2.
ENDDAT(1)=ESE
ENDDAT(2)=0
C
C
C----- DUCTILITIES AND EXCURSION RATIOS
C
1100 RETURN
C
C----- PRINT MAXIMUM ELEMENT LOADS
C
1200 CONTINUE
LOAD2=0
IF (FIRST) WRITE(108,494)
& ' COLUMN (LOAD) REPRESENTS THE DOF WHICH HAS',
& ' MAXIMUM VALUE'
IF (FIRST) WRITE(108,491) ' MAXIMUM VALUES FOR ALL STEPS '
& ' NOTE: MAXIMUM VALUES WITH THE OTHER DOFS FORCES'
& ' ARE PRINT OUT AT THE SAME TIME'
DO 309 I=1,11
LOAD2=I-1
IF (PMAX(I+12+I) EQ 0. ) GO TO 309
WRITE(108,492) IELNO,LOAD2,MODE1,(PMAX(I+12+I),J=1,6),
& MODEJ,(PMAX(I+12+I),J=7,12)
309 CONTINUE
C
C----- RESET ELEMENT FORCES
C
1300 CONTINUE
C----- ZERO MATRICES
DO 1340 I=1,12
PMAX(I+12+I)=0
F(I)=0
P(I)=0
R(I)=0
RT(I)=0
DO 1310 I=1,12
SE(I)=SE(I)+LENGTH
C
WRITE(108,1330) IELNO
1330 FORMAT (' STABILITY ELEMENT ..... ELEMENT #',I6,
& ' INTERNAL FORCES AND HYSTERESIS MODELS ARE RESET TO ZERO')
& ' FALSE- FALSE.
& ' LSTYP=0
& ' CALL MATLIB(2, LSTYP, FLEM, SE, EPSE, DUCT, EXCR, PT, F, RT, R,
& ' & CALL MATLIB(2, LSTYP, FLEM, SE, EPSE, DUCT, SE, IELNO, DAMAGE)
C
RETURN
C----- DAMAGE INDEX
C
1400 DAMAGE = 0.
C
RETURN
END
C----- SUBROUTINE ELEM09
& (IOPT, FIRST, PRINT, NAME, AXIAL,
& ID, IDOP, COORD, COSINE, JTCOS,
& CONST, COORD, REL,
& KGDATA, PGEOM, IELD, IELNO,
& RINPUT, PUB, ESE, EPSE, DAMAGE,
& DUCTPAG, IELTYP, IELDOP, MYXZ, STY,
& ISEMI, PKG, R,
& FT, LM, CTS, CT, BS,
& BE, NODE1, NODEJ, JOINT1, JOINTJ,
& REL, LENGTH, KODOP, LKG, LMKG,
& ISE, ENDDAT, PMAX, IAPAG, STBPAG,
& MATMLI, S23, ROIEL1, A,
& SE, STIFF, FEM )
C
COMMON /IDSTEP/ JSTEP
COMMON /BSA/ ICONF,RSAP(12)
REAL ICONF,LENGTH
INTEGER IAPAG,STBPAG,REL
LOGICAL FIRST,PRINT,FEMPLG,FALSE,AXIAL,BTEST
CHARACTER*80 NAME
INCLUDE 'COMMON'
DIMENSION KGDATA(3),S23(0:11),IDOP(MAXNOE,6),COORD(MAXNOE,6)
DIMENSION COORD(3),CONST(MAXNOE,6),ID(MAXNOE)
DIMENSION ASAT(12,12),SAT(12,12),STIFF(156),JTCOS(MAXNOE)
DIMENSION A(12,12),S(12,12),LM(12),LMT(12),LMKX(12),PMAX(144)
DIMENSION CTS(3,3),CTE(3,3),VK(3),VT(3)
DIMENSION BE(1,3),BE(1,3),SE(18),ENDDAT(2),STY(10)
DIMENSION B(12),DISPL(NDOP,NLOAD),P(12),RT(12),PT(12),FUB(NDOP)
DIMENSION FORCE(NDOP,NLOAD),IELD(5,MAXELD),ELD(12,MAXELD)
DIMENSION RINPUT(100),FEM(12,NLOAD),GFEM(12),TYP(6)
DIMENSION DUCT(1,6),EXCR(6,6),P(6),V(12),VL(12),EFUB(12),UP(12)
DIMENSION MYXZ(6),ISEMI(6),MATMLI(6),ROIEL1(4),RIEL1(4)
C
C----- VARIABLES
C----- GLOBAL VARIABLES
C IOPT - 1, INITIALIZE BEAM COLUMN ELEMENT
& 2, CALCULATE GEOMETRIC STIFFNESS
& 3, FORM STIFFNESS
& 4, CALCULATE INCREMENTAL FORCES
& 5, CALCULATE TOTAL FORCES AND ENERGIES
C UNIT - OUTPUT FILE UNIT # FOR PRINTING OUTPUT
C FIRST - FLAG, FIRST=TRUE, PRINT HEADERS
C PRINT - FLAG, PRINT=TRUE, PRINT DATA
C NDOP - # OF GLOBAL DOF
C NLOAD - # OF LOAD COMBINATIONS
C MAXNOE - # ROW DIMENSION OF NODE ARRAY
C NNODE - # OF NODES
C ID - ARRAY OF EXTERNAL NODE NUMBERS
C IDOP - ARRAY OF DEGREES OF FREEDOM
C COORD - ARRAY OF COORDINATES
C COSINE - ARRAY OF DIRECTION COSINES OF NODES
C CONST - ARRAY OF CONSTRAINT TRANSFORMATIONS
C DISPL - GLOBAL DISPLACEMENT MATRIX
C IZMAT - LOCATION OF MATERIAL DATA IN GLOBAL STORAGE VECTOR
C NAME - ELEMENT NAME
C IELNO - ELEMENT NUMBER
C RINPUT - INPUT DATA
C STIFF - OUTPUT ELEMENT STIFFNESS (UPPER TRIANGULAR FORM)
C LM - OUTPUT LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C----- ELEMENT VARIABLES
C H,O,AJ,AJ,C,BI,BJ,S22,S26,S212,S33,S35,S311,D
& - INDIVIDUAL TERMS IN THE ELEMENT STIFFNESS (S) MATRIX
C PKG - INPUT LOAD FOR FORMING GEOMETRIC STIFFNESS MATRIX
C R - INCREMENTAL DISPLACEMENTS
C RT - TOTAL DISPLACEMENTS
C FT - INCREMENTAL FORCES
C LM - TOTAL FORCES
C LOCAL - LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C CTS - LOCAL TO GLOBAL ROTATION MATRIX START
C CTE - LOCAL TO GLOBAL ROTATION MATRIX END
C BS - LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX START
& BS - LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX END
C STIFF - ELEMENT STIFFNESS
C IELTYP - ELEMENT TYPE = 9
C NODE1 - EXTERNAL JOINT NUMBER, END J
& NODEJ - EXTERNAL JOINT NUMBER, END J
& JOINT1 - INTERNAL JOINT NUMBER, END I
& JOINTJ - INTERNAL JOINT NUMBER, END J
&----- TEMPORARY VARIABLES
C VX - VECTOR DEFINING THE ELEMENT X AXIS
C VY - VECTOR DEFINING THE ELEMENT Y AXIS

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IF (LL.EQ.2) L0=1
IF (MYX(LL-1) EQ. 9 .AND. MYX(LL-2) EQ. 9) .OR.
  (MYX(LL-1) EQ. 11 .AND. MYX(LL-2) EQ. 11) .OR.
  (MYX(LL-1) EQ. 6 .AND. MYX(LL-2) EQ. 6) THEN
  STY(LL-1)-SE(ISEMLI(LL-1))
  STY(LL-2)-SE(ISEMLI(LL-2))
  IF MYX(LL-1) EQ. 9 THEN
    SA1-SE(ISEMLI(LL-1)+60)
    SA2-SE(ISEMLI(LL-2)+60)
  ELSE IF MYX(LL-1) EQ. 11 THEN
    SA1-SE(ISEMLI(LL-1)+30)
    SA2-SE(ISEMLI(LL-2)+30)
  ELSE IF MYX(LL-1) EQ. 6 THEN
    SA1-SE(ISEMLI(LL-1)+47)/LENGTH
    SA2-SE(ISEMLI(LL-2)+47)/LENGTH
  STY(LL-1)-STY(LL-1)/LENGTH
  STY(LL-2)-STY(LL-2)/LENGTH
  EI-SE(ISEMLI(LL-1)+48)
  TERM1=(LENGTH**2)/(12*EI*EI)
  TERM2=LENGTH/(3*EI)
ENDIF
IF (MYX(LL-1) NE. 6) THEN
  TERM1=3./(SA1**2)
  TERM2=2./SA1
ENDIF
FLX1=0.
IF (SA1 NE. 0.) FLX1=(1/STY(LL-1))/(1/SA1)
FLX2=0.
IF (SA2 NE. 0.) FLX2=(1/STY(LL-2))/(1/SA2)
DCOP=TERM1 + TERM2*(FLX1+FLX2) + FLX1*FLX2
DCOP=1/DCOP
S23(LL-8)=DCOP*(TERM2 + FLX2)
S23(LL-12)=DCOP*(TERM2 + FLX1)
S23(LL-9)=DCOP*(0.5*TERM2)
ELSE IF MYX(LL-1) EQ. 10 .AND. MYX(LL-2) EQ. 10 THEN
  ELAS=SE(ISEMLI(LL-1)+4)
  SP =SE(ISEMLI(LL-1)+5)
  EIY2=SE(ISEMLI(LL-1)+1)
  STY(LL-7)-SE(ISEMLI(LL-1)+3)
  STY(LL-8)-SE(ISEMLI(LL-2)+3)
  IF (STY(LL-7) EQ. 1 .OR. STY(LL-8) EQ. 1) STYPAG=1
  IF (KKO.EQ.0) THEN
    SE(ISEMLI(LL-1)+12)-SE(ISEMLI(LL-1)+1)*LENGTH/(6*EIY2)
    SE(ISEMLI(LL-2)+12)-SE(ISEMLI(LL-2)+1)*LENGTH/(6*EIY2)
  ENDIF
  IF (S23 EQ. 0) THEN
    S23(LL-8)=4*EIY2/LENGTH
    S23(LL-12)=4*EIY2/LENGTH
    S23(LL-9)=2*EIY2/LENGTH
  ELSE
    IF (STY(LL-7) EQ. 0 .AND. STY(LL-8) EQ. 0) THEN
      S23(LL-8)=-SP*(4*EIY2/LENGTH)
      S23(LL-12)=-SP*(4*EIY2/LENGTH)
      S23(LL-9)=-SP*(2*EIY2/LENGTH)
    ELSE IF (STY(LL-7) EQ. 1 .AND. STY(LL-8) EQ. 1) THEN
      S23(LL-8)=-SP*(4*EIY2/LENGTH)
      S23(LL-12)=-SP*(4*EIY2/LENGTH)
      S23(LL-9)=-SP*(2*EIY2/LENGTH)
    ELSE IF (STY(LL-7) EQ. 0 .AND. STY(LL-8) EQ. 1) THEN
      S23(LL-8)=-SP*(4*EIY2/LENGTH)
      S23(LL-12)=-SP*(4*EIY2/LENGTH)
      S23(LL-9)=-SP*(2*EIY2/LENGTH)
    ENDIF
  ENDIF
  ELSE
    IF (LL.EQ.0) THEN
      WRITE(108,99) MYX(LL-1),MYX(LL-2),IELNO
      FORMAT(1X,'** INVALID MATERIAL TYPE MYA',I2,' MYB',I2,
        & ' FOR ELEMENT NO. ',I5,'**')
    ELSE
      WRITE(108,79) MYX(LL-1),MYX(LL-2),IELNO
      FORMAT(1X,'** INVALID MATERIAL TYPE MZA',I2,' MZB',I2,
        & ' FOR ELEMENT NO. ',I5,'**')
    ENDIF
  ENDIF
CONTINUE
205 C
C FOR ANIAL IN X AXIS LL=6
C FOR TORSION IN X AXIS LL=5
DO 210 LL=5,6
  L0=0
  IF (LL.EQ.5) L0=7
  IF MYX(LL) EQ. 10 THEN
    ELAS=SE(ISEMLI(LL)+4)
    SP =SE(ISEMLI(LL)+5)
    EAG=SE(ISEMLI(LL)+1)
    STY(LL)-SE(ISEMLI(LL)+3)
    IF (STY(LL) EQ. 1) STYPAG=1
    IF (KKO.EQ.0) SE(ISEMLI(LL)+12)-SE(ISEMLI(LL)+1)*LENGTH/EAG
    IF (ELAS EQ. 0) THEN
      S23(LL)=EAG/LENGTH
    ELSE
      IF (STY(LL) EQ. 0) THEN
        S23(LL)-SP*(EAG/LENGTH)
      ELSE
        ENDIF
      ELSE IF MYX(LL) EQ. 13 THEN
        S23(LL)-SE(ISEMLI(LL))
      ELSE
        IF (LL.EQ.6) THEN
          WRITE(108,89) MYX(LL),IELNO
          FORMAT(1X,'** INVALID MATERIAL TYPE MFXA',I2,
            & ' FOR ELEMENT NO. ',I5,'**')
        ELSE
          WRITE(108,99) MYX(LL),IELNO
          FORMAT(1X,'** INVALID MATERIAL TYPE MXXA',I2,
            & ' FOR ELEMENT NO. ',I5,'**')
        ENDIF
      ENDIF
    CONTINUE
210 C
  S23(1)=(S23(10)+2*S23(11)+S23(13))/(LENGTH**2)
  S23(2)=(S23(10)+S23(11))/LENGTH
  S23(3)=(S23(13)+S23(11))/LENGTH
  S23(4)=(S23(18)+2*S23(9)+S23(12))/(LENGTH**2)
  S23(5)=(S23(18)+S23(9))/LENGTH
  S23(6)=(S23(12)+S23(9))/LENGTH
C
C ASSEMBLE LOCAL STIFFNESS MATRIX
DO 72 I=1,12
DO 73 J=1,12
  S(I,J)=0
  S(1,1)=S23(0)
  S(1,7)=-S23(0)
  S(2,2)=-S23(0)
  S(2,6)=-S23(2)
  S(2,8)=-S23(1)
  S(2,12)=-S23(3)
  S(3,3)=-S23(4)
  S(3,5)=-S23(5)
  S(3,9)=-S23(4)
  S(3,11)=-S23(6)
  S(4,4)=-S23(7)
  S(4,10)=-S23(7)
  S(5,5)=-S23(8)
  S(5,9)=-S23(5)
  S(5,11)=-S23(9)
  S(6,6)=-S23(0)
  S(6,8)=-S23(2)
  S(7,7)=-S23(0)
  S(7,12)=-S23(0)
  S(8,8)=-S23(1)
  S(8,12)=-S23(3)
  S(9,9)=-S23(4)
  S(9,11)=-S23(6)
  S(10,10)=-S23(7)
  S(10,12)=-S23(7)
  S(11,11)=-S23(8)
  S(11,12)=-S23(9)
  S(12,12)=-S23(13)
DO 78 J=1,12
  DO 78 I=1,12
    S(I,J)=S(I,J)
C
C ***** CALCULATE SAT
CALL MULTM(3,12,SAT,S,A)
C ***** CALCULATE ASAT
CALL MULTM(0,12,ASAT,A,SAT)
C ***** DEGREES OF FREEDOM
C REMOVE DOP'S THAT ARE CONSTRAINED TO THE SAME DOF...
DO 75 I=1,6
  IF (IDOP(JOINTI,1) NE. IDOP(JOINTJ,1)) THEN
    LMT(I)-IDOP(JOINTI,1)
    LMT(I+6)-IDOP(JOINTJ,1)
  ELSE
    LMT(I)=0
    LMT(I+6)=0
  ENDIF
75 CONTINUE
C ***** CONVERT TO HALF STORAGE MODE BY COLUMNS
C
L=0
IELDOP=0
DO 90 I=1,12
  IF (LMT(I).EQ.0) GO TO 90
  IELDOP=IELDOP+1
  L=IELDOP-LMT(I)
  DO 80 J=1,12
    IF (LMT(J).EQ.0) GO TO 80
    L=L-1
    STUFF(L)-ASAT(I,J)
80 CONTINUE
90 CONTINUE
C ***** IOPT=3, KK0=1, RETURN **
IF (KK0.EQ.1) RETURN
C ***** ASSEMBLE GEOMETRIC STIFFNESS MATRIX FOR A UNIT LOAD...
L0=L
IF (PKG.EQ.0 .AND. KGDATA(1) NE. 2) THEN
  AXIAL=.FALSE.
  REL=IDOP(REL,6)
  GO TO 180
ENDIF
C ***** DETERMINE IF AXIAL DEFORMATION IS CONSIDERED
BIT #6 OF REL IS TRUE IF AXIAL DEFORMATION IS CONSIDERED
DO 999 I=1,3
  CE=CTE(I,1)
  CE=CTE(I,1)
  IF (ABS(CE).LE. .0001 .AND. ABS(CE).LE. .0001) GO TO 999
  IS-IDOP(JOINTI,1)
  IE-IDOP(JOINTJ,1)
  IF (IS.EQ.1) GO TO 999
  REL=ISSET(REL,6)
999 CONTINUE
C ***** AXIAL-BTEST(REL,6)
IF (KGDATA(1) NE. 0 .AND. KGDATA(2) NE. 0 .AND. BTEST(REL,6) THEN
  C ***** DETERMINE THE AXIAL LOAD TO BE USED FOR KG...
  IF (KGDATA(1).EQ.1) THEN
    PGEOM = PKG
  ELSE IF (KGDATA(1).EQ.2) THEN
    PGEOM = 0.50*(P(1)-P(7))
  ENDIF
C ***** ASSEMBLE LOCAL GEOMETRIC STIFFNESS MATRIX FOR A UNIT LOAD...
ZERO LOCAL KG MATRIX
DO 998 I=1,12
  DO 998 J=1,12
    S(I,J)=0.
998 C ***** LUMPED MASS FORM OF GEOMETRIC STIFFNESS
IF (KGDATA(2).EQ.1) THEN
  P1=1/LENGTH
  S(2,2)=P1
  S(2,8)=P1
  S(8,8)=P1
  S(2,3)=P1
  S(3,3)=P1
  S(3,9)=P1
  S(9,9)=P1
C ***** CONSISTENT MASS FORM OF GEOMETRIC STIFFNESS
ELSE IF (KGDATA(2).EQ.2) THEN
C ***** SET CONSISTENT MASS GEOMETRIC STIFFNESS COEFFICIENTS...
GAZ=2*LENGTH/15.
GBZ=GAZ
GCD=LENGTH/30.
GDZ=0.10
GEZ=0.10
GFZ=1.2/LENGTH
GAV=2*LENGTH/15
GBV=GAV
GCV=LENGTH/30
GDV=0.10
GFV=1.2/LENGTH
S(2,2)=GFZ
S(2,8)=GDE
S(8,8)=GFE
S(2,12)=GEZ
S(8,6)=GAZ
S(8,12)=GDE
S(6,12)=GCD
S(8,8)=GFE
S(8,12)=GDE
S(11,12)=GDE
S(3,3)=GFV
S(3,5)=GDY
S(3,9)=GCV
S(3,11)=GEV
S(5,5)=GAY
S(5,9)=GDY
S(5,11)=GCV
S(9,9)=GFV
S(9,11)=GEV
S(11,11)=GBY
ENDIF
DO 70 I=1,12
DO 70 J=1,12
  S(I,J)=S(I,J)
C ***** CALCULATE SAT
CALL MULTM(3,12,SAT,S,A)
C ***** CALCULATE ASAT
CALL MULTM(0,12,ASAT,A,SAT)
C ***** CONVERT TO HALF STORAGE MODE BY COLUMNS
KDOF=0
DO 1090 I=1,12

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IF (LMT(I) EQ 0 ) GO TO 1090
KGDOP=KGDOP+1
LXKG(KGDOP)-LMT(I)
DO 1090 J=1,3
IF (LMT(J) EQ 0 ) GO TO 1080
L=L-1
STIFF(L)-ASAT(I,J)
CONTINUE
CONTINUE
ENDIF
C
RELEASE UNUSED STORAGE
180 IREL=ISS-L
C
PRINT COLUMN DATA
IF (FIRST) WRITE(108,192)
191 WRITE(108,193) NAME,IELNO,NOEEL,NOEEL,LENGTH,
* VY(1),VY(2),VY(3),XS,*XE,PKG
WRITE(108,194) MATMLI(6),MATMLI(5),MATMLI(1),MATMLI(2),
* MATMLI(3),MATMLI(4)
C
192 FORMAT(/' ELEMENT 09, IESBEAN ELEMENT //
* T2, ' START END, 'JK, 'LENGTH',3X,'(---)', ' Y-AXIS ',
* L2('---'), ' START DIST END DIST PKG')
193 FORMAT(1X,A15,216,18,16,1X,1P,610,4,0P,
* P9.5,' I',SP,P9.5,' J',P9.5,' K',1P,68,3G12.4)
194 FORMAT(16X,' MATERIAL : (PK )',12,1X,' (MX )',12,1X,
* '(M1-A)',12,1X,' (M1-B)',12,1X,' (M2-A)',12,1X,
* '(M2-B)',12)
RETURN
C----- DETERMINE GEOMETRIC STIFF -----
200 CONTINUE
C XG IS DETERMINED FOR A UNIT LOAD WITH LOPT=1,
C THE ACTUAL LOAD (PGEOM) IS DETERMINED HERE.
C XG IS MULTIPLIED BY PGEOM WHEN THE TOTAL STIFFNESS IS ASSEMBLED
C NEGATIVE PGEOM IS COMPRESSIGN...
AXIAL-BTEST(REL,6)
C----- DETERMINE THE AXIAL LOAD TO BE USED FOR XG...
IF (XGDATA(1).EQ.1) THEN
PGEOM = PKG
ELSE IF (XGDATA(1).EQ.2) THEN
PGEOM = 0.50*(P(1)-P(7))
ENDIF
ENDIF
C
RETURN
C----- DETERMINE STIFFNESS -----
300 CONTINUE
STY(1),STY(2),STY(3),STY(4),STY(5),STY(6),STY(7),STY(8),
STY(9),AND STY(10) CONTAIN STIFFNESSES, WHICH WERE DETERMINED IN
THE LAST CALL TO MA_TLIB
IF STY(1) - SE(ISEMLI(1)) FOR MAT09,MAT11,MAT06
STY(2) - SE(ISEMLI(2)) FOR MAT09,MAT11,MAT06
STY(3) - SE(ISEMLI(3)) FOR MAT09,MAT11,MAT06
STY(4) - SE(ISEMLI(4)) FOR MAT09,MAT11,MAT06
STY(5) - SE(ISEMLI(5)) FOR MAT10
STY(6) - SE(ISEMLI(6)) FOR MAT10
OR
IF STY(7) - SE(ISEMLI(7)) FOR MAT10
STY(8) - SE(ISEMLI(8)) FOR MAT10
STY(9) - SE(ISEMLI(9)) FOR MAT10
STY(10) - SE(ISEMLI(10)) FOR MAT10
STY(5) - SE(ISEMLI(5)) FOR MAT10
STY(6) - SE(ISEMLI(6)) FOR MAT10
THEN
THE STIFFNESS HAS NOT CHANGED, RETURN
DO 330 L=1,3,2
L=L-1
IF (MYZX(L).NE. MYZX(L1)) GO TO 330
IF (MYZX(L).EQ.9 OR MYZX(L1).EQ.11) THEN
IF (STY(L).NE. SE(ISEMLI(L))) GO TO 3
ELSE IF (MYZX(L).EQ.6) THEN
IF (STY(L).NE. SE(ISEMLI(L))/LENGTH) OR
STY(L1).NE. SE(ISEMLI(L1))/LENGTH) GO TO 3
ELSE IF (MYZX(L).EQ.10) THEN
IF (STY(L1).NE. SE(ISEMLI(L1)) OR
IF (STY(L1).NE. SE(ISEMLI(L1)) GO TO 3
ENDIF
ENDIF
330 CONTINUE
IF (STY(5).NE. SE(ISEMLI(5))) GO TO 3
IF (MYZX(6).EQ.10) THEN
IF (STY(6).NE. SE(ISEMLI(6))) GO TO 3
ELSE IF (MYZX(6).EQ.13) THEN
IF (STY(6).NE. SE(ISEMLI(6))) GO TO 3
ENDIF
RETURN
C----- CONSTRUCT THE STIFFNESS MATRIX -----
300-1
GOTO 110
C----- DETERMINE INCREMENTAL FORCES -----
400 CONTINUE
IF (PRINT AND FIRST) WRITE(108,491) STEPID
C----- CALL BMLCA TO DETERMINE THE FIXED END FORCES
IF (MAXELD GT. 0) THEN
CALL BMLCA(IELNO,PBFLG,LENGTH,REL,MAXELD,NELD,NLOAD,
* PEM,IELD,ELD)
ELSE
PEMFLG=.FALSE.
ENDIF
C
LOOP FOR EACH LOAD
DO 430 LOAD=1,NLOAD
C----- TRANSFORM FRAME DISPL INTO LOCAL DISPL
DO 410 J=1,3
R(1)=0
R(1-3)=0
R(1-6)=0
R(1-9)=0
DO 410 J=1,3
D1=DISPL(IDOP(JOINT1),J,LOAD)
D2=DISPL(IDOP(JOINT2),J,LOAD)
D3=DISPL(IDOP(JOINT3),J,LOAD)
D4=DISPL(IDOP(JOINT4),J,LOAD)
R(1)=R(1)+D1*CTE(1,1)+D2*-BS(J,I)*D2
R(1-3)=R(1-3)+D1*CTE(1,2)+D2*-BS(J,I)*D2
R(1-6)=R(1-6)+D1*CTE(1,3)+D2*-BS(J,I)*D2
R(1-9)=R(1-9)+D1*CTE(1,4)+D2*-BS(J,I)*D2
410
C----- INCREMENTAL FORCES
F(1)=S23(0)*(R(1)-R(7))
F(2)=S23(1)*(R(2)-R(8))-S23(2)*(R(6)-S23(3))*R(12)
F(3)=S23(4)*(R(3)-R(9))-S23(5)*(R(5)-S23(6))*R(11)
F(4)=S23(7)*(R(4)-R(10))
F(5)=S23(5)*(R(3)-R(9))-S23(8)*(R(5)-S23(9))*R(11)
F(6)=S23(2)*(R(2)-R(8))-S23(10)*(R(6)-S23(11))*R(12)
F(7)=S23(0)*(R(1)-R(7))
F(8)=S23(1)*(R(2)-R(8))-S23(2)*(R(6)-S23(3))*R(12)
F(9)=S23(4)*(R(3)-R(9))-S23(5)*(R(5)-S23(6))*R(11)
F(10)=S23(7)*(R(4)-R(10))
F(11)=S23(5)*(R(3)-R(9))-S23(8)*(R(5)-S23(9))*R(11)
F(12)=S23(2)*(R(2)-R(8))-S23(10)*(R(6)-S23(11))*R(12)
C----- RELATIVE ROTATION FOR BENDINGS
R(5)-R(8)-(R(3)-R(9))/LENGTH
R(6)-R(9)-(R(6)-R(9))/LENGTH
R(11)-R(11)-(R(3)-R(9))/LENGTH
R(12)-R(12)-(R(8)-R(12))/LENGTH

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C
SUBTRACT OF FIXED END FORCES
IF (PEMFLG) THEN
DO 420 I=1,12
IF (I.EQ.1 OR I.EQ.4 OR I.EQ.7 OR I.EQ.10) GOTO 420
P(I)=P(I)-PEM(I,LOAD)
420 CONTINUE
ENDIF
C
DETERMINE MAXIMUM VALUES FOR MULTIPLE LOAD CASES
IF (NLOAD GT. 1) THEN
DO 425 J=0,11
IF (ABS(P(J)).LE.ABS(PMAX(I*12-12+J))) GOTO 425
DO 426 J=0,11
426 PMAX(I*12-11+J)=P(J-1)
425 CONTINUE
ENDIF
C
PRINT FORCES AND DEFORMATIONS
IF (PRINT) WRITE(108,492) IELNO,LOAD,NOEEL,(R(J),J=1,6),
* NOEEL,(R(J),J=7,12)
IF (PRINT) WRITE(108,492) IELNO,LOAD,NOEEL,(P(J),J=1,6),
* NOEEL,(P(J),J=7,12),STBPAQ
430 CONTINUE
C----- THIS IS FOR SOLOSA USE ONLY
IF (ICOMM EQ. 1 OR ICOMM EQ. 2) THEN
DO 431 I=1,12
RSAP(I)=P(I)
431
ENDIF
C
CHECK STIFFNESS
THIS IS PERFORMED ONLY FOR INELASTIC ANALYSIS
P(1)=PT(5)+P(5)
P(2)=PT(11)+P(11)
P(3)=PT(6)+P(6)
P(4)=PT(12)+P(12)
P(5)=PT(10)+P(10)
P(6)=PT(7)+P(7)
IF (.NOT.ELASTIC) THEN
C
CHECK INTERACTIVE STRENGTH CRITERIA
PV = SE(ISEMLI(6)+1)
TMP1 = SE(ISEMLI(1)+1)
TMP2 = SE(ISEMLI(3)+1)
DO 432 I=1,4
432 TMP(I)=SE(ISEMLI(I)+1)
IF (IAPAG EQ. 3) THEN
CALL ITERS(P(6),P(1),P(2),TMP,PY,P(3),P(4),TMP2,IAPAG)
IF (IAPAG EQ. 2 OR IAPAG EQ. 3) THEN
X1=IAPAG-1
X2=IAPAG-1
SE(ISEMLI(X1)+1)*ABS(P(X1))*SE(ISEMLI(X1)+8)
SE(ISEMLI(X2)+1)*ABS(P(X2))*SE(ISEMLI(X2)+8)
SE(ISEMLI(X1)+2)*SE(ISEMLI(X1)+1)
SE(ISEMLI(X2)+2)*SE(ISEMLI(X2)+1)
ELSE IF (IAPAG EQ. 4) THEN
DO 433 I=1,4
SE(ISEMLI(I)+1)*TMP(I)*SE(ISEMLI(I)+8)
SE(ISEMLI(I)+2)*SE(ISEMLI(I)+1)
433 CONTINUE
ENDIF
C
ASSIGN AXIAL RESIDUAL STRENGTH
IF (SE(ISEMLI(6)+1).EQ. SE(ISEMLI(6)+2).AND.IAPAG.NE.1) THEN
SE(ISEMLI(6)+1)=SE(ISEMLI(6)+2)
SE(ISEMLI(6)+2)=SE(ISEMLI(6)+8)
WRITE(108,705) IELNO,JSTEP,IAPAG
FORMAT(1X,' ELEMENT ',15,' LOCAL BUCKLING OCCURRED ',
* ' AT STEP ',15,' IAPAG-',15,' ')
705
ENDIF
ELSE IF (IAPAG EQ. 2 OR IAPAG EQ. 3) THEN
IF (IAPAG EQ. 2) THEN
X1=2
X2=4
ELSE
X1=1
X2=3
ENDIF
CALL ITERS(P(6),P(1),P(2),TMP,PY,P(3),P(4),TMP2,IAPAG)
IF (IAPAG EQ. 4) THEN
SE(ISEMLI(X1)+1)*ABS(P(X1))*SE(ISEMLI(X1)+8)
SE(ISEMLI(X2)+1)*ABS(P(X2))*SE(ISEMLI(X2)+8)
SE(ISEMLI(X1)+2)*SE(ISEMLI(X1)+1)
SE(ISEMLI(X2)+2)*SE(ISEMLI(X2)+1)
WRITE(108,705) IELNO,JSTEP,IAPAG
ENDIF
ELSE IF (IAPAG EQ. 5) THEN
CALL ITERS(P(6),P(1),P(2),TMP,PY,P(3),P(4),TMP2,IAPAG)
IF (IAPAG EQ. 3) THEN
WRITE(108,706) IELNO,JSTEP,IAPAG
FORMAT(1X,' ELEMENT ',15,' MODEL OVERSTRENGTH ',
* ' AT STEP ',15,' IAPAG-',15,' ')
706
ENDIF
ENDIF
ENDIF
C
TRANSFER PERMANENT HYSTERESIS DATA TO TEMPERARY STORAGE
ISE2=ISE2/I
DO 522 I=1,ISE2
522 SE(I)=ISE2-SE(I)
C
CALCULATE INELASTIC ROTATION
DO 510 I=1,2
I=I-1
IF (MYZX(I).NE. MYZX(I1)) GOTO 310
IF (MYZX(I).NE.6 OR MYZX(I1).NE.9 OR MYZX(I).NE.11) GOTO 310
STY(I)=SE(ISEMLI(I))
IF (MYZX(I) EQ. 9) THEN
SA1=SE(ISEMLI(I)+60)
SA2=SE(ISEMLI(I)+30)
ELSE IF (MYZX(I) EQ. 11) THEN
SA1=SE(ISEMLI(I)+60)
SA2=SE(ISEMLI(I)+30)
ELSE IF (MYZX(I) EQ. 6) THEN
SA1=SE(ISEMLI(I)+47)/LENGTH
SA2=SE(ISEMLI(I)+47)/LENGTH
STY(I1)=STY(I)/LENGTH
STY(I)=STY(I1)/LENGTH
ENDIF
PLX=C
IF (SA1.NE.0) PLX2=(1/STY(I1))-(1/SA1)
PLX=C
IF (SA2.NE.0) PLX2=(1/STY(I1))-(1/SA2)
RIEL(I)=ROIEL(I)+PLX*P(6-1)
RIEL(I1)=ROIEL(I1)+PLX2*P(10-1)
310 CONTINUE
C
SET UP TEMPORARY TOTAL LOADS,DISP'S
BENDING IN A END Y DIRECTION I=1
BENDING IN B END Y DIRECTION I=2
BENDING IN A END X DIRECTION I=3
BENDING IN B END X DIRECTION I=4
TORSION IN A END X DIRECTION I=5
AXIAL IN A END X DIRECTION I=6
DO 525 I=1,6
IF (I.LE.4) THEN
X1=I-1
D=ST(K11)-R(K11)
DL=ST(K11)
IF (I.EQ.1) THEN
P=RIEL(I)+P(1)/SE(ISEMLI(I)+60)
PL=RIEL(I1)+PL/SE(ISEMLI(I1)+60)

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ELSE IF (MYXZ(1) .EQ. 11) THEN
D-RIELI(1) = P(1)/SE(ISEM(1)) + 30
DL-ROELI(1) = PL/SE(ISEM(1)) + 30
ELSE IF (MYXZ(1) .EQ. 6) THEN
IF SE(ISEM(1)) .GT. 0 THEN
D-RIELI(1)/LENGTH = P(1)/SE(ISEM(1)) + 47
DL-ROELI(1)/LENGTH = PL/SE(ISEM(1)) + 47
ENDIF
ENDIF
ELSE
K11=10
IF (EQ 6) K11=7
PL=PT(K11)
D-RT(K11)+R(K11)-RT(K11-6)-R(K11-6)
DL-RT(K11)-RT(K11-6)
ENDIF
CALL MATLIB(3,LETTYP,FALSE,ESEE,EPSE,DUCT(1,1),EXCR(1,1),
P(1),PL,D,DL,0,0,0,0,LELEM,LMAT,MATLI(1),MATTYP,
SE(ISE2+ISEM(1)),IELNO,DAMAGE)
IF (I.LE.4) THEN
IF (MATTYP .EQ. 9) THEN
IF (SE(ISE2+ISEM(1))) .NE. SE(ISEM(1)) NEWK=.TRUE.
ROELI(1)=D-P(1)/SE(ISEM(1))+60
ELSE IF (MATTYP .EQ. 11) THEN
IF (SE(ISE2+ISEM(1))) .NE. SE(ISEM(1)) NEWK=.TRUE.
ROELI(1)=D-P(1)/SE(ISEM(1))+30
ELSE IF (MATTYP .EQ. 6) THEN
IF (SE(ISE2+ISEM(1))) .NE. SE(ISEM(1)) NEWK=.TRUE.
IF (SE(ISEM(1))) .GT. 0 THEN
ROELI(1)=(D-P(1)/SE(ISEM(1)))+LENGTH
ENDIF
ELSE IF (MATTYP .EQ. 10) THEN
IF (SE(ISE2+ISEM(1))) .NE. SE(ISEM(1)) NEWK=.TRUE.
ENDIF
ELSE IF (EQ 5) THEN
IF (SE(ISE2+ISEM(1))) .NE. SE(ISEM(1)) NEWK=.TRUE.
ENDIF
ELSE IF (MYXZ(6) .EQ. 10) THEN
IF (SE(ISE2+ISEM(1))) .NE. SE(ISEM(1)) NEWK=.TRUE.
ELSE IF (MYXZ(6) .EQ. 13) THEN
IF (SE(ISE2+ISEM(1))) .NE. SE(ISEM(1)) NEWK=.TRUE.
ENDIF
ENDIF
CONTINUE
ENDIF
525 CONTINUE
ENDIF
C----- CALCULATE THE UNBALANCED MEMBER FORCES ON THE JOINT
C
UP(5) = -(PT(5)+P(5)-P(1))
UP(11) = -(PT(11)+P(11)-P(2))
UP(6) = -(PT(6)+P(6)-P(3))
UP(12) = -(PT(12)+P(12)-P(4))
UP(10) = -(PT(10)+P(10)-P(8))
UP(7) = -(PT(7)+P(7)-P(6))
UP(9) = (UP(5)+UP(11))/LENGTH
UP(3) = -UP(9)
UP(2) = (UP(6)+UP(12))/LENGTH
UP(8) = -UP(2)
UP(1) = -UP(7)
UP(4) = -UP(10)
DO 440 J=1,IELDOP
K=LN(I)
EPUB(I)=0
DO 460 J=1,12
460 EPUB(I)+PUB(J) = A(I,J)+UP(J)
PUB(K)+PUB(K)+EPUB(I)
440 CONTINUE
C----- ADJUST P FOR UNBALANCED LOAD P
DO 445 I=1,12
445 P(I)=P(I)+UP(I)
491 FORMAT (' IESD BEAM FORCES ... 5X/7X,A/,
' ELEMENT LOAD NODE 7X,AXIAL,11X,'FY',13X,'FZ',
' 11X,'TORSION',10X,'MY',13X,'MZ STBPAQ')
492 FORMAT (' /,110,15,2X,'FORCE',16,1P,6G15.6,
' /,15X '1,2X,'FORCE',16, 6G15.6,3X,11)
493 FORMAT (' /,110,15,2X,'DISPL',16,1P,6G15.6,
' /,15X '2X,'DISPL',16, 6G15.6)
494 FORMAT (7X,A)
RETURN
C----- DETERMINE TOTAL FORCES
500 CONTINUE
IF (PRINT AND FIRST) WRITE(108,491) STEPID
IF (ICOMM .NE. 1 AND ICOMM .EQ. 2) THEN
C----- TOTAL FORCES AND DISPLACEMENTS
DO 510 I=1,12
PT(I)=P(I)+F(I)
RT(I)=RT(I)+R(I)
510 CONTINUE
DO 512 I=1,12
IF (ABS(PT(I)).GT.ABS(FMAX(I*12-12+I))) THEN
511 FMAX(I*12-11+3)=PT(I*12-11+3)
512 CONTINUE
ELSE
DO 513 I=1,12
513 PT(I)=RT(I)+R(I)
ENDIF
LOAD=1
IF (PRINT) WRITE(108,493) IELNO,LOAD,MODEI,(RT(J),J=1,6),
NODEJ,(RT(J),J=7,12)
IF (PRINT) WRITE(108,492) IELNO,LOAD,MODEI,(PT(J),J=1,6),
NODEJ,(PT(J),J=7,12),STBPAQ
C----- TRANSPER TEMPORARY HYSTERESIS DATA TO PERMANENT STORAGE
ISE2=ISE/2
IF (NOT ELSTIC) THEN
DO 415 I=1,ISE2
415 SE(I)=SE(I)+ISE2
ENDIF
RETURN
C----- DETERMINE FIXED END FORCES, AND ADD TO FORCES
600 IF (MAXELD.LE.0) RETURN
CALL BMLQA(IELNO,FEMPLG,LENGTH,REL,MAXELD,NELD,FEM,ELD)
FEMLO=FEMPLG
IF (NOT FEMPLG) RETURN
DO 620 LOAD=1,NLOAD
DO 610 I=1,5
GFM(I)=0
GFM(I+3)=0
GFM(I+6)=0
GFM(I+9)=0
DO 610 J=1,3
D1=FEM(J,LOAD)
D2=FEM(J+3,LOAD)
D3=FEM(J+6,LOAD)
D4=FEM(J+9,LOAD)
GFM(I)=GFM(I)+GFM(I)+CTE(I,J)*D1
GFM(I+3)=GFM(I+3)+GFM(I+3)+CTE(I,J)*D2
GFM(I+6)=GFM(I+6)+GFM(I+6)+CTE(I,J)*D3
GFM(I+9)=GFM(I+9)+GFM(I+9)+CTE(I,J)*D4
610 DO 620 J=1,6

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JI-IDOF(JOINTI,J)
JU-IDOF(JOINTJ,J)
FORCE(JI,LOAD)+FORCE(JI,LOAD)+GFM(J)
620 FORCE(JJ,LOAD)+FORCE(JJ,LOAD)+GFM(J+6)
C
RETURN
C----- WRITE MEMBER FORCES TO OUTPUT FILE
700 CONTINUE
C
WRITE (UNIT,710) NODEI,(PT(J),J=1,6),NODEJ,(PT(J),J=7,12),STBPAQ
WRITE (UNIT,710) NODEI,(RT(J),J=1,6),NODEJ,(RT(J),J=7,12)
710 FORMAT (16,1P,6G12.5,16,1P,6G12.5,11)
C
RETURN
C----- READ AND PRINT MEMBER FORCES FROM OUTPUT FILE
800 CONTINUE
BACKSPACE(UNIT)
READ (UNIT,*) ITYPE,IELNO,IWRITE,TO,DT
WRITE(108,805)
ISTEP=IWRITE
810 READ (UNIT,815,END=830) NODEI,(PT(J),J=1,6),NODEJ,
,(PT(J),J=7,12),STBPAQ
815 FORMAT (/,110,1P,6G12.5,16,6G12.5,11)
C
ISTEP=ISTEP-IWRITE
T=TO-DT*ISTEP
IF (MOD(ISTEP,INC).EQ.0) THEN
WRITE(108,820) ISTEP,T,NODEI,(PT(J),J=1,6),
NODEJ,(PT(J),J=7,12),STBPAQ
820 FORMAT (/,110,1P,6G15.6,10P,16,1P,6G15.6,15X,16,6G15.6,3X,11)
820 FORMAT (25X,'DISP',1P,6G15.6,31X,6G15.6)
ENDIF
GO TO 810
C
805 FORMAT (' IESD BEAM FORCES ... /
' STEP TIME NODE 7X,AXIAL,11X,'FY',
' 11X,'FZ',11X,'TORSION',10X,'MY',13X,'MZ STBPAQ')
820 FORMAT (/,110,1P,6G15.6,10P,16,1P,6G15.6,15X,16,6G15.6,3X,11)
920 FORMAT (25X,'DISP',1P,6G15.6,31X,6G15.6)
C
930 RETURN
C----- DETERMINE THE LENGTH OF THE MATERIAL DATA
900 CONTINUE
ISE=0
C----- LOOP FOR EACH MATERIAL
DO 910 I=1,6
ISEM(I)=ISE+1
MATI = RINPUT(I)
CALL MATLIB(0,LETTYP,FALSE,ESEE,EPSE,DUCT,EXCR,
PT,F,RT,R,0,0,0,0,LELEM,LMAT,MATLI,MATTYP,SE,IELNO,DAMAGE)
910 ISE=ISE+LELEM
C----- DOUBLE THE STORAGE, TO ALLOW FOR TEMPORARY VALUES
ISE=ISE*2
C
RETURN
C----- DETERMINE THE TOTAL STRAIN ENERGY
1000 CONTINUE
ESEE=0
EPSE=0
C
/* ENERGY FORMULATION IS NOT AVAILABLE FOR ELEM9 ELEMENT */
DO 1010 I=1,12
ESEE+ESEE+(PT(I)-P(I))*R(I)-RT(I))/2.
1010 ESEE=ESEE
ENDAT(1) = ESEE
ENDAT(2) = 0
C
RETURN
C----- DUCTILITIES(BASED ON DEFINITION ONE ONLY)
1100 CONTINUE
NOTE ELEM9 WAS LAST CALLED TO CALC INCREMENTAL DISPL,
THE ENERGIES ASSOCIATED WITH INCR DISPL ARE STORED IN
THE ADDRESSES: ISEM(1)+ISE/2
ISEM(2)+ISE/2
ISEM(3)+ISE/2
ISEM(4)+ISE/2
ISEM(5)+ISE/2
ISEM(6)+ISE/2
C----- BENDING ENERGY, DUCTILITY AND EXCURSION
ISE2=ISE/2
C----- BENDING IN A END Y DIRECTION I=1
C----- BENDING IN B END Y DIRECTION I=2
C----- BENDING IN A END Z DIRECTION I=3
C----- BENDING IN B END Z DIRECTION I=4
C----- TORSION IN A END X DIRECTION I=5
C----- AXIAL IN A AND X DIRECTION I=6
DO 1105 I=1,6
CALL MATLIB(4,LETTYP,FALSE,ESEE,EPSE,DUCT(1,1),EXCR(1,1),
PT,F,RT,R,0,0,0,0,LELEM,LMAT,MATLI(1),MATTYP,
SE(ISE2+ISEM(I)),IELNO,DAMAGE)
1105 CONTINUE
DO 1109 I=1,6
IF (ABS(DUCT(I,1))) LT. 1.) DUCT(1,1)=0.
CONTINUE
IF (DUCFAG .EQ. 0) THEN
IF (ABS(DUCT(1,1))) GE 1 .OR. ABS(DUCT(1,2)) GE 1
IF (ABS(DUCT(1,3))) GE 1 .OR. ABS(DUCT(1,4)) GE 1
IF (ABS(DUCT(1,5))) GE 1 .OR. ABS(DUCT(1,6)) GE 1 THEN
DUCFAG=ABS(DUCT(1,1))
IF (DUCFAG .LT. ABS(DUCT(1,2))) DUCFAG=ABS(DUCT(1,2))
IF (DUCFAG .LT. ABS(DUCT(1,3))) DUCFAG=ABS(DUCT(1,3))
IF (DUCFAG .LT. ABS(DUCT(1,4))) DUCFAG=ABS(DUCT(1,4))
IF (DUCFAG .LT. ABS(DUCT(1,5))) DUCFAG=ABS(DUCT(1,5))
IF (DUCFAG .LT. ABS(DUCT(1,6))) DUCFAG=ABS(DUCT(1,6))
ENDIF
ENDIF
IF (PRINT AND FIRST) WRITE(108,1191)
IF (PRINT) WRITE(108,1192) IELNO,DUCT(1,1),J=1,6)
1192 FORMAT (6X,15,1X,6P11.5)
1191 FORMAT (' IESDBEAM DUCTILITY(BASED ON DEFINITION 1) /,6X,
' ELEM', I, MYA: MYB: MZA:
' MCB: MK: 1 1 1 1 1 1)
C
RETURN
C----- PRINT MAXIMUM ELEMENT LOADS
1200 CONTINUE
LOAD=0
IF (FIRST) WRITE(108,494)
/* COLUMN (LOAD) REPRESENTS THE DOP WHICH'
/* HAS MAXIMUM VALUE'
IF (FIRST) /* MAXIMUM VALUES FOR ALL STEPS '
/* NOTE MAXIMUM VALUES WITH THE OTHER DOPS FORCES '
/* ARE PRINT OUT AT THE SAME TIME '
DO 309 I=0,11
LOAD=I+1
IF (FMAX(I*12-1+I) .EQ. 0) GO TO 309
WRITE(108,492) IELNO,LOAD,MODEI,(FMAX(I*12-1+J),J=1,6),
NODEJ,(FMAX(I*12-1+J),J=7,12),STBPAQ
309 CONTINUE
RETURN
C----- RESET ELEMENT FORCES
1300 CONTINUE

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ELEM9630
ELEM9640
ELEM9650
ELEM9660
ELEM9670
ELEM9680
ELEM9690
ELEM9700
ELEM9710
ELEM9720
ELEM9730
ELEM9740
ELEM9750
ELEM9760
ELEM9770
ELEM9780
ELEM9790
ELEM9800
ELEM9810
ELEM9820
ELEM9830
ELEM9840
ELEM9850
ELEM9860
ELEM9870
ELEM9880
ELEM9890
ELEM9900
ELEM9910
ELEM9920
ELEM9930
ELEM9940
ELEM9950
ELEM9960
ELEM9970
ELEM9980
ELEM9990
ELEM1000
ELEM1010
ELEM1020
ELEM1030
ELEM1040
ELEM1050
ELEM1060
ELEM1070
ELEM1080
ELEM1090
ELEM1100
ELEM1110
ELEM1120
ELEM1130
ELEM1140
ELEM1150
ELEM1160
ELEM1170
ELEM1180
ELEM1190
ELEM1200
ELEM1210
ELEM1220
ELEM1230
ELEM1240
ELEM1250
ELEM1260
ELEM1270
ELEM1280
ELEM1290
ELEM1300
ELEM1310
ELEM1320
ELEM1330
ELEM1340
ELEM1350
ELEM1360
ELEM1370
ELEM1380
ELEM1390
ELEM1400
ELEM1410
ELEM1420
ELEM1430
ELEM1440
ELEM1450
ELEM1460
ELEM1470
ELEM1480
ELEM1490
ELEM1500
ELEM1510
ELEM1520
ELEM1530
ELEM1540
ELEM1550
ELEM1560
ELEM1570
ELEM1580
ELEM1590
ELEM1600
ELEM1610
ELEM1620
ELEM1630
ELEM1640
ELEM1650
ELEM1660
ELEM1670
ELEM1680
ELEM1690
ELEM1700
ELEM1710
ELEM1720
ELEM1730
ELEM1740
ELEM1750
ELEM1760
ELEM1770
ELEM1780
ELEM1790
ELEM1800
ELEM1810
ELEM1820
ELEM1830
ELEM1840
ELEM1850
ELEM1860
ELEM1870
ELEM1880
ELEM1890
ELEM1900
ELEM1910
ELEM1920
ELEM1930
ELEM1940
ELEM1950
ELEM1960
ELEM1970
ELEM1980
ELEM1990
ELEM2000
ELEM2010
ELEM2020
ELEM2030
ELEM2040
ELEM2050
ELEM2060
ELEM2070
ELEM2080
ELEM2090
ELEM2100
ELEM2110
ELEM2120
ELEM2130
ELEM2140
ELEM2150
ELEM2160
ELEM2170
ELEM2180
ELEM2190
ELEM2200

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```

I2LM-IC=40
CALL ELE08
4 (IOPT ,FIRST ,PRINT ,NAME ,IELNO
4 N2(I2ID) ,N2(I2IDOP) ,2(I2ICORD) ,2(I2ICOS) ,N2(I2JCOS)
4 2(I2CNST) ,2(IDISP) ,2(I2VEL) ,2(I2FUB) ,IREL
4 RINPUT ,EISE ,EISE ,DAMAGE ,DUCFAG
4 N2(IC 1) ,N2(IC 2) ,N2(IC 3) ,N2(IC 4) ,N2(IC 5)
4 N2(IC 6) ,2(IC 7) ,2(IC 8) ,2(IC 9) ,2(IC 10)
4 2(IC 11) ,2(IC 12) ,N2(IC 13) ,2(IC 14)
4 N2(IC 15) ,2(IC 16) ,N2(IC 17) ,N2(IC 18) ,2(IC 19)
4 2(IC 20) ,2(IC 21) ,2(IC 22)
MSTOR =N2(IC-12)
IELDOP=N2(IC-11)
IELMKG=0
IKXKG=0
KGDOP =0
C
ELSE IF (IELTYP.EQ.9) THEN
I2LM-IC=73
I2XE-IC=458-N2(IC-141)
I2LMKG-IC=129
IF (MAXELD.LE.0) MAXELD=1
CALL ELE09
4 (IOPT ,FIRST ,PRINT ,NAME ,AXIAL
4 N2(I2ID) ,N2(I2IDOP) ,2(I2ICORD) ,2(I2ICOS) ,N2(I2JCOS)
4 2(I2CNST) ,2(IDISP) ,2(ILOAD) ,IREL
4 N2(I2KGD-1) ,PGEOM ,N2(I2IELD) ,2(I2ELD) ,IELNO
4 RINPUT ,EISE ,EISE ,DAMAGE
4 DUCFAG ,N2(IC 1) ,N2(IC 2) ,2(IC 4)
4 N2(IC-18) ,2(IC-24) ,2(IC-25) ,2(IC-27) ,2(IC-49)
4 2(IC-61) ,N2(I2LM) ,2(IC-85) ,2(IC-94) ,2(IC-103)
4 2(IC-112) ,N2(IC-121) ,N2(IC-122) ,N2(IC-123) ,N2(IC-124)
4 N2(IC-125) ,2(IC-126) ,N2(IC-127) ,N2(IC-128) ,N2(I2LMKG)
4 N2(IC-141) ,2(IC-142) ,2(IC-144) ,N2(IC-145) ,N2(IC-149)
4 N2(IC-190) ,2(IC-196) ,2(IC-210) ,2(IC-214)
4 2(IC-458) ,2(I2XE) ,2(I2W)
C
MSTOR =N2(IC-141)
IELDOP=N2(IC-11)
IELMKG-IC=129
IKXKG=I2XE-N2(IC-128)
KGDOP =N2(IC-127)
C
ELSE IF (IELTYP.EQ.12) THEN
AXIAL =FALSE
I2LM-IC=50
I2XE-IC=540-N2(IC-104)
IF (MAXELD.LE.0) MAXELD=1
CALL ELE12
4 (IOPT ,FIRST ,PRINT ,IREL ,NAME
4 N2(I2ID) ,N2(I2IDOP) ,2(I2ICORD) ,2(I2ICOS) ,N2(I2JCOS)
4 2(I2CNST) ,2(IDISP) ,2(I2FUB) ,EISE ,EISE
4 EISE ,DAMAGE ,N2(IC 1) ,N2(IC 2) ,2(IC 2)
4 2(IC 14) ,2(IC 26) ,2(IC 36) ,N2(I2LM) ,2(IC 62)
4 2(IC 71) ,2(IC 80) ,2(IC 89) ,N2(IC 98) ,N2(IC 99)
4 N2(IC-100) ,N2(IC-101) ,N2(IC-102) ,2(IC-103) ,IELNO
4 N2(IC-104) ,2(IC-105) ,2(IC-107) ,2(IC-151) ,2(IC-155)
4 N2(IC-539) ,2(IC-540) ,2(I2XE)
C
N2(IC-62)-1
MSTOR =N2(IC-104)
IELDOP=N2(IC-11)
IELMKG=0
IKXKG=0
KGDOP =0
C
ELSE IF (IELTYP.EQ.14) THEN
I2XE-IC=167-N2(IC-5)
CALL ELE14
4 (IOPT ,FIRST ,PRINT ,PGEOM ,NAME
4 N2(I2ID) ,N2(I2IDOP) ,2(I2ICORD) ,2(I2ICOS) ,N2(I2JCOS)
4 2(I2CNST) ,2(IDISP) ,2(I2FUB) ,IREL ,N2(I2KGD-1)
4 IELNO ,RINPUT ,AXIAL ,EISE ,EISE
4 N2(IC 1) ,N2(IC 1) ,N2(IC 2) ,N2(IC 5) ,N2(IC 6)
4 N2(IC 10) ,2(IC 14)
4 2(IC 17) ,2(IC 20) ,2(IC 23) ,2(IC 26) ,2(IC 29)
4 2(IC 32) ,N2(IC 33) ,N2(IC 34) ,N2(IC 58) ,2(IC 82)
4 2(IC-154) ,2(IC-156) ,2(IC-156) ,DAMAGE ,DUCFAG
4 N2(IC-159) ,2(IC-160) ,2(IC-161) ,2(IC-167) ,2(I2XE)
MSTOR =N2(IC-5)
IELDOP=N2(IC-11)
I2LM-IC=34
I2LMKG-IC=58
I2XEKG-IC=167-N2(IC-5) + N2(IC-33)-1
KGDOP =N2(IC-159)
ELSE
WRITE(108,10) IELNO
ERR=.TRUE.
10 FORMAT(' ELEMENT ',I6,' IS NOT AVAILABLE')
ENDIF
LETTY=IELTYP
RETURN
END
C
SUBROUTINE WMTR2(A,IR,IC,NAME,ICM)
DIMENSION A(100,10)
CHARACTER(*) NAME
CALL OPTINT(NAME,'DUMP',0,'TRUE',FALSE)
WRITE(108,*) 'PRINTING MATRIX: ',NAME
DO 50 ICG=1,IC,6
IF (ICG.GT.1) WRITE(108,*) ' '
K=KIN(ICG-5,IC)
WRITE(108,10) (J,J=ICG,K)
DO 20 I=1,IR
IF (IDUMP.NE.0) THEN
DO 15 J=1,IC,K
IF (A(I,J).NE.0) WRITE(IDUMP,*) I,J,A(I,J),NAME
ENDIF
WRITE(108,30) I,(A(I,J),J=ICG,K)
50 CONTINUE
10 FORMAT(' COLUMN',I7,IS,(ISX,IS))
20 FORMAT(' ROW',I4,6F20.6)
RETURN
END
C
SUBROUTINE VECTOR CROSS PRODUCT ...
V3 = V1 X V2, VECTOR CROSS PRODUCT ...
C
SUBROUTINE VCROSS(V3,V1,V2)
DIMENSION V1(3),V2(3),V3(3)
V3(1) = V1(2)*V2(3) - V1(3)*V2(2)
V3(2) = -V1(1)*V2(3) + V1(3)*V2(1)
V3(3) = V1(1)*V2(2) - V1(2)*V2(1)
RETURN
END
C
SUBROUTINE ELEM0
4 (IOPT ,FIRST ,PRINT ,PGEOM ,NAME
4 ID ,IDOP ,CORO ,COSINE ,JTCOS
4 CONST ,DISPL ,VELOC ,FUB ,IREL
4 KGDATA ,IELNO ,RINPUT ,EISE
4 PSE ,DAMAGE ,DUCFAG ,IELDOP
4 ISEB ,ISE ,JK ,MATS ,R
4 RT ,F ,V ,VT
4 PKG ,LKG ,LM ,LXKG ,A
4 SE ,SS ,SA ,KGDOP ,LENGTH
4 PHAX ,SE ,STIFF
IMPLICIT REAL(A-H,O-Z)

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C
COMMON /BSA/ICOM,RSAP(12)
REAL ICOS,LENGTH
LOCAL FIRST,PRINT,FALSE,AXIAL
CHARACTER*10 NAME
$INCLUDE 'ICOMN.D'
DIMENSION IDOP(MAXNO,6),COORD(MAXNO,6),COSINE(3,3),NCOS,AC(6,3)
DIMENSION CONST(MAXNO,6),ID(MAXNO),JTCOS(MAXNO),JK(4),LMKG(24)
DIMENSION SAT(3,24),STIFF(600),KGDATA(3),SAT2(6,24),S(6,6),LM(24)
DIMENSION CT(3,3),B6(3,3),VT(3),VNB(3),VNT(3),VZB(3)
DIMENSION AT(24,6),ISEB(3),MATS(3),EISE(3),PSE(3),DMS(3)
DIMENSION SE(ISE),AA(4,6,3),RINPUT(100),DUCT(3,3),EXCR(6,3)
DIMENSION F(3),R(3),V(3),PT(3),RT(3),VT(3),P(3),VELOC(INDOP)
DIMENSION DISPL(NDOP),LOAD, PUB(NDOP),EPUB(24),PMAK(6),A(24,3)
C
C VARIABLES:
C----- GLOBAL VARIABLES
C IOPT = 1, INITIALIZE BEAM COLUMN ELEMENT
C = 2, CALCULATE GEOMETRIC STIFFNESS
C = 3, FORM STIFFNESS
C = 4, CALCULATE INCREMENTAL FORCES
C = 5, CALCULATE TOTAL FORCES AND ENERGIES
C IUNIT = OUTPUT FILE UNIT # FOR PRINTING OUTPUT
C FIRST = FLAG, FIRST = TRUE, PRINT HEADERS
C PRINT = FLAG, PRINT = TRUE, PRINT DATA
C NDOP = # OF GLOBAL DOP
C NLOAD = # OF LOAD COMBINATIONS
C R = LOCAL DIMENSION OF NODE ARRAY
C NNODE = # OF NODES
C ID = ARRAY OF EXTERNAL NODE NUMBERS
C IOP = ARRAY OF DEGREES OF FREEDOM
C COOR = ARRAY OF COORDINATES
C COSINE = ARRAY OF DIRECTION COSINES OF NODES
C CONST = ARRAY OF CONSTRAINT TRANSFORMATIONS
C DISPL = GLOBAL DISPLACEMENT MATRIX
C N = REAL GLOBAL STORAGE VECTOR
C NZ = INEGR GLOBAL STORAGE VECTOR
C IZMAT = LOCATION OF MATERIAL DATA IN GLOBAL STORAGE VECTOR
C NAME = ELEMENT NAME
C IELNO = ELEMENT NUMBER
C RINPUT = INPUT DATA
C STIFF = OUTPUT ELEMENT STIFFNESS (UPPER TRIANGULAR FORM)
C LM = OUTPUT LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C----- ELEMENT VARIABLES
C H,K,G,AI,AS,B,IL,B,82,826,8212,833,835,8311,D
C = INDIVIDUAL TERMS IN THE ELEMENT STIFFNESS (6) MATRIX
C PKG = INPUT LOAD FOR FORMING GEOMETRIC STIFFNESS MATRIX...
C R = INCREMENTAL DISPLACEMENTS
C FT = TOTAL DISPLACEMENTS
C IF = INCREMENTAL FORCES
C FT = TOTAL FORCES
C LM = LOCAL TO GLOBAL STIFFNESS MAPPING MATRIX
C S = LOCAL TO GLOBAL ROTATION MATRIX START
C CTE = LOCAL TO GLOBAL ROTATION MATRIX END
C PTE = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX START
C BE = LOCAL TO GLOBAL ROTATED ECCENTRICITY/CONSTRAINT MATRIX END
C STIFF = ELEMENT STIFFNESS
C IELTYP = ELEMENT TYPE = 10
C NODEI = EXTERNAL JOINT NUMBER, END I
C NODEJ = EXTERNAL JOINT NUMBER, END J
C JOINTI = INTERNAL JOINT NUMBER, END I
C JOINTJ = INTERNAL JOINT NUMBER, END J
C----- TEMPORARY VARIABLES
C VY = VECTOR DEFINING THE ELEMENT X AXIS
C VY = VECTOR DEFINING THE ELEMENT Y AXIS
C COSLOC = LOCAL COSINE MATRIX
C A = GLOBAL TO LOCAL FORCE TRANSFORMATION MATRIX
C SAT = ELEMENT STIFFNESS AT LOCAL COORD, AT ENDS OF FLEXIBLE PART
C ASAT = PRODUCT OF S*A
C----- CHOOSE OPTION
IF (IOPT.LT.1 .OR. IOPT.GT.14) THEN
WRITE(108,*) 'INVALID OPTION IN ELE-02, IOPT=',IOPT
STOP
ENDIF
C
GO TO (100,200,300,400,500,600,700,800,900,1000,1100,
1200,1300,1400) IOPT
C
100 CONTINUE
C----- INTERPRETATE INPUT DATA
IELTYP = 3
MATS(1) = RINPUT(1)
MATS(2) = RINPUT(2)
MATS(3) = RINPUT(3)
NODEI = RINPUT(4)
NODEJ = RINPUT(5)
NODES = RINPUT(6)
NODEA = RINPUT(7)
ALPH = RINPUT(8)
PKG = RINPUT(9)
C
GET INTERNAL NODE # ASSOCIATED WITH EXTERNAL NODE #'S
JK(1)=IQUICK(NODEI,ID,NNODE)
JK(2)=IQUICK(NODEJ,ID,NNODE)
JK(3)=IQUICK(NODES,ID,NNODE)
JK(4)=IQUICK(NODEA,ID,NNODE)
C
GET GLOBAL TO LOCAL TRANSFORMATION MATRIX
DO 105 J=1,3
DEFINE VECTOR X, AT BOTTOM OF THE WALL
VXT(J)=COORD(JK(4),J)-COORD(JK(3),J)
C
DEFINE VECTOR Y, AT TOP OF THE WALL
VYT(J)=COORD(JK(1),J)-COORD(JK(2),J)
C
CALCULATE Y VECTOR ...
VY(J)=(COORD(JK(1),J)-COORD(JK(2),J)-
COORD(JK(3),J)-COORD(JK(4),J))/2.
105 CONTINUE
C
CALCULATE AVERAGE LENGTH,ALPHA AND BETA ...
LENGTH=SQRT(VY(1)**2+VY(2)**2+VY(3)**2)
IF (LENGTH.LE.0) THEN
WRITE(108,101) IELNO,LENGTH
101 FORMAT('X,20(')',I6,' ',NCR,' LENGTH IS LE 0 /
5X,' IELNO=',I5,5X,' LENGTH',I6,6F
5X,' REVERSE INPUT ...')
LENGTH=1.0
ENDIF
C
ALPHA=ALPH/LENGTH
BETA = LENGTH*ALPHA
C
NORMALIZE Y VECTOR ...
VY(1)=VY(1)/LENGTH
VY(2)=VY(2)/LENGTH
VY(3)=VY(3)/LENGTH
C
CALCULATE THE WIDTH ...
CALL VCROSS(VZT,VY,VXT)
CALL VCROSS(VZB,VY,VXB)
WT = SORT(VZT(1)**2 + VZT(2)**2 + VZT(3)**2)
WB = SORT(VZB(1)**2 + VZB(2)**2 + VZB(3)**2)
W = (WT-VB)/2.0
C
CHECK FOR TWIST
TWIST=ACOS(WCOS(VZT,VZB))*57.2958

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IF (ABS(TWIST).GT. 5.0) WRITE(108,102) TWIST
102 FORMAT(5X,10(' '), 'WARNING THE WALL IS TWISTED-',IP,G12.4,
' DEGREES, REVISE INPUT AND RERUN')
C
C----- CALCULATE VK, AND NORMALIZE
CALL VCROSS(VX,VY,VZB)
VK=SQRT(VX(1)**2+VX(2)**2+VX(3)**2)
VX(1)=VX(1)/VK
VX(2)=VX(2)/VK
VX(3)=VX(3)/VK
C
C----- CHECK FOR TWIST
C
C----- GET LOCAL TO GLOBAL TRANSFORMATION MATRICES
C----- JOINT #1 .. #4
DO 30 I=1,4
CALL ROTN2X(VX,VY,COSINE(1,1),JTCOS(JK(L)),CT,
0,0,0,BS,CONST(JK(L),1),MAXNCD)
DO 20 J=1,3
AA(L,I,J)=CT(I,J)
AA(L,1,3)=BS(I,J)
20 CONTINUE
C----- ZERO MATRICES
DO 25 I=1,3
PMA(I)=0
PMB(I)=0
PT(I)=0
R(I)=0
RT(I)=0
V(I)=0
VT(I)=0
25 CONTINUE
DO 27 I=1,34
LM(I)=0
LMKG(I)=0
DO 26 J=1,3
A(I,J)=0.00
DO 27 K=1,6
AT(I,K)=0.00
27 CONTINUE
C----- ASSEMBLE TRANSFORMATION MATRIX A - AS X A4 X A3
C NOTE: AT IS DIMENSIONED AS A 24X6 MATRIX, BUT ONLY THE FIRST IELDOP
ROW'S CONTAIN INFORMATION. THE BALANCE OF THE MATRIX IS
UNDEFINED. THE GLOBAL DOP CORRESPONDING TO THE ROW'S OF A
ARE STORED IN VECTOR LM. AGAIN, ONLY THE FIRST IELDOP ROW'S
OF LM ARE DEFINED.
IELDOP=0
DO 50 J=1,6
C
C REMOVE DOP'S THAT ARE CONSTRAINED TO THE SAME DOP..
IF ((IDOP(JK(1),J).EQ.IDOP(JK(4),J)) OR
(IDOP(JK(1),J).EQ.IDOP(JK(3),J)) OR
(IDOP(JK(2),J).EQ.IDOP(JK(4),J)) OR
(IDOP(JK(2),J).EQ.IDOP(JK(3),J))) GO TO 50
C
IELDOP=IELDOP+1
LMTI=IDOP(JK(1),J)
K0=I-1
IF (I.EQ.4) K0=I-2
AT(IELDOP,K0)=AA(I,J,2)
IF (I.EQ.1.OR.I.EQ.4) AT(IELDOP,K0-1)=AA(I,J,1)
C
LM(IELDOP)=LMTI
IF (AT(IELDOP,1).EQ.0) AND (AT(IELDOP,2).EQ.0) AND
(AT(IELDOP,3).EQ.0) AND (AT(IELDOP,4).EQ.0) AND
(AT(IELDOP,5).EQ.0) AND (AT(IELDOP,6).EQ.0)) THEN
IELDOP=IELDOP-1
ELSE IF (IELDOP.GT.1) THEN
DO 49 I=1,IELDOP-1
IF (LM(I).EQ.LM(IELDOP)) THEN
DO 48 J=1,6
AT(I,J)=AT(I,J)+AT(IELDOP,J)
AT(IELDOP,J)=0
IELDOP=IELDOP-1
GO TO 50
49 CONTINUE
ENDIF
ENDIF
50 CONTINUE
C----- AS21 FOR STIFFNESS ONLY...
AC(1,1)=0.
AC(2,1)=0.
AC(3,1)=1./W
AC(4,1)=-1./W
AC(5,1)=0.
AC(6,1)=1./W
AC(1,2)=1.
AC(2,2)=ALPHA/W
AC(3,2)=-ALPHA/W
AC(4,2)=BETA/W
AC(5,2)=1.
AC(6,2)=BETA/W
AC(1,3)=0.
AC(2,3)=1./2.
AC(3,3)=1./2.
AC(4,3)=-1./2.
AC(5,3)=0.
AC(6,3)=-1./2.
DO 55 I=1,IELDOP
DO 55 J=1,3
A(I,J)=0.
DO 55 K=1,6
A(I,J)=A(I,J)+AT(I,K)*AC(K,J)
55 CONTINUE
C----- GET SPRING STIFFNESS
FALSE=FALSE
LSTYP=0
DO 56 I=1,3
CALL MATLIB(2,LSTYP,FALSE,PSEB(I),DUCT(1,I),EXCR(1,I),
PT,F,RT,R,G,0.,LELM,LMAT,MATB(1),MATTP,
SE(ISEB(I)),IELNO,DAMAGE)
56 CONTINUE
C
ESE=0.
PSE=0.
C----- SET UP THE LOCAL STIFFNESS MATRIX
SB=SE(ISEB(1))
SS=SE(ISEB(2))
SA=SE(ISEB(3))
C
C----- CALCULATE SAT
SBL=SB/LENGTH
SSL=SS/LENGTH
SAL=SA/LENGTH
DO 60 J=1,IELDOP
SAT(1,J)=SBL*A(J,1)
SAT(2,J)=SSL*A(J,2)
60 SAT(3,J)=SAL*A(J,3)
C----- CALCULATE ASAT AND CONVERT TO HALF STORAGE MODE BY COLUMNS
1 3 6 10 15 21 28 36 45 55 66 78 300 NUMBER INDICATES
2 5 9 14 20 27 35 44 54 65 77 LOCATION OF DATA
4 8 13 19 26 34 43 53 64 76 IN ARRAY STIFF
7 12 18 25 32 42 52 63 75
11 17 24 32 41 51 62 74
16 23 31 40 50 61 73
22 30 39 49 60 72
29 38 48 59 71
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ELE01630 C 37 47 58 70
ELE01640 C 46 57 69
ELE01650 C 56 68
ELE01660 C 67
ELE01670 C
ELE01680 C
ELE01690 C
ELE01700 C
L=0
ELE01710 DO 70 J=1,IELDOP
ELE01720 DO 70 I=J,1,-1
ELE01730 L=L+1
ELE01740 STIFF(L)=0
ELE01750 DO 70 K=1,3
70 STIFF(L)=STIFF(L)+A(I,K)*SAT(K,J)
C
C ASSEMBLE GEOMETRIC STIFFNESS MATRIX FOR A UNIT LOAD.
ELE01760 C
ELE01770 C
ELE01780 C
ELE01790 C
ELE01800 C
ELE01810 C
ELE01820 IF (PKG.EQ.0 AND KGDATA(1).NE.2) GO TO 180
ELE01830 IF (KGDATA(1).EQ.0 OR KGDATA(2).EQ.0) GO TO 180
ELE01840 AXIAL=.TRUE.
C
C----- DETERMINE THE AXIAL LOAD TO BE USED FOR KG..
ELE01850 C
ELE01860 C
ELE01870 IF (KGDATA(1).EQ.1) THEN
ELE01880 PGEOM = PKG
ELE01890 ELSE IF (KGDATA(1).EQ.2) THEN
ELE01900 PGEOM = -P(3)
ELE01910 ENDFIF
C
C----- ASSEMBLE TRANSFORMATION MATRIX A
ELE01920 C
ELE01930 C
ELE01940 C
ELE01950 C
ELE01960 C
ELE01970 C
ELE01980 C
ELE01990 DO 140 I=1,24
ELE02000 DO 140 K=1,6
140 AT(I,K)=0.00
ELE02010 KGDOP=0
ELE02020 DO 145 I=1,4
ELE02030 DO 145 J=1,6
ELE02040 KGDOP=KGDOP+1
ELE02050 LMTI=IDOP(JK(1),J)
ELE02060 K0=I-1
ELE02070 IF (I.EQ.4) K0=I-2
ELE02080 AT(KGDOP,K0)=AA(I,J,3)
ELE02090 IF (I.EQ.1.OR.I.EQ.4) AT(KGDOP,K0-1)=AA(I,J,1)
ELE02100 LMKG(KGDOP)=LMTI
ELE02110 IF (AT(KGDOP,1).EQ.0) AND (AT(KGDOP,2).EQ.0) AND
ELE02120 (AT(KGDOP,3).EQ.0) AND (AT(KGDOP,4).EQ.0) AND
ELE02130 (AT(KGDOP,5).EQ.0) AND (AT(KGDOP,6).EQ.0)) THEN
ELE02140 KGDOP=KGDOP-1
ELE02150 ELSE IF (KGDOP.GT.1) THEN
ELE02160 IF (LMKG(I).EQ.LMKG(KGDOP)) THEN
ELE02170 DO 148 J=1,6
ELE02180 AT(I,J)=AT(I,J)+AT(KGDOP,J)
ELE02190 AT(KGDOP,J)=0
ELE02200 KGDOP=KGDOP-1
148 GO TO 145
ELE02210 ENDFIF
149 CONTINUE
ELE02220 ENDFIF
145 CONTINUE
ELE02230 C
ELE02240 C
ELE02250 C
ELE02260 C
ELE02270 C
ELE02280 C
ELE02290 C
ELE02300 C
ELE02310 C
ELE02320 C
ELE02330 C
ELE02340 C
ELE02350 C
ELE02360 C
ELE02370 C
ELE02380 C
ELE02390 C
ELE02400 C
ELE02410 C
ELE02420 C
ELE02430 C
ELE02440 C
ELE02450 C
ELE02460 C
ELE02470 C
ELE02480 C
ELE02490 C
ELE02500 C
ELE02510 C
ELE02520 C
ELE02530 C
ELE02540 C
ELE02550 C
ELE02560 C
ELE02570 C
ELE02580 C
ELE02590 C
ELE02600 C
ELE02610 C
ELE02620 C
ELE02630 C
ELE02640 C
ELE02650 C
ELE02660 C
ELE02670 C
ELE02680 C
ELE02690 C
ELE02700 C
ELE02710 C
ELE02720 C
ELE02730 C
ELE02740 C
ELE02750 C
ELE02760 C
ELE02770 C
ELE02780 C
ELE02790 C
ELE02800 C
ELE02810 C
ELE02820 C
ELE02830 C
ELE02840 C
ELE02850 C
ELE02860 C
ELE02870 C
ELE02880 C
ELE02890 C
ELE02900 C
ELE02910 C
ELE02920 C
ELE02930 C
ELE02940 C
ELE02950 C
ELE02960 C
ELE02970 C
ELE02980 C
ELE02990 C
ELE03000 C
C----- SB, SS AND SA CONTAIN THE STIFFNESSES, WHICH WERE DETERMINED IN
ELE03010 C
ELE03020 C
ELE03030 C
ELE03040 C
ELE03050 C
ELE03060 C
ELE03070 C
ELE03080 C
192 FORMAT(' ELEMENT 03, R/C SHEAR WALL ELEMENT '// T1,
' ELEM', BEND, SHEAR, AXIAL, JOINT, JOINT,
' JOINT', JOINT, LENGTH, WIDTH,
' ALPHA', PKG, '/,T1,
' MATL', MATL, MATL, '#1', '#2',
' #3', '#4')
193 FORMAT(1X,A15.816,2X,1P,G12.4)
194 RETURN
C----- DETERMINE GEOMETRIC STIFF -----
200 CONTINUE
C
C KG IS DETERMINED FOR A UNIT LOAD WITH IOPT=1.
C THE ACTUAL LOAD (PGEOM) IS DETERMINED HERE.
C KG IS MULTIPLIED BY PGEOM WHEN THE TOTAL STIFFNESS IS ASSEMBLED
C NEGATIVE PGEOM IS COMPRESSION...
C
C AXIAL=.TRUE.
C----- DETERMINE THE AXIAL LOAD TO BE USED FOR KG..
IF (KGDATA(1).EQ.1) THEN
PGEOM = PKG
ELSE IF (KGDATA(1).EQ.2) THEN
PGEOM = -P(3)
ENDIF
C
C RETURN
C
C----- DETERMINE STIFFNESS -----
300 CONTINUE
C
C----- SB, SS AND SA CONTAIN THE STIFFNESSES, WHICH WERE DETERMINED IN
ELE04470 C
ELE04480 C
ELE04490 C
ELE04500 C
ELE04510 C
ELE04520 C
ELE04530 C
ELE04540 C
ELE04550 C
ELE04560 C
ELE04570 C
ELE04580 C
ELE04590 C
ELE04600 C
ELE04610 C
ELE04620 C
ELE04630 C
ELE04640 C
ELE04650 C
ELE04660 C
```

```

C..... SET UP THE LOCAL STIFFNESS MATRIX
SB= SE(ISEB(1))
SS= SE(ISEB(2))
SA= SE(ISEB(3))
SBL= SE(ISEB(1))/LENGTH
SSL= SE(ISEB(2))/LENGTH
SAL= SE(ISEB(3))/LENGTH
C
C..... CALCULATE SAT
DO 360 J=1,IELDOP
SAT(1,J)=SBL*A(J,1)
SAT(2,J)=SS*A(J,2)
360 SAT(3,J)=SAL*A(J,3)
C
C..... CALCULATE ASAT, STORE IN UPPER TRIANGULAR MATRIX
L=0
DO 370 J=1,IELDOP
DO 370 I=J,1,-1
L=L+1
STIFF(L)=0
DO 370 K=1,3
STIFF(L)=STIFF(L)+A(I,K)*SAT(K,J)
C
RETURN
C..... DETERMINE INCREMENTAL FORCES
400 CONTINUE
IF (PRINT.AND.FIRST) WRITE(108,491) STEPID
C
C..... SET UP THE COMPONENT STIFFNESSES
SB= SE(ISEB(1))
SS= SE(ISEB(2))
SA= SE(ISEB(3))
C
DO 430 LOAD=1,NLOAD
C
C..... TRANSFORM FRAME DISPL INTO LOCAL DISPL
DO 410 I=1,3
R(I)=0
V(I)=0
DO 410 J=1,IELDOP
K=LM(J)
R(I)=R(I)+A(J,I)*DISPL(K,LOAD)
410 V(I)=V(I)+A(J,I)*VELOC(K)
C
C..... INCREMENTAL FORCES
P(1)=SB*R(1)/LENGTH
P(2)=SS*R(2)/LENGTH
P(3)=SA*R(3)/LENGTH
C
C..... CHECK STIFFNESS AND CALCULATE UNBALANCED FORCES
THIS IS PERFORMED ONLY FOR INELASTIC ANALYSIS
P(1)=PT(1)+F(1)
P(2)=PT(2)+F(2)
P(3)=PT(3)+F(3)
IF (.NOT.ELSTIC) THEN
C
C..... TRANSFER PERMANENT HYSTERESIS DATA TO TEMPORARY STORAGE...
ISE2=ISE/2
DO 415 I=1,ISE2
SE(I)=ISE2+SE(I)
415 DO 420 I=1,3
C
C..... BENDING .....
C..... SHEAR .....
C..... AXIAL .....
C
C..... SET UP TEMPORARY TOTAL LOADS, DISPL'S AND VELOC
PL=PT(I)
D=(RT(I)-R(I))/LENGTH
DL=RT(I)-R(I)
VC=VT(I)-V(I)
VL=VT(I)
C
C..... CALL MESH MODEL TO GET NEW STIFFNESS AND
THE TOTAL FORCE P AT DISPL D
CALL MATLIB(3,LSSTYP,FALSE,ESEB(I),PSEB(I),DUCT(1,I),EXCR(1,I),
P(1),PL,D,DL,VC,VL,LELEM,LMAT,MATB(I),MATTYP,
SE(ISE2+ISEB(I)),IELNO,DAMAGE)
C
IF (SE(ISE2+ISEB(I)).NE.SE(ISEB(I))) NEWK=.TRUE.
420 CONTINUE
ENDIF
C
IF (ELSTIC) THEN
ESE=0
DO 425 I=1,3
ESEB(I)=0.50*(PT(I)+F(I))*(RT(I)+R(I))
ESE=ESE+ESEB(I)
PSE=0
ELSE
ESE=(ESEB(1)+ESEB(2)+ESEB(3))*LENGTH
PSE=(PSEB(1)+PSEB(2)+PSEB(3))*LENGTH
ENDIF
C
C..... SAVE MAXIMUM LOADS
IF (NLOAD.GT.1) THEN
DO 389 I=1,3
IF (ABS(F(I)).GT.ABS(PMAX(I))) THEN
PMAX(I)=F(I)
FMAX(I)=R(I)
389 CONTINUE
ENDIF
C
C..... PRINT DATA
IF (PRINT) WRITE(108,392) IELNO,LOAD,ID(JK(1)),ID(JK(2)),
ID(JK(3)),ID(JK(4)),(PT(I),R(I),I=1,3)
392 FORMAT (I10,15,2X,4I5,1P,6G15.6)
C
C..... CALCULATE THE UNBALANCED MEMBER FORCE ON A JOINT.
IF (LOAD.EQ.1) THEN
IF (LOAD.EQ.1) THEN
UBM=- ( F(1)+PT(1)-P(1) )
UBS=- ( F(2)+PT(2)-P(2) )
UBA=- ( F(3)+PT(3)-P(3) )
DO 440 I=1,IELDOP
K=LM(I)
EPUB(I)=A(I,1)*UBM+A(I,2)*UBS+A(I,3)*UBA
440 FUB(K)=FUB(K)+EPUB(I)
ENDIF
P(1)=P(1)+F(1)
P(2)=P(2)+F(2)
P(3)=P(3)+F(3)
430 CONTINUE
C
THIS IS FOR SOLOSA ONLY
IF (ICOMN.EQ.1 OR ICOMN.EQ.2) THEN
DO 431 I=1,3
RSAP(I)=F(I)
431 ENDIF
C
491 FORMAT (/ ' R/C SHEAR WALL FORCES. ' ,5X,A/
' ELEMENT LOAD,'T17,' JNT-1',' JNT-2',' JNT-3',' JNT-4',
' MOMENT ' , ' ROTATION ' , ' SHEAR ' ,
' SHEAR DISPL ' , ' AXIAL ' , ' AXIAL DISPL ' //)
C
IOPT=IOPT
RETURN
C..... DETERMINE TOTAL FORCES, ENERGIES ECT
IF (ICOMN.NE.1.AND.ICOMN.NE.2) THEN
IF (PRINT.AND.FIRST) WRITE(108,491) STEPID
C
C..... TOTAL FORCES, DISPLACEMENTS AND VELOCITIES
DO 510 I=1,3
PT(I)=PT(I)+F(I)
RT(I)=RT(I)+R(I)
VT(I)=VT(I)+V(I)
IF (ABS(PT(I)).GT.ABS(PMAX(I))) THEN
FMAX(I)=PT(I)
RMAX(I)=RT(I)
ENDIF
ENDIF
C..... DETERMINE FIXED END FORCES, AND ADD TO FORCE
600 RETURN
C..... FIXED END FORCES ARE NOT AVAILABLE FOR R/C SHEAR WALL
C..... WRITE MEMBER FORCES TO OUTPUT FILE
700 CONTINUE
C..... BENDING ENERGY, DUCTILITY AND EXCURSION
IF (ELSTIC) THEN
ESE=0
DO 704 I=1,3
ESEB(I)=0.50*(PT(I)+F(I))*(RT(I)+R(I))
ESE=ESE+ESEB(I)
PSE=0
ELSE
ESE=(ESEB(1)+ESEB(2)+ESEB(3))*LENGTH
PSE=(PSEB(1)+PSEB(2)+PSEB(3))*LENGTH
ENDIF
IP (PRINT) WRITE (IUNIT,710) (ID(JK(I)),I=1,4)
WRITE (IUNIT,711) (PT(I),RT(I),I=1,3),ESE,PSE
710 FORMAT (4I5,1P,6E10.3/(8E10.3) )
711 FORMAT (1P,8E10.3)
C
RETURN
C..... READ AND PRINT MEMBER FORCES FROM OUTPUT FILE
800 CONTINUE
SX=0
SXX=0
SY=0
SXY=0
SN=0
SNX=0
SNY=0
BACKSPACE(IUNIT)
READ (IUNIT,' ') ITYPE,IELNO,IWRITE,TO,DT
READ (IUNIT,710) NODE1,NODE2,NODE3,NODE4
ISTEP=IWRITE
AVEMV=0
ICOUNT=0
C
810 READ (IUNIT,815,END=930) (P(I),R(I),I=1,3),ESE,PSE
815 FORMAT (8E10.3)
ISTEP=ISTEP-IWRITE
T=TO-DT*ISTEP
IF (ISTEP.EQ.0) WRITE(108,805) NODE1,NODE2,NODE3,NODE4
IF (NODE1.NE.0) THEN
WRITE(108,820) ISTEP,T,(P(I),R(I),I=1,3),ESE,PSE
805 SX = SX + F(2)
820 SXX = SXX + F(2)*F(2)
SY = SY + F(1)
830 SXY = SXY + F(1)*F(2)
SN = SN + 1
IF (F(2).NE.0) THEN
RATMV=RATMV+F(1)/F(2)
AVEMV=AVEMV+RATMV
ICOUNT=ICOUNT+1
ENDIF
GO TO 810
C
830 CONTINUE
AVEMV=AVEMV/MAX(ICOUNT,1)
WRITE(108,831) AVEMV,ICOUNT
IF (SY.NE.0) THEN
RM=SY/SX
ICOUNT=SN
WRITE(108,833) RM,ICOUNT
ENDIF
DM = SN*SXX + SX*SX
DB = SN*SXY + SX*SN
IF (DM.NE.0.AND.DB.NE.0) THEN
RM = (SXY*SX + SY*SXY) / DB
ICOUNT=SN
WRITE(108,832) RM,ICOUNT
ENDIF
831 FORMAT (/T10, ' AVERAGE MOMENT/SHEAR RATIO=' ,IP,G12.4,5X,OP,16,
' POINTS WERE USED, ' POINTS WITH V=0 ARE EXCLUDED')
832 FORMAT (T10, ' REGRESSION MOMENT-SHEAR EQU: '
' M=' ,IP,G12.4, ' V=' ,IP,G12.4,5X,OP,16, ' POINTS WERE USED')
833 FORMAT (T10, ' REGRESSION MOMENT-SHEAR EQU: '
' M=' ,IP,G12.4, ' V=' ,IP,G12.4,5X,OP,16, ' POINTS WERE USED')
805 FORMAT (/ ' R/C SHEAR WALL FORCES. ' ,
' STEP TIME ' , ' MOMENT ' , ' ROTATION ' ,
' SHEAR ' , ' SHEAR DISPL ' ,
' AXIAL ' , ' AXIAL DISPL ' ,
' ESE ' , ' PSE ' //)
820 FORMAT (16,1P,G12.4,6G16.5,2G12.4)
RETURN
C
C..... DETERMINE THE LENGTH OF THE MATERIAL DATA
900 CONTINUE
ISE=0
DO 910 I=1,3
ISEB(I)=ISE+SE(I)
CALL MATLIB(0,LSSTYP,FALSE,ESE,PSE,DUCT(1,2),EXCR(1,2),
PT,F,RT,R,0,0,LELEM,LMAT,MAT,MATTYP,SE,IELNO,DAMAGE)
910 ISE=ISE+LELEM
C
C..... DOUBLE THE STORAGE, TO ALLOW FOR TEMPORARY VALUES
ISE=ISE*2
C
RETURN
C..... DETERMINE THE STRAIN ENERGY
1000 CONTINUE
C..... DUCTILITEIS AND EXCURSION RATIOS

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1100 CONTINUE
C ... NOTE ELE03 WAS LAST CALLED TO CALC INCORPORATED DISPL.
C THE ENERGIES ASSOCIATED WITH INCR DISPL ARE STORED IN
C THE ADDRESSES: ISEB=ISE/2
C ISEB=ISE/2
C ISEA=ISE/2
C ... BENDING ENERGY, DUCTILITY AND EXCURSION ...
DO 1105 I=1,3
ISEB=ISE/2
DO 1105 I=1,3
CALL MATLIB(4,LSTTYP,FALSE,ESEB(I),PSEB(I),DUCT(1,I),EXCR(1,I),
FT,P,RT,R,0,0,LELEM,LMAT,MATB(I),MATTYP,
SE(ISEB(I)+ISE/2),IELNO,DAMAGE)
1105 CONTINUE
C ... SHEAR ENERGY, DUCTILITY AND EXCURSION ...
C ... AXIAL ENERGY, DUCTILITY AND EXCURSION ...
IF ( DUCFAG EQ 0 AND ( ABS(DUCT(1,1)) GE 1 OR
ABS(DUCT(1,2)) GE 1 OR ABS(DUCT(1,3)) GE 1 ) THEN
DUCFAG=ABS(DUCT(1,1))
IF ( DUCFAG LT ABS(DUCT(1,2)) ) DUCFAG=ABS(DUCT(1,2))
IF ( DUCFAG LT ABS(DUCT(1,3)) ) DUCFAG=ABS(DUCT(1,3))
ENDIF
C
IF (ELSTIC) THEN
ESE=0
DO 1106 I=1,3
ESE(I)=0.5*(PT(I)-P(I))*(RT(I)-R(I))
ESE=ESE+ESE(I)
PSE=0.0
ELSE
ESE=(ESEB(1)+ESEB(2)+ESEB(3))*LENGTH
PSE=(PSEB(1)+PSEB(2)+PSEB(3))*LENGTH
ENDIF
C
IF (PRINT.AND.FIRST) WRITE(108,1191)
IF (PRINT) WRITE(108,1192) IELNO,(DOCT(I,J),EXCR(I,J),I=1,3),
$
1192 FORMAT (1X,15,18F7.2)
1191 FORMAT (/ / R/C SHEAR WALL DUCTILITY AND EXCURSION RATIOS*/6X,
$
$ BENDING COMPONENT
$ SHEAR COMPONENT
$ AXIAL COMPONENT
$ DEPN 1
$ DEPN 2
$ DEPN 3
$ DUCT EXCR DUCT EXCR DUCT EXCR
$ DUCT EXCR DUCT EXCR DUCT EXCR
$ DUCT EXCR DUCT EXCR DUCT EXCR
RETURN
C
----- WRITE MAXIMUM AND MINIMUM VALUES
1200 CONTINUE
IF (FIRST) WRITE(108,491)
$
$ 'MAXIMUM LOADS AND DISPL AT MAXIMUM LOADS'
$
$ 'NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY'
WRITE(108,492) IELNO,LOAD,ID(JK(1)),ID(JK(2)),ID(JK(3)),
$
$ ID(JK(4)),FMAX(1),FMAX(3-1),I=1,3)
RETURN
C
----- RESET INTERNAL FORCES
1300 CONTINUE
C
C ... ZERO MATRICES
DO 1310 I=1,ISE
1310 SE(I)=0
DO 1325 I=1,3
FMAX(I)=0
F(I)=0
FT(I)=0
R(I)=0
RT(I)=0
V(I)=0
VT(I)=0
1325 WRITE(108,1330) IELNO
1330 FORMAT (' SHEAR WALL ... ELEMENT #',I6,
$
$ ' INTERNAL FORCES AND HYSTERESIS MODELS ARE RESET TO ZERO')
DO 1335 I=1,3
CALL MATLIB(2,LSTTYP,FALSE,ESEB(I),PSEB(I),DUCT(1,I),EXCR(1,I),
FT,P,RT,R,0,0,LELEM,LMAT,MATB(I),MATTYP,
SE(ISEB(I)),IELNO,DAMAGE)
1305 CONTINUE
C
RETURN
C
----- DAMAGE INDEX
1400 IF (ELSTIC) RETURN
C ... BENDING ENERGY, AND DAMAGE INDEX
DAMAGE=0
DO 1405 I=1,3
CALL MATLIB(5,LSTTYP,FALSE,ESEB(I),PSEB(I),DUCT(1,I),EXCR(1,I),
$
$ FMAX(1),0,FMAX(1-3),0,0,LELEM,LMAT,MATB(I),
$
$ MATTYP,SE(ISEB(I)),IELNO,DAMB(I))
1405 CONTINUE
C
ESE=(ESEB(1)+ESEB(2)+ESEB(3))*LENGTH
PSE=(PSEB(1)+PSEB(2)+PSEB(3))*LENGTH
DAMAGE=DAMAGE+LENGTH
IF (FIRST) WRITE(108,1420)
1410 FORMAT (1X,15,1P,3F14.5,2G14.5)
1420 FORMAT (/6X,
$
$ BENDING COMPONENT
$ SHEAR COMPONENT
$ AXIAL COMPONENT
$ DAMAGE
$ DAMAGE
$ DAMAGE
RETURN
END
C
-----
SUBROUTINE ELE03
C IOPT = FIRST , PRINT , NAME , IELNO
C ID = IDOP , COORD , COSINE , JTCOS
C CNST = DISPL , VELOC , PUB , IREL
C RINPUB = ESE , EPSE , DAMAGE , DUCFAG
C IELTYP = IELDOP , NODEI , NODEJ
C JOINTJ = R , FT
C V = VT , ISE , H , LENGTH
C MAT = A , LM , KTYPE , FMAX
C SE = STIFF
C
IMPLICIT REAL(A-H,O-Z)
COMMON /RSA/ICOND,RSAP(12)
COMMON /IDSEI/STEP
REAL ICOWN,LENGTH
LOGICAL FIRST,PRINT,FALSE,BTEST
CHARACTER*80 NAME
CHARACTER*8 TYPE(1)
INCLUDE:'COMMON'
DIMENSION IDOP(MAXNOD,6),COORD(MAXNOD,6),COSINE(3,3,NCOS)
DIMENSION CNST(MAXNOD,6),ID(MAXNOD),JTCOS(MAXNOD)
DIMENSION SAT(12,12),STIFF(FM,4),S(2,2),LM(12),FMAX(2)
DIMENSION CTS(3,3),CTE(3,3),VX(3),VY(3),VELOC(DNDF)
DIMENSION BS(3,3),BE(3,3),SE(ISE)
DIMENSION DISPL(DNDF,NLOAD),PUB(DNDF)
DIMENSION RINPUB(100),WORK(6),DUCT(3),EXCR(6)
DATA TYPE/' AXIAL'/
C
C ... ZERO MATRICES
F=0
PT=0
R=0
RT=0
V=0
VT=0
FMAX(1)=0
FMAX(2)=0
C ... ASSEMBLE TRANSFORMATION MATRIX A
C NOTE: A IS DIMENSIONED AS A 12X2 MATRIX, BUT ONLY THE FIRST IELDOP
C ROWS CONTAIN INFORMATION. THE BALANCE OF THE MATRIX IS
C UNDEFINED. THE GLOBAL DOP CORRESPONDING TO THE ROWS OF A
C ARE STORED IN VECTOR LM. AGAIN, ONLY THE FIRST IELDOP ROWS
C OF LM ARE DEFINED.
IELDOP=0
DO 50 I=1,12
C
C ... REMOVE DOP'S THAT ARE CONSTRAINED TO THE SAME DOP.

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J=I
IP (1,GT,6) J=I-6
IP (1,IDOF(JOINTI,J),EQ,IDOF(JOINTJ,J)) GO TO 50
IP (1,LE,6) THEN
  LMTI-IDOF(JOINTI,1)
ELSE
  LMTI-IDOF(JOINTJ,J)
ENDIF
C
A1=0
A2=0
IF (1,LE,3) THEN
  IF (KTYPE,LE,3) A1-CTS(1,KTYPE)
ELSE IF (1,LE,6) THEN
  IF (KTYPE,LE,3) THEN
    A1-BS(1-3,KTYPE)
  ELSE
    A1-CTS(1-3,KTYPE-3)
  ENDIF
ELSE IF (1,LE,9) THEN
  IF (KTYPE,LE,3) A2-CTE(1-6,KTYPE)
ELSE
  IF (KTYPE,LE,3) THEN
    A2-BS(1-3,KTYPE)
  ELSE
    A2-CTE(1-3,KTYPE-3)
  ENDIF
ENDIF
IF (A1,EQ,0 .AND. A2,EQ,0) GO TO 50
IELDOP=IELDOP-1
A(IELDOP,1)=A1
A(IELDOP,2)=A2
LM(IELDOP)=LMTI
C
50 CONTINUE
C----- GET GOEL STRUT STIFFNESS
FALSE=.FALSE.
LSTTYP=0
CALL MATLIB(2,LSTTYP,FALSE,ESE,EPSE,DUCT,EXCR,
  & FT,F,RT,R,0,0,LELEM,LMAT,MAT,MATTF,SE,IELNO,DAMAGE)
C----- H IS THE STRUT STIFFNESS
H = SE(1)
HM= SE(1)
S(1,1)= HM
S(1,2)= HM
S(2,1)= HM
S(2,2)= HM
C----- CALCULATE SAT
DO 60 J=1,1
  DO 60 J=1,IELDOP
    SAT(I,J)=0
  DO 60 K=1,2
    SAT(I,J)=SAT(I,J)+S(I,K)*A(J,K)
  60
C----- CALCULATE ASAT AND CONVERT TO HALF STORAGE MODE BY COLUMNS
L=0
DO 70 J=1,IELDOP
  DO 70 I=J,1,-1
    L=L+1
    STIFF(L)=0
    DO 70 K=1,2
      STIFF(L)=STIFF(L)+A(I,K)*SAT(K,J)
  70
C----- RELEASE UNUSED STORAGE
180 IREL=78-L
C----- PRINT COLUMN DATA
IF (FIRST) WRITE (108,192)
191 WRITE(108,193) NAME,IELNO,MAT,NODEI,NODEJ,TYPE(KTYPE),LENGTH,
  & VY(1),VY(2),VY(3),XS,XE
C
192 FORMAT(/' ELEMENT NO, ONE (LOCAL) DOP STRUT ELEMENT//
  & T22,' MATL START END,2X,' TYPE---,6X,' LENGTH',3X,
  & 11,'-',1) Y-AXIS ',12,'-',1) START DIST END DIST')
193 FORMAT(1X,A15,4E,2X,A9,3X,1P,G12.4,9F,
  & P9.5,1',EP,P9.5,' J',P9.5,' K',1P,SE,3G12.4)
RETURN
C----- DETERMINE GEOMETRIC STIFF -----
300 RETURN
C THE GEOMETRIC STIFFNESS MATRIX IS NOT AVAILABLE FOR THIS ELEMENT
C----- DETERMINE STIFFNESS -----
300 CONTINUE
C----- H CONTAINS THE STRUT STIFFNESS, WHICH WAS DETERMINED IN THE
LAST CALL TO MATLIB
IF (H,SE(1)) THEN THE STIFFNESS HAS NOT CHANGED, RETURN
C
IF (H,EQ,SE(1)) RETURN
H = SE(1)
HM= SE(1)
S(1,1)= HM
S(1,2)= HM
S(2,1)= HM
S(2,2)= HM
C----- CALCULATE SAT
DO 360 J=1,1
  DO 360 J=1,IELDOP
    SAT(I,J)=0
  DO 360 K=1,2
    SAT(I,J)=SAT(I,J)+S(I,K)*A(J,K)
  360
C----- CALCULATE ASAT, STORE IN UPPER TRIANGULAR MATRIX
L=0
DO 370 J=1,IELDOP
  DO 370 I=J,1,-1
    L=L+1
    STIFF(L)=0
    DO 370 K=1,2
      STIFF(L)=STIFF(L)+A(I,K)*SAT(K,J)
  370
C----- RETURN
C----- DETERMINE INCREMENTAL FORCES -----
400 CONTINUE
IF (PRINT AND FIRST) WRITE(108,491) STEPID
C----- LOOP FOR EACH LOAD
DO 430 LOAD=1,NLOAD
C----- TRANSFORM FRAME DISPL INTO LOCAL DISPL
C----- LOCAL DISPL ARE AT THE END END OF THE STRUT, THUS
C----- POSITIVE IS TENSION AND NEGATIVE IS COMPRESSION
R=0
V=0
DO 410 I=1,IELDOP
  J=LM(I)
  A1= A(I,1)
  A2= A(I,2)
  DISP= DISPL(J,LOAD)
  VEL= VELOC(J)
  R= R + (A1,2) A(I,1) + DISPL(J,LOAD)
  V= V + (A1,2) A(I,1) + VELOC(J)
  410
C----- INCREMENTAL FORCES
P= R*P
C----- CHECK STIFFNESS AND CALCULATE UNBALANCED FORCES
THIS IS PREPARED ONLY FOR INELASTIC ANALYSIS
P=PT-P

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C ROW'S CONTAIN INFORMATION. THE BALANCE OF THE MATRIX IS
C UNDEFINED. THE GLOBAL DOP CORRESPONDING TO THE ROW'S OF A
C ARE STORED IN VECTOR LM. AGAIN, ONLY THE FIRST IELDOP ROW'S
C OF LM ARE DEFINED.
C IELDOP=0
C DO 50 J=1,6
C DO 50 J=1,6
C
C REMOVE DOP'S THAT ARE CONSTRAINED TO THE SAME DOP..
C IF ( (IDOP(JK(1),J).EQ.IDOP(JK(4),J)) OR
C & (IDOP(JK(2),J).EQ.IDOP(JK(3),J)) OR
C & (IDOP(JK(2),J).EQ.IDOP(JK(4),J)) OR
C & (IDOP(JK(2),J).EQ.IDOP(JK(3),J)) ) GO TO 50
C
C L0=I-1
C IF (I.EQ.4) L0=I-2
C IELDOP=IELDOP-1
C LMTI=IDOP(JK(1),J)
C AT(IELDOP,L0)=AA(I,J,2)
C IF (I.EQ.1.GR.1.EQ.4) AT(IELDOP,L0-1)=AA(I,J,1)
C
C LM(IELDOP)=LMTI
C IF ( (AT(IELDOP,1).EQ.0).AND.(AT(IELDOP,2).EQ.0).AND
C & (AT(IELDOP,3).EQ.0).AND.(AT(IELDOP,4).EQ.0).AND
C & (AT(IELDOP,5).EQ.0).AND.(AT(IELDOP,6).EQ.0)) THEN
C IELDOP=IELDOP-1
C ELSE IF (IELDOP=1) THEN
C DO 49 I=1,IELDOP-1
C IF (LM(I).NE.LM(IELDOP)) GOTO 49
C DO 49 JJ=1,6
C AT(IELDOP,JJ)=AT(I,II,JJ)+AT(IELDOP,JJ)
C AT(IELDOP,JJ)=0
C IELDOP=IELDOP-1
C GO TO 50
C CONTINUE
C ENDP
C
C 50 CONTINUE
C
C ..... A2A1 FOR STIFFNESS ONLY...
C DO 52 I=1,6
C DO 52 J=1,3
C AC(I,J)=0
C AC(I,1)=1
C AC(2,1)=ALPHA/W
C AC(3,1)=-ALPHA/W
C AC(4,1)=BETA/W
C AC(5,1)=-BETA/W
C AC(6,1)=BETA/W
C AC(2,2)=1
C AC(6,2)=-1
C AC(3,3)=1
C
C DO 55 I=1,IELDOP
C DO 55 J=1,3
C A(I,J)=0
C DO 55 K=1,6
C A(I,J)=A(I,J)+AT(I,K)*AC(K,J)
C
C ..... GET SPRING STIFFNESSES
C FALSE=FALSE
C LETTYP=0
C DO 58 I=1,3
C CALL MATLIB(2,LETTYP,FAISE,ESSE(I),PSES(I),DUCT(1,1),EXCR(1,1),
C & PT,P,RT,R,0,0,LELEN,LMAT,MATS(I),MATTYP,
C & SE(ISES(1)),IELNO,DAMAGE)
C CONTINUE
C
C TO CHECK KXL,CYCLE,COL,COL,COS
C
C ESE=0
C PSE=0
C
C ..... SET UP THE LOCAL STIFFNESS MATRIX
C SS=SE(ISES(1))
C SA1=SE(ISES(2))
C SA2=SE(ISES(3))
C
C ..... CALCULATE SAT
C SALL=SAL/(LENGTH*2)
C SALL2=SA2/(LENGTH*2)
C SLL=SS
C DO 60 J=1,IELDOP
C SAT(1,J)=SLL*A(J,1)
C SAT(2,J)=SALL*A(J,2)
C SAT(3,J)=SALL2*A(J,3)
C
C ..... CALCULATE ASAT AND CONVERT TO HALF STORAGE MODE BY COLUMNS
C
C 1 3 6 10 15 21 28 36 45 55 66 78 300. NUMBER INDICATES
C 2 5 9 14 20 27 35 44 54 65 77 LOCATION OF DATA
C 4 8 13 19 26 34 43 53 64 76 IN ARRAY STIFF
C 7 12 18 25 33 42 52 63 75
C 11 17 24 32 41 51 62 74
C 16 23 31 40 50 61 73
C 22 30 39 49 60 72
C 29 38 48 59 71
C 37 47 58 70
C 46 57 69
C 56 68
C 67
C
C L=0
C DO 70 J=1,IELDOP
C DO 70 I=J,1
C L=L+1
C STIFF(L)=0
C DO 70 K=1,3
C 70 STIFF(L)=STIFF(L)+A(I,K)*SAT(K,J)
C
C ..... ASSEMBLE GEOMETRIC STIFFNESS MATRIX FOR A UNIT LOAD
C
C LKG=L-1
C IF (PKG.EQ.0.AND.KGDATA(1).NE.2) GO TO 180
C IF (KGDATA(1).EQ.0.OR.KGDATA(2).EQ.0) GO TO 180
C AXIAL=TRUE
C
C ..... DETERMINE THE AXIAL LOAD TO BE USED FOR KG...
C IF (KGDATA(1).EQ.1) THEN
C PGEOM=PKG
C ELSE IF (KGDATA(1).EQ.2) THEN
C PGEOM=-F(2)
C ENDP
C
C ..... ASSEMBLE TRANSFORMATION MATRIX A
C NOTE: AT IS DIMENSIONED AS A 12X6 MATRIX, BUT ONLY THE FIRST IELDOP
C ROW'S CONTAIN INFORMATION. THE BALANCE OF THE MATRIX IS
C UNDEFINED. THE GLOBAL DOP CORRESPONDING TO THE ROW'S OF A
C ARE STORED IN VECTOR LM. AGAIN, ONLY THE FIRST IELDOP ROW'S
C OF LM ARE DEFINED.
C DO 140 I=1,24
C DO 140 X=1,6
C AT(I,X)=0
C KGDOP=0
C DO 145 I=1,4
C DO 145 J=1,6
C
C KGDOP=KGDOP-1
C L0=I-1
C IF (I.EQ.4) L0=I-2
C IELDOP=IELDOP-1
C LMTI=IDOP(JK(1),J)
C AT(IELDOP,L0)=AA(I,J,2)

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ELE12100 IF (I.EQ.1.GR.1.EQ.4) AT(IELDOP,L0-1)=AA(I,J,1)
ELE12110
ELE12120
ELE12130
ELE12140
ELE12150
ELE12160
ELE12170
ELE12180
ELE12190
ELE12200
ELE12210
ELE12220
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ELE15000
ELE15010

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PI-P
DI-D
ENDIF
CALL HYST03(PI,DI,PL,DL,1.,NSEG,NI,SMAT(6),SMAT(J1),SMAT(6),
& SMAT(J1),SI,SMAT(3),SE(1),SE(2),SE(3),SE(4),SE(5),SE(6),SE(7),
& SE(8),SE(9),SE(10),SE(11),SE(12),SE(13),SE(14),SE(15),SE(16),
& SE(17),SMAT(4),SE(16),SE(13),SE(14),SE(18),SE(21),SE(24))
GO TO 310
ENDIF
C ----- SAVE MAXIMUM DISPL AND IT'S LOAD
IF (ABS(D).GT.ABS(SE(36)*NI)) THEN
SE(35)=3*NI-P
SE(36)=3*NI-D
ENDIF
C ----- TRANSFER ENERGIES FOR BOTH IOPT=3 AND 4.
EPESE=SE(30)
EPSE=SE(33)
C ----- TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT=3 AND 4.
DO 410 I=1,3
DUCT(I)=MAX(SE(17+I),SE(20+I))
EXCR(I)=SE(23+I)
410 EXCR(I)=SE(23+I)
RETURN
C ----- DAMAGE INDEX -----
500 EPESE=SE(30)
EPSE=SE(33)
NSEGS=SMAT(1)
NI=SMAT(2)
DMAX=SMAT(5)+2*NSEGS
DY=SMAT(4)
BDAM=SMAT(5)
P=SE(35)+3*NI
D=SE(36)+3*NI
DI=SMAT(6)+NSEGS
IF (DY.LE.DI) THEN
PY=SMAT(6)*DY/DI
ELSE
DO 510 I=1,NSEGS
P1=SMAT(4+I)
P2=SMAT(5+I)
D1=SMAT(1+NSEGS)
D2=SMAT(5+I+NSEGS)
IF (DY.GE.D1.AND.DY.LE.D2) THEN
FY=P1*(DY-D1)/(P2-P1)/(D2-D1)
GO TO 520
ENDIF
510 CONTINUE
520 DAMAGE=ABS(DI)/DMAX+BDAM/(PY*DMAX)*EPSE
RETURN
END
C ----- SUBROUTINE MAT04 (IOPT,IELNO,PIRST,MAT,SE,P,PL,D,DL,V,VL,
& LELEM,LMAT,SMAT,EPESE,EPSE,DUCT,EXCR,DAMAGE,NSOLN)
LOGICAL PIRST
DIMENSION SE(100),SMAT(20),DUCT(3),EXCR(6)
CHARACTER*40 TYPE
C ----- IF (IOPT.NE.1) LELEM=32*3*SMAT(2)+2
IF (IOPT.EQ.0) RETURN
C ----- VARIABLES -----
C ----- GLOBAL VARIABLES -----
C IOPT = 1, INITIALIZE MATERIAL
C = 2, GET STIFFNESS
C RINPUT = INPUT DATA
C SMAT = INTERNAL STORAGE
C SE = OUTPUT STIFFNESS
C ----- SHEAR DATA -----
SMAT(1)=NSEGS, NUMBER OF BACKBONE SEGMENTS
SMAT(2)=NI, NUMBER OF SMALL AMPLITUDE LOOPS
SMAT(3)=CSE, CONSTRAINT STRAIN ENERGY...
SMAT(4)=DY, YIELD DISPLACEMENT
SMAT(4+1)=PP, BACKBONE SHEAR FOR POINT 1
SMAT(4+I+NSEG)=DP, BACKBONE DISPLACEMENT FOR POINT I
SMAT(5+2*NSEG)=BDAM, BETA FOR DAMAGE INDEX
GO TO (100,200,300,400,500) IOPT
100 CONTINUE
C ----- INPUT DATA FOR SHEAR HYSTERESIS MODEL -----
C NSEG = NUMBER OF BACKBONE SEGMENTS
C NI = NUMBER OF SMALL LOOP POINTS
C PP = BACKBONE LOAD
C DP = BACKBONE DISPLACEMENT
READ(105,*) TYPE,NSEG,NI,DY,BDAM
READ(105,*) (SMAT(4+I),I=1,NSEG)
READ(105,*) (SMAT(4+I+NSEG),I=1,NSEG)
LELEM=32*3*NI+2
LMAT=5+2*NSEG
SMAT(1)=NSEG
SMAT(2)=NI
SMAT(4)=DY
SMAT(5+2*NSEG)=BDAM
C ----- CALC CONSTANT STRAIN ENERGY -----
DL=0
PL=0
DO 10 I=1,NSEG
P=SMAT(4+I)
D=SMAT(4+I+NSEG)
IF (D.LE.DY) THEN
CSE=CSE+0.5*(P-PL)*(D-DL)
PL=P
DL=D
ELSE IF (D.EQ.DL) THEN
GO TO 20
ELSE
PY=PL-(DY-DL)*(P-PL)/(D-DL)
CSE=CSE+0.5*(P-PL)*(DY-DL)
GO TO 20
ENDIF
10 CONTINUE
20 SMAT(3)=CSE
IF (FIRST) WRITE(106,65)
WRITE(106,67) MAT,NI,DY,BDAM,(SMAT(4+X),SMAT(4+X+NSEG),X=1,NSEG)
WRITE(106,*)
65 FORMAT(////, SHEAR HYSTERESIS MODEL DATA - UNIT LENGTH MEMBER
& /, /, BACKBONE CURVE POINTS MAT NI DY
& 5X,BETA,6X,STRESS,6X,STRAIN')
67 FORMAT( (28X,214.2X,4G15.6)/(68X,2G15.6))
RETURN
C ----- GET STIFFNESS TERMS -----
200 CONTINUE
C ----- SHEAR MODEL DATA STORAGE FOR MEMBER -----
SE(1)=X
SE(2)=RULE
SE(3)=DIR

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SE(4)=A
SE(5)=PMG
SE(6)=PMH
SE(7)=DMG
SE(8)=DMH
SE(9)=BACKCB
SE(10)=SR1
SE(11)=SR2
SE(12)=SR3
SE(13)=SRM
SE(14)=IP
SE(15)=SEQUNLOAD
SE(16)=UPOS(1)
SE(17)=UPOS(2)
SE(18)=UPOS(3)
SE(19)=UNEG(1)
SE(20)=UNEG(2)
SE(21)=UNEG(3)
SE(22)=EXCR(1)
SE(23)=EXCR(2)
SE(24)=EXCR(3)
SE(25)=EXCR(4)
SE(26)=EXCR(5)
SE(27)=EXCR(6)
SE(28)=ESE
SE(29)=ESE
SE(30)=ESE
SE(31)=PSE
SE(32)=PSE-OLD
SE(32+1)=PR(1)
SE(32+NI)=PR(NI)
SE(32+1+NI)=DR(1)
SE(32+2+NI)=DR(NI)
SE(32+3+NI)=FR(1)
SE(32+3+NI)=FR(NI)
SE(33)=3*NI+P_MAX
SE(34)=3*NI+D_MAX
NSEG=SMAT(1)
NI=SMAT(2)
SI=SMAT(5)/SMAT(5+NSEG)
J1=NSEG+5
J3=NI+33
J4=2*NI+33
J5=3*NI+33-1.-2
DO 210 I=1,35
210 SE(I)=0
CALL HYST04(P,D,PL,DL,V,NSEG,NI,
& SMAT(5),SMAT(J1),SMAT(5),SMAT(J1),SI,SMAT(3),SMAT(4),
& SE(1),SE(2),SE(3),SE(4),SE(5),SE(6),
& SE(7),SE(8),SE(9),SE(10),SE(11),SE(12),
& SE(13),SE(14),SE(15),SE(16),SE(19),SE(22),
& SE(28),SE(13),SE(14),SE(18),SE(21),SE(24))
RETURN
C ----- GET STIFFNESS TERMS -----
300 CONTINUE
NSEG=SMAT(1)
C ----- MODIFIED TO SATISFY THE PORTION OF DROPPING DOWN OF BACKBONE
C CURVE AFTER THE LAST POINT (I.E. THE NSEG'S POINT)
C ON JULY 8, 1994 BASED ON YANG'S WORK
IF (NSOLN.EQ.4) THEN
IP=(SE(1).EQ.0.AND.PL.EQ.0) THEN
P=0
SE(1)=0
RETURN
ENDIF
JJJ=4+2*NSEG
IF (ABS(D).GT.SMAT(JJJ).AND.ABS(DL).LT.SMAT(JJJ)) THEN
SMAT(JJJ)=SIGN(SMAT(JJJ),D)
P1=PL*(SMAT(JJJ)*D)/SE(1)
DI=SMAT(JJJ)
CALL STRENG(SE(30),SE(33),SE(34),P1,PL,DL,SE(17))
P=0
AFTER REACHING FAILURE DUCTILITY, ENFORCE P=0.
D=SIGN(SMAT(JJJ),D)
DDUCT=ABS(D)/SMAT(JJJ)
RETURN
ENDIF
IF (ABS(DL).GT.SMAT(JJJ).AND.ABS(D).GT.SMAT(JJJ)) THEN
ENFORCED THE FORCE=0. AFTER FAILURE DUCTILITY
P=0
RETURN
ENDIF
ENDIF
MODIFICATION END
NI=SMAT(2)
SI=SMAT(5)/SMAT(5+NSEG)
J1=NSEG+5
J3=NI+33
J4=2*NI+33
DV=V+VVL
C SET VELOCITY FLAG TO 1 TO REMOVE INFL OF HIGH FREQ VELOC
DV=V+VVL
ICVC=0
310 STIFF=SE(1)
P=PL*(D-DL)*STIFF
A=SE(4)
IF (A.EQ.0.AND.SE(2).EQ.0) THEN
A=SIGN(SMAT(5),P)
IF (P.LT.PL.AND.PL.GT.0.AND.P.GT.0) A=C
IF (P.GT.PL.AND.PL.LT.0.AND.P.LT.0) A=C
ENDIF
ICVC=ICVC+1
C IS P WITHIN LIMITS ?
IF (ICVC.GT.10) THEN
WRITE(108,*) 'MORE THAN 10 CYCLES IN MAT04, IELNO=',IELNO
ELSE IF ((PL.LE.P.AND.P.LT.A).OR.(PL.GE.P.AND.P.GT.A)).OR.
& (DL.EQ.D) THEN
CALL STRENG(SE(28),SE(31),SE(32),P,PL,DL,SE(15))
CALL HYST04(P,D,PL,DL,DV,NSEG,NI,
& SMAT(5),SMAT(J1),SMAT(5),SMAT(J1),SI,SMAT(3),SMAT(4),
& SE(1),SE(2),SE(3),SE(4),SE(5),SE(6),
& SE(7),SE(8),SE(9),SE(10),SE(11),SE(12),
& SE(13),SE(14),SE(15),SE(16),SE(19),SE(22),
& SE(28),SE(13),SE(14),SE(18),SE(21),SE(24))
C IS P GREATER THAN LIMITS ?
ELSE IF (((PL.LE.A.AND.A.LE.P).OR.(PL.GE.A.AND.A.GE.P)).AND.
& (STIFF.NE.0.001)) THEN
DI=DL-(A-PL)/STIFF
P1=A
CALL STRENG (SE(28),SE(31),SE(32),P1,PL,DL,SE(15))
DL=DI
PL=P1
IF (ABS(P-PL).GT.0.0005*ABS(P-PL)) THEN
P1=PL*(P-PL)*.001
DI=DL-(D-DL)*.001
ELSE
PI=P
DI=D
ENDIF
CALL HYST04(PI,DI,PL,DL,1.00,NSEG,NI,
& SMAT(5),SMAT(J1),SMAT(5),SMAT(J1),SI,SMAT(3),SMAT(4),
& SE(1),SE(2),SE(3),SE(4),SE(5),SE(6),
& SE(7),SE(8),SE(9),SE(10),SE(11),SE(12),
& SE(13),SE(14),SE(15),SE(16),SE(19),SE(22),
& SE(28),SE(13),SE(14),SE(18),SE(21),SE(24))
GO TO 310
C IS P LESS THAN LIMITS ?
ELSE IF ((P.LT.PL.AND.PL.LE.A).OR.(P.GT.PL.AND.PL.GE.A)).THEN
IF (ABS(P-PL).GT.0.0005*ABS(P-PL)) THEN
P1=PL*(P-PL)*.001
DI=DL-(D-DL)*.001
ELSE
PI=P

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MAT00890
MAT00900
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MAT01560
MAT01570
MAT01580
MAT01590
MAT01600
MAT01610
MAT01620
MAT01630
MAT01640
MAT01650
MAT01660
MAT01670
MAT01680
MAT01690
MAT01700
MAT01710
MAT01720
MAT01730
MAT01740
MAT01750
MAT01760
MAT01770
MAT01780
MAT01790
MAT01800
MAT01810
MAT01820
MAT01830
MAT01840
MAT01850
MAT01860
MAT01870
MAT01880
MAT01890
MAT01900
MAT01910
MAT01920
MAT01930
MAT01940
MAT01950
MAT01960
MAT01970
MAT01980
MAT01990
MAT02000
MAT02010
MAT02020
MAT02030
MAT02040
MAT02050
MAT02060
MAT02070
MAT02080
MAT02090
MAT02100
MAT02110
MAT02120
MAT02130
MAT02140
MAT02150
MAT02160
MAT02170
MAT02180
MAT02190
MAT02200
MAT02210
MAT02220
MAT02230
MAT02240
MAT02250
MAT02260
MAT02270
MAT02280
MAT02290
MAT02300
MAT02310
MAT02320
MAT02330
MAT02340

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C
C ..... RESERVE STORAGE FOR ELEMENT
LELEM=24
IF (IOPT.EQ.0) RETURN
C
C ..... VARIABLES:
C ..... GLOBAL VARIABLES
C IOPT = 1, INITIALIZE MATERIAL
C IOPT = 2, GET STIFFNESS
C KINPUT = INPUT DATA
C SMAT = INTERNAL STORAGE
C SE = OUTPUT STIFFNESS
C
C GO TO (100,200,300,400,500) IOPT
C
C 100 CONTINUE
C ..... INTERPERTATE INPUT DATA
C KE = SMAT(2)
C PY = SMAT(3)
C KIE = SMAT(4)
C DY = SMAT(5)
C CSE = SMAT(6)
C DMAX = SMAT(7)
C BDAM = SMAT(8)
C
C READ(105,*) TYPE ,(SMAT(1),I=1,3),DUCMAX,BDAM
C
C LMAT = 7
C SMAT(4)=SMAT(2)/SMAT(1)
C SMAT(5)=SMAT(4)*SMAT(2)/2
C SMAT(6)=SMAT(4)*DUCMAX
C SMAT(7)=BDAM
C
C IF (FIRST) WRITE(108,191)
WRITE(108,192) MAT,(SMAT(K),K=1,3),DUCMAX,BDAM
C
191 FORMAT (///' BILINEAR HYSTERESIS MODEL PARAMETERS'//
& ' MAT',5X,
& ' KE',10X, ' PY',10X, ' KIE', 7X, 'MAX DUCT',10X, ' BETA')
192 FORMAT (1X,15,7G15.6/)
C
RETURN
C ..... INITIALIZE STIFFNESS
C ..... ELASTO-PLASTIC MODEL DATA STORAGE FOR MEMBER .....
C SE(1)=K
C SE(2)=RULE
C SE(3)=EPSE_OLD
C SE(4)=A, UPPER LIMIT BEFORE RULE CHANGE
C SE(5)=B, LOWER LIMIT BEFORE RULE CHANGE
C SE(6)=EBSE(1)
C SE(7)=EBSE(2)
C SE(8)=EBSE(3)
C SE(9)=EPSE
C SE(10)=KEY
C SE(11)=UPOS(1)
C SE(12)=UPOS(2)
C SE(13)=UPOS(3)
C SE(14)=UNEG(1)
C SE(15)=UNEG(2)
C SE(16)=UNEG(3)
C SE(17)=EXCR(1)
C SE(18)=EXCR(2)
C SE(19)=EXCR(3)
C SE(20)=EXCR(4)
C SE(21)=EXCR(5)
C SE(22)=EXCR(6)
C SE(23)=P_MAX
C SE(24)=D_MAX
C
INITIALIZE RULE #1 ELASTIC BEHAVIOR
C
DO 210 I=1,22
SE(21)=0
210 SE(21)=0
CALL HYST07(0.,0.,0.,0.,SE(1),SE(2),SE(3),SE(4),SE(5),V,V,L,
& SMAT(1),SMAT(2),SMAT(3),SMAT(4),SMAT(5),
& SE(6),SE(9),SE(10),SE(11),SE(14),SE(17))
C
RETURN
C ..... GET STIFFNESS TERMS AND ACTUAL FORCE
C 300 CONTINUE
C ..... CHECK TO SEE IF DD = 0 ?
P=P
D=D
IF (D.EQ.DL) RETURN
C ..... CALL HYST07 TO CALC NEW STIFFNESS AND GET LOAD P
CALL HYST07(P,D,PL,DL,SE(1),SE(2),SE(3),SE(4),SE(5),V,V,L,
& SMAT(1),SMAT(2),SMAT(3),SMAT(4),SMAT(5),
& SE(6),SE(9),SE(10),SE(11),SE(14),SE(17))
C
SE1=SE(1)
C ..... TRANSFER ENERGIES FOR BOTH IOPT-3 AND 4.
IF (ABS(D).GT.ABS(SE(24))) THEN
SE(23)=P
SE(24)=D
ENDIF
C ..... TRANSFER ENERGIES FOR BOTH IOPT-3 AND 4.
400 ESE=SE(6)
EPSE=SE(9)
C ..... TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT-3 AND 4
DO 410 I=1,3
DUCT(I)=MAX(SE(10+I),SE(11+I))
EXCR(I)=SE(16+I)
410 EXCR(I)=SE(16+I)
RETURN
C ..... DAMAGE INDEX
500 ESE=SE(6)
EPSE=SE(9)
PY = SMAT(2)
DY = SMAT(4)
DMAX = SMAT(6)
BDAM = SMAT(7)
P = SE(23)
D = SE(24)
DAMAGE=ABS(D)/DMAX + BDAM / (PY*DMAX) + EPSE
C
RETURN
END
C
C ..... SUBROUTINE MAT08(IOPT,FIRST,MAT,SE,P,PL,D,DL,LELEM,
& LMAT,SMAT,ESE,EPSE,DUCT,EXCR,DAMAGE)
C
LOGICAL FIRST
DIMENSION SE(100),SMAT(9),DUCT(3),EXCR(6)
CHARACTER*80 TYPE
C ..... RESERVE STORAGE FOR ELEMENT
LELEM=39
IF (IOPT.EQ.0) RETURN
C
C ..... VARIABLES

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C ..... GLOBAL VARIABLES
C IOPT = 1, INITIALIZE MATERIAL
C IOPT = 2, GET STIFFNESS
C KINPUT = INPUT DATA
C SMAT = INTERNAL STORAGE
C SE = OUTPUT STIFFNESS
C
C GO TO (100,200,300,400,500) IOPT
C
C 100 CONTINUE
C ..... INTERPERTATE INPUT DATA
C Z = SMAT(1)
C A = SMAT(2)
C R = SMAT(3)
C YS = SMAT(4)
C PY = SMAT(5)
C CC = SMAT(6)
C DMAX = SMAT(7) : ULTIMATE DISPLACEMENT = DUCMAX*DELY
C EDAM = SMAT(8) : DATA PARAMETER
C SHAPE = SMAT(9) : CROSS SECTION SHAPE
C
C 1: FOR BOX OR ANGLE SECTION
C 2: FOR I SHAPE SECTION
C 3: FOR BOX SECTION CONSIDERING
C LOCAL BUCKLING. THIS IS ONLY FOR MEMBER
C WITH KL/R LESS THAN 40 CASE
C
C READ(105,*) TYPE ,(SMAT(1),I=1,4), SHAPE,DUCMAX, BDAM
C
C LMAT = 9
P1=1.415936
SMAT(5)=SMAT(2)*SMAT(4)
SMAT(6)=SQRT(2.*PI*PI*SMAT(1)/SMAT(4))
SMAT(7)=DUCMAX
SMAT(8)=BDAM
SMAT(9)=SHAPE
C
IF (FIRST) WRITE(108,191)
WRITE(108,192) MAT,(SMAT(K),K=1,4),SHAPE,DUCMAX,BDAM
C
191 FORMAT (///' GOEL HYSTERESIS MODEL PARAMETERS'//
& ' MAT',5X,
& ' E',6X, ' A',6X, ' R',6X, ' YG',
& ' A',6X, ' SHAPE',6X, ' MAX DUC', 6X, ' BETA')
192 FORMAT (1X,15,7G15.6/)
C
RETURN
C ..... INITIALIZE STIFFNESS TERMS
C 200 CONTINUE
C ..... GOEL HYSTERESIS MODEL DATA STORAGE FOR MEMBER .....
C SE(1)=K
C SE(2)=K EFFECT SLENDER COEFFICIENT (TRANSFERRED FROM ELE08)
C SE(3)=NOCY NO. CP CYCLE
C SE(4)=RULE NUMBER
C SE(5)=EL MEMBER LENGTH (TRANSFERRED FROM ELE08)
C SE(6)=IRV 0:REVRSC=FALSE 1:REVRSC=TRUE
C SE(7)=IDUC 0:HC=-12 1:HC=LT=-12
C SE(8)=PBOT
C SE(9)=DBOT
C SE(10)=PTOP
C SE(11)=DTOP
C SE(12)=KRD UNLOADING EQUIVALENT STIFFNESS
C SE(13)=DUCT(1)
C SE(14)=DUCT(2)
C SE(15)=DUCT(3)
C SE(16)=EXCR(1)
C SE(17)=EXCR(2)
C SE(18)=EXCR(3)
C SE(19)=EXCR(4)
C SE(20)=EXCR(5)
C SE(21)=EXCR(6)
C SE(22)=DELY YIELDING DISPLACEMENT
C SE(23)=EBSE(1)
C SE(24)=EBSE(2)
C SE(25)=EBSE(3)
C SE(26)=EPSE
C SE(27)=PSBOLD
C SE(28)=CSE
C SE(29)=DMAX ULTIMATE DISPLACEMENT
C SE(30)=D_MAX MAXIMUM ELEMENT DISPLACEMENT FOR DAMAGE INDEX
C SE(31)=P_MAX ELEMENT LOAD CORRESPONDING TO D_MAX
C SE(32)=HA CURRENT CONTROL POINT HA
C SE(33)=VA CURRENT CONTROL POINT VA
C SE(34)=HC CURRENT CONTROL POINT HC
C SE(35)=VC CURRENT CONTROL POINT VC
C SE(36)=RSN CURRENT RESIDUAL STRAIN VALUE
C SE(37)=D_MAX CURRENT MAXIMUM AXIAL DISPLACEMENT FOR DUCTILITIES
C SE(38)=REGIN CURRENT REGINE ZONE NUMBER
C SE(39)=PREGIN PREVOUT REGIN ZONE NUMBER
C
C SE(1)=1
C SE(2)=1
C SE(3)=1
C DO 210 I=1,6,28
210 SE(I)=0
C SE(37)=0
C SE(38)=1
C SE(39)=1
C
CALL HYST08(0.,0.,0.,0.,SE(5),SE(2),SMAT(1),SMAT(2),SMAT(3),
& SMAT(4),SMAT(5),SMAT(9),SMAT(6),SE(3),SE(4),SE(1),SE(6),
& SE(7),SE(12),SE(13),SE(14),SE(15),SE(16),SE(17),SE(18),
& SE(19),SE(8),SE(9),SE(10),SE(11),SE(12),SE(13),
& SE(16),SE(22),SE(23),SE(26),SE(27),SE(28))
C SE(28)=SE(22)*SMAT(5)/2
C SE(29)=SE(22)*SMAT(7)
C SE(30)=0
C SE(31)=0
C
C RETURN
C ..... GET STIFFNESS TERMS
C 300 CONTINUE
C ..... CALL HYST07 TO CALC NEW STIFFNESS STIFF
CALL HYST07(P,PL,D,DL,SE(5),SE(2),SMAT(1),SMAT(2),SMAT(3),
& SMAT(4),SMAT(5),SMAT(9),SMAT(6),SE(3),SE(4),SE(1),SE(6),
& SE(7),SE(12),SE(13),SE(14),SE(15),SE(16),SE(17),SE(18),
& SE(19),SE(22),SE(23),SE(26),SE(27),SE(28))
C
C ..... STORE MAXIMUM DISPLACEMENT AND CORRESPONDING LOAD
IF (ABS(D).GT.ABS(SE(13))) THEN
SE(30)=D
SE(31)=P
ENDIF
C
C ..... GET ENERGIES, DUCTILITY AND EXCURSION FOR BOTH IOPT-3 AND 4
400 CONTINUE
C ..... TRANSFER ENERGIES FOR BOTH IOPT-3 AND 4.
ESE=SE(23)
EPSE=SE(26)
C ..... TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT-3 AND 4
DO 410 I=1,3
DUCT(I)=SE(12+I)
EXCR(I)=SE(15+I)
410 EXCR(I)=SE(15+I)
C
RETURN
C ..... DAMAGE INDEX
500 CONTINUE
ESE=SE(23)

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EPSE-SE(26) MAT01630 SE(58)=0 MAT01370
DMAX-SE(29) MAT01640 SE(59)=0 MAT01380
D-SE(30) MAT01650 SE(61)-SE(1) MAT01390
P-SE(31) MAT01660 MAT01400
PY-SMAT(5) MAT01670 RETURN MAT01410
BDAM-SMAT(8) MAT01680 ----- GET STIFFNESS TERMS ----- MAT01420
DAMAGE-ABS(D)/DMAX + BDAM / (PY*DMAX) * EPSE MAT01690 300 CONTINUE MAT01430
C RETURN MAT01700 SE(2)=P*PL MAT01440
C MAT01710 SE(3)=D*DL MAT01450
C MAT01720 MAT01460
C-----
C CALL HYST09 TO CALC NEW STIFFNESS STIF MAT01470
C CALL HYST09(IOPT,SE(1),P,D,PL,DL,SE(3),SE(4),SE(5),SE(6),SE(7), MAT01480
A SE(8),SE(9),SE(10),SE(11),SE(12),SE(13),SE(14),SE(15),SE(16), MAT01490
A SE(17),SE(18),SE(19),SE(20),SE(21),SE(22), MAT01500
A SE(23),SE(24),SE(25),SE(26),SE(27),SE(28),SE(29),SE(30), MAT01510
A SE(31),SE(32),SE(33),SE(34),SE(35),SE(36),SE(37),SE(38), MAT01520
A SE(39),SE(41),SE(60),SE(62),SE(63),SE(64),SMAT(3) ) MAT01530
C-----
C STORE MAXIMUM DISPLACEMENT AND CORRESPONDING LOAD MAT01540
C IF(ABS(DI.DI)-ABS(SE(59))) THEN MAT01550
C SE(58)=D MAT01560
C SE(59)=P MAT01570
C ENDIF MAT01580
C-----
C GET ENERGIES, DUCTILITY AND EXCURSION FOR BOTH IOPT-3 AND 4 ---- MAT01590
C 400 CONTINUE MAT01600
C-----
C TRANSFER ENERGIES FOR BOTH IOPT-3 AND 4 MAT01610
C ESE=SE(51) MAT01620
C EPSE=SE(54) MAT01630
C-----
C TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT-3 AND 4. MAT01640
C DO 410 I=1,3 MAT01650
C DUCT(I)=SE(40+I) MAT01660
C EXCR(I)=SE(43+I) MAT01670
C 410 EXCR(I)=SE(43+I) MAT01680
C RETURN MAT01690
C-----
C DAMAGE INDEX MAT01700
C 500 CONTINUE MAT01710
C...DAMAGE INDEX IS NOT AVAILABLE MAT01720
C DAMAGE=0. MAT01730
C RETURN MAT01740
C END MAT01750
C-----
C SUBROUTINE MAT10(IOPT, FIRST, MAT, SE, P, PP, D, DL, LELEM, LMAT, MAT01760
A SMAT, ESE, EPSE, DUCT, DAMAGE) MAT01770
C-----
C LOGICAL FIRST MAT01780
C DIMENSION SE(100), SMAT(9), DUCT(3) MAT01790
C CHARACTER*80 TYPE MAT01800
C-----
C RESERVE STORAGE FOR ELEMENT MAT01810
C LELEM=66 MAT01820
C IF (IOPT.EQ.0) RETURN MAT01830
C-----
C VARIABLES: MAT01840
C-----
C GLOBAL VARIABLES MAT01850
C IOPT = 1, INITIALIZE MATERIAL MAT01860
C = 2, INITIALIZE STIFFNESS MAT01870
C = 3, GET STIFFNESS TERM MAT01880
C = 4, GET ENERGIES, DUCTILITY AND EXCURSION MAT01890
C = 5, CALCULATE DAMAGE INDEX MAT01900
C RINPUT - INPUT DATA MAT01910
C SMAT - INTERNAL STORAGE MAT01920
C SE - OUTPUT STIFFNESS MAT01930
C-----
C GO TO (100,200,300,400,500),IOPT MAT01940
C-----
C 100 CONTINUE MAT01950
C-----
C INTERPERTATE INPUT DATA MAT01960
C HA = SMAT( 1) MAT01970
C VA = SMAT( 2) MAT01980
C DUCHMAX = SMAT( 3) : FAILURE DUCTILITY MAT01990
C BDAM = SMAT( 4) : BATA PARAMETER MAT02000
C-----
C READ(105,*) TYPE ,(SMAT(I),I=1,2), DUCHMAX, BDAM MAT02010
C LMAT = 4 MAT02020
C SMAT(3)=DUCHMAX MAT02030
C SMAT(4)=BDAM MAT02040
C IF (FIRST) WRITE(108,191) MAT02050
C WRITE(108,192) MAT,(SMAT(K),K=1,2),DUCHMAX,BDAM MAT02060
191 FORMAT (//// PINO-SUAREZ TOWER LONG DIR. HYST. MODEL PARAMETERS / MAT02070
A ' MAT ',X, ' HA,EX, ' VA,EX, ' MAX DUC', ' EX, ' BETA') MAT02080
192 FORMAT (1X,13,2X,6012.6/) MAT02090
C RETURN MAT02100
C-----
C INITIALIZE STIFFNESS TERMS ----- MAT02110
C 200 CONTINUE MAT02120
C-----
C PINO-SUAREZ TOWER LONG GIRDER MODEL DATA STORAGE FOR MEMBER MAT02130
C SE( 1) = STIF MEMBER MOMENT-ROTATION STIFFNESS MAT02140
C SE( 2) = DP LOAD INCREMENT MAT02150
C SE( 3) = DX DISPLACEMENT INCREMENT MAT02160
C SE( 4) = RV INDEX FOR TOP OR BOTTOM ENVELOPE MAT02170
C RV=0 THEN HIGH= TRUE MAT02180
C RV=1 THEN HIGH= FALSE. MAT02190
C SE( 5) = RULE RULE NUMBER MAT02200
C SE( 6) = PTOP MAT02210
C SE( 7) = DTOP MAT02220
C SE( 8) = PBOT MAT02230
C SE( 9) = DBOT MAT02240
C SE(10) = SPTOP MAT02250
C SE(11) = SDTOP MAT02260
C SE(12) = SFBOT MAT02270
C SE(13) = SDBOT MAT02280
C SE(14) = SSPTOP MAT02290
C SE(15) = SSDBOT MAT02300
C SE(16) = SSPBOT MAT02310
C SE(17) = SSDBOT MAT02320
C SE(18) = HB MAT02330
C SE(19) = VB MAT02340
C SE(20) = HD MAT02350
C SE(21) = VD MAT02360
C SE(22) = HE MAT02370
C SE(23) = VE MAT02380
C SE(24) = HAP MAT02390
C SE(25) = VAP MAT02400
C SE(26) = HBP MAT02410
C SE(27) = VBP MAT02420
C SE(28) = SSHD MAT02430
C SE(29) = SSTD MAT02440
C SE(30) = SBHE MAT02450
C SE(31) = SBVE MAT02460
C SE(32) = SSSPTP MAT02470
C SE(33) = SSSDBT MAT02480
C SE(34) = SSSPBT MAT02490
C SE(35) = SSSDBT MAT02500
C SE(36) = SHD MAT02510
C SE(37) = SVD MAT02520
C SE(38) = SHB MAT02530
C SE(39) = SVE MAT02540
C SE(40) = XEQ UNLOADING EQUIVALENT STIFFNESS MAT02550
C SE(41) = DUCT(1) MAT02560
C SE(42) = DUCT(2) MAT02570
C SE(43) = DUCT(3) MAT02580
C SE(44) = EXCR(1) MAT02590
C SE(45) = EXCR(2) MAT02600
C SE(46) = EXCR(3) MAT02610
C SE(47) = EXCR(4) MAT02620
C SE(48) = EXCR(5) MAT02630
C SE(49) = EXCR(6) MAT02640
C SE(50) = DELY YIELDING DISPLACEMENT MAT02650
C SE(51) = ESE(1) MAT02660
C SE(52) = ESE(2) MAT02670
C SE(53) = ESE(3) MAT02680
C SE(54) = EPSE MAT02690
C SE(55) = PSBOLD MAT02700
C SE(56) = CSE MAT02710
C SE(57) = DMAX ULTIMATE DISPLACEMENT MAT02720
C SE(58) = P_MAX MAXIMUM ELEMENT DISPLACEMENT FOR DAMAGE INDEX MAT02730
C SE(59) = P_MAX ELEMENT LOAD CORRESPONDING TO D_MAX MAT02740
C SE(60) = DMAX CURRENT MAXIMUM AXIAL DISPLACEMENT FOR DUCTILITIES MAT02750
C SE(61) = INITIAL STIFFNESS MAT02760
C SE(62) = DISP FACTOR FACD MAT02770
C SE(63) = MOMENT FACTOR FACV MAT02780
C SE(64) = ROTATION STIFFNESS FACTOR=FACV/FACD MAT02790
C SE(65) = HA MAT02800
C SE(66) = VA MAT02810
C-----
C DO 210 I=1,61 MAT02820
C SE(I)=0 MAT02830
C SE(60)=SMAT(1) MAT02840
C SE(5)=1 MAT02850
C SE(66)= SMAT(1) MAT02860
C SE(66)= SMAT(2) MAT02870
C CALL HYST09(IOPT,SE(1),0,0,0,0,SE(3),SE(4),SE(5),SE(6),SE(7), MAT02880
A SE(8),SE(9),SE(10),SE(11),SE(12),SE(13),SE(14),SE(15),SE(16), MAT02890
A SE(17),SE(18),SE(19),SE(20),SE(21),SE(22), MAT02900
A SE(23),SE(24),SE(25),SE(26),SE(27),SE(28),SE(29),SE(30), MAT02910
A SE(31),SE(32),SE(33),SE(34),SE(35),SE(36),SE(37),SE(38), MAT02920
A SE(39),SE(41),SE(60),SE(62),SE(63),SE(64),SMAT(3) ) MAT02930
C SE(56)=SE(50)*SMAT(2)/ MAT02940
C SE(57)=SE(22)*SMAT(3) MAT02950
C-----
C SE(1)= SMAT(3)*SMAT(4) MAT02960
C SE(2)= SMAT(5) MAT02970
C SE(3)= SMAT(5) MAT02980
C SE(4)= 0 MAT02990
C SE(5)= SMAT(1) MAT03000
C SE(6)= SMAT(2) MAT03010
C SE(7)= FAILURE DUCTILITY MAT03020
C SE(8)= PARAMETER FOR ANG'S DAMAGE INDEX MAT03030
C SE(9)= RESIDUAL STRENGTH FOR ELAS-2 MAT03040
C SE(10)= CURRENT DUCTILITY MAT03050
C SE(11)= SMAT(4) MAT03060
C SE(12)= TMP*SMAT(5) MAT03070
C SE(13)= DELY YIELDING DISPLACEMENT MAT03080
C FOR ROTATION SE(13)=TMP*LENGTH/(SEI) MAT03090
C FOR AXIAL SE(13)=TMP*LENGTH/EI MAT03100
C FOR TORSION SE(13)=TMP*LENGTH/EI MAT03110
C SE(14)= DMAX CURRENT MAXIMUM DISPLACEMENT FOR DUCTILITY MAT03120
C SE(15)= YIELDING DISPLACEMENT INDEX MAT03130
C 0: DELY HAS NOT BEEN DETERMINED MAT03140
C 1: DELY HAS BEEN DETERMINED MAT03150
C SE(1)= SMAT(3)*SMAT(4) MAT03160
C SE(2)= SMAT(5) MAT03170
C SE(3)= SMAT(5) MAT03180
C SE(4)= 0 MAT03190
C SE(5)= SMAT(1) MAT03200
C SE(6)= SMAT(2) MAT03210
C SE(7)= SMAT(6) MAT03220
C SE(8)= SMAT(7) MAT03230

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SMAT(4)-LIBN MAT10699 4 IX,IMATER '-,1P,110,9X,'RATIO3 '-,1P,G10.3/ MAT12150
SMAT(9)-INER MAT10700 4 IX,IR '-,1P,110,9X,'G '-,1P,G10.3/ MAT12160
SMAT(10)-INER MAT10701 4 IX,'GRNZE '-,1P,G10.3,9X,'ISTIF '-,1P,I10 // MAT12180
SMAT(12)-IREV1 MAT10720 C 195 FORMAT (IX,'MAT NO. '-,13,2X,' WIDE-FLANGE SECTION'// MAT12190
SMAT(13)-IREV3 MAT10730 4 IX,'NSEG '-,1P,110,9X,'YS '-,1P,G10.3/ MAT12200
SMAT(14)-IREV3 MAT10740 4 IX,'EM '-,1P,110,9X,'LIBN '-,1P,I10 // MAT12210
SMAT(15)-IREV4 MAT10750 4 IX,'BOX HEIGHT '-,1P,G10.3,9X,'BOX WILTH '-,1P,G10.3/ MAT12220
SMAT(16)-IECOP MAT10760 4 IX,'ECCX0 '-,1P,G10.3,9X,'ECCY0 '-,1P,G10.3/ MAT12230
SMAT(22)-IAUTO MAT10770 4 IX,'WEB THICK. '-,1P,G10.3,9X,'DUMMY VAR. '-,1P,G10.3/ MAT12240
SMAT(23)-IMATER MAT10780 4 IX,'FLA THICK. '-,1P,G10.3,9X,'IREV1 '-,1P,I10 // MAT12250
SMAT(25)-IR MAT10790 4 IX,'IREV2 '-,1P,110,9X,'IREV3 '-,1P,I10 // MAT12260
SMAT(26)-ISTIF MAT10810 4 IX,'IREV4 '-,1P,110,9X,'IECOP '-,1P,G10.3/ MAT12270
MAT10820 4 IX,'NUMP '-,1P,110,9X,'SMALL '-,1P,G10.3/ MAT12280
MAT10830 4 IX,'RATIO3 '-,1P,G10.3,9X,'RATIO0 '-,1P,G10.3/ MAT12290
MAT10840 4 IX,'TOTA '-,1P,G10.3,9X,'IAUTO '-,1P,I10 // MAT12300
MAT10850 4 IX,'IMATER '-,1P,110,9X,'RATIO3 '-,1P,G10.3/ MAT12310
MAT10860 4 IX,'IR '-,1P,110,9X,'G '-,1P,I10 // MAT12320
MAT10870 4 IX,'GRNZE '-,1P,G10.3,9X,'ISTIF '-,1P,I10 // MAT12330
MAT10880 C 199 FORMAT (IX,'MAT NO. '-,13,' IS NOT AVAILABLE', MAT12340
MAT10890 4 'LIBN-',G10.3,' CHECK LIBN '/' MAT12350
MAT10900 MAT12360
MAT10910 MAT12370
MAT10920 MAT12380
MAT10930 MAT12390
MAT10940 I2=11*NSEG-12 MAT12400
MAT10950 I3=12*(NSEG-1)*6 MAT12410
MAT10960 I4=13*NSEG MAT12420
MAT10970 I5=14*2 MAT12430
MAT10980 I6=15*2 MAT12440
MAT10990 I7=16*NSEG*NUMP MAT12450
MAT11000 I8=17*NSEG*NUMP MAT12460
MAT11010 I9=18*NUMP MAT12470
MAT11020 I10=19*NUMP MAT12480
MAT11030 I11=110*NSEG*9*3 MAT12490
MAT11040 I12=11*NSEG MAT12500
MAT11050 I13=112*(NSEG-1)*6 MAT12510
MAT11060 I14=113*(NSEG-1)*6 MAT12520
MAT11070 I15=114*(NSEG-1)*6 MAT12530
MAT11080 I16=115*NSEG MAT12540
MAT11090 I17=116*NSEG*12 MAT12550
MAT11100 I18=117*NSEG*12 MAT12560
MAT11110 I19=118*NSEG*12 MAT12570
MAT11120 I20=119*NSEG*12 MAT12580
MAT11130 I21=120*NSEG*NUMP MAT12590
MAT11140 I22=121*NSEG*NUMP MAT12600
MAT11150 I23=122*NSEG*NUMP MAT12610
MAT11160 I24=123*NUMP MAT12620
MAT11170 I25=124*NSEG*9 MAT12630
MAT11180 I26=125*NSEG*9 MAT12640
MAT11190 I27=126*(NSEG-1)*6**2 MAT12650
MAT11200 I28=127*(NSEG-1)*6*(NSEG-1)*6-11**2 MAT12660
MAT11210 I29=128*(NSEG-1)*6-1 MAT12670
MAT11220 I30=129*144 MAT12680
MAT11230 I31=130*144 MAT12690
MAT11240 I32=131*(NSEG-1)*6 MAT12700
MAT11250 I33=132*(NSEG-1)*6**2 MAT12710
MAT11260 I34=133*NSEG*144 MAT12720
MAT11270 I35=134*2 MAT12730
MAT11280 I36=135*2 MAT12740
MAT11290 I37=136*(NSEG-1)*6 MAT12750
MAT11300 I38=137*(NSEG-1)*6 MAT12760
MAT11310 I39=138*(NSEG-1)*6 MAT12770
MAT11320 I40=139*(NSEG-1)*6**2 MAT12780
MAT11330 I41=140*NUMP MAT12790
MAT11340 I42=141*NSEG*NUMP MAT12800
MAT11350 I43=142*NSEG*12 MAT12810
MAT11360 I44=143*NUMP MAT12820
MAT11370 I45=144*NSEG*NUMP MAT12830
MAT11380 I46=145*NSEG*NUMP MAT12840
MAT11390 I47=146*NSEG*NUMP MAT12850
MAT11400 I48=147*NSEG*NUMP MAT12860
MAT11410 I49=148*NSEG*NUMP MAT12870
MAT11420 I50=149*NSEG*NUMP MAT12880
MAT11430 I51=150*NSEG*NUMP MAT12890
MAT11440 IP (XG0,ED,2) GOTO 205 MAT12900
MAT11450 C 196 MAT12910
MAT11460 C 197 MAT12920
MAT11470 C 198 MAT12930
MAT11480 C 199 MAT12940
MAT11490 C 200 MAT12950
MAT11500 C 201 MAT12960
MAT11510 C 202 MAT12970
MAT11520 C 203 MAT12980
MAT11530 C 204 MAT12990
MAT11540 C 205 MAT13000
MAT11550 C 206 MAT13010
MAT11560 C 207 MAT13020
MAT11570 C 208 MAT13030
MAT11580 C 209 MAT13040
MAT11590 C 210 MAT13050
MAT11600 C 211 MAT13060
MAT11610 C 212 MAT13070
MAT11620 C 213 MAT13080
MAT11630 C 214 MAT13090
MAT11640 C 215 MAT13100
MAT11650 C 216 MAT13110
MAT11660 C 217 MAT13120
MAT11670 C 218 MAT13130
MAT11680 C 219 MAT13140
MAT11690 C 220 MAT13150
MAT11700 C 221 MAT13160
MAT11710 C 222 MAT13170
MAT11720 C 223 MAT13180
MAT11730 C 224 MAT13190
MAT11740 C 225 MAT13200
MAT11750 C 226 MAT13210
MAT11760 C 227 MAT13220
MAT11770 C 228 MAT13230
MAT11780 C 229 MAT13240
MAT11790 C 230 MAT13250
MAT11800 C 231 MAT13260
MAT11810 C 232 MAT13270
MAT11820 C 233 MAT13280
MAT11830 C 234 MAT13290
MAT11840 C 235 MAT13300
MAT11850 C 236 MAT13310
MAT11860 C 237 MAT13320
MAT11870 C 238 MAT13330
MAT11880 C 239 MAT13340
MAT11890 C 240 MAT13350
MAT11900 C 241 MAT13360
MAT11910 C 242 MAT13370
MAT11920 C 243 MAT13380
MAT11930 C 244 MAT13390
MAT11940 C 245 MAT13400
MAT11950 C 246 MAT13410
MAT11960 C 247 MAT13420
MAT11970 C 248 MAT13430
MAT11980 C 249 MAT13440
MAT11990 C 250 MAT13450
MAT12000 C 251 MAT13460
MAT12010 C 252 MAT13470
MAT12020 C 253 MAT13480
MAT12030 C 254 MAT13490
MAT12040 C 255 MAT13500
MAT12050 C 256 MAT13510
MAT12060 C 257 MAT13520
MAT12070 C 258 MAT13530
MAT12080 C 259 MAT13540
MAT12090 C 260 MAT13550
MAT12100 C 261 MAT13560
MAT12110 C 262 MAT13570
MAT12120 C 263 MAT13580
MAT12130 C 264 MAT13590
MAT12140 C 265 MAT13600

```


C RULE 3 - UNLOADING ON TENSION BEFORE YIELD

```
300 IF (P.GT.D) GO TO 330
IF (P.GT.D) GO TO 330
310 IF (K.C.NE.KT1) THEN
CALL INTSCT(D1,P1,D,P,KC,DX,FX,KT1)
ELSE
P1=P
D1=D
ENDIF
IF (P.LE.P1) GO TO 320
S=KC
NRL=3
KRLN=3.11
A=PY
RETURN
320 S=KT1
NRL=4
KRLN=3.12
A=PY
RETURN
330 IF (P.GE.PMAX) GO TO 200
S=KC
NRL=3
KRLN=3.2
A=FX
RETURN
```

C RULE 4 - LOADING TOWARDS Y FROM FX.

```
400 IF (IDR) 500,500,410
410 IF (ABS(P).GE.PY) GO TO 100
S=KT1
NRL=4
KRLN=4.1
A=PY
RETURN
```

C RULE 5 - LOADING AND UNLOADING BETWEEN BRANCHES O-Y AND FX-Y

```
500 IF (K.C.NE.KT1) THEN
CALL INTSCT(DITENS,PITENS,D,P,KC,DYT,PY,KT1)
CALL INTSCT(DICOMP,PICOMP,D,P,KC,DX,FX,KT1)
ELSE
PITENS=P
DITENS=D
PICOMP=P
DICOMP=D
ENDIF
IF (P.LE.PICOMP) GO TO 330
IF (P.GE.PITENS) GO TO 210
S=KC
NRL=5
KRLN=5.1
IF (POTPL) A=PITENS
IF (PLTPL) A=PICOMP
RETURN
```

C RULE 6 - LOADING AFTER TENSION YIELD

```
600 IP (IDR) 630,630,610
610 S=KT2
NRL=6
KRLN=6.1
A=S*P
C..... CALC UNLOADING CURVE FOR ENERGY INFO
PMAK=AMAX1(P,PMAK)
DMAK=AMAX1(D,DMAK)
KR=KC*(DMAK/DYT)**(-1.*ALPHA)
FX = PMAK*PY
DK = DMAK*PY/KR
KS = (PY*FX)/(DX*DYC)
C..... FORCE KR GE KS, TO PREVENT UNSTABLE LOOPS...
IF (KS GT KR) KR=(PMAK*PY)/(DMAK*DYC)
SEQ=KR
RETURN
630 PMAK=AMAX1(P,PMAK)
DMAK=AMAX1(D,DMAK)
KR=KC*(DMAK/DYT)**(-1.*ALPHA)
FX = PMAK*PY
DK = DMAK*PY/KR
KS = (PY*FX)/(DX*DYC)
C..... FORCE KR GE KS, TO PREVENT UNSTABLE LOOPS...
IF (KS GT KR) KR=(PMAK*PY)/(DMAK*DYC)
K5=KR
DK=DMAK*PY/KR
ENDIF
DP =DYC + BETA*(DX*DYC)
FP =FX (DX-DP)*KS
KT = (-2*PY*FP)/(DP*2*DYC)
DQ =DYC + PY/KR
S=KR
NRL=7
KRLN=6.2
A=FX
RETURN
```

C RULE 7 - UNLOADING AFTER YIELD

```
700 SEQ=KR
IF (IDR) 730,730,710
710 IF (P.GE.PMAX) GO TO 600
S=KR
NRL=7
KRLN=7.1
A=PMAK
RETURN
730 IF (P.LE.FX) GO TO 800
S=KR
NRL=7
KRLN=7.2
A=FX
RETURN
```

C RULE 8 - ON BRANCH BETWEEN DX & DP

```
800 IF (POTPL) THEN
IF (K.EQ.KS) GO TO 1200
GO TO 1300
ENDIF
IF (P.LE.FP) GO TO 900
S=KS
NRL=8
KRLN=8.1
A=FP
RETURN
```

C RULE 9 - ON BRANCH FP-Y

```
900 IF (POTPL) GO TO 1300
IF (P.LE.(-2*PY)) GO TO 1000
S=KT
NRL=9
KRLN=9.1
A=-2*PY
RETURN
```

C RULE 10 - ON THE COMPRESSION CURVE AFTER YIELDING

```
1000 SEQ= 1 / ( 1./KC + (4./KR + 4./KC)*(PY/PI)**2 )
IF (IDR) 1030,1030,1010
1010 S=KC
NRL=10
KRLN=10.1
```

```
A=10*P
RETURN
1030 IF (ABS(P).LE.PY) GO TO 1100
S=KC
NRL=10
KRLN=10.2
A=-PY
RETURN
```

C RULE 11 - UNLOADING BETWEEN Y & Q

```
1100 SEQ=KR
IF (ABS(P).GT.PY) GO TO 1000
IF ( P.GE.0.) GO TO 1200
S=KR
NRL=11
KRLN=11.1
IF (POTPL) A=0
IF (PLTPL) A=-PY
RETURN
```

C RULE 12 - LOADING FROM Q TO M

```
1200 IF (PLTPL) THEN
IF (KS.EQ.KR AND P.GT.FP) GO TO 800
GO TO 1300
ENDIF
IF ( P.GE.PMAX) GO TO 600
S=KS
NRL=12
KRLN=12.1
A=PMAK
RETURN
```

C RULE 13 - LOADING AND UNLOADING INSIDE Q-M-D-P-Y-Y

```
1300 IF (KR.NE.KS) THEN
CALL INTSCT(DITENS,PITENS,D,P,KR,DMAK,PMAK,KS)
CALL INTSCT(DICOMP,PICOMP,D,P,KR,DX,FX,KS)
ELSE
IF (POTPL) GO TO 1200
PITENS=P
DITENS=D
PICOMP=P
DICOMP=D
ENDIF
IF (P.LE.PITENS) THEN
IF (PICOMP.LE.FP) THEN
IF (K.C.NE.KT) THEN
CALL INTSCT(DICOMP,PICOMP,D,P,KC,DX,FP,KT)
ELSE
IF (PLTPL) GO TO 900
PICOMP=P
DICOMP=D
ENDIF
IF (KR.EQ.KS AND PLTPL) GO TO 800
ELSE
S2=KR
ENDIF
IF (POTPL) THEN
IF (P.GT.PITENS) GO TO 1200
A=PITENS
KRLN=13.1
ENDIF
IF (PLTPL) THEN
IF (P.LT.PICOMP) GO TO 800
A=PICOMP
KRLN=13.2
ENDIF
S=MAX(KR,S2)
NRL=13
RETURN
END
```

C-----BENDING HYSTERESIS MODEL-----HYSTO3-----

SUBROUTINE HYSTO3(D,P,DL,VEL,NB,NI,PC,DC,FR,DB,SI,CSE,
& K,RULE,DIR,A,PMG,PMG,DMG,DMH,BACKS,ALPHA1,ALPHA2,
& IR,FR,DR,FR,SE,SY,ESE,FSE,PSEOLD,UPOS,UNEG,EXCR)

IMPLICIT REAL(A-H,K,O-Z)
INTEGER I1,I2,I3,I4,I5,I6,I7,I8,I9,I10,I11,I12,I13,I14,I15,I16,I17,I18,I19,I20,I21,I22,I23,I24,I25,I26,I27,I28,I29,I30,I31,I32,I33,I34,I35,I36,I37,I38,I39,I40,I41,I42,I43,I44,I45,I46,I47,I48,I49,I50,I51,I52,I53,I54,I55,I56,I57,I58,I59,I60,I61,I62,I63,I64,I65,I66,I67,I68,I69,I70,I71,I72,I73,I74,I75,I76,I77,I78,I79,I80,I81,I82,I83,I84,I85,I86,I87,I88,I89,I90,I91,I92,I93,I94,I95,I96,I97,I98,I99,I100

INPUT VARIABLES
I = CURRENT LOAD
D = CURRENT DISPLACEMENT
VEL = LAST LOAD
NB = PRODUCT OF LAST AND CURRENT VELOCITY
NI = NUMBER OF BACKBONE CURVE POINTS
NI = MAXIMUM NUMBER OF SECONDARY LOOPS
PC = CRACKING LOAD
DC = CRACKING DISPLACEMENT
PB(5) = BACKBONE CURVE LOADING POINT J, J=1,NB
DB(5) = BACKBONE CURVE DISPLACEMENT POINT J, J=1,NB
SI = INITIAL STIFFNESS, PC/DC

OUTPUT VARIABLES
...AN INITIAL VALUE OF ZERO, OR FALSE
Y = STIFFNESS
RULE = RULE NUMBER, STORED IN THE FORMAT RN,DIR UPON EXIT OF THIS SUBROUTINE
A = LOAD LIMIT BEFORE NEXT RULE CHANGE, IN THE CURRENT DIR
PMG = MAXIMUM FAST LOAD IN THE POSITIVE DIRECTION
PMH = MAXIMUM FAST LOAD IN THE NEGATIVE DIRECTION
DMG = MAXIMUM FAST DISPLACEMENT IN THE POSITIVE DIRECTION
DMH = MAXIMUM FAST DISPLACEMENT IN THE NEGATIVE DIRECTION
FR(1) = SECONDARY LOOP LOADING POINT, I
FR(11) = SECONDARY LOOP DISPLACEMENT POINT, I
FR(1) = SECONDARY LOOP FLAG
IF = NUMBER OF SECONDARY LOOPS ACTIVE
ALPHA1 = POSITIVE LOAD DEGRADING STIFFNESS FACTOR
ALPHA2 = NEGATIVE LOAD DEGRADING STIFFNESS FACTOR
BACKB = BACKBONE LOADING FLAG

INTERNAL VARIABLES
J = BACKBONE CURVE POINT # IN THE CURRENT DIRECTION
JO = TEMPORARY VARIABLE
I,T = SECONDARY LOOP NUMBER
DIR = CURRENT DIRECTION
DIR=1, POSITIVE LOADING
DIR=2, POSITIVE UNLOADING
DIR=3, NEGATIVE LOADING
DIR=4, NEGATIVE UNLOADING
DIR = LAST DIRECTION
IRULE = INTEGER VALUE OF RULE #
IRULE = INTEGER VALUE OF LAST RULE #
REVERSL = LOAD REVERSAL FLAG
AP = ABSOLUTE VALUE OF THE LOAD, P
PM = MAXIMUM FAST LOAD IN THE CURRENT DIRECTION
DM = MAXIMUM FAST DISPLACEMENT IN THE CURRENT DIRECTION
Q1 = MAXIMUM FAST DISPLACEMENT IN BOTH DIRECTIONS
Q2 = ONE QUARTER OF PM, USED BY UNLOADING CURVES
Q3 = THREE QUARTERS OF PM, USED BY UNLOADING CURVES
DQ1 = DISPLACEMENT AT Q1 ON UNLOADING BRANCH
DQ3 = DISPLACEMENT AT Q3 ON UNLOADING BRANCH

```

C D01 = INTERMEDIATE DISPLACEMENT INTERCEPT OF UNLOADING BRANCH HYST0710
C D02 = INTERMEDIATE DISPLACEMENT INTERCEPT OF UNLOADING BRANCH HYST0710
C DS = DISPLACEMENT INTERCEPT OF UNLOADING BRANCH HYST0710
C DP = DIFFERENCE IN LOAD, USED FOR CALCULATING K HYST0740
C DD = DIFFERENCE IN DISPLACEMENT, USED FOR CALCULATING K HYST0750
C PK = LOAD INTERCEPT OF LOADING AND BACKBONE CURVE HYST0760
C DX = DISPLACEMENT INTERCEPT OF LOADING AND BACKBONE CURVE HYST0770
C PC3 = PC/3 HYST0780
C DC3 = DISPLACEMENT AT PC3 HYST0790
C P2,D2 = INTERSECTION OF LOADING AND UNLOADING CURVES HYST0800
C P1,D1 = SMALL LOOP UNLOADING POINTS HYST0810
C X = MAXIMUM DISPLACEMENT HYST0820
C X8 = STIFFNESS FROM RULE 8 HYST0830
C FLAG1 = UPPER SECONDARY LOOP FLAG HYST0840
C FLAG2 = LOWER SECONDARY LOOP FLAG HYST0850
C ALPHAA = DEGRADING FACTOR FOR FIRST CYCLE HYST0860
C ALPHAB = DEGRADING FACTOR FOR SUBSEQUENT CYCLES HYST0870
C S = TEMPORARY STIFFNESS HYST0880
C S1 = UNLOADING STIFFNESS FROM PS1 HYST0890
C S2 = UNLOADING STIFFNESS FROM PS2 HYST0900
C S3 = UNLOADING STIFFNESS FROM PS3 HYST0910
C S0 = STIFFNESS BETWEEN CURRENT AND RELOADING POINT HYST0920
C S02 = STIFFNESS BETWEEN PA AND D0 HYST0930
C S03 = STIFFNESS BETWEEN PB AND D0 HYST0940
C HYST0950
INTERNAL FUNCTIONS
C PC2 = DISPLACEMENT INTERCEPT OF LOADING AND UNLOADING CURVE HYST0970
C PS1 = UNLOADING STIFFNESS WHEN P > .75 PMAX HYST0980
C PS2 = UNLOADING STIFFNESS WHEN .75 PMAX > P > .25 PMAX HYST0990
C PS3 = UNLOADING STIFFNESS WHEN .25 PMAX > P HYST1000
C PSL = LOADING STIFFNESS WHEN P < PC/3 WITHOUT REVERSAL... HYST1010
C HYST1020
SU BROUCTIONS CALLED
C DIRECT = DETERMINES THE VALUES OF DIR AND DIRL HYST1030
C INTXCT = DETERMINES THE INTERSECTION OF TWO LINES IN POINT-SLOPE HYST1040
C FORM HYST1050
C KMIN = DETERMINES THE STIFFNESS BASED ON DP,DD AND K_DEFAULT HYST1060
C HYST1070
PS1(X) = SI * (DC/X) ** .294 HYST1080
PS2(X) = SI * (0.8344 * (DC/X - 1) ** 1) HYST1090
PS3(X) = SI * (0.9092 * (DC/X - 1) ** 1) HYST1100
PSL(X) = SI * (DC/X) ** .285 HYST1110
PD2(X) = DM - .05 * PM / (PS1(X)) HYST1120
C 1000 IF (IR.GT.0) WRITE(108,1020) (I,FR(I),DR(I),FR(I)-1,I,IR) HYST1130
1020 FORMAT('SI ',I,' G13.5',' FR-',G13.5,' DR-',G13.5, HYST1140
' FR-',A) HYST1150
C AP=ABS(P) HYST1160
C ..... DETERMINE IF ELASTIC HYST1170
IF (RULE.EQ.0 .AND. AP.LT.PC) THEN HYST1180
K=SI HYST1190
SE=X HYST1200
RETURN HYST1210
ENDIF HYST1220
C ..... DETERMINE IF WALL IS INACTIVE, IF SO RETURN HYST1230
IF (RULE.LT.0) RETURN HYST1240
C ..... DETERMINE CURRENT DIRECTION HYST1250
RULE=INT(RULE) HYST1260
DIR =DIR HYST1270
CALL DIRECT(DIR,DIRL,P,PL,VEL) HYST1280
DIR =DIR HYST1290
C ..... DETERMINE MAXIMUM AND MINIMUM VALUES HYST1300
PMG=MAX(P,PMG,PC) HYST1310
DMG=MAX(D,DMG,DC) HYST1320
PMH=MIN(P,PMH,PC) HYST1330
DMH=MIN(D,DMH,DC) HYST1340
DMAX=MAX(DMG,DMH) HYST1350
PMAX=MAX(PMG,PMH) HYST1360
C ..... SET MAXIMUM AND MINIMUM VALUES... HYST1370
IF (DIR.EQ.1 .OR. DIR.EQ.2) THEN HYST1380
PM=PMG HYST1390
DM=DMG HYST1400
ELSE IF (DIR.EQ.3 .OR. DIR.EQ.4) THEN HYST1410
PM=PMH HYST1420
DM=DMH HYST1430
ENDIF HYST1440
C ..... CALCULATE EQUIVALENT UNLOADING STIFFNESS FOR ENERGY... HYST1450
IF (BACKB .OR. RULE.EQ.1) THEN HYST1460
S1=PS1(DMAX) HYST1470
S2=PS2(DMAX) HYST1480
S3=PS3(DMAX) HYST1490
D0 = DM - PM * (DMG - DMH) / (PMH - PMG) HYST1500
Q1 = PM / 4 HYST1510
Q2 = 3 * Q1 HYST1520
Q3 = DM - .025 * PM / S1 HYST1530
S02 = KMIN((DQ3 - D0) / Q3, S1) HYST1540
DQ1 = DQ3 - 0.50 * PM / MAX(S2, S02) HYST1550
S03 = KMIN((DQ1 - D0) / Q1, S1) HYST1560
SE = 0.5 * PM ** 2 / ((PM - Q3) * 0.5 * (DM - DQ3) + (Q3 + Q1) * 0.5 * (DQ3 - DQ1) HYST1570
+ (Q1) * 0.5 * (Q1) / MAX(S3, S03) ) HYST1580
ENDIF HYST1590
C ..... CHECK TO SEE IF LAST RULE IS STILL IN EFFECT HYST1600
IF (DIR.EQ.DIRL .AND. HYST1610
((DIR.EQ.1 .AND. P.LT.A) .OR. (DIR.EQ.2 .AND. P.GT.A) .OR. HYST1620
(DIR.EQ.3 .AND. P.GT.A) .OR. (DIR.EQ.4 .AND. P.LT.A)) THEN HYST1630
RETURN HYST1640
ENDIF HYST1650
C ..... CALC EXCURSION RATIO AT EVERY ZERO CROSSING... HYST1660
IF (P*PL.LE.0) CALL DNE(0,UPCS,EXCR,DY,DLI,ESE,FSE,PSOLD,CSE) HYST1670
C ..... ERASE SMALL LOOP FLAGS HYST1680
IF (IR.GT.0) THEN HYST1690
IT=IR HYST1700
DO 10 I=1,IT HYST1710
IF ((FR(I).EQ.'L' .AND. (P.LE.FR(I) .OR. D.LE.DR(I))) .OR. HYST1720
(FR(I).EQ.'7' .AND. (P.GE.FR(I) .OR. D.GE.DR(I)))) THEN HYST1730
IR=I-1 HYST1740
GO TO 15 HYST1750
ENDIF HYST1760
10 CONTINUE HYST1770
ENDIF HYST1780
15 I=IR-1 HYST1790
C ..... IF (DIR.EQ.1 .OR. DIR.EQ.3) THEN HYST1800
LOADING RULES RULES 1,6 HYST1810
LOADING ON BACKBONE RULE HYST1820
IF (BACKB .OR. RULE.EQ.0) THEN HYST1830
X=0 HYST1840
DO 18 J=2,NB HYST1850
S1=ABS(D) HYST1860
DP=PB(J)-AP HYST1870
IF (AP.LT.PB(J) .AND. ((DP*DD).GT.0) THEN HYST1880
S=DP/DD HYST1890
IF (S.GT.K) THEN HYST1900
K=S HYST1910
A=SIGN(PB(J),P) HYST1920
ENDIF HYST1930
ENDIF HYST1940
18 CONTINUE HYST1950
RULE=1 HYST1960
BACKB=.TRUE HYST1970
IR=0 HYST1980
IF (DIR.EQ.1) THEN HYST1990
ELSE ALPHAI=ALPHAA HYST2000
ELSE ALPHAI=ALPHAA HYST2010
ENDIF HYST2020
C ..... UNLOADING INSIDE SMALL POSITIVE LOOPS RULE 5 HYST2030
I=I-1 HYST2040
IF (I.GT.0) THEN HYST2050
IF (NOT ((DIR.EQ.2 .AND. FR(I).EQ.'L') .OR. HYST2060
(DIR.EQ.4 .AND. FR(I).EQ.'7')) .GO TO 70 HYST2070
IF ((DIR.EQ.2 .AND. (P.GT.Q3) .AND. FR(I).GT.Q3) .OR. HYST2080

```



```

C BEE ELASTIC STRAIN ENERGY HYST6430 S-YU HYST1890
C PSE PLASTIC STRAIN ENERGY HYST6440 NREL-3 HYST1900
C PSEOLD PLASTIC STRAIN ENERGY AT LAST ZERO CROSSING HYST6450 HRLN-4.12 HYST1910
C UPOS POSITIVE DUCTILITY INFO. HYST6460 GO TO 2000 HYST1920
C UNEG NEGATIVE DUCTILITY INFO. HYST6470 A-PI HYST1930
C EXCR EXCURSION RATIO INFO. HYST6480 B=0 HYST1940
C HYST6490 B=S1 HYST1950
C JI # FOR CURRENT DIRECTION FOR LOADING AND UNLOADING HYST6500 NREL-4 HYST1960
C # FOR NEW DIRECTION FOR REVERSAL HYST6510 HRLN-4.2 HYST1970
C # - 2 FOR POSITIVE LOAD # - 1 FOR NEGATIVE LOAD HYST6520 GO TO 2000 HYST1980
C IREVSL SIGN OF LOAD IN NEW DIRECTION HYST6530 IF (DI.LE.DS) IDRVO-1 HYST1990
C ISGN SIGN OF THE CURRENT LOAD FOR LOADING AND UNLOADING HYST6540 IF (DI.GT.DS) IDRVO-2 HYST2000
C SIGN OF THE NEW LOAD FOR REVERSAL HYST6550 IF (ABS(UM>IDRVO,1)).GT.DC) GO TO 450 HYST2010
C HYST6560 A-PC*IREVSL HYST2020
C HYST6570 B=0 HYST2030
C 8000 IF ((PI.EQ.PS.OR.DI.EQ.DS).AND.RNRL.NE.0) RETURN HYST6580 S=S1 HYST2040
C HYST6590 NREL-15 HYST2050
C GET DIRECTION AND RTN IF CURRENT STATE IS WITHIN LIMITS.. HYST6600 HRLN-4.31 HYST2060
C CALL INDIRECT (IDR,IDRV,IDRVO,PI,PS,VI,VS) HYST6610 GO TO 2000 HYST2070
C IF (A.GT.B.AND.PI.LE.A.AND.PI.GT.B.AND.IDR.EQ.IDRO) RETURN HYST6620 X0=DI HYST2080
C IF (A.LT.B.AND.PI.GT.A.AND.PI.LT.B.AND.IDR.EQ.IDRO) RETURN HYST6630 X0M=(X0-UM>IDRVO,2)/(X0-UM>IDRVO,1) HYST2090
C # CALC EXCURSION RATIO AT EVERY ZERO CROSSING... HYST6650 X0Y=(X0-UM>IDRVO,2)/(X0-DY*ISGN) HYST2100
C IF ((PI*PS.LE.0)CALL.DNE(C,UPOS,EXCR,DY,DI,ESE,PSE,PSEOLD,CSE) HYST6660 IF ((PY.GT.ABS(UM>IDRVO,2)).AND.X0Y.GT.X0M) GO TO 460 HYST2110
C HYST6670 A=UM>IDRVO,2 HYST2120
C HYST6680 B=0 HYST2130
C HYST6690 S2=X0M HYST2140
C HYST6700 S=S2 HYST2150
C HYST6710 NREL-6 HYST2160
C HYST6720 HRLN-4.32 HYST2170
C HYST6730 GO TO 2000 HYST2180
C HYST6740 U=(J1,1)-DY*ISGN HYST2190
C HYST6750 U=(J1,2)-PY*ISGN HYST2200
C HYST6760 A=UM>(J1,2) HYST2210
C HYST6770 B=0 HYST2220
C HYST6780 S2=X0Y HYST2230
C HYST6790 S=S2 HYST2240
C HYST6800 NREL-6 HYST2250
C HYST6810 HRLN-4.33 HYST2260
C HYST6820 GO TO 2000 HYST2270
C HYST6830 HYST2280
C HYST6840 HYST2290
C 500 IF (IDR) 530,540,510 HYST2300
C 510 IF (ABS(PI).GE.ABS(UM>(J1,2))) GO TO 200 HYST2310
C A=UM>(J1,2) HYST2320
C B=PI HYST2330
C B=S1 HYST2340
C NREL-5 HYST2350
C HRLN-5.11 HYST2360
C GO TO 2000 HYST2370
C 530 A-PI HYST2380
C B=C HYST2390
C S=S1 HYST2400
C NREL-5 HYST2410
C HRLN-5.2 HYST2420
C GO TO 2000 HYST2430
C 540 IF (DI.LE.DS) IDRVO=2 HYST2440
C IF (DI.GT.DS) IDRVO=1 HYST2450
C IF (ABS(UM>IDRVO,1)).GT.DC) GO TO 450 HYST2460
C A-PC*IREVSL HYST2470
C B=0 HYST2480
C S=S1 HYST2490
C NREL-14 HYST2500
C HRLN-5.11 HYST2510
C GO TO 2000 HYST2520
C HYST2530
C HYST2540
C HYST2550
C HYST2560
C HYST2570
C HYST2580
C HYST2590
C HYST2600
C HYST2610
C HYST2620
C HYST2630
C HYST2640
C HYST2650
C HYST2660
C HYST2670
C HYST2680
C HYST2690
C HYST2700
C HYST2710
C HYST2720
C HYST2730
C HYST2740
C HYST2750
C HYST2760
C HYST2770
C HYST2780
C HYST2790
C HYST2800
C HYST2810
C HYST2820
C HYST2830
C HYST2840
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C HYST2970
C HYST2980
C HYST2990
C HYST3000
C HYST3010
C HYST3020
C HYST3030
C HYST3040
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C HYST3070
C HYST3080
C HYST3090
C HYST3100
C HYST3110
C HYST3120
C HYST3130
C HYST3140
C HYST3150
C HYST3160
C HYST3170
C HYST3180
C HYST3190
C HYST3200
C HYST3210
C HYST3220
C HYST3230
C HYST3240
C HYST3250
C HYST3260
C HYST3270
C HYST3280
C HYST3290
C HYST3300
C HYST3310
C HYST3320
C HYST3330
C HYST3340

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940 A-U0
B=0
X200=(PI-U0)/(DI-U0D)
S=X200
NRL=10
NRLN=9.3
GO TO 2000
C ----- TAKEDA RULE 10.0 ----- LOADING TOWARDS POINT U0
1000 IF (IDR) 1030,1030,1010
1010 IF (ABS(PI).GE.ABS(U0)) GO TO 610
A-U0
B=PI
X200=(PI-U0)/(DI-U0D)
S=X200
NRL=10
NRLN=10.11
GO TO 2000
1030 U2=PI
A=PI
B=0
S=PI
NRL=11
NRLN=10.2
GO TO 2000
C ----- TAKEDA RULE 11.0 ----- UNLOADING FROM POINT U2 AFTER RULE 10
1100 IF (IDR) 1130,1140,1110
1110 IF (ABS(PI).GE.ABS(U2)) GO TO 1010
A=U2
B=PI
S=PI
NRL=11
NRLN=11.11
GO TO 2000
1130 A=PI
B=0
S=PI
NRL=11
NRLN=11.2
GO TO 2000
1140 A=U1
B=0
X301=(PI-U1)/(DI-U1D)
S=X301
NRL=12
NRLN=11.3
GO TO 2000
C ----- TAKEDA RULE 12.0 ----- LOADING TOWARDS POINT U1
1200 IF (IDR) 1230,1230,1210
1210 IF (ABS(PI).GE.ABS(U1)) GO TO 810
A=U1
B=PI
X301=(PI-U1)/(DI-U1D)
S=X301
NRL=12
NRLN=12.11
GO TO 2000
1230 U3=PI
A=PI
B=0
S=PI
NRL=13
NRLN=12.2
GO TO 2000
C ----- TAKEDA RULE 13.0 ----- UNLOADING FROM POINT U3 AFTER RULE 12
1300 IF (IDR) 1330,1340,1310
1310 IF (ABS(PI).GE.ABS(U3)) GO TO 1210
A=U3
B=PI
S=PI
NRL=13
NRLN=13.11
GO TO 2000
1330 A=PI
B=0
S=PI
NRL=13
NRLN=13.2
GO TO 2000
C ----- TAKEDA RULE 14.0 ----- LOADING IN THE UNCRACKED DIRECTION AFTER POINT U3
1400 IF (IDR) 1430,1440,1410
1410 IF (ABS(PI).GE. PC) GO TO 210
A=PC*ISGN
B=0
S=PI
NRL=14
NRLN=14.11
GO TO 2000
1430 A=PI
B=0
S=PI
NRL=14
NRLN=14.2
GO TO 2000
1440 A=(UM(J1,2))
B=0
S=PI
NRL=5
NRLN=14.3
GO TO 2000
C ----- TAKEDA RULE 15.0 ----- UNLOADING IN THE UNCRACKED DIRECTION AFTER POINT U3
1500 IF (IDR) 1530,1540,1510
1510 IF (ABS(PI).GE. PC) GO TO 1520
A=PC*ISGN
B=PI
S=PI
NRL=15
NRLN=15.11
GO TO 2000
1520 PQ=PC*ISGN
DQ=(PQ-PI)/S1-DI
QY=(PY*ISGN-PQ)/(DY*ISGN-DQ)
A=PY*ISGN
B=PQ
S=QY
NRL=16
NRLN=15.12
GO TO 2000
1530 A=PI
B=0
S=PI
NRL=15
NRLN=15.2
GO TO 2000
1540 IF (ABS(PI).GE.ABS(UM(J1,2))) GO TO 1550
A=UM(J1,2)
B=0
S=PI
NRL=4
NRLN=15.31
GO TO 2000
1550 A=PU*IRVSL
B=0
S=PI
NRL=3
NRLN=15.32
GO TO 2000
C ----- TAKEDA RULE 16.0 ----- LOADING TOWARDS THE YIELD POINT AFTER
1600 IF (IDR) 1630,1630,1610
1610 IF (ABS(PI).GE. PY) GO TO 300
A=PY*ISGN
B=PI
S=QY
NRL=16
HYST3350
HYST3360
HYST3370
HYST3380
HYST3390
HYST3400
HYST3410
HYST3420
HYST3430
HYST3440
HYST3450
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HYST3900
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HYST3930
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HYST3950
HYST3960
HYST3970
HYST3980
HYST3990
HYST4000
HYST4010
HYST4020
HYST4030
HYST4040
HYST4050
HYST4060
HYST4070
HYST4080
HYST4090
HYST4100
HYST4110
HYST4120
HYST4130
HYST4140
HYST4150
HYST4160
HYST4170
HYST4180
HYST4190
HYST4200
HYST4210
HYST4220
HYST4230
HYST4240
HYST4250
HYST4260
HYST4270
HYST4280
HYST4290
HYST4300
HYST4310
HYST4320
HYST4330
HYST4340
HYST4350
HYST4360
HYST4370
HYST4380
HYST4390
HYST4400
HYST4410
HYST4420
HYST4430
HYST4440
HYST4450
HYST4460
HYST4470
HYST4480
HYST4490
HYST4500
HYST4510
HYST4520
HYST4530
HYST4540
HYST4550
HYST4560
HYST4570
HYST4580
HYST4590
HYST4600
HYST4610
HYST4620
HYST4630
HYST4640
HYST4650
HYST4660
HYST4670
HYST4680
HYST4690
HYST4700
HYST4710
HYST4720
HYST4730
HYST4740
HYST4750
HYST4760
HYST4770
HYST4780
HYST4790
HYST4800
NRLN=16.11
GO TO 2000
1630 UM(J1,1)-DY
UM(J1,2)-PY
U0=PI
UD0=D1
A=PI
B=0
S=PI
S=QY
X0=D1-PI/S2
NRL=7
NRLN=16.2
GO TO 2000
C ----- CALCULATE MAXIMUM PREVIOUS DEFORMATION
2000 UM1=MIN(UM(1,1),D1)
UM2=MIN(UM(1,2),PI)
UM3=MAX(UM(2,1),D1)
UM4=MAX(UM(2,2),PI)
NRL=NRL
IDR0=IDR
RF=0
IF (IDR.GE.0) ACHNG=A
IF (IDR.LT.0) ACHNG=B
RETURN
END
C ----- HYST07 -----
SUBROUTINE HYST07 (PC,DC,FL,DL,X,RULE,PSEOLD,A,B,VI,VL,S,PY,KIE,
A
DY,CSE,ESE,PSE,PSEOLD,PC,FL,DC,DL,S)
REAL X,KIE
DIMENSION UPOS(3),UMEG(3),EMCK(6),ESE(3)
C
SU_BROU_TINE ELSPLS - ELASTO-PLASTIC BILINEAR HYSTERESIS MODEL
C
PC - CURRENT LOAD
PEQ - MODIFIED CURRENT LOAD
DC - CURRENT DISPL.
PL - LAST LOAD
DL - LAST DISPL.
PY - YIELD LOAD
PVEQ - MODIFIED YIELD LOAD
S - ELASTIC STIFFNESS
X - CURRENT STIFFNESS (OUTPUT)
RULE - CURRENT RULE NUMBER. USED TO TRACE MODEL (OUTPUT)
NRL - NEXT RULE NUMBER (OUTPUT)
VI - NOT USED
VL - NOT USED
KIE - INELASTIC STIFFNESS
KEY=KEY
PC=X*(DC-DL)*PL
C ----- CALC EXCURSION RATIO AT EVERY ZERO CROSSING ...
IF (PC*PL.EQ.0) CALL DNE(0,UPOS,EMCK, DY,DL,ESE,PSE,PSEOLD,CSE)
C
C ----- ELASTIC REGION
IF (RULE.LE.1 .AND. ABS(PC).LT.PY) THEN
K=S
RULE=1
A=PY
B=PY
PC=S*DC
CALL STRENG (ESE,PSE,PSEOLD,PC,FL,DC,DL,S)
KEY=0
RKEY=KEY
GO TO 1000
ENDIF
C ----- INELASTIC REGION
DIR=DC-DL
IF (DIR.EQ.0) RETURN
PEQ=PC-KIE*DC
PEQL=PL-KIE*DL
PVEQ=PY*(1-KIE/S)
DVEL=VI*VL
C ----- LOAD AT OR ABOVE POSITIVE YIELD
IF (RULE.EQ.1) THEN
IF (DIR.GT.0) THEN
POSITIVE YIELD ...
CALL STRENG (ESE,PSE,PSEOLD, PY,FL, DY,DL,S)
PL=PY
DL=DI
GO TO 210
ELSE
NEGATIVE YIELD ...
CALL STRENG (ESE,PSE,PSEOLD, -PY,FL, -DY,DL,S)
PL=PY
DL=-DY
GO TO 220
ENDIF
ELSE
IF (RULE.EQ.2.10) GO TO 210
IF (RULE.EQ.2.20) GO TO 230
IF (RULE.EQ.2.30) GO TO 330
ENDIF
WRITE(108,*) 'INVALID RULE IN HYST07-BILINEAR HYSTERESIS MODEL'
C ----- LOAD AT OR ABOVE POSITIVE YIELD, ON BACKBONE CURVE
210 IF (DIR.LE.0) GO TO 230
K=KIE
RULE=2.10
PC=PEQ*DC*KIE
B=PC
A=PC*10
CALL STRENG (ESE,PSE,PSEOLD,PC,FL,DC,DL,S)
RKEY=1
IF (DVEL.LT.0) THEN
X=S
B=PC
A=PC
ENDIF
GO TO 1000
C ----- LOAD AT OR BELOW NEGATIVE YIELD, ON BACKBONE CURVE
220 IF (DIR.GE.0) GO TO 230
X=KIE
RULE=2.20
PC=PVEQ*DC*KIE
B=PC
A=PC*10
CALL STRENG (ESE,PSE,PSEOLD,PC,FL,DC,DL,S)
RKEY=2
IF (DVEL.LT.0) THEN
X=S
B=PC
A=PC
ENDIF
GO TO 1000
C ----- LOAD IN LINEAR RANGE BETWEEN BACKBONE CURVES
230 X=S
RULE=2.3
PC=S*(DC-DL)*PL
A=PC+PVEQ*PEQ
B=PC-PVEQ*PEQ
IF (PC.GT.A) THEN
DA=DC*(A-PC)/S
CALL STRENG (ESE,PSE,PSEOLD, A,FL,DA,DL,S)
DL=DA

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IP(PREGIN.NE.REGIN) REVRSC-.FALSE.
STIP-SLOP1*ES
IP(DMAX.LT.0.AND.DE.LE.DMAX) THEN
RSM=((0.55*ABS(DE)/ESR)+0.0001*ABS(DE)**2)*RFAC
ELSE IP(DMAX.GT.0) THEN
RSM=((0.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC
ENDIF
-----
IP(.NOT.REVRSC) THEN
EKI-SLOP1*ES
ELSE IF(REVRSC) THEN
EKI-ES*((PTOP.PL)/(DTOP-DEL))
ENDIF
-----
ELSE IF(DDE.GT.0) THEN
IP(.NOT.REVRSC) THEN
RULE=7
STIP-SLOP1*ES
-----
PLA-PL-SLOP2*DDEL
DLA-DEL-DDEL
DBOT-DLA
PBOT-PLA
PL-PLA-SLOP1*DDEL
P-PL*PY
-----
PTOP-VE*(SLOP4*(PBOT-VE-SLOP1*DBOT
SLOP1*HD)/(SLOP4-SLOP1))
DTOP-(PBOT-VE+SLOP5*HE-SLOP1*DBOT)
/(SLOP4-SLOP1)
EKI-ES*SLOP1
ELSE IF(REVRSC) THEN
RULE=12
DBOT-DEL
PBOT-P/PY
SLOP7*(PTOP-PBOT)/(DTOP-DBOT)
STIP-SLOP7*ES
EKI-ES*SLOP7
ENDIF
ENDIF
ELSE IF(DEL.LT.HH.AND.DEL.GE.HB) THEN
IP(DDE.LE.0) THEN
STIP-SLOP2*ES
IP(DMAX.LT.0.AND.DE.LE.DMAX) THEN
RSM=((0.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC
ELSE IF(DMAX.GT.0) THEN
RSM=((0.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC
ENDIF
-----
IP(.NOT.REVRSC) THEN
EKI-SLOP1*ES
ELSE IF(REVRSC) THEN
EKI-ES*((PTOP.PL)/(DTOP-DEL))
ENDIF
-----
ELSE IF(DDE.GT.0) THEN
IP(.NOT.REVRSC) THEN
RULE=8
STIP-SLOP1*ES
-----
PLA-PL-SLOP2*DDEL
DLA-DEL-DDEL
DBOT-DLA
PBOT-PLA
PL-PLA-SLOP1*DDEL
P-PL*PY
-----
PTOP-VD*(SLOP4*(PBOT-VD-SLOP1*DBOT
SLOP1*HD)/(SLOP4-SLOP1))
DTOP-(PBOT-VD+SLOP4*HD-SLOP1*DBOT)
/(SLOP4-SLOP1)
EKI-ES*SLOP1
ELSE IF(REVRSC) THEN
RULE=11
DBOT-DEL
PBOT-P/PY
PTOP-VD
DTOP-HD
SLOP7*(PTOP-PBOT)/(DTOP-DBOT)
STIP-SLOP7*ES
EKI-ES*SLOP7
ENDIF
ENDIF
ELSE IF(DEL.LT.HB) THEN
IP(PREGIN.NE.REGIN) REVRSC-.FALSE
RULE=3
EKI-ES*SLOP1
ENDIF
IF(REVRSC) IRV=1.
IP(.NOT.REVRSC) IRV=0.
CALL STRENG(ESE,PSE,PSOLD,P,PLA,DE,DELA,EXI)
IF(ABS(DEMAX.LT.ABS(DE)) THEN
CALL DNE(1, DUC, EKCR, DELY, DA, ESE, PSE, PSOLD, CSE)
DEMAX-DE
ENDIF
RETURN
ENDIF
(3) RULE 3
-----
IP(RULE.EQ.3) THEN
IF(DEL.GT.HC) THEN
IF(DDE.LE.0) THEN
IP(PREGIN.NE.REGIN) REVRSC-.FALSE.
STIP-SLOP3*ES
IP(DMAX.LT.0.AND.DE.LE.DMAX) THEN
RSM=((0.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC
ELSE IF(DMAX.GT.0) THEN
RSM=((0.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC
ENDIF
-----
IP(.NOT.REVRSC) THEN
EKI-SLOP1*ES
ELSE IF(REVRSC) THEN
EKI-ES*((PTOP.PL)/(DTOP-DEL))
ENDIF
-----
ELSE IF(DDE.GT.0) THEN
IP(.NOT.REVRSC) THEN
RULE=9
STIP-SLOP1*ES
-----
PLA-PL-SLOP2*DDEL
DLA-DEL-DDEL
DBOT-DLA
PBOT-PLA
PL-PLA-SLOP1*DDEL
P-PL*PY
-----
PTOP-VD*(SLOP4*(PBOT-VD-SLOP1*DBOT
SLOP1*HD)/(SLOP4-SLOP1))
DTOP-(PBOT-VD+SLOP4*HD-SLOP1*DBOT)
/(SLOP4-SLOP1)
EKI-ES*SLOP1
ELSE IF(REVRSC) THEN
RULE=10
DBOT-DEL
PBOT-P/PY
SLOP7*(PTOP-PBOT)/(DTOP-DBOT)
STIP-SLOP7*ES
EKI-ES*SLOP7
ENDIF
ENDIF
ELSE IF(DEL.LE.12) THEN
REVRSC-.FALSE.
RULE=4
-----
HYST2690 STIP-SLOP4*ES
HYST2700 EKI-ES*SLOP4
HYST2710 ENDF
HYST2720 ELSE IF(DEL.LE.HC) THEN
HYST2730 IP(DDE.LT.0) THEN
HYST2740 RULE=3
HYST2750 HC-DEL
HYST2760 VC-PL
HYST2770 IDUC=1
HYST2780 STIP-SLOP6*ES
HYST2790 EKI-ES*((VD-VC)/(HD-HC))
HYST2800 IF(DMAX.LT.0.AND.DE.LE.DMAX) THEN
HYST2810 RSM=((0.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC
HYST2820 ELSE IF(DMAX.GT.0) THEN
HYST2830 RSM=((0.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC
HYST2840 ENDF
HYST2850 ELSE IF(DDE.GT.0) THEN
HYST2860 RULE=4
HYST2870 STIP-SLOP4*ES
HYST2880 EKI-ES*SLOP4
HYST2890 ENDF
HYST2900 IF(REVRSC) IRV=1.
HYST2910 IF(.NOT.REVRSC) IRV=0.
HYST2920 CALL STRENG(ESE,PSE,PSOLD,P,PLA,DE,DELA,EXI)
HYST2930 IF(ABS(DEMAX.LT.ABS(DE)) THEN
HYST2940 CALL DNE(1, DUC, EKCR, DELY, DA, ESE, PSE, PSOLD, CSE)
HYST2950 DEMAX-DE
HYST2960 ENDF
HYST2970 RETURN
HYST2980 ENDF
(4) RULE 7
-----
IP(RULE.EQ.7) THEN
IF(DEL.GT.DBOT.AND.DEL.LT.DTOP.AND.P.GT.PMAX) THEN
HYST2990 STIP-SLOP1*ES
HYST3000 EKI-ES*SLOP1
HYST3010 ELSE IF(DEL.LT.DBOT.OR.P.LE.PMAX) THEN
HYST3020 REVRSC-.FALSE.
HYST3030 PTOP=0
HYST3040 DTOP=0
HYST3050 PBOT=PL
HYST3060 DBOT-DEL
HYST3070 S1-PL/PY
HYST3080 S2-PMAX/PY
HYST3090 HA=(DELA/DELY)*((S2-S1)/(SLOP1))
HYST3100 VA=S2
HYST3110 SLOP2=(VB-VA)/(HB-HA)
HYST3120 ENDF
HYST3130 STIP-SLOP2*ES
HYST3140 RULE=2
HYST3150 PL-VA-SLOP2*(DBOT-HA)
HYST3160 P-PL*PY
-----
EKI-ES*((PTOP.PL)/(DTOP-DEL))
ELSE IF(DEL.GT.DTOP.AND.PL.LT.VE) THEN
HYST3170 STIP-SLOP5*ES
HYST3180 RULE=5
HYST3190 EKI-ES*SLOP1
HYST3200 ELSE IF(DEL.GT.DTOP.AND.PL.GE.VE) THEN
HYST3210 STIP-SLOP6*ES
HYST3220 RULE=6
HYST3230 EKI-ES*SLOP1
HYST3240 ENDF
HYST3250 IF(REVRSC) IRV=1.
HYST3260 IF(.NOT.REVRSC) IRV=0
HYST3270 CALL STRENG(ESE,PSE,PSOLD,P,PLA,DE,DELA,EXI)
HYST3280 RETURN
HYST3290 ENDF
(5) RULE 8
-----
IP(RULE.EQ.8) THEN
IF(DEL.GT.DBOT.AND.DEL.LT.DTOP) THEN
HYST3300 STIP-SLOP1*ES
HYST3310 EKI-ES*SLOP1
HYST3320 ELSE IF(DEL.LT.DBOT) THEN
HYST3330 STIP-SLOP2*ES
HYST3340 RULE=2
HYST3350 PL-PROT*(DBOT-DEL)*SLOP2
HYST3360 P-PL*PY
-----
EKI-ES*((PTOP.PL)/(DTOP-DEL))
ELSE IF(DEL.GT.DTOP) THEN
HYST3370 STIP-SLOP4*ES
HYST3380 EKI-ES*SLOP1
HYST3390 RULE=4
HYST3400 EKI-ES*SLOP1
HYST3410 ENDF
HYST3420 IF(REVRSC) IRV=1.
HYST3430 IF(.NOT.REVRSC) IRV=0
HYST3440 CALL STRENG(ESE,PSE,PSOLD,P,PLA,DE,DELA,EXI)
HYST3450 RETURN
HYST3460 ENDF
(6) RULE 9
-----
IP(RULE.EQ.9) THEN
IF(DEL.GT.DBOT.AND.DEL.LT.DTOP) THEN
HYST3470 STIP-SLOP1*ES
HYST3480 EKI-ES*SLOP1
HYST3490 ELSE IF(DEL.LT.DBOT) THEN
HYST3500 STIP-SLOP3*ES
HYST3510 RULE=3
HYST3520 PL-PROT*(DBOT-DEL)*SLOP3
HYST3530 P-PL*PY
-----
EKI-ES*((PTOP.PL)/(DTOP-DEL))
ELSE IF(DEL.GT.DTOP) THEN
HYST3540 STIP-SLOP4*ES
HYST3550 EKI-ES*SLOP1
HYST3560 RULE=4
HYST3570 EKI-ES*SLOP1
HYST3580 ENDF
HYST3590 IF(REVRSC) IRV=1.
HYST3600 IF(.NOT.REVRSC) IRV=0
HYST3610 CALL STRENG(ESE,PSE,PSOLD,P,PLA,DE,DELA,EXI)
HYST3620 RETURN
HYST3630 ENDF
(7) RULE 4
-----
IP(RULE.EQ.4) THEN
IF(DEL.GE.HC1.AND.DEL.LT.HD) THEN
HYST3640 IF(DDE.GE.0) THEN
HYST3650 STIP-SLOP4*ES
HYST3660 EKI-ES*SLOP1
HYST3670 ELSE IF(DDE.LT.0) THEN
HYST3680 PREP-VD*(SLOP4*(VB-VD-SLOP1*HB
HYST3690 SLOP1*HD)/(SLOP4-SLOP1))
HYST3700 DREP-(VB-VD-SLOP4*HD-SLOP1*HB)/(SLOP4-SLOP1)
HYST3710 IF(DEL.LT.DREP) THEN
HYST3720 PTOP-P/PY
HYST3730 DTOP-DEL
HYST3740 DBOT=(SLOP3*HB-VB-PTOP-SLOP1*DEL)/(SLOP3-SLOP1)
HYST3750 PBOT-VB-SLOP3*(DBOT-HB)
HYST3760 STIP-SLOP1*ES
HYST3770 RULE=9
HYST3780 REVRSC=.TRUE.
HYST3790 EKI-ES*SLOP1
HYST3800 ELSE IF(DEL.GE.DREP) THEN
HYST3810 PTOP-P/PY
HYST3820

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DTOP=DEL
DBOT=(SLOP2*HA+VA*PTOP-SLOP1*DEL)/(SLOP2-SLOP1)
PBOT=VA-SLOP2*(DBOT-HA)
STIP=SLOP1*ES
RULE=5
REVRSC=.TRUE.
EXI=ES*SLOP1
ENDIF
ELSE IF ( DEL .GT. HD .AND. DDE .GT. 0 ) THEN
STIP=SLOP5*ES
RULE=5
ELSE IF ( DEL .LT. HC ) .AND. DEL .GE. HC ) THEN
REVRSC=.FALSE.
RULE=4
STIP=SLOP4*ES
EXI=ES*SLOP4
ELSE IF ( DEL .LT. HC ) THEN
REVRSC=.FALSE.
RULE=3
HC=DEL
VC=PL
IDUC=1
STIP=SLOP5*ES
EXI=ES*(VD-VC)/(HD-HC)
ENDIF
IF ( REVRSC ) IRV=1
IF ( .NOT. REVRSC ) IRV=0
CALL STRENG(ESE,PSE,PSEOLD,P,PLA,DE,DELA,EXI)
IF ( ABS(DEMAX) .LT. ABS(DE) ) THEN
CALL DNE(1, DUC, EXCR, DELV, DA, ESE, PSE, PSEOLD, CSE )
DEMAX=DE
ENDIF
RETURN
ENDIF
(8) RULE 5
IF ( RULE EQ. 5 ) THEN
IF ( DEL .LT. HE ) THEN
IF ( DDE .GE. 0 ) THEN
STIP=SLOP5*ES
EXI=ES*SLOP1
ELSE IF ( DDE .LT. 0 ) THEN
BREP=VA-SLOP1*HA
HREP=(VD-SLOP5*HD-BREP)/(SLOP1-SLOP5)
IF ( DEL .GE. HREP ) CYCLE=CYCLE+1
PTOP=H/PTOP
DTOP=DEL
DBOT=(SLOP2*HA+VA*PTOP-SLOP1*DEL)/(SLOP2-SLOP1)
PBOT=VA-SLOP2*(DBOT-HA)
STIP=SLOP1*ES
RULE=7
REVRSC=.TRUE.
EXI=ES*SLOP1
ENDIF
ELSE IF ( DEL .GE. HE .AND. DDE .GT. 0 ) THEN
STIP=SLOP6
RULE=6
EXI=ES*SLOP1
ENDIF
IF ( REVRSC ) IRV=1
IF ( .NOT. REVRSC ) IRV=0
CALL STRENG(ESE,PSE,PSEOLD,P,PLA,DE,DELA,EXI)
IF ( ABS(DEMAX) .LT. ABS(DE) ) THEN
CALL DNE(1, DUC, EXCR, DELV, DA, ESE, PSE, PSEOLD, CSE )
DEMAX=DE
ENDIF
RETURN
ENDIF
(9) RULE 6
IF ( RULE EQ. 6 ) THEN
IF ( DDE .GE. 0 ) THEN
STIP=SLOP6
EXI=ES*SLOP1
ELSE IF ( DDE .LT. 0 ) THEN
STIP=SLOP1*ES
RULE=1
CYCLE=CYCLE+1
REVRSC=.FALSE.
EXI=ES*SLOP1
ENDIF
IF ( REVRSC ) IRV=1
IF ( .NOT. REVRSC ) IRV=0
CALL STRENG(ESE,PSE,PSEOLD,P,PLA,DE,DELA,EXI)
IF ( ABS(DEMAX) .LT. ABS(DE) ) THEN
CALL DNE(1, DUC, EXCR, DELV, DA, ESE, PSE, PSEOLD, CSE )
DEMAX=DE
ENDIF
RETURN
ENDIF
(10) RULE 10
IF ( RULE EQ. 10 ) THEN
IF ( DEL .GT. DBOT .AND. DEL .LT. DTOP ) THEN
STIP=SLOP7*ES
EXI=ES*SLOP7
ELSE IF ( DEL .LT. DBOT ) THEN
STIP=SLOP3*ES
REVRSC=.FALSE.
PTOP=0
DTOP=0
PBOT=0
DBOT=0
EXI=ES*((PTOP-PL)/(DTOP-DEL))
ELSE IF ( DEL .GT. DTOP ) THEN
STIP=SLOP4*ES
RULE=4
EXI=ES*SLOP1
ENDIF
IF ( REVRSC ) IRV=1
IF ( .NOT. REVRSC ) IRV=0
CALL STRENG(ESE,PSE,PSEOLD,P,PLA,DE,DELA,EXI)
RETURN
ENDIF
(11) RULE 11
IF ( RULE EQ. 11 ) THEN
IF ( DEL .GT. DBOT .AND. DEL .LT. DTOP ) THEN
STIP=SLOP7*ES
EXI=ES*SLOP7
ELSE IF ( DEL .LT. DBOT ) THEN
STIP=SLOP2*ES
RULE=2
REVRSC=.FALSE.
PTOP=0
DTOP=0
PBOT=0
DBOT=0
EXI=ES*((VD-PL)/(HD-DEL))
ELSE IF ( DEL .GT. DTOP ) THEN
STIP=SLOP5*ES
RULE=5
EXI=ES*SLOP1
ENDIF
IF ( REVRSC ) IRV=1
IF ( .NOT. REVRSC ) IRV=0
CALL STRENG(ESE,PSE,PSEOLD,P,PLA,DE,DELA,EXI)
RETURN
ENDIF

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SE(15)- DXP MAT11320 IF (.NOT. TNPTPG) SE(16)-0 MAT12780
SE(16)- TNPTPG MAT11330 IF (POSTPG) SE(17)-1 MAT12790
SE(17)- POSTPG MAT11340 IF (.NOT. POSTPG) SE(17)-0 MAT12800
SE(18)- COL MAT11350 IF (SRFLAG) SE(14)-1 MAT12810
SE(19)- RAT MAT11360 IF (.NOT. SRFLAG) SE(14)-0 MAT12820
SE(40)- RAT1 MAT11370 SE( 3)-LRLU MAT12830
SE(41)- SQE MAT11380 SE(11)-CYCLEN MAT12840
SE(42)- SR MAT11390 SE( 2)-LRLU MAT12850
SE(43)- SRPS MAT11400 SE(16)-IOPT2 MAT12860
SE(44)- SRFLAG MAT11410 RETURN MAT12870
-USE - EQUIVALENT UNLOADING STIFFNESS MAT12880
SE(45)-USE MAT11430 C ----- TRANSFER ENERGIES FOR BOTH IOPT-3 AND 4. MAT12890
SE(46)- IOPT2 MAT11440 C (NEGLECT DUCTILITIES CALCULATION AT PRESENT STAGE) MAT12900
-IOPT2 USED IN HYST14 SUBRTN (DNCKUL,DISSEI SUBRTN IN HYST14) MAT11450 400 EPSE=SE(52) MAT12910
MAT11460 EPSE=SE(55) MAT12920
SE(47)- IOPT MAT11470 C ----- TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT-3 & 4. MAT12930
SE(48)- PSQE MAT11480 DO 410 I=1,3 MAT12940
SE(49)- KLAST MAT11490 DUCT(I)=MAX(SE(71-I),SE(74-I)) MAT12950
SE(50)- DPT MAT11500 EXCR(I)=SE(56-I) MAT12960
SE(51)- DPT MAT11510 410 EXCR(I-3)=SE(59-I) MAT12970
SE(52)- ESE MAT11520 RETURN MAT12980
SE(53)- ESE MAT11530 C ----- MAT13000
SE(54)- ESE MAT11540 ----- DAMAGE INDEX ----- MAT13010
SE(55)- PSE MAT11550 500 EPSE=SE(52) MAT13020
SE(56)- PSE OLD MAT11560 EPSE=SE(55) MAT13030
SE(57)- EXCR(1) MAT11570 C ----- DMAX IS THE LARGER OF DMAX OR DM IN HYST14 SUBRTN MAT13040
SE(58)- EXCR(2) MAT11580 DMAX=MAX(SE(5),SE(6)) MAT13050
SE(59)- EXCR(3) MAT11590 PY=SMAT(6) MAT13060
SE(60),SE(61),SE(62) SEE ONE SUBRTN MAT11600 C ----- DDU - FAILURE DISPLACEMENT OF BACKBONE CURVE MAT13070
SE(60)- EXCR(4) MAT11610 DDU=SMAT(11) MAT13080
SE(61)- EXCR(5) MAT11620 BDAM=SMAT(5)*NSEC MAT13090
SE(62)- EXCR(6) MAT11630 DAMAGE=DDMAX/DDU*BDAM/(PY*DDU)*EPSE MAT13100
SE(63)- FC MAT11640 RETURN MAT13110
SE(64)- FY MAT11650 END C ----- MAT13000
SE(65)- FU MAT11660 C ---- TO CALCULATE AVAILABLE STIFFNESS USED IN THE HYSTERESIS LOOP C ----- MAT13020
SE(66)- FF MAT11670 C ----- MAT13030
SE(67)- DC MAT11680 C SUBROUTINE OPNSTP(FC,FY,FU,FF,DC,DY,DU,DP,SC,SCY,SOV, MAT13040
SE(68)- DY MAT11690 & SYU,SUP,SCU,SOB,SOE) MAT13050
SE(69)- DU MAT11700 C ----- MAT13060
SE(70)- DP MAT11710 C ----- MAT13070
SE(71)- DSINCL MAT11720 C COMPUTE STIFFNESS S(OC) MAT13080
SE(72)- UPOS(1) MAT11730 SOC=FC/DC C ----- MAT13090
SE(73)- UPOS(2) MAT11740 C COMPUTE STIFFNESS S(CY) MAT13100
SE(74)- UPOS(3) MAT11750 SCY=(FY-FC)/(DY-DC) C ----- MAT13110
SE(75)- UNEG(1) MAT11760 C COMPUTE STIFFNESS S(OY) MAT13120
SE(76)- UNEG(2) MAT11770 SOY=FY/DY C ----- MAT13130
SE(77)- UNEG(3) MAT11780 C COMPUTE STIFFNESS S(YU) MAT13140
SE(78)- LENGTH MAT11790 SYU=(FU-FY)/(DU-DY) C ----- MAT13150
SE(79)- WIDTH MAT11800 C COMPUTE STIFFNESS S(OP) MAT13160
SE(80)- SHEAR SPAN LENGTH RATIO: SMAT(11)*NSEC OR MQD MAT11810 SUP=(FU-FY)/(DP-DC) C ----- MAT13170
MAT11820 C COMPUTE STIFFNESS S(CU) MAT13180
MAT11830 SCU=(FU-FC)/(DU-DC) C ----- MAT13190
MAT11840 C COMPUTE STIFFNESS S(OB) ; B IS MIDPT. OF PT. A AND PT. Y MAT13200
MAT11850 C PT. A - INTERSECTION OF EXTENSION OF OC WITH HORIZONTAL LINE THRU Y MAT13210
MAT11860 XA=PY/SOC MAT13220
MAT11870 XB=(XA-DY)/2 MAT13230
MAT11880 SOB=PY/XB MAT13240
MAT11890 C COMPUTE STIFFNESS S(OE) ; E IS MIDPT. OF PT. D AND PT. U MAT13250
MAT11900 C PT. D - INTERSECTION OF EXTENSION OF OY WITH HORIZONTAL LINE THRU U MAT13260
MAT11910 XD=FU/SOY MAT13270
MAT11920 XE=(XD-DU)/2 MAT13280
MAT11930 SOE=PY/XE MAT13290
MAT11940 RETURN MAT13300
MAT11950 END MAT13310
MAT11960 C ----- MAT13000
MAT11970 C -- THIS HYSTERESIS SUBROUTINE FOR PERFORATED SHEAR WALL. . . HYST14. . . . . HYST0200
MAT11980 ----- HYST0300
MAT11990 ----- HYST0400
MAT12000 C SUBROUTINE HYST14(FP ,DP ,FFP ,DPP ,H , MAT12010
MAT12010 & A ,OPC ,ODC ,OPY ,ODY ,OPU ,ODU , MAT12020
MAT12020 & H ,OC ,OSOC ,OSCY ,OSOB ,K ,LRLU , MAT12030
MAT12030 & KKL ,DSINCL ,DMAX ,DM ,KP ,SOC , MAT12040
MAT12040 & SCY ,SYU ,SUF ,SOB ,SOE ,FUO , MAT12050
MAT12050 & DUC ,KEYDS1 ,FU17 ,ARAL ,CYCLEN ,CSE , MAT12060
MAT12060 & FU00 ,TNPTPG ,POSTPG ,COL ,SQE ,SR , MAT12070
MAT12070 & USE ,IOPT2 ,PSQE ,KLAST ,DPTT ,DPT , MAT12080
MAT12080 & ESE ,PSE ,PSEOLD ,EXCR ,FC ,FY , MAT12090
MAT12090 & FU ,FF ,DC ,DY ,DU ,DP , MAT12100
MAT12100 & DSINCL ,UPOS ,UNEG ) MAT12110
MAT12110 C ----- MAT12120
MAT12120 C INTERC CYCLEN MAT12130
MAT12130 REAL K,KE,DUO(4),FUO(4),KLAST,KEYDS1,KEYDS2 MAT12140
MAT12140 LOGICAL UPOS(3),UNEG(3),EXCR(6),SE(3) MAT12150
MAT12150 RETURN MAT12160
MAT12160 C ----- MAT12170
MAT12170 C ----- MAT12180
MAT12180 C ----- MAT12190
MAT12190 C ----- MAT12200
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MAT16370 C ----- MAT16380
MAT16380 C ----- MAT16390
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MAT16980 C ----- MAT16990
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MAT17210 C ----- MAT17220
MAT17220 C ----- MAT17230
MAT17230 C ----- MAT17240
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MAT1873
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DSINC-DP-DPP
C 11 IP WALL FAIL(K=0) AT FORCE=0 THEN STILL K=0 AND RETURN
IP(K.EQ.0) THEN
  WRITE(10,900)
900 FORMAT(IX,'*** WALL FAIL, AND STOP THE PROGRAM')
  STOP
END IF
C
C 12 WHEN DISPLACEMENT INCREMENT CHANGE SIGN, REMEMBER TWO PREVIOUS
MAX. DISP. OF TWO HALF CYCLES
C
C 13 IF (DSINCL-DSINC).LT.0) THEN
KEYDS1=KEYDS2
KEYDS2=DPP
C
C 14 TO MEMORIZE THE POINTS AT WHICH DSINC<DSINCL < 0.
IP(POSTPG) THEN
IP(KXLLST.EQ.2 .AND. KXL.EQ.5) THEN
  PUD(2)=0
  DUO(2)=0
END IF
IP(KXLLST.EQ.4 .AND. KXL.EQ.6) THEN
  PUD(1)=0
  DUO(1)=0.
END IF
IP(DSINC.LT.0) THEN
IP(KXLLST.EQ.8 OR KXLLST.EQ.10) THEN
  PUD(1)=MAX(PUD(1),PPP)
  DUO(1)=MAX(DUO(1),KEYDS2)
ELSE
  PUD(1)=PPP
  DUO(1)=KEYDS2
END IF
IP(DSINC.GT.0) THEN
IP(KXLLST.EQ.7 OR KXLLST.EQ.9) THEN
  PUD(2)=MIN(PUD(2),PPP)
  DUO(2)=MIN(DUO(2),KEYDS2)
ELSE
  PUD(2)=PPP
  DUO(2)=KEYDS2
END IF
END IF
ELSE
IP(KXLLST.EQ.2 AND KXL.EQ.5) THEN
  PUD(1)=0.
  DUO(1)=0.
END IF
IP(KXLLST.EQ.4 AND KXL.EQ.6) THEN
  PUD(2)=0.
  DUO(2)=0.
END IF
IP(DSINC.GT.0) THEN
IP(KXLLST.EQ.7 OR KXLLST.EQ.9) THEN
  PUD(1)=MIN(PUD(1),PPP)
  DUO(1)=MIN(DUO(1),KEYDS2)
ELSE
  PUD(1)=PPP
  DUO(1)=KEYDS2
END IF
IP(DSINC.LT.0) THEN
IP(KXLLST.EQ.8 OR KXLLST.EQ.10) THEN
  PUD(2)=MAX(PUD(2),PPP)
  DUO(2)=MAX(DUO(2),KEYDS2)
ELSE
  PUD(2)=PPP
  DUO(2)=KEYDS2
END IF
END IF
C
C 15 IF ((ABS(KEYDS2) LT.DC) .AND. (KXL.EQ.6) .AND. (KEYDS2.GT.0)) THEN
GO TO 23
END IF
C 16 IF ((ABS(KEYDS2) LT.DC) .AND. (KXL.EQ.5) .AND. (KEYDS2.LT.0)) THEN
KXL=4
GO TO 23
END IF
C 17 IF ((KXL.EQ.2) .AND. ((DM.GE.DC) OR (DMAX.GE.DC))) THEN
KXL=8
GO TO 23
END IF
C 18 IF ((KXL.EQ.2) .AND. (DM.LT.DC) .AND. (DMAX.LT.DC)) THEN
KXL=1
GO TO 23
END IF
C 19 IF ((KXL.EQ.4) .AND. ((DM.GE.DC) OR (DMAX.GE.DC))) THEN
KXL=7
GO TO 23
END IF
C 20 IF ((KXL.EQ.4) .AND. (DM.LT.DC) .AND. (DMAX.LT.DC)) THEN
KXL=3
GO TO 23
END IF
C
C 21 IF (KXL.EQ.1) THEN
KXL=2
GO TO 23
ELSE IF (KXL.EQ.3) THEN
KXL=4
GO TO 23
ELSE IF ((KXL.EQ.2) .AND. (PP.LE.0) .AND. (DPP.NE.0)
.AND. (DSINC.LT.0)) THEN
KXL=5
GO TO 23
ELSE IF ((KXL.EQ.4) .AND. (PP.LE.0) .AND. (DPP.NE.0)
.AND. (DSINC.GT.0)) THEN
KXL=6
GO TO 23
END IF
IP(KXL.EQ.8) THEN
KXL=2
GO TO 23
END IF
IP(KXL.EQ.7) THEN
KXL=4
GO TO 23
END IF
IP(KXL.EQ.10) THEN
KXL=2
GO TO 23
END IF
IP(KXL.EQ.9) THEN
KXL=3
GO TO 23
END IF
IP(KXL.EQ.5) THEN
KXL=10
PSEOLD=AREAL
GO TO 13
END IF
IP(KXL.EQ.6) THEN
KXL=9
PSEOLD=AREAL
GO TO 13
END IF
C
C 22 IF (KEYDS2.LT.0.0 .AND. (ABS(KEYDS2) LT.DC) .AND. (KXL.EQ.5)) KXL=4
IF (KEYDS2.GT.0.0 .AND. (KEYDS2 LT.DC) .AND. (KXL.EQ.6)) KXL=2
END IF
C
C 23 IF (KXL.EQ.10) .AND. ((DSINC<DSINCL) LT.0) THEN
IP(POSTPG) THEN

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HYST0830 PFFF-FUO(1)
HYST0840 DDDD-DUO(1)
HYST0850 ELSE
HYST0860 PFFF-FUO(2)
HYST0870 DDDD-DUO(2)
HYST0880 END IF
HYST0890 PP1-PPPP*0.9
HYST0900 DP1-DDDD
HYST0910 KP=(PP1-0)/(DP1-DPP)
HYST0920 X=KP
HYST0930 USE=MAX(K,SOB)
HYST0940 IP(POSTPG) CYCLEN-CYCLEN+1
HYST0950 IOPT2=3
HYST0960 CALL DD1SSI(IOPT2,ESE,PSE,PSEOLD,DPP,PPP,DP,AREAL,
HYST0970 K,USE,KXL,PC,DC,FC,PY,DY,DM,DMAX,ODC,SOB,PUO0)
HYST0980 PFP=0.(DP-DPP)*KP
HYST0990 PFP=0.
HYST1000 LRULE=51
HYST1010 CALL NEWSTI(ODC,ODY,ODU,OPC,OPY,OPU,AREAL,DM,FC,PY,
HYST1020 FU,PF,DC,DY,DU,DF,HO,H,OSOC,OSCY,SOC,
HYST1030 SCY,SVU,SUP,SR,SOB,SOE,SO6,PSQ6,DPT,SDP)
HYST1040 CALL DINTER(PC,FY,FU,PF,PPP,DC,DY,DU,DF,DPP,SOC,SCY,SVU,SUP,K,
HYST1050 PP,DF,DSINC,KXL,LRULE,POSTPG,DUO,FUO)
HYST1060 DMAX=ABS(DP)
HYST1070 DSINCL=DSINC
HYST1080 RETURN
HYST1090 END IF
HYST1100 IP(KXL.EQ.10) THEN
HYST1110 IP(POSTPG) THEN
HYST1120 DDKK=DUO(1)
HYST1130 ELSE
HYST1140 DDKK=DUO(2)
HYST1150 END IF
HYST1160 IP(ABS(DP).LT.ABS(DDKK)) THEN
HYST1170 LRULE=51
HYST1180 CALL DINTER(PC,FY,FU,PF,PPP,DC,DY,DU,DF,DPP,SOC,SCY,SVU,SUP,
HYST1190 X,PP,DF,DSINC,KXL,LRULE,POSTPG,DUO,FUO)
HYST1200 END IF
HYST1210 IP(ABS(DP).GT.ABS(DDKK)) THEN
HYST1220 K=PP*0.5
HYST1230 IP(K.NE.KLAST) THEN
HYST1240 IP(POSTPG) PP=PPP*(DUO(1)-DPP)*KLAST+(DP-DUO(1))*K
HYST1250 IP(.NOT.POSTPG) PP=PPP*(DUO(2)-DPP)*KLAST+(DP-DUO(2))*K
HYST1260 END IF
HYST1270 LRULE=53
HYST1280 CALL DINTER(PC,FY,FU,PF,PPP,DC,DY,DU,DF,DPP,SOC,SCY,SVU,SUP,
HYST1290 X,PP,DF,DSINC,KXL,LRULE,POSTPG,DUO,FUO)
HYST1300 END IF
HYST1310 DSINCL=DSINC
HYST1320 DMAX=MAX(ABS(DP),DMAX)
HYST1330 IOPT2=1
HYST1340 CALL DD1SSI(IOPT2,ESE,PSE,PSEOLD,DPP,PPP,DP,AREAL,
HYST1350 K,USE,KXL,PC,DC,FC,PY,DY,DM,DMAX,ODC,SOB,PUO0)
HYST1360 RETURN
HYST1370 END IF
C
C 24 IF (KXL.EQ.9) .AND. ((DSINC<DSINCL) LT.0) THEN
IP(POSTPG) THEN
  PFFF=FUO(2)
  DDDD=DUO(2)
ELSE
  PFFF=FUO(1)
  DDDD=DUO(1)
END IF
END IF
PP1-PPPP*0.9
DP1-DDDD
KP=(PP1-0)/(DP1-DPP)
USE=MAX(K,SOB)
IP(.NOT.POSTPG) CYCLEN-CYCLEN+1
IOPT2=3
CALL DD1SSI(IOPT2,ESE,PSE,PSEOLD,DPP,PPP,DP,AREAL,
K,USE,KXL,PC,DC,FC,PY,DY,DM,DMAX,ODC,SOB,PUO0)
PFP=0.(DP-DPP)*KP
PFP=0.
LRULE=51
CALL NEWSTI(ODC,ODY,ODU,OPC,OPY,OPU,AREAL,DM,FC,PY,
FU,PF,DC,DY,DU,DF,HO,H,OSOC,OSCY,SOC,
SCY,SVU,SUP,SR,SOB,SOE,SO6,PSQ6,DPT,SDP)
CALL DINTER(PC,FY,FU,PF,PPP,DC,DY,DU,DF,DPP,SOC,SCY,SVU,SUP,K,
PP,DF,DSINC,KXL,LRULE,POSTPG,DUO,FUO)
DMAX=ABS(DP)
DSINCL=DSINC
RETURN
END IF
IP(KXL.EQ.9) THEN
IP(POSTPG) THEN
  DDKK=DUO(2)
ELSE
  DDKK=DUO(1)
END IF
IP(ABS(DP).LT.ABS(DDKK)) THEN
  LRULE=52
  CALL DINTER(PC,FY,FU,PF,PPP,DC,DY,DU,DF,DPP,SOC,SCY,SVU,SUP,
  K,PP,DF,DSINC,KXL,LRULE,POSTPG,DUO,FUO)
END IF
IP(ABS(DP).GT.ABS(DDKK)) THEN
  X=KP*0.5
  IP(K.NE.KLAST) THEN
  IP(POSTPG) PP=PPP*(DUO(2)-DPP)*KLAST+(DP-DUO(2))*K
  IP(.NOT.POSTPG) PP=PPP*(DUO(1)-DPP)*KLAST+(DP-DUO(1))*K
END IF
LRULE=53
CALL DINTER(PC,FY,FU,PF,PPP,DC,DY,DU,DF,DPP,SOC,SCY,SVU,SUP,
K,PP,DF,DSINC,KXL,LRULE,POSTPG,DUO,FUO)
END IF
DMAX=MAX(ABS(DP),DMAX)
DSINCL=DSINC
IOPT2=1
CALL DD1SSI(IOPT2,ESE,PSE,PSEOLD,DPP,PPP,DP,AREAL,
K,USE,KXL,PC,DC,FC,PY,DY,DM,DMAX,ODC,SOB,PUO0)
RETURN
HYST3410 END IF
C 25 CHECK IF PASS TURNING POINT
23 CALL DCURD(KXL,DSINC,PP,PPP,DM,FC,PY,FU,PF,X,DF,DMAX,
  DPP,DC,DY,DU,CYCLEN,AREAL,PMAX,TNPTPG,ODC,ODU,OPU,
  DP,HO,H,OSOC,POSTPG,OSCY,SOC,SCY,SVU,SUP,SR,SOB,SOE,
  SQ6,OPC,OPY,PSQ6,DPT,DPPT,KXLLST,USE,PSEOLD,UPOS,ESE,PSE,
  CSE,EKCR,IOPT2)
C
C 26 IF (X.EQ.0) .AND. (PP.EQ.0) .AND. (PP.EQ.FP) GO TO 77
C
C 27 TO CALCULATE THE STIFFNESS IN EACHLOAD STEP
CALL DNCKUL(PC,FY,FU,PF,DC,DY,DU,DF,PP,DP,DMAX,DM,KP,SOC,
  SCY,SVU,SUP,SOB,SOE,KXL,LRULE,PPP,DPP,FUO,DUO,DSINC,K,FU17,
  CSOB,AREAL,PUO0,POSTPG,PSEOLD,PSE,ESE,HO,H,OPC,OPY,OPU,ODC,
  ODY,ODU,SO6,OSOC,OSCY,SR,KEYDS2,COL,PSQ6,KLAST,DPPT,DPT,
  PPT,TNPTPG,KXLLST,LRULE,IOPT2,USE)
C
C 28 IF (X.EQ.0) RETURN
77 IP(TNPTPG) THEN
  IP(KXL.EQ.1 OR KXL.EQ.8 OR KXL.EQ.10 OR KXL.EQ.3)
  OR KXL.EQ.5 OR KXL.EQ.9 OR KXL.EQ.6) THEN
  IP(ABS(FP).LE.(0.7*PY) .AND. KXL.NE.1 .AND. KXL.NE.3) THEN
  USE=K
  IOPT2=3
  END IF
  IP(ABS(FP).GT.(0.7*PY)) IOPT2=1
  IP(ABS(FP)) IOPT2,ESE,PSE,PSEOLD,DPT,PPT,DP,PP,AREAL,
  K,USE,KXL,PC,DC,FC,PY,DY,DM,DMAX,ODC,SOB,PUO0)
  ELSE IF (KXL.EQ.2 OR KXL.EQ.4) THEN
  IOPT2=3
  CALL DD1SSI(IOPT2,ESE,PSE,PSEOLD,DPT,PPT,DP,PP,AREAL,
  K,USE,KXL,PC,DC,FC,PY,DY,DM,DMAX,ODC,SOB,PUO0)
  END IF
  TNPTPG=FALSE.

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ELSE IF((KXL.NE.5 AND .KXL.NE.6) OR
((KXL.EQ.5 OR .KXL.EQ.6) AND ((DP*PP).GT.0))) THEN
IF(ABS(PP).LE.(0.7*FY)) AND .KXL.NE.1 AND .KXL.NE.3) THEN
USE=X
IOPT2=3
END IF
IF(ABS(PP).GT.(0.7*FY)) IOPT1=1
CALL DDISS1(IOPT2,ESE,PSE,PSEOLD,DPP,PPP,DP,PP,AREAL,
K,USE,KXL,PC,DC,FY,DY,DM,DMAX,CDC,SCB,PU00)
ELSE IF((KXL.EQ.5 OR .KXL.EQ.6) AND ((DP*PP).LT.0)) THEN
USE=Y2A5
CALL DDISS1(IOPT2,ESE,PSE,PSEOLD,DPP,PPP,DP,PP,AREAL,
K,USE,KXL,PC,DC,FY,DY,DM,DMAX,CDC,SCB,PU00)
END IF
C DSINCL=DSINC
C CALC DIRCTLITY
IF(KXL.EQ.1 OR .KXL.EQ.8 OR .KXL.EQ.10) THEN
DLI=MAX(DP,DPP)
CALL DNE(1,UPOS,EXCR,ODY,DLI,ESE(2),PSE,PSEOLD,CSE)
END IF
IF(KXL.EQ.3 OR .KXL.EQ.7 OR .KXL.EQ.9) THEN
DLI=MIN(DP,DPP)
CALL DNE(1,UNEG,EXCR,-ODY,DLI,ESE(3),PSE,PSEOLD,CSE)
END IF
66 RETURN
END
C-----
C-- SUBROUTINE DNCKUL SUBRTN (FIND STIFFNESS AND FORCE IN EACH STEP)
C-----
SUBROUTINE DNCKUL(PC,FY,PU,PP,DC,DY,DU,DP,DF,PP,DP,DMAX,DM,XP,SC,
SCY,SU,SUP,SCB,SOB,KXL,LRULE,PPF,DPF,PU0,DUP,DSINC,K,PU17,
A,OSOB,AREAL,PU00,POSTPG,PSEOLD,PSE,ESE,HO,H,OPC,OPY,OPU,ODC,
A,ODY,ODU,SOB,OSOC,OSCY,SR,KEYDS2,COI,PSQ6,KLAST,DPTT,DPT,
A,PPT,TNPTPG,KXLLST,LRULST,IOPT2,USE)
REAL K,DU0(2),PU0(2),KP,DSINC,KLAST,KEYDS2
DIMENSION ESE(3)
LOGICAL TNPTPG,POSTPG
C 1. .... RULE 1.0
LOAD
IF((KXL.EQ.1) OR (KXL.EQ.3)) THEN
LRULE=10
IF(ABS(DP).LE.DC) THEN
K=SOC
USE=X
IOPT2=3
RETURN
ELSE IF((ABS(DP).GT.DC) AND (ABS(DP).LE.DY)) THEN
DDT=SIGN(DC,DP)
K=SCY
DNCK0240
DNCK0250
DNCK0260
DNCK0270
DNCK0280
DNCK0290
DNCK0300
DNCK0310
DNCK0320
DNCK0330
DNCK0340
DNCK0350
DNCK0360
DNCK0370
DNCK0380
DNCK0390
DNCK0400
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DNCK0670
DNCK0680
DNCK0690
DNCK0700
DNCK0710
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DNCK0800
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DNCK0980
DNCK0990
DNCK1000
DNCK1010
DNCK1020
DNCK1030
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DNCK1050
DNCK1060
DNCK1070
DNCK1080
DNCK1090
DNCK1100
DNCK1110
DNCK1120
DNCK1130
DNCK1140
DNCK1150
DNCK1160
DNCK1170
DNCK1180
DNCK1190
DNCK1200
IF(ABS(DP).LE.DC) THEN
K=SOC
USE=X
IOPT2=3
RETURN
ELSE IF((ABS(DP).GT.DC) AND (ABS(DP).LE.DY)) THEN
DDT=SIGN(DC,DP)
K=SCY
DNCK0240
DNCK0250
DNCK0260
DNCK0270
DNCK0280
DNCK0290
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DNCK1110
DNCK1120
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DNCK1160
DNCK1170
DNCK1180
DNCK1190
DNCK1200
IF(ABS(DP).LE.(0.7*FY)) AND (ABS(DP).LE.DY) THEN
DDT=SIGN(DY,DP)
K=SYU
IOPT2=1
ELSE IF(ABS(DP).GT.DY) AND (ABS(DP).LE.DU)) THEN
DDT=SIGN(DY,DP)
K=SYU
IOPT2=1
ELSE IF(ABS(DP).GT.DU) AND (ABS(DP).LE.DF) THEN
DDT=SIGN(DU,DP)
K=SUPT
IOPT2=1
ELSE IF(ABS(DP).GT.DF) AND (ABS(DP).LE.(20*DY)) THEN
DDT=SIGN(DP,DP)
K=(10.1*FU)/PP/((20*DY)/DP)
USE=OSOC
ELSE IF(ABS(DP).GT.(20*DY)) THEN
DDT=SIGN(20*DY,DP)
K=0.0001*SCY
USE=OSOC
END IF
IF(KLAST.EQ.K) RETURN
IF(TNPTPG) THEN
PPF=PP
PP=PPPP*(DDT-DPP)*KLAST+(DP-DDT)*K
PPT=PPPP*(DDT-DPP)*KLAST
DPT=DDT
END IF
RETURN
END IF
C 2. .... UNLOAD
IF((KXL.EQ.2) OR (KXL.EQ.4)) THEN
IOPT2=3
IF(DM.LT.ODC AND .DMAX.LT.ODC) THEN
C ..... RULE 2.1
LRULE=21
K=SOC
IF(KXLLST.EQ.5 OR .KXLLST.EQ.6 OR .KXLLST.EQ.7
OR .KXLLST.EQ.8) THEN
DPI=DDT
PFI=PP
CALL DDISS1(3,ESE,PSE,PSEOLD,DPP,PPP,DPT,0,AREAL,
K,USE,KXL,PC,DC,FY,DY,DM,DMAX,CDC,SCB,PU00)
PP=0.-K*DPTT
DPTT=0
DPP=DPT
PPP=0
DP=DDI
PP=PFI
END IF
IOPT2=3
RETURN
END IF
IF(LRULE.EQ.54 OR .LRULE.EQ.23) THEN
K=KLAST
RETURN
END IF
C ..... RULE 2.2
IF(KXLLST.EQ.5 OR .KXLLST.EQ.6 OR .KXLLST.EQ.7 OR
KXLLST.EQ.8 OR .KXLLST.EQ.9 OR .KXLLST.EQ.10) THEN
SKI=(0.-PPP)/(DPP/3)
IF((DM.GE.CDC) AND (ABS(KEYDS2).LE.DC)) THEN
DNCK0870
DNCK0880
DNCK0890
DNCK0900
DNCK0910
DNCK0920
DNCK0930
DNCK0940
DNCK0950
DNCK0960
DNCK0970
DNCK0980
DNCK0990
DNCK1000
DNCK1010
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DNCK1080
DNCK1090
DNCK1100
DNCK1110
DNCK1120
DNCK1130
DNCK1140
DNCK1150
DNCK1160
DNCK1170
DNCK1180
DNCK1190
DNCK1200
IF(KXLLST.EQ.7 OR .KXLLST.EQ.8 OR .KXLLST.EQ.9) THEN
K=MAX(KP,SKI)
C ..... RULE 2.3
LRULE=23
USE=X
PP=PP+DP-DPP)*K
IF(PP*PP.LT.0) THEN
IF(PP.GT.0) K=PC/(DC-DPT)
IF(PP.LT.0) K=PC/(DC-DPP)
PP=0.-K*DPTT
IF(KXL.EQ.2) KXL=5
IF(KXL.EQ.4) KXL=6
END IF
RETURN
END IF
IF(ABS(PPP).LE.(0.7*FY)) AND (KXLLST.EQ.7 OR
KXLLST.EQ.8 OR .KXLLST.EQ.9 OR .KXLLST.EQ.10) THEN
K=MAX(KLAST,OSOB)
C ..... RULE 2.4
LRULE=24
USE=X
PP=PP+DP-DPP)*K
IF(PP*PP.LT.0) THEN
DPT=PP/(DC-DPT)
CALL NEWSTI(ODC,ODY,ODU,OPC,OPY,OPU,AREAL,DM,PC,FY,
FU,PP,DC,DY,DU,DP,HO,H,OSOC,OSCY,SCC,SCY,
SCY,SU,SUP,SR,SCB,SOB,SOE,PSQ6,DPT,SDP)
IF(SQ6.LE.0.3 AND .SQ6.GT.0) THEN
DJP=SIGN(DC,-DPT)
FJP=SIGN(PC,DJP)
K=FJP/(DJP-DPT)
ELSE
K=SR
END IF
END IF
IF(PP*PP.LT.0) THEN
DPT=PP/(DC-DPT)
DPP=0.
PPP=0.
PP=0.-(DP-DPT)*K
RETURN
END IF
END IF
C 3. .... REVERSAL LOADING
IF((KXL.EQ.5) OR (KXL.EQ.6)) THEN
IF((SQ6.LE.COI) OR (LRULE.EQ.31)) THEN
LRULE=31
IF(DM.LT.DC AND .DMAX.LT.DC) THEN
K=KLAST
PSE=0
DPP=0
PPP=0
USE=X
DNCK1210
DNCK1220
DNCK1230
DNCK1240
DNCK1250
DNCK1260
DNCK1270
DNCK1280
DNCK1290
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DNCK1310
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DNCK2110
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DNCK2160
DNCK2170
DNCK2180
DNCK2190
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DNCK2240
DNCK2250
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DNCK2500
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DNCK2580
DNCK2590
DNCK2600
DNCK2610
DNCK2620
DNCK2630
DNCK2640
DNCK2650
DNCK2660

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IOPT2-3
IF (KCL.EQ.5) KKL-3
IF (KCL.EQ.6) KKL-1
RETURN
END IF
IF (KCL.NE.KKLLST) THEN
DJP=SIGN(DC,DPT)
FJP=SIGN(PC,DJP)
IF (DPT.NE.0) THEN
K-FJP/(DJP-DPP)
ELSE
K-FJP/(DJP-DPP)
END IF
IF (DPT.NE.0) THEN
IOPT2-3
CALL DDISSI (IOPT2,ESE,PSE,PSOLD,DPP,PPP,DPT,0,AREAL,
X,USE,KKL,PC,DC,PY,DY,DM,DMAX,ODC,SOB,PU00)
PP=0.-K*DPTT
DPTT=0
DPP=DPT
PPP=0
END IF
IF (DP*PP) GT.0) THEN
USE=(0.-PP)/(DP/3*DP)
END IF
IOPT2-3
RETURN
END IF
C ..... RULE 3.2
LRULE-3
IF (PP*PPP) LT.0) THEN
K-SR
END IF
IF (DPT.NE.0) THEN
IOPT2-3
CALL DDISSI (IOPT2,ESE,PSE,PSOLD,DPP,PPP,DPT,0,AREAL,
K,USE,KKL,PC,DC,PY,DY,DM,DMAX,ODC,SOB,PU00)
PP=0.-K*DPTT
DPTT=0
DPP=DPT
PPP=0
END IF
IF (DP*PP) GT.0) THEN
USE=(0.-PP)/(DP/3*DP)
END IF
IOPT2-3
RETURN
END IF
C4. RELOADING AFTER UNLOADING.....
IF (KCL.EQ.7) OR (KCL.EQ.8) THEN
IF (DM.LT.ODC AND DMAX.LT.ODC) THEN
K-KLAST
USE-K
IOPT2-3
IF (KCL.EQ.7) KKL-3
IF (KCL.EQ.8) KKL-1
RETURN
END IF
IF (KCL.NE.KKLLST) THEN
IF (POSTPG) THEN
FFFF=FU0(1)
GGGG=FU0(2)
DDDD=DU0(1)
EEEE=DU0(2)
ELSE
FFFF=FU0(2)
GGGG=FU0(2)
DDDD=DU0(2)
GGGG=FU0(1)
EEEE=DU0(1)
END IF
IF (DSINC GT.0) THEN
FU09=0.9*FFFF
IF (ABS(FFFF) LT.0.7*FY) DPT=DDDD-(FFFF-FU09)/OSOC
IF (ABS(FFFF) GE.0.7*FY) DPT=DDDD
END IF
IF (DSINC LT.0) THEN
FU09=0.9*GGGG
IF (ABS(GGGG) LT.0.7*FY) DPT=EEEE-(GGGG-FU09)/OSOC
IF (ABS(GGGG) GE.0.7*FY) DPT=EEEE
END IF
IF (LRULE EQ.54) THEN
KP-KLAST
ELSE
SKP=(FU09-PPP)/(DPT-DPP)
KP=MAX(SKP,KLAST)
END IF
TNPTFG=.TRUE.
END IF
IF (ABS(DP) LE ABS(DPT)) THEN
LRULE=41
K-KP
USE-K
IOPT2-3
IF (TNPTFG) THEN
PP=PPP+(DP-DPP)*K
END IF
ELSE
IF (LRULE EQ.42) GO TO 688
IF (LRULE EQ.54) THEN
KP-KLAST
END IF
IF (LRULE NE.41) THEN
X=MAX(0.5*KP,KLAST)
PP=PPP+(DP-DPP)*K
IOPT2-1
DPP=DPT
PPP=PPP
LRULE=42
TNPTFG=FALSE.
GO TO 688
END IF
C ..... RULE 4.2
LRULE=42
K=0.5*KP
IOPT2-1
IF (X.NE.KLAST) TNPTFG=.TRUE.
IF (TNPTFG) THEN
PP=PPP+(DPT-DPP)*KLAST-(DP-DPT)*K
FPT=PPP-(DPT-DPP)*KLAST
END IF
688 CALL DINTER(PC,PY,PU,FP,PPP,DC,DY,DU,DP,DPP,SOC,SCY,SYU,SUP,
K,FP,DP,DSINC,KKL,LRULE,POSTPG,DUO,PU0)
IF (DM.GE.ODC AND ABS(KRYDSD) LE DC) THEN
SKL=10.-FP/(DP/3*DP)
USE=MAX(KP,SKL)
IOPT2-3
ELSE IF (ABS(PP) LE (0.7*FY)) THEN
SKL=0.-FP/(DP/3*DP)
USE=MAX(KLAST,OSOB)
USE=MAX(USE,SKL)
IOPT2-3
ELSE IF (ABS(PP) GT.(0.7*FY)) THEN
IOPT2-1
END IF
C
RETURN
END IF
99 FORMAT(2P11,5,I6,P9,4,I3)
199 FORMAT(2P11,5,I6,I5)
299 FORMAT(5P12,4)
RETURN
END
C-----DCUR0202
DNCK2670
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DNCK3970
DNCK3980
DNCK3990
DNCK4000
DNCK4010
DNCK4020
DNCK4030
DNCK4040
DNCK4050
DNCK4060
DNCK4070
DNCK4080
DNCK4090
DNCK4100
DCUR0010
DCUR0020
C -- SUBROUTINE DCUR0 SUBRTN (TO COMPUTE CURRENT DISPL,DM,DMAX) DCUR0030
C -----DCUR0030----- DCUR0040
SUBROUTINE DCUR0(KKL,DSINC,PP,PPP,DM,PC,PY,PU,FP,K,DP,DMAX, DCUR0050
4 DPP,DC,DY,DU,CYCLN,AREAL,PMAX,TNPTFG,ODC,ODY,ODU,OPU, DCUR0060
4 DP,HO,H,OSOC,POSTPG,OSCY,SOC,SCY,SYU,SUP,SR,SOB,SOE, DCUR0070
4 SQ6,OPC,OPY,PSQ6,DPT,DPTT,KKLLST,USE,PSOLD,UPOS,ESE,PSE, DCUR0080
4 CSE,EXCR,IOPT2) DCUR0090
C
C DPP ----DISPL BEFORE DISPL INCREMENT ADDED DCUR0100
LOGICAL TNPTFG,POSTPG DCUR0110
INTEGER CYCLN DCUR0120
REAL K DCUR0130
DIMENSION ESE(3),EXCR(6),UPOS(3) DCUR0140
C
C IF ((KKLLST EQ.1 AND DSINC GT.0) OR DCUR0150
4 (KKLLST EQ.3 AND DSINC LT.0)) THEN DCUR0160
IF (ABS(DP)-DC) LT ABS(DSINC) AND ABS(DP) GT.DC) TNPTFG=.TRUE. DCUR0170
IF (ABS(DP)-DY) LT ABS(DSINC) AND ABS(DP) GT.DY) TNPTFG=.TRUE. DCUR0180
IF (ABS(DP)-DU) LT ABS(DSINC) AND ABS(DP) GT.DU) TNPTFG=.TRUE. DCUR0190
IF (ABS(DP)-DP) LT ABS(DSINC) AND ABS(DP) GT.DP) TNPTFG=.TRUE. DCUR0200
IF (ABS(DP)-(2*DY)) LT ABS(DSINC) AND ABS(DP) GT.(2*DY)) DCUR0210
4 TNPTFG=.TRUE. DCUR0220
GO TO 131 DCUR0230
IF ((KCL.EQ.5 OR KCL.EQ.6) AND ((DP*PP) GT.0)) THEN DCUR0240
IF (ABS(DP)-DC) LT ABS(DSINC)) THEN DCUR0250
IF (ABS(DP) GT DC) THEN DCUR0260
TNPTFG=.TRUE. DCUR0270
IF (KCL.EQ.5) KKL=3 DCUR0280
IF (KCL.EQ.6) KKL=1 DCUR0290
END IF DCUR0300
GO TO 131 DCUR0310
END IF DCUR0320
131 DMAX=MAX(ABS(DP),DMAX) DCUR0330
C
IF ((PP.EQ.0) OR ((PP*PPP) LT.0)) THEN DCUR0340
DM=DMAX(DM,DMAX) DCUR0350
C
C DPTT,DPT (0,DPT) (0,0) DCUR0360
C -----DCUR0360----- DCUR0370
C .....DPTT:C DCUR0380
C .....DSINC:C DCUR0390
C
IF (PP*PPP) LT.0) THEN DCUR0400
DPT=DPP-PPP/K DCUR0410
DPTT=DP-DPT DCUR0420
CALL DNE(0,UPOS,EXCR,ODY,DP,ESE,PSE,PSOLD,CSE) DCUR0430
IF X-SUP AND REACH FORCE=0 THEN SET FORCE=0 AND DP=DPTT DCUR0440
C
C AFTER THAT K=0 DCUR0450
IF (K EQ.(-0.0001*SCY)) THEN DCUR0460
DP=DP DCUR0470
PP=0 DCUR0480
K=0 DCUR0490
STOP DCUR0500
END IF DCUR0510
IF (DM.LT.ODC AND DMAX.LT.ODC) THEN DCUR0520
IOPT2-3 DCUR0530
ELSE DCUR0540
IOPT2-2 DCUR0550
END IF DCUR0560
C
CALL DDISSI (IOPT2,ESE,PSE,PSOLD,DPP,PPP,DPT,0,AREAL, DCUR0570
K,USE,KKL,PC,DC,PY,DY,DM,DMAX,ODC,SOB,PU00) DCUR0580
IF ((POSTPG) AND (PPP LT.0)) THEN DCUR0590
CYCLN=CYCLN+1 DCUR0600
PMAX=0 DCUR0610
DMAX=ABS(DP) DCUR0620
END IF DCUR0630
IF (.NOT. POSTPG AND PPP GT.0) THEN DCUR0640
CYCLN=CYCLN+1 DCUR0650
PMAX=0 DCUR0660
DMAX=ABS(DP) DCUR0670
END IF DCUR0680
IF (.NOT. POSTPG AND PPP GT.0) THEN DCUR0690
CYCLN=CYCLN+1 DCUR0700
PMAX=0 DCUR0710
DMAX=ABS(DP) DCUR0720
END IF DCUR0730
IF (DM GT.ODC) THEN DCUR0740
CALL NEWSTI (ODC,ODY,ODU,OPC,OPY,OPU,AREAL,DM,PC,PY, DCUR0750
FU,PP,DC,DY,DU,DP,HO,H,OSOC,OSCY,SCY, DCUR0760
SCY,SYU,SUP,SR,SOB,SOE,SQ6,PSQ6,DPT,SDP) DCUR0770
END IF DCUR0780
IF (KCL.EQ.2) KKL=5 DCUR0790
IF (KCL.EQ.4) KKL=6 DCUR0800
END IF DCUR0810
IF ((KCL.EQ.7) AND (KKLLST EQ.4)) OR DCUR0820
4 ((KCL.EQ.8) AND (KKLLST EQ.2)) THEN DCUR0830
CYCLN=CYCLN+1 DCUR0840
DMAX=DM,DMAX) DCUR0850
END IF DCUR0860
RETURN DCUR0870
C-----DCUR0870----- DCUR0880
C -- NEWSTI SUBRTN (FIND STIFFNESS WHEN PASSING HORIZ. AXIS (FORCE=0) NEWSTI020
C -----NEWSTI020----- NEWSTI030
SUBROUTINE NEWSTI (ODC,ODY,ODU,OPC,OPY,OPU,AREAL,DM,PC,PY, NEWSTI040
FU,PP,DC,DY,DU,DP,HO,H,OSOC,OSCY,SCY, NEWSTI050
SCY,SYU,SUP,SR,SOB,SOE,SQ6,PSQ6,DPT,SDP) NEWSTI060
C
SQ6=AREAL/(OPU*ODU) NEWSTI070
TOL=1E-10 NEWSTI080
IF (ABS(DM) LT.ODC AND SQ6 LT.TOL) SQ6=0. NEWSTI090
PSPD=SDP NEWSTI100
SDP=DPT NEWSTI110
ALPHA=HO/H NEWSTI120
IF ((SQ6 GT.0.) AND (SQ6 LE.0.3)) THEN NEWSTI130
DC=DC NEWSTI140
PC=PC NEWSTI150
ELSE NEWSTI160
B=-0.25473*SQ6 NEWSTI170
SR=(0.27988*10**B)*OSOC NEWSTI180
DC=ABS(SDP)*0.25 NEWSTI190
DC=MAX(DC,DC) NEWSTI200
IF (DC GT.(1.5*ODC)) THEN NEWSTI210
DC=1.5*ODC NEWSTI220
END IF NEWSTI230
IF (ABS(SDP) GT.DU) THEN NEWSTI240
PC=0.98*PC NEWSTI250
SR=PC/(ABS(SDP)*DC) NEWSTI260
END IF NEWSTI270
C
PC=SB*(ABS(SDP)-DC) NEWSTI280
END IF NEWSTI290
IF (SQ6 NE.0.) THEN NEWSTI300
B=0.20102*SQ6 NEWSTI310
BB=1.578*10**B NEWSTI320
IF (BB GT.1.) BB=1 NEWSTI330
SCY=BB*OSCY NEWSTI340
END IF NEWSTI350
COEPI=(OPY-OPC)/(ODY-ODC) NEWSTI360
COEFP=(OPU-OPY)/(ODU-ODY) NEWSTI370
SYU=(COEPI/COEFP)*SCY NEWSTI380
IF (SQ6 LE.0) THEN NEWSTI390
DY=DY NEWSTI400
ELSE IF ((SQ6 GT.0.) AND (SQ6 LE.0.312)) THEN NEWSTI410
DY=ODY NEWSTI420
ELSE NEWSTI430
DY=(1.5299+0.4546*LOG(SQ6))*ODY NEWSTI440
END IF NEWSTI450
IF (SQ6 EQ.0.) THEN NEWSTI460
DU=DU NEWSTI470
ELSE NEWSTI480
DU=DY*(ODU/ODY) NEWSTI490
DU=MAX(DU,1.5*DY) NEWSTI500
END IF NEWSTI510
PY=PC*(DY-DC)*SCY NEWSTI520
PU=PI*(DU-DY)*SYU NEWSTI530
SUP=-0.5*SCY NEWSTI540

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PP=(PC*PY)/2
DP=DU*(PP*PU)/SUP
SOC=PC/DC
SOY=PY/DY
SCU=(PU*PC)/(DU*DC)
KA=PY/SOC
KB=(KA*DY)/2
SCB=PY/XB
XD=PU/SOY
KE=(KD*DU)/2
SCB=PU/KE
PQ6=SQ6
RETURN
END
-----
C --- SUBROUTINE DDISS1 SUBRTN (AREA1-ENERGY DISSIPATION)
C --- CALCULATE THE ELASTIC STRAIN ENERGY (ESE(1)) AND
C --- PLASTIC STRAIN ENERGY (PSE)
-----
SUBROUTINE DDISS1(IOPTR, ESE, PSE, PSEOLD, DPP, FPP, DP, PP, AREAL,
& K, USE, KKL, PC, DC, PY, DY, DM, DMAX, CDC, SCB, PUC0)
REAL X
DIMENSION ESE(3)
C AREA1 REPRESENTING AREA ENCLOSED BY THE HYSTERESIS RESPONSE
C USE, ESE(1), ESE(2), ESE(3), PSE, PSEOLD SEE STRENG SUBRTN
C USE - EQUIVALENT UNLOADING STIFFNESS
C --- CALCULATE THE EQUIVALENT UNLOADING STIFFNESS
IF(IOPTR.EQ.1 AND K.NE.0.) THEN
IF((ABS(PUC0).GT.0.*PY) OR (KXL.EQ.7 OR KXL.EQ.8 OR
& KXL.EQ.9 OR KXL.EQ.10)) THEN
DDPP=SIGN(DC, DP)
PPPP=SIGN(PC, DPPP)
SK1=(PPPP*PUC0*.85)/(DDPP*DP)
SK1=MAX(SK1, SCB)
DP1=DP*(PP/6*PP*.85)/SK1
PVP=SIGN(PY, DP1)
DVP=SIGN(DY, PVP)
SK2=(PVP*PP/6)/(DVP*DP1)
DP2=DP1*(0.-PP/6)/SK2
AA=0.5*(0.85*PP*PP/6)+(DP*DP1)+0.5*PP/6*(DP1*DP2)
USE=(PP*2)/(2*ABS(AA))
ELSE IF(ABS(PP).LE.0.*PY) THEN
SK1 = PC/DC
PP1=SIGN(PC/3, PP)
DP1=DP*(PP1*PP)/SK1
DCP = SIGN(DC, DP)
PCP = SIGN(PC, DCP)
SK2 = (PCP*PP1)/(DCP*DP1)
DP2 = DP1*(0.-PP1)/SK2
AA=0.5*(PP1*PP)+(DP*DP1)+0.5*PP1*(DP1*DP2)
USE = (PP*2)/(2*ABS(AA))
END IF
IF(IOPTR.EQ.2 AND K.NE.0.) THEN
IF(DM.LE.ODC AND DMAX.LE.CDC AND (KXL.EQ.1 OR
& KXL.EQ.3 OR KXL.EQ.5 OR KXL.EQ.6)) THEN
USE=K
ELSE IF((DP*PP).GT.0.) THEN
USE=(0.-PP)/(DP/3*DP)
END IF
CALL STRENG(ESE, PSE, PSEOLD, PP, FPP, DP, DPP, USE)
AREA1=ESE(1)*PSE
RETURN
END
-----
C --- SUBROUTINE DINTER SUBRTN (CHECK IF NEW CURRENT TRACK INTERSECT
C --- IF YES, FIND CONNECT PT. (UPDATED CURRENT PT.)
-----
SUBROUTINE DINTER(PC, PY, PU, PP, FPP, DC, DY, DU, DP, DPP, SOC, SCY, SYU,
& SUP, X, FP, DP, DSINC, KXL, LRULE, POSTFG, DUC, PUC)
REAL X, KJ, DUC(4), PUC(4)
LOCAL POSTFG
C
IF(ABS(DP).LT.DC) THEN
KJ=PP/DY
IF(KJ.LT.SOC) RETURN
IF(KJ.EQ.SOC) THEN
IF(DSINC.GT.0) KXL=1
IF(DSINC.LT.0) KXL=3
RETURN
END IF
IF(KJ.GT.SOC AND KJ.GT.SOC) THEN
IF(LRULE.EQ.51 OR LRULE.EQ.52) THEN
X=(K*DPP*PP)/(K*SOC)
Y=X*SOC
ADP=SIGN(X, DPP)
APP=SIGN(Y, DPP)
PP=APP*SOC*(DP*ADP)
IF(DSINC.GT.0) KXL=1
IF(DSINC.LT.0) KXL=3
K=SOC
END IF
C --- FOR RELOAD DURING REVERSAL STAGE
IF(LRULE.EQ.41 OR LRULE.EQ.42) THEN
IF(DP.GT.0) THEN
IF(POSTFG) THEN
PPPP=PU(1)
GGGG=DU(1)
ELLS=DU(1)
PPPP=PU(2)
GGGG=DU(2)
END IF
ELSE
IF(POSTFG) THEN
PPPP=PU(2)
GGGG=DU(2)
ELLS=DU(2)
PPPP=PU(1)
GGGG=DU(1)
END IF
END IF
IF(POSTFG) THEN
PPPP=PU(2)
GGGG=DU(2)
ELLS=DU(2)
PPPP=PU(1)
GGGG=DU(1)
END IF
END IF
IF(ABS(DP).GT.ABS(GGGG) AND (DP*GGGG).GT.0) THEN
Y=PPPP*(GGGG*DP)/K
SK=(ABS(PC)*ABS(Y))/(ABS(DC)*ABS(GGGG))
PP=Y*(DP*GGGG)*SK
IF(DP.GT.0) KXL=5
IF(DP.LT.0) KXL=5
K=SK
RETURN
END IF
END IF
END IF
IF(ABS(DP).GE.DC AND (ABS(DP).LT.DY)) THEN
IF(DP.GT.0) THEN
IF(POSTFG) THEN
PPPP=PU(1)
GGGG=DU(1)
ELLS=DU(1)
PPPP=PU(2)
GGGG=DU(2)
END IF
ELSE
IF(POSTFG) THEN
PPPP=PU(2)
GGGG=DU(2)
ELLS=DU(2)
PPPP=PU(1)
GGGG=DU(1)
END IF
END IF
IF(DP.GT.0) THEN
PP=APP*SU*(DU*ADP)+SUP*(DP*DU)+SPG*(DP*DP)
IF(DSINC.GT.0) KXL=1
IF(DSINC.LT.0) KXL=3
K=SPG
RETURN
END IF
ADP=SIGN(X, DPP)
APP=SIGN(Y, DPP)
IF(DP.GT.0) PP=APP*SU*(DU*ADP)+SUP*(DP*DU)+SPG*(DP*DP)
IF(DP.LT.0) PP=APP*SU*(DU*ADP)+SUP*(DP*DU)+SPG*(DP*DP)
IF(DSINC.GT.0) KXL=1
IF(DSINC.LT.0) KXL=3
K=SPG
RETURN
END IF
IF(ABS(X).LT.DU) THEN
X=(PY*DY*SU)*ABS(DPP)*ABS(PP)/(X*SU)
Y=ABS(PP)*K*ABS(X)*K*ABS(DPP)
IF(ABS(X).LT.DU) THEN
X=(PY*DY*SU)*ABS(DPP)*ABS(PP)/(X*SU)
Y=ABS(PP)*K*ABS(X)*K*ABS(DPP)
ADP=SIGN(X, DPP)
APP=SIGN(Y, DPP)
IF(DP.GT.0) PP=APP*SU*(DU*ADP)+SUP*(DP*DU)+SPG*(DP*DP)
IF(DP.LT.0) PP=APP*SU*(DU*ADP)+SUP*(DP*DU)+SPG*(DP*DP)
IF(DSINC.GT.0) KXL=1
IF(DSINC.LT.0) KXL=3
K=SPG
RETURN
END IF
IF(ABS(X).LT.DP AND ABS(X).GT.DU) THEN
X=(PP*DP*SU)*ABS(DPP)*ABS(PP)/(K*SU)

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Y=ABS(FPP)*K*ABS(X)-K*ABS(DPP)
ADP=SIGN(X,DPP)
APP=SIGN(Y,DPP)
IF(DP.GT.0) PP=APP*SGH*(DP-ADP)+SPG*(DP-DP)
IF(DP.LT.0) PP=APP*SGH*(-DP-ADP)+SPG*(DP-DP)
IF(DSINC.GT.0) KKL=1
IF(DSINC.LT.0) KKL=3
K=SPG
RETURN
END IF
ADP=SIGN(X,DPP)
APP=SIGN(Y,DPP)
PP=APP*SGH*(DP-ADP)
IF(DSINC.GT.0) KKL=1
IF(DSINC.LT.0) KKL=3
K=SPG
END IF
RETURN
END IF
DH=20*DY-ID.0.1*PU)/(1-0.001*SCY)
SGH=0.001*SCY
SPG=(0.1*PU-PP)/(20*DY-DP)
IF(ABS(DP).GE.(20*DY) .AND. ABS(DP).LT.DH) THEN
  IF(ABS(DPP).EQ.(20*DY) .AND. ABS(FP).LT.(0.1*PU)) RETURN
  KU=(ABS(FP)/(0.1*PU))/(ABS(DP)-(20*DY))
  IF(KU.LT.0 .AND. KU.LT.SGH) RETURN
  IF(KU.EQ.SGH) THEN
    IF(DSINC.GT.0) KKL=1
    IF(DSINC.LT.0) KKL=3
    RETURN
  END IF
  IF(KU.GT.SGH) THEN
    X=(0.1*PU-20*DY*SGH+K*ABS(DPP)-ABS(FPP))/(X-SGH)
    Y=ABS(FPP)+K*ABS(X)-K*ABS(DPP)
    IF(ABS(X).LT.DP) THEN
      CC ... TRY WHETHER IT IS LOCATED IN BTN P AND G ...
      X=(0.1*PU-DG*SGH+K*ABS(DPP)-ABS(FPP))/(K-SPG)
      Y=ABS(FPP)+K*ABS(X)-K*ABS(DPP)
      IF(ABS(X).LT.DP) THEN
        X=(PU-DU*SGH+K*ABS(DPP)-ABS(FPP))/(X-SUP)
        Y=ABS(FPP)+K*ABS(X)-K*ABS(DPP)
        ADP=SIGN(X,DPP)
        APP=SIGN(Y,DPP)
        IF(DP.GT.0) PP=APP*SGH*(DP-ADP)+SPG*(DP-DP)+SGH*(DP-DG)
        IF(DP.LT.0) PP=APP*SGH*(-DP-ADP)+SPG*(DP-DP)+SGH*(DP-DG)
        IF(DSINC.GT.0) KKL=1
        IF(DSINC.LT.0) KKL=3
        K=SGH
        RETURN
      END IF
      IF(ABS(X).LT.(20*DY)) THEN
        X=(PP-DP*SGH+K*ABS(DPP)-ABS(FPP))/(K-SPG)
        Y=ABS(FPP)+K*ABS(X)-K*ABS(DPP)
        ADP=SIGN(X,DPP)
        APP=SIGN(Y,DPP)
        PP=APP*SGH*(DP-ADP)
        IF(DSINC.GT.0) KKL=1
        IF(DSINC.LT.0) KKL=3
        K=SGH
        RETURN
      END IF
      IF(ABS(DP).GE.DH) THEN
        X=0.
        IF(DSINC.GT.0) KKL=1
        IF(DSINC.LT.0) KKL=3
        RETURN
      END IF
      RETURN
    END
  END IF
  IF(ABS(DP).GE.DH) THEN
    X=0.
    IF(DSINC.GT.0) KKL=1
    IF(DSINC.LT.0) KKL=3
    RETURN
  END IF
  RETURN
END IF
END IF
IF(ABS(DP).GE.DH) THEN
  X=0.
  IF(DSINC.GT.0) KKL=1
  IF(DSINC.LT.0) KKL=3
  RETURN
END IF
RETURN
END IF
***** HYST12 *****
THIS SUBROUTINE IS USED FOR NONLINEAR BEAM-COLUMN ANALYSIS,
WHICH CAN ANALYZE THE POSTBUCKLING, UNLOADING, AND RELOADING
HYSTERETIC LOOPS.
-----
SUBROUTINE HYST12(IOPT,NSEG,VG,EM,LIBN,HH,UU,
& F1,IRV1,IRV2,IRV3,IRV4,IECOP,EC20,EC30,EC40,NUMP,ELS,
& SMALL,RATIO,RATIOV,TOTA,IMD,ISPE,DELTP2,S2,
& SP,ISTART,IR,G,ISTIP,IAUTO,IMATER,RATIOV,
& QRNZE,RENZE
& NP,IPM,BETA,ECX,ECY,U,V,UO,VO,GCD,
& EL,DI,DD,DOO,TL,XLOC,FLOC,PGLOB,XGLO,
& ED,EL,E2,TE,FR,TIR,SEASAT,STP,MDIAG,
& SEASAT,FRUT,DP,DDOXY,PEK,ECCK,ECY,
& FFI,FACTOR,DPCOPY,ASAT,S,DE,
& AO,B,STR,DEP,ROP,STRP,TEP,REP,SREF,FUL,FLP)
IOPT = 1 : FORMULATE THE INITIAL MEMBER STIFFNESS
IOPT = 2 : FORMULATE THE NONLINEAR MEMBER STIFFNESS
RATIO = INITIAL IMPERFECTION OF THE MEMBER IN X DIRECTION
RATIOV = INITIAL IMPERFECTION OF THE MEMBER IN Y DIRECTION
IECOP = ECCENTRIC LOAD OPTION
WW = ECCENTRIC DISTANCE IN X DIRECTION
ZZ = ECCENTRIC DISTANCE IN Y DIRECTION
NMP = TOTAL SECTIONAL POINT ON THE SEGMENT CROSS-SECTION
ASAT = MEMBER GLOBAL STIFFNESS MATRIX
SEASAT = CONDENSED MATRIX
CONMT = RESIDUAL MATRIX
DD = MEMBER'S DISPLACEMENT INCREMENT AT EACH D.O.F.
NSEG = TOTAL NO OF SEGMENTS IN THE MEMBER
IDOP = TOTAL D.O.F. OF THE MEMBER
ELS = TOTAL LENGTH OF THE MEMBER
LIBN = THE CROSS-SECTION LIBRARY NO
FLP = FLAG FOR JUDGING LOCAL BUCKLING
0 = NO LOCAL BUCKLING
1 = LOCAL BUCKLING OCCURRED
NOMEN = SYSTEM MEMBER NO
RNZE = INDEX FOR LIMITING NORM OF DISPLACEMENT
0 = BEE LT. QRNZE
1 = BEE GT. QRNZE
ECX = DISTANCE FROM REFERENCE COORDINATE (U,V,W) ORIGIN TO
ECCENTRIC LOAD LOCATION IN X DIRECTION
ECY = DISTANCE FROM REFERENCE COORDINATE (U,V,W) ORIGIN TO
ECCENTRIC LOAD LOCATION IN Y DIRECTION
REAL INEB,INEM
DIMENSION BETA(NSEG),ECX(2),ECY(2),IPM(IDOP),MDIAG(IDOP*1)
DIMENSION U(NSEG,NUMP),V(NSEG,NUMP),UO(NUMP),VO(NUMP)
DIMENSION GCD(NSEG,3),D1(IDOP),D2(IDOP),ASAT(IDOP,IDOP)
DIMENSION DDO(IDOP),EL(NSEG),TL(NSEG),XLOC(NSEG,12),DP(IDOP)
DIMENSION FLOC(NSEG,12),PGLOB(NSEG,12),XGLO(NSEG,12)
DIMENSION E1(NSEG,NUMP),E2(NSEG,NUMP),TE(NUMP),FR(NSEG,9)
DIMENSION E(NSEG,9),DE(NSEG,NUMP),SEASAT(IDOP,IDOP)
DIMENSION TIR(NSEG,9),SK(12,12),SREF(NSEG,NUMP)
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DINT2260 DIMENSION DPCOPY(IDOP),F1(NUMP),E9(NSEG,NUMP),NP(NSEG,12)
DINT2270 DIMENSION FACTOR(IDOP),SEASAT(12,12),STP(JSTR)
DINT2280 DIMENSION DDOXY(IDOP,IDOP),SREF(NSEG,NUMP),STR(NSEG,NUMP)
DINT2290 DIMENSION R(12,12),PSK(NSEG,12,12),TEP(NSEG,NUMP)
DINT2300 DIMENSION ECX(2),ECY(2),AO(NUMP),B(NUMP),FFI(IDOP)
DINT2310 DIMENSION DEP(NSEG,NUMP),EOP(NSEG,NUMP),STRP(NSEG,NUMP)
C
C DEFINE PARAMETERS
DINT2320 FLP=0
DINT2330 BY=YS/EM
DINT2340
DINT2350
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DO 329 J=1,12
XLOC(I,J)=0.
FLOC(I,J)=0.
XGLOB(I,J)=0.
FGLOB(I,J)=0.
329 CONTINUE
C INITIALIZE THE SECTIONAL STRAIN AND BETA VALUES
DO 125 I=1,NSEG
BETA(I)=0
DO 125 J=1,NMPP
E0(I,J)=0
E1(I,J)=EY
E2(I,J)=EY
TE(J)=EM
125 CONTINUE

```

```

DO 112 I=1,NSEG
C FINE THE ROTATION MATRIX DUE TO INITIAL IMPERFECTION EFFECT
PR(I,1)=1
PR(I,2)=0
PR(I,3)=0
PR(I,4)=0
PR(I,5)=1
PR(I,6)=0
PR(I,7)=0
PR(I,8)=0
PR(I,9)=1

```

```

130 K00=0
KA=D1(NP(I,1))*GCD(I,1,1)+D00(NP(I,1))
VA=D1(NP(I,2))*GCD(I,1,2)+D00(NP(I,2))
ZA=D1(NP(I,3))*GCD(I,1,3)+D00(NP(I,3))
ROZA=D1(NP(I,6))*D00(NP(I,6))
XB=D1(NP(I,7))*GCD(I,2,1)+D00(NP(I,7))
YB=D1(NP(I,8))*GCD(I,2,2)+D00(NP(I,8))
ZB=D1(NP(I,9))*GCD(I,2,3)+D00(NP(I,9))
ROZB=D1(NP(I,12))*D00(NP(I,12))
DAA=DD(NP(I,1))
DVA=DD(NP(I,2))
DZA=DD(NP(I,3))
DROKA=DD(NP(I,4))
DROVA=DD(NP(I,5))
DROZA=DD(NP(I,6))
DXB=DD(NP(I,7))
DYB=DD(NP(I,8))
DZB=DD(NP(I,9))
DROXB=DD(NP(I,10))
DROYB=DD(NP(I,11))
DROZB=DD(NP(I,12))
IF (K00 EQ 1) GOTO 135

```

```

C VARIABLES FOR THE ROTATIONAL MATRIX DUE TO INITIAL IMPERFECTION
VX0=PR(I,4)
VY0=PR(I,5)
VZ0=PR(I,6)
WX0=PR(I,7)
WY0=PR(I,8)
WZ0=PR(I,9)

```

```

C CALCULATE ROTATIONAL MATRIX OF SEGMENT
CALL ROMA(XA, YA, ZA, ROZA, XB, YB, ZB, ROZB, VX0, VY0, VZ0, WX0,
WY0, WZ0, I, R, PR, BETA, TL, NSEG)

```

```

110 L0=0
DO 110 J=1,3
DO 110 K=1,3
L0=L0+1
TIR(I,L0)=R(I,J,K)

```

```

C GENERATE THE INITIAL ROTATIONAL MATRIX OR PREVIOUS ROTATIONAL
C MATRIX

```

```

C FIND SEGMENTS' PRINCIPAL AXIS AND MATERIAL PROPERTIES
C ( SUBROUTINE STRATE AND JUDEL ARE CALLED BY PRINC )
CALL PRINC(0,0,0,0,0,0,1,ST,TOTA,LISN,EM,EY,G,YS,IOPF,NMPP,NSEG,
& EA,EI0,EIV,GKT,IREV1,IREV2,IREV3,IREV4,UCO,VCO,IMATER
& A0,B,BETA,EO,E1,E2,U,V,UO,VO,WE,E,DE,INEB,INEM,PJL
& RATI03,IR,STR,DEP,EOP,STRP,TEP,EREP,SREP,PLP)

```

```

C CALCULATE ECCENTRICITIES OF TWO END SEGMENTS
IF (I.EQ 1.OR.I.EQ.NSEG.AND.I.EQ.OC.EQ.1) THEN
CALL ECCENT(NSEG,UCO,VCO,I,0,0,0,ECN,ECY,ECCX,ECCY)
ENDIF

```

```

C CALCULATE INITIAL ROTATION MATRIX OF SEGMENT
K00=1
GOTO 130

```

```

135 VX0=TIR(I,4)
VY0=TIR(I,5)
VZ0=TIR(I,6)
WX0=TIR(I,7)
WY0=TIR(I,8)
WZ0=TIR(I,9)

```

```

C CALCULATE ROTATIONAL MATRIX OF SEGMENT
CALL ROMA(XA, YA, ZA, ROZA, XB, YB, ZB, ROZB, VX0, VY0, VZ0, WX0,
WY0, WZ0, I, R, PR, BETA, TL, NSEG)
DEC=0
DCU=0
DCV=0

```

```

C FORMULATE THE SEGMENT'S LOCAL STIFFNESS MATRIX
CALL ELESTI(NSEG, I, ISTIP, EA, EI0, EIV, GKT, EL, FLOC, PSK, SK)

```

```

C TRANSFER SEGMENT'S LOCAL STIFFNESS TO GLOBAL STIFFNESS
CALL MULTM(0,12,DUMY,SK,R)
CALL MULTM(1,12,SK,R,DUMY)

```

```

C ASSEMBLING SEGMENT GLOBAL STIFFNESS TO MEMBER
C CENTROID STIFFNESS MATRIX
DO 85 J=1,12
DO 85 JU=1,12
ASAT(NP(I,J),NP(I,JU))-ASAT(NP(I,J),NP(I,JU))+SK(J,JU)
85 CONTINUE

```

```

112 CONTINUE
GOTO 555

```

```

300 CONTINUE
IF (ISTART LT 5) THEN
ISTART=ISTART+1
ELSE
ISTART=5
ENDIF

```

```

426 DO 426 I=1,IDOP
FACTOR(I)=0

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C DISPLACEMENT INCREMENT TRANSFORMATION DUE TO ECCENTRIC LOADS
IP( I.EQ.OC.EQ.1 ) THEN
DO 913 I=1,12
DP(I)=DD(I)
913 CONTINUE
DO 525 I=1,12
DD(I)=0
DO 525 KK=1,12
DD(I)=DD(I)+TPJT(I,KK)*DP(KK)
525 CONTINUE
ENDIF

```

```

C CONVERT MEMBER END DISPLACEMENT INCREMENT TO THE CORRESPONDING
C MEMBER'S DOP DIRECTION
DO 837 I=1,12
DD(IDOP-12-I)=DD(I)
837 CONTINUE
C BACK SUBSTITUTE TO GET MEMBER'S INTERNAL DOP DISPLACEMENT INCREMENT
L1=IDOP-12-1
L2=IDOP
DO 838 I=L1,L2
FACTOR(I)=DD(I)
IMD=MDIAG(IDOP-1)-1
CALL GAUSS2(4, IDOP, L1, L2, MDIAG, STP, DPCOPY, FACTOR, JSTOR)
DO 901 I=1, L1-1
DPCOPY(I)=0
DO 902 I=L1, L2
DP(I)=DPCOPY(I)
DO 848 I=IDOP-13+1, IDOP
FACTOR(I)=DD(I)

```

```

-----
--- BACK SUBSTITUTE BEGIN ---
IP ( IDOP NE 12 ) THEN
DO 148 I=12
CALL GAUSS2(5, IDOP, 1, L2, MDIAG, STP, DPCOPY, FACTOR, JSTOR)
ENDIF

```

```

C ADD GLOBAL DISPLACEMENT RATE TO TOTAL DISPLACEMENT
DO 903 I=1, IDOP
D(I)=FACTOR(I)
903 D(I)=D(I)+DD(I)
DO 219 I=1, NSEG
DO 219 J=1, 12
XGLO(I,J)=DD(NP(I,J))
219 CONTINUE

```

```

C FIND EACH SEGMENT INTERNAL FORCE IN GLOBAL DIRECTIONS
C ADJUST THE INITIAL IMPERFECTION DEFLECTION FOR NEW CYCLE, ISPE=1
IP( ISPE EQ 1 ) THEN
PI=3.1415926
DO 210 I=1, IDOP
DO(I)=0
210 CONTINUE
ELS=0
DO 215 I=1, NSEG
SZA=D1(NP(I,2))*GCD(I,1,3)
SZA=D1(NP(I,9))*GCD(I,2,3)
EL(I)=SZA-SZA
ELS=ELS+EL(I)
215 CONTINUE
ASSUME INITIAL IMPERFECTION RATIO BASED ON MEMBER MIDDLE POINT
DISPLACEMENT
IDUM=(NSEG/2)-1+5
RATIOX=D1(IDUM)/ELS
RATIOY=D1(IDUM+1)/ELS
DO 215 I=1, NSEG
D00(NP(I,1))=(RATIOX*ELS)*SIN(PI*
& (D1(NP(I,3))*GCD(I,1,3)-D1(NP(I,3)))/ELS)
& D00(NP(I,7))=(RATIOX*ELS)*SIN(PI*
& (D1(NP(I,9))*GCD(I,2,3)-D1(NP(I,3)))/ELS)
& D00(NP(I,2))=(RATIOY*ELS)*SIN(PI*
& (D1(NP(I,3))*GCD(I,1,3)-D1(NP(I,3)))/ELS)
& D00(NP(I,8))=(RATIOY*ELS)*SIN(PI*
& (D1(NP(I,9))*GCD(I,2,3)-D1(NP(I,3)))/ELS)
215 CONTINUE

```

```

C GENERATE THE SEGMENT LENGTH (INCLUDING THE INITIAL IMPERFECTION)
DO 922 I=1, NSEG
EL(I)=SQRT((D1(NP(I,3))*GCD(I,1,3)-D00(NP(I,2)))**2
& +(D00(NP(I,7))-D00(NP(I,1)))**2)
TL(I)=EL(I)
922 CONTINUE
WRITE(108,915)
WRITE(108,916)
WRITE(108,917) (I, EL(I), I=1, NSEG)
915 FORMAT(/I,X,'THE SEGMENT LENGTH:')
916 FORMAT(I,X,'SEGMENT LENGTH')
917 FORMAT(5,F15.6)
ENDIF

```

```

-----
C FIND EACH SEGMENT INTERNAL FORCE IN GLOBAL DIRECTIONS
DO 90 I=1, NSEG
CALL CLDAP(R, PR, I, NSEG, XGLO, PSK, SK, FGLOB)
C FIND SEGMENT DEFORMATION AND INCREMENTS IN MEMBER COORDINATES
C DIRECTIONS
IP ( ISPE EQ 0 OR ISPE EQ 1 ) THEN
IP ( ISPE EQ 0 ) GOTO 330
IP ( ISPE EQ 1 ) GOTO 335
D1(NP(I,J))=0
335 CONTINUE
330 KA=D1(NP(I,1))*GCD(I,1,1)+D00(NP(I,1))
XA=D1(NP(I,2))*GCD(I,1,2)+D00(NP(I,2))
ZA=D1(NP(I,3))*GCD(I,1,3)+D00(NP(I,3))
ROZA=D1(NP(I,6))*D00(NP(I,6))
XB=D1(NP(I,7))*GCD(I,2,1)+D00(NP(I,7))
YB=D1(NP(I,8))*GCD(I,2,2)+D00(NP(I,8))
ZB=D1(NP(I,9))*GCD(I,2,3)+D00(NP(I,9))
ROZB=D1(NP(I,12))*D00(NP(I,12))
DAA=DD(NP(I,1))
DVA=DD(NP(I,2))
DZA=DD(NP(I,3))
DROKA=DD(NP(I,4))
DROVA=DD(NP(I,5))
DROZA=DD(NP(I,6))
DXB=DD(NP(I,7))
DYB=DD(NP(I,8))
DZB=DD(NP(I,9))
DROXB=DD(NP(I,10))
DROYB=DD(NP(I,11))
DROZB=DD(NP(I,12))
ENDIF

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C
C VARIABLES FOR THE INITIAL ROTATIONAL MATRIX
VX0-TIR(1,4)
VY0-TIR(1,5)
VZ0-TIR(1,6)
WX0-TIR(1,7)
WY0-TIR(1,8)
WZ0-TIR(1,9)
C
C CALCULATE SEGMENT CURVATURE RATE IN PRINCIPAL AXES DIRECTIONS
CALL CURVA(NSEG,I,DBC,DCU,DCV,R,EL,FR,DNA,DYA,DZA,DROXA,DROYA,
DROZA,DXB,DYB,DEB,DROXB,DROYB,DROZB)
C
C CALCULATE EACH SEGMENT'S PRINCIPAL AXIS AND MATERIAL PROPERTIES
CALL PRINC(DBC,DCU,DCV,I,ST,TOTA,LIBM,EM,EY,G,YB,IOP,T,NUM,NSEG,
EA,EIU,EIV,GKT,IREV1,IREV2,IREV3,IREV4,UCO,VCO,IMATER,
AO,B,BETA,BO,EL,E2,U,V,UO,V0,TE,E,DE,INEB,INEM,FJL,
RATIO3,IR,STR,DEP,ROP,STRP,TEP,REP,SREP,FLP)
C
C CALCULATE ECCENTRICITIES OF TWO END SEGMENTS
IF (I.EQ.1.OR.I.EQ.NSEG.AND.I.EQOP.EQ.1) THEN
CALL ECCENT(NSEG,UCO,VCO,I,ROZA,ROZB,ECK,EYX,ECCY,ECCZ)
ENDIF
C
C CALCULATE SEGMENT CURRENT ROTATIONAL MATRIX
CALL ROMA(XA,YA,ZA,ROZA,XB,YB,ZB,ROZB,VX0,VY0,VZ0,WX0,
WY0,WZ0,I,R,FR,BETA,T,L,NSEG)
C
C CALCULATE EACH SEGMENT LOCAL FORCE
DO 710 I=1,12
PLOC(I,II)=0
DO 710 KK=1,12
710 PLOC(I,II)=PLOC(I,II)+R(I,II)*PGLCB(I,XX)
C
C CALCULATE ELEMENT STIFFNESS MATRIX
CALL ELBST(NSEG,I,ISTIP,EA,EIU,EIV,GKT,EL,PLOC,PSK,SK)
C
C CALCULATE SYSTEM STIFFNESS MATRIX
CALL MULTM(0,12,DUMMY,SK,R)
CALL MULTM(1,12,SK,R,DUMMY)
C
C ASSEMBLING SEGMENT GLOBAL STIFFNESS TO MEMBER
CENTROID STIFFNESS MATRIX
DO 805 J=1,12
DO 805 JJ=1,12
ASAT(NP(I,J),NP(I,JJ))-ASAT(NP(I,J),NP(I,JJ))*SK(J,JJ)
805 CONTINUE
90 CONTINUE
C
C CALCULATE MEMBER'S LOAD INCREMENTS BASED ON 'K'*DD
DO 5021 II=1,12DOP
DP(II)=0
FP1(II)=0
DO 5021 K=1,12DOP
DP(II)=DP(II)+SEASAT(II,K)*DD(K)
5021 CONTINUE
C
C CALCULATE MEMBER'S TOTAL FORCE IN THE GLOBAL DIRECTION
DO 804 J=1,NSEG
DO 804 K=1,12
FP1(NP(J,K))+FP1(NP(J,K))*PGLCB(J,K)
804 CONTINUE
C
C CALCULATE STABILITY MEMBER'S STIFFNESS PARAMETER AND
DISPLACEMENT NORM
IF (IAUTO EQ. 1) THEN
DO 977 NM=1,12DOP
DPCOPY(NM)=D1(NM)
DO 977 NN=1,12DOP
DUMMY(NM,NN)=SEASAT(NM,NN)
977 IF (ISTART EQ. 2) THEN
CALL SNORM(I,FP1,DD,DP,DUMMY,DELTP2,S2,IDOP,IPM)
ENDIF
IF (ISTART EQ. 3) THEN
CALL SNORM(I,FP1,DD,DP,DUMMY,DELTP3,SJ,IDOP,IPM)
CALL SNORM(I,DPCOPY,DD,DD,DUMMY,REE,DMY,IDOP,IPM)
IP (S2*DELTP2.NE.0) SP=(DELTP3/DELTP2)**2*(S2/SJ)
NREB=ABS(REE/CRNE2)
ENDIF
ENDIF
C
C MEMORIZE THE STABILITY ELEMENT STIFFNESS
555 DO 987 I=1,12DOP
DO 987 J=1,12DOP
SEASAT(I,J)+ASAT(I,J)
987 CONTINUE
C
C CONVERT ASAT MATRIX TO HALF STORAGE AND BANDED MODE
CALL SKY(ASAT,STP,JSTOR,IDOP,MDIAG,IMD)
C
C FIND CONDENSED STIFFNESS MATRIX CORRESPONDING TO TWO END 12 DOP
IF (IDOP.NE.12) THEN
LG-IDOP-12
CALL GAUSS2(1,1DOP,1,L2,MDIAG,STP,DPCOPY,FACTOR,JSTOR)
ENDIF
C
C CONDENSE ASAT MATRIX TO GET CONDENSED MATRIX CORRESPONDING TO
TWO ENDS TWELVE DOP DIRECTIONS
DO 205 J=L1,1DOP,-1
DO 205 I=L1,L1-1
IJ=MDIAG(J)*J-I
IF (I3 LT MDIAG(J-1) .OR. I3 EQ. MDIAG(J)) THEN
REASAT(I=L1-1,J=L1-1)+STP(IJ)
REASAT(J=L1-1,I=L1-1)+REASAT(I=L1-1,J=L1-1)
205 ENDIF
CONTINUE
IF (IECOP EQ. 1) CALL TRANX(REASAT,ECCX,ECCY,TPJT,SK)
RETURN
END
C
SUBROUTINE SNORM(IOPT,BR,DSR,DBR,TK,DELTP,DEN,NOP,IPM)
C
C CALCULATE NORM OF LOAD INCREMENT AND NORM OF DISPLACEMENT INCREMENT
IOPT = 1 - CALCULATE DELTP FOR NORM OF LOAD AND DEN
IOPT = 2 - CALCULATE DELTP FOR NORM OF DISPLACEMENT
C
C BR = LOAD VECTOR
C DSR = DISPLACEMENT INCREMENT VECTOR

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SNOR0040
SNOR0050
SNOR0060
SNOR0070
SNOR0080
SNOR0090
SNOR0100
SNOR0110
SNOR0120
SNOR0130
C
C DSR = LOAD VECTOR INCREMENT VECTOR
C TK = STIFFNESS MATRIX
C DELTP: NORM OF LOAD OR DISPLACEMENT
C DEN = DEN = TRANSPOSE OF IDR = TK * DSR
C IPM = TRANSVERSE AND ROTATION DOP IDENTIFICATION INDEX
C I FOR TRANSVERSE
C I FOR ROTATION
C
C NOTE: ST(NM), BR(NM), DSR(NM), DBR(NM), TK(NM,MM)
C (PM(NM))
C
C DIMENSION DSR(NOP),DBR(NOP),BR(NOP),TK(NOP,NOP),IPM(NOP)
C
C SKIM STIFFNESS TERMS
DO 80 I=1,NOP
DO 80 J=1,NOP
IF (ABS(TK(I,J)) .LT. 1.E-7) TK(I,J)=0
80 CONTINUE
C
C SEARCH MAXIMUM FORCE OR DISPLACEMENT INCREMENT
PMAX=0
PMSAL=0
DO 25 I=1,NOP
DO 25 J=1,NOP
IF (IPM(I) EQ. 0) THEN
IF (ABS(BR(I)) GT. ABS(PMAX)) THEN
PMAX=BR(I)
ELSE IF (IPM(I) NE. 0) THEN
IF (ABS(TK(I,J)) GT. ABS(TPMAX)) THEN
TPMAX=TK(I,J)
ENDIF
ENDIF
25 CONTINUE
C
C FIND NORM OF DSR
DELTP=0
DO 32 I=1,NOP
IF (IPM(I) EQ. 0) THEN
IF (IPM(I) EQ. 0) GO TO 32
DELTP=DELTP+(ABS(DSR(I)/PMAX))**2
ELSE IF (IPM(I) NE. 0) THEN
IF (TPMAX EQ. 0) GO TO 32
DELTP=DELTP+(ABS(DSR(I)/TPMAX))**2
ENDIF
32 CONTINUE
DELTP=SQR(DELTP/NOP)
IF (IOPT.EQ.2) GOTO 200
C
C DSRT=**((TK)*TK)-DSR*TK
DEN=0
DO 68 J=1,NOP
DO 68 K=1,NOP
DEN=DEN+DSR(K)*TK(K,J)*DSR(K)
68 CONTINUE
200 RETURN
END
C
C MATRIX SOLUTION OF EQUATION AX=P, WHERE THE UPPER TRIANGULAR MATRIX
C OF THE SYMMETRIC MATRIX A IS STORED BY SKYLINES. P IS STORED AS
C A FULL MATRIX. THE MATRIX P IS REPLACED BY X AFTER BACK
C SUBSTITUTION IS COMPLETE. BOTH A AND P ARE ALTERED
C GUYAN OPTION: IF GUYAN IS TRUE, MASS MATRIX IS ALSO CONDENSED
SUBROUTINE GAUSS2(IOPT,N,L1,L2,MD,A,P,FACTOR,ISTOR)
DIMENSION A(ISTOR),FACTOR(N),P(N),MD(N*1)
IJI(I,J)+MD(J)*J-I
C
C MATRIX STORAGE SCHEME
C THE MATRIX A IS BANDED AND SYMMETRIC. THUS ONLY THE SKYLINE ABOVE
C AND THE MAIN DIAGONAL NEEDS TO BE STORED.
C THE MATRIX A IS STORED IN A LINEAR ARRAY BY COLUMNS. THE VALUE ON
C THE MAIN DIAGONAL IS STORED IN THE LINEAR ARRAY FIRST
C ADDRESSES OF THE LINEAR ARRAY ELEMENTS CONTAINING THE MAIN DIAGONAL
C TERMS ARE STORED IN MD.
C EXAMPLE: LET B BE THE 5X5 FULL SYMMETRIC MATRIX BELOW.
B =
1 3 1 1 1
2 5 1 1 1
3 4 7 1 1
4 6 8 1 1
5 6 8 1 1
NOTE: B(1,3),B(1,4) AND B(2,4) ARE ZERO.
SYMM. 6 9 MD=1, 2, 4, 6, 8, 13
C
C THUS, B(1,3)+A(MD(3))-A(4)
B(I,J)+A(MD(J)*J-I) IF J>I AND (MD(J)-J-I)MD(J+1)
C
C MATRIX P IS NOT SYMMETRIC, AND IS STORED AS A FULL MATRIX
C
C VARIABLE TABLES:
C IOPT = OPTION NUMBER
C N = NUMBER OF DEGREES OF FREEDOM OF A MATRIX
C L1 = FIRST ROW TO BE WORKED WITH
C L2 = LAST ROW TO BE WORKED WITH
C MD = ADDRESS OF THE MAIN DIAGONAL TERMS OF A
C P = LOAD MATRIX ON INPUT, SOLN (X) OF AX=P ON COMPLETION OF
IOPT=4
C
C FACTOR = TEMPORARY STORAGE MATRIX
C L = ROW NUMBER FOR ELIMINATION
C I = ROW NUMBER
C J = COLUMN NUMBER
C
C L1=MAX(L1,1)
C L2=MIN(L2,N)
C
C GO TO (10,370,376,310,410),IOPT
10 CONTINUE
IOPT=1. REDUCE A AND P
GAUSSIAN ELIMINATION IS USED TO REDUCE THE A AND P MATRICES
FROM ROW L1 TO ROW L2. IF L1 IS NOT 1, IT IS ASSUMED THAT
A AND P ARE ALLREADY REDUCED FROM 1 TO L1.
DO 100 L=L1,L2
IF (L.EQ.N) GO TO 100
C
C TEST FOR ZERO PIVOT.
IF (A(MD(L)).EQ.0) THEN
LJ=L
GOTO 375
ENDIF
C
C CALCULATE THE FACTORS EACH ROW IS MULTIPLIED BY
DO 30 I=(L+1),N
LI=I(L,I)
IF (LI.LT.MD(I+1)) THEN
FACTOR(I)=A(LI)/A(MD(L))
ELSE
FACTOR(I)=0
ENDIF
30 CONTINUE
C
C REDUCE A BY COLUMNS (J) EACH ROW (I)
DO 40 J=(L+1),N
LJ=I(L,J)
IF (LJ.GE.MD(J+1)) GO TO 40
DO 25 I=(L+1),J
IJ=I(L,I)
A(IJ)-A(IJ)*FACTOR(I)*A(LJ)

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GAUS1000

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35 CONTINUE
40 CONTINUE
C ..... REDUCE P BY COLUMNS (J) EACH ROW (I)...
DO 45 I=L1,L2,N
P(I)=P(I)-FACTOR(I)*P(L)
45
100 CONTINUE
RETURN
C .....
310 CONTINUE
IOPT=4, REDUCED MULTIPLICATION
SOLVE FOR V IN S*D=P WHERE S HAS BEEN REDUCED BY GAUSSIAN
ELIMINATION.
THE RESULTING LOAD IS STORED IN P.
THE DISPLACEMENTS ARE INPUT IN D.
MULTIPLICATION ONLY TAKES PLACE BETWEEN ROWS L1 AND L2.
C .....
ONLY ONE LOAD CASE...
ZERO P
DO 320 J=L1,L2
IF (A(MD(J),EQ.0) THEN
LJ=J
GOTO 375
ENDIF
P(J)=0
320
C ..... MULTIPLY S_RED * DISP - P
DO 340 I=L1,L2,N
DO 340 J=L1,L2,N
IF (I.LE.J) IJ=MD(J)+J-I
IF (I.GT.J) IJ=MD(I)+I-J
P(I)=P(I)+A(IJ)*FACTOR(J)
340
400 CONTINUE
RETURN
410 CONTINUE
IOPT=5, BACK SUBSTITUTION
SOLVE FOR D IN S*D=P
L1=1
L2=1
L3=1
WHERE S IS THE CONDENSED MATRIX BY GAUSS ELIMINATION,
AND IT IS A NON-POSITIVE DEFINITIVE MATRIX
D AND P ARE THE REDUCED DISPLACEMENT AND LOAD MATRIX
P ARE INPUT IN ARRAY P
D ARE INPUT IN ARRAY FACTOR
C ..... ONLY ONE LOAD CASE...
DO 720 J=L1,L2
IF (A(MD(J),EQ.0) THEN
LJ=J
GOTO 375
ENDIF
720 CONTINUE
C ..... CALCULATE DISPLACEMENT MATRIX , BACK SUBSTITUTION ONLY
TAKES PLACE BETWEEN L1 AND L2
L3=N
DO 350 I=L2,L1,-1
FACTOR(I)=0.
DO 350 J=L1,L3,+1
IJ=MD(J)+J-I
IF (I.EQ. MD(I)) THEN
FACTOR(I)=FACTOR(I)+P(I)/A(MD(I))
ELSE IF (IJ.LT. MD(I+1)) THEN
FACTOR(I)=FACTOR(I)-A(IJ)*FACTOR(J)/A(MD(I))
ENDIF
350 CONTINUE
GOTO 370
375 WRITE(108,2000) LJ,MD(LJ),A(MD(LJ))
2000 FORMAT (//1X,50(' '))
& * ER, * FOR IN SUBROUTINE GAUSS', T51,'//
& * PIVOT IS LE 0 ..... ROW: IS, T51,'//
& * STORAGE LCN: IS, T51,'//
& * ..... PIVOT: IS, P15.6, T51,'//
& * ..... SOLUTION IS ABORTED', T51,'//
& 1X,50(' '))
370 STOP
RETURN
END
C .....
SUBROUTINE CURVA (NSEG,ISEG,DBC,DCU,DCV,E,EL,FR,DXA,DYA,DZA,
DIMENSION R(12,12),DA(6),DDA(6),DB(6),DDB(6),FR(NSEG,9),EL(NSEG)
C .....
DO 52 I=1,12
DO 52 J=1,12
R(I,J)=0.
52 CONTINUE
C .....
I=1
72 R(I,1)=PR(ISEG,1)
R(I,1)=PR(ISEG,2)
R(I,2)=PR(ISEG,3)
R(I,1)=PR(ISEG,4)
R(I,1,1)=PR(ISEG,5)
R(I,1,2)=PR(ISEG,6)
R(I,2,1)=PR(ISEG,7)
R(I,2,1)=PR(ISEG,8)
R(I,2,1)=PR(ISEG,9)
I=I+1
IF (I.EQ.13) GO TO 62
GO TO 72
62 DA(1)=DXA
DA(2)=DYA
DA(3)=DZA
DA(4)=DROXA
DA(5)=DROYA
DA(6)=DROZA
DB(1)=DXB
DB(2)=DYB
DB(3)=DZB
DB(4)=DROXB
DB(5)=DROYB
DB(6)=DROZB
C .....
DO 710 II=1,6
DDA(II)=0
DO 710 KK=1,6
DDA(II)=DDA(II)+R(II,KK)*DA(KK)
710
DO 720 II=1,6
DDB(II)=0
DO 720 KK=1,6
DDB(II)=DDB(II)+R(II,KK)*DB(KK)
720
C .....
-- FIND SEGMENT BASIC DEFORMATION INCREMENTS --
DEC=(DDB(3)-DDA(3))/EL(ISEG)
DCU=(DDB(4)-DDA(4))/EL(ISEG)
DCV=(DDB(5)-DDA(5))/EL(ISEG)
RETURN
END
C .....
SUBROUTINE CLDAP (PRM,FR,I,NSEG,XGLO,PSK,SK,PGLOB)
C .....
CALCULATE SEGMENT GLOBAL FORCES

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SECT0610

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SUBROUTINE SECT2 (RAD, ECCXO, ECCYO, INEB, ST, UO, VO, AO, B, ECCXO,
&
ECCYO, NUMP, IECCP)
C
C THIS SUBROUTINE IS FOR DEFINING SECTIONAL COORDINATES FOR
C THE TUBE CROSS-SECTION
C
C RAD = RADIUS OF THE TUBE (BASED ON CENTER LINE)
C INEB = TOTAL NO. OF ELEMENTS FOR 1/4 OF THE CIRCLE
C ST = THICKNESS
C
C REAL INEB
C DIMENSION UO (NUMP), VO (NUMP), AO (NUMP), B (NUMP)
C NUMP=4*INEB
C PI=2.*3.1415926
C SITA=(PI/4.)/(2*INEB)
C TEMP=2*SITA
C DO 25 I=1, INEB
C VO(I)=SQRT(RAD*ST/(1.-TAN(SITA)*TAN(SITA)))
C UO(I)=VO(I)*TAN(SITA)
C 25 SITA=SITA+TEMP
C DO 30 I=INEB+1, 2*INEB
C VO(I)=VO(I-INEB)
C UO(I)=UO(I-INEB)
C 30 DO 35 I=2*INEB+1, 3*INEB
C VO(I)=VO(I-2*INEB)
C UO(I)=UO(I-2*INEB)
C 35 DO 40 I=3*INEB+1, 4*INEB
C VO(I)=VO(I-3*INEB)
C UO(I)=UO(I-3*INEB)
C 40 DO 50 I=1, NUMP
C B(I)=(PI*ST)/NUMP
C AO(I)=B(I)*ST
C 50 CONTINUE
C IF (IECCP .EQ. 1) THEN
C ECCXO=ECCXO
C ECCYO=ECCYO
C ENDIF
C RETURN
C END

SUBROUTINE SECT3 (HH, UU, ECCXO, ECCYO, INEB, INEH, UO, VO, AO, B, ECCXO,
&
ECCYO, NUMP, IECCP)
C
C THIS SUBROUTINE IS FOR DEFINING SECTIONAL COORDINATES FOR
C THE RECTANGULAR CROSS-SECTION
C
C HH = SECTIONAL HIGH
C UU = SECTIONAL WIDTH
C INEB = TOTAL NO. OF ELEMENTS FOR HALF WIDTH
C INEH = TOTAL NO. OF ELEMENTS FOR HALF HIGH
C B(1) = HH/2 (LARGER LENGTH)
C B(2) = UU/2 (SMALLER LENGTH)
C NOTE : B(1) AND B(2) ARE FOR CALCULATING TORSIONAL RIGIDITY
C
C REAL INEB, INEH
C DIMENSION UO (NUMP), VO (NUMP), AO (NUMP), B (NUMP)
C NUMP=4*(INEB+INEH)
C ELB=HH/(2*INEB)
C ELV=UU/(2*INEH)
C
C DO 50 M=1, INEB
C II=(M-1)*INEH+1
C UO(II)=(UU/2)-(ELB/2)*(M-1)*ELB
C VO(II)=(HH/2)-(ELV/2)
C AO(II)=ELB+ELB
C 50 CONTINUE
C DO 60 I=(M-1)*INEH+2, M*INEH
C J=J+1
C UO(I)=UO(II)
C VO(I)=VO(II)
C AO(I)=ELB+ELB
C 60 CONTINUE
C 50 CONTINUE
C IGBT=INEB*INEH
C DO 70 I=1, IGBT+1, 2*IGBT
C UO(I)=UO(I-IGBT)
C VO(I)=VO(I-IGBT)
C AO(I)=ELB+ELB
C 70 CONTINUE
C DO 80 I=2*IGBT+1, 3*IGBT
C UO(I)=UO(I-2*IGBT)
C VO(I)=VO(I-2*IGBT)
C AO(I)=ELB+ELB
C 80 CONTINUE
C DO 90 I=3*IGBT+1, 4*IGBT
C UO(I)=UO(I-3*IGBT)
C VO(I)=VO(I-3*IGBT)
C AO(I)=ELB+ELB
C 90 CONTINUE
C IF (HH .GE. UU) THEN
C B(1)=HH/2
C B(2)=UU/2
C ELSE IF (HH .LT. UU) THEN
C B(1)=UU/2
C B(2)=HH/2
C ENDIF
C
C IF (IECCP .EQ. 1) THEN
C ECCXO=ECCXO
C ECCYO=ECCYO
C ENDIF
C RETURN
C END

SUBROUTINE SECT4 (HH, UU, ECCXO, ECCYO, INEB, ST, UO, VO, AO, B, ECCXO,
&
ECCYO, NUMP, IREV1, IREV2, IREV3, IREV4, IECCP)
C
C THIS SUBROUTINE IS FOR DEFINING SECTIONAL COORDINATES FOR
C THE WIDE-PLANGE SECTION
C
C ST = PLANGE THICKNESS
C INEB = WEB THICKNESS
C HH = SECTION HIGH
C UU = SECTION WIDTH
C IREV1 = NO. OF PLANGE COLUMN IN THE REFERENCE AXIS U DIRECTION
C IREV2 = NO. OF PLANGE ROW IN THE REFERENCE AXIS U DIRECTION
C IREV3 = NO. OF WEB COLUMN IN THE REFERENCE AXIS V DIRECTION
C IREV4 = NO. OF WEB ROW IN THE REFERENCE AXIS V DIRECTION
C
C REAL INEB
C DIMENSION UO (NUMP), VO (NUMP), AO (NUMP), B (NUMP)
C NUMP=IREV1*IREV2*2 + IREV3*IREV4
C WEBHIGH = HH
C 2*ST
C SS = UU/IREV2
C TT = ST/IREV1
C SSS = INEB/IREV3
C TTT = WEBHIGH/IREV4
C B(1) = (2*UU*ST**3 + WEBHIGH*INEB**3)/3
C
C ... CALCULATE TOP FLANGE ELEMENT COORDINATES
C X=0
C DO 50 I=1, IREV1
C DO 50 J=1, IREV2
C K=K+1
C UO(K)=(UU/2) + 0.5*SS + (J-1)*SS
C VO(K)=(HH/2) - 0.5*TT - (I-1)*TT
C AO(K)=SS*TT
C 50 CONTINUE
C
C ... CALCULATE BOTTOM FLANGE ELEMENT COORDINATES
C DO 60 I=1, IREV1
C DO 60 J=1, IREV2
C K=K+1
C UO(K)=(-UU/2) + 0.5*SS + (J-1)*SS

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VO(K)=(-HH/2) + 0.5*TT - (I-1)*TT
C AO(K)=SS*TT
C 60 CONTINUE
C
C ... CALCULATE WEB ELEMENT COORDINATES
C DO 70 I=1, IREV4
C DO 70 J=1, IREV3
C K=K+1
C UO(K)=(-INEB/2) + 0.5*SSS + (J-1)*SSS
C VO(K)=(WEBHIGH/2) - 0.5*TTT - (I-1)*TTT
C AO(K)=SSS*TTT
C 70 CONTINUE
C
C IF (IECCP .EQ. 1) THEN
C ECCXO=ECCXO
C ECCYO=ECCYO
C ENDIF
C RETURN
C END

SUBROUTINE SECT5 (BB, T, D, NCS, NPS, TI, UO, VO, AO, B, NUMP, IECCP)
C
C THIS SUBROUTINE IS FOR DEFINING SECTIONAL COORDINATES FOR
C THE OPEN WEB JOIST PLATE-CHORD CROSS-SECTION (BOTTOM)
C
C BB = PLATE WIDTH
C T = PLATE THICKNESS
C TI = CHORD THICKNESS
C NPS = TOTAL NO. OF ELEMENTS FOR HALF PLATE WIDTH
C NCS = TOTAL NO. OF ELEMENTS FOR EACH ANGLE CHORD'S TOP
C D = THE ANGLE'S LENGTH
C
C REAL NCS, NPS
C DIMENSION UO (NUMP), VO (NUMP), AO (NUMP), B (NUMP)
C IF (IECCP .EQ. 1) THEN
C WRITE (108, *) '1', SECT5 LIBRARY FOR ECCENTRIC LOAD IS NOT
C $ AVAILABLE
C IECCP=0
C ENDIF
C NUMP=4*NCS+2*NPS-2
C S=((0.5*BB**2+2)-(4.*TI*T*D*D+TI**2-.TI*D**2-.2*TI**2-TI**3))
C / (BB*T+4.*TI*D-.2*TI**2)
C DS=D-TI/(4.*NCS)
C UO(1)=(BB/2)-(TI/2.)
C VO(1)=-(8*(T+0.5*TI))
C AO(1)=TI**2
C B(1)=TI
C DO 52 I=2, NCS+1
C UO(I)=(0.5*BB-D-DS)-2.*DS*(I-1)
C VO(I)=-(8*(T+0.5*TI))
C AO(I)=2.*DS*TI
C B(I)=2.*DS
C 52 CONTINUE
C DO 54 I=NCS+2, 2*NCS-1
C UO(I)=0.5*BB-0.5*TI
C VO(I)=(D-T)*DS-2.*DS*(I-NCS-1)
C AO(I)=2.*DS*TI
C B(I)=2.*DS
C 54 CONTINUE
C UO(2*NCS-2)=UO(1)
C VO(2*NCS-2)=VO(1)
C AO(2*NCS-2)=AO(1)
C B(2*NCS-2)=B(1)
C DO 56 I=2*NCS+3, 3*NCS-2
C UO(I)=-(0.5*BB-D-DS)-2.*DS*(I-2*NCS-2)
C VO(I)=-(8*(T+0.5*TI))
C AO(I)=2.*DS*TI
C B(I)=2.*DS
C 56 CONTINUE
C DO 58 I=3*NCS+3, 4*NCS-2
C UO(I)=0.5*BB-0.5*TI
C VO(I)=(D-T)*DS-2.*DS*(I-3*NCS-2)
C AO(I)=2.*DS*TI
C B(I)=2.*DS
C 58 CONTINUE
C DBB=0.5*BB/(2.*NPS)
C DO 60 I=4*NCS+3, 4*NCS-3-NPS-1
C UO(I)=0.5*BB-DBB-2.*DBB*(I-4*NCS-2)
C VO(I)=-(8*(T+0.5*TI))
C AO(I)=2.*DBB*TI
C B(I)=2.*DBB
C 60 CONTINUE
C UO(4*NCS-2)=UO(1)
C VO(4*NCS-2)=VO(1)
C AO(4*NCS-2)=AO(1)
C B(4*NCS-2)=B(1)
C 64 CONTINUE
C RETURN
C END

SUBROUTINE SECT6 (RAD, ECCXO, ECCYO, INEB, ST, UO, VO, AO, B, ECCXO,
&
ECCYO, NUMP, IECCP)
C
C THIS SUBROUTINE IS FOR DEFINING SECTIONAL COORDINATES FOR
C THE TUBE CROSS-SECTION WHICH HAS TWO LAYERS
C
C RAD = RADIUS OF THE TUBE (BASED ON CENTER LINE)
C INEB = TOTAL NO. OF ELEMENTS FOR 1/4 OF THE CIRCLE FOR
C EACH LAYER
C ST = THICKNESS
C
C REAL INEB
C DIMENSION UO (NUMP), VO (NUMP), AO (NUMP), B (NUMP)
C NUMP=8*INEB
C NRUMP=NUMP/2
C RAD1=RAD*(ST/4.)
C RAD2=RAD*(ST/4.)
C PI=2.*3.1415926
C SITA=(PI/4.)/(2*INEB)
C TEMP=2*SITA
C DO 25 I=1, INEB
C VO(I)=SQRT(RAD1*RAD1/(1.-TAN(SITA)*TAN(SITA)))
C UO(I)=VO(I)*TAN(SITA)
C 25 SITA=SITA+TEMP
C DO 30 I=INEB+1, 2*INEB
C VO(I)=VO(I-INEB)
C UO(I)=UO(I-INEB)
C 30 DO 35 I=2*INEB+1, 3*INEB
C VO(I)=VO(I-2*INEB)
C UO(I)=UO(I-2*INEB)
C 35 DO 40 I=3*INEB+1, 4*INEB
C VO(I)=VO(I-3*INEB)
C UO(I)=UO(I-3*INEB)
C 40 DO 50 I=1, NRUMP
C VO(I)=NRUMP+VO(I-2*INEB-NRUMP)
C UO(I)=UO(I-2*INEB-NRUMP)
C 50 CONTINUE
C IF (IECCP .EQ. 1) THEN
C ECCXO=ECCXO
C ECCYO=ECCYO
C ENDIF
C RETURN
C END

SUBROUTINE SECT7 (D, ECCXO, ECCYO, T, UO, VO, AO, B, ECCXO, ECCYO, NUMP,
&
L1, N1, L2, N2, IECCP)

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C THIS SUBROUTINE IS FOR DEFINING SECTIONAL COORDINATES FOR
THE ANGLE CROSS SECTION
D = ANGLE SIDE TOTAL LENGTH
T = THICKNESS
ECCX0 = THE X-DISTANCE FROM ECCENTRIC LOAD TO SHAPE CENTER
ECCY0 = THE Y-DISTANCE FROM ECCENTRIC LOAD TO SHAPE CENTER
N1 = NO. OF COLUMN IN X DIRECTION
N2 = NO. OF ROW IN Y DIRECTION
L1 = NO. OF ROW IN X DIRECTION
L2 = NO. OF COLUMN IN Y DIRECTION
IECOP = ECCENTRIC LOAD OPTION
1: ECCENTRIC LOAD APPLIED
2: WITHOUT ECCENTRIC LOAD
-----
DIMENSION UO(NUMP), VO(NUMP), AO(NUMP), B(NUMP)
BB=(D*T*D**2*T**2)/(4.*D.*D.*T)
SV1=(T/N1)*0.5
SK1=(D/L1)*0.5
DO 101 J=1,N1
  II=J-1
  DO 101 I=1,L1
    M=I+L1-1
    UO(M)=BB*(D*(2*I-1)*SK1)
    VO(M)=BB*(2*J-1)*SV1
    AO(M)=SV1*SK1*4.
    B(M)=2.*SK1
101 CONTINUE
SK2=(T/N2)*0.5
SV2=(D*(D-T)/L2)*0.5
DO 201 J=1,N2
  II=J-1
  DO 201 I=1,L2
    M=I+L2-1*(N1*L1)
    UO(M)=BB*(2*J-1)*SK2
    VO(M)=BB*(D*(2*I-1)*SV2)
    AO(M)=SV2*SK2*4.
    B(M)=2.*SV2
201 CONTINUE
NUMP=L1*N1+L2*N2
IF(IECOP.EQ.1) THEN
  ECCX0=ECCX0
  ECCY0=ECCY0
ENDIF
RETURN
END
-----
SUBROUTINE SECT9(D,ECCX0,ECCY0,T,UO,VO,AO,B,ECCX0,ECCY0,NUMP,
S
L1,N1,L2,N2)
C THIS SUBROUTINE IS FOR DEFINING SECTIONAL COORDINATES FOR
THE ANGLE CROSS SECTION (BOTTOM CHORD)
D = ANGLE SIDE TOTAL LENGTH
T = THICKNESS
ECCX0 = THE X-DISTANCE FROM ECCENTRIC LOAD TO SHAPE CENTER
ECCY0 = THE Y-DISTANCE FROM ECCENTRIC LOAD TO SHAPE CENTER
N1 = NO. OF COLUMN IN X DIRECTION
N2 = NO. OF ROW IN Y DIRECTION
L1 = NO. OF ROW IN X DIRECTION
L2 = NO. OF COLUMN IN Y DIRECTION
-----
DIMENSION UO(NUMP), VO(NUMP), AO(NUMP), B(NUMP)
BB=(D*T*D**2*T**2)/(4.*D.*D.*T)
SV1=(T/N1)*0.5
SK1=(D/L1)*0.5
DO 101 J=1,N1
  II=J-1
  DO 101 I=1,L1
    M=I+L1-1
    UO(M)=BB*(D*(2*I-1)*SK1)
    VO(M)=BB*(2*J-1)*SV1
    AO(M)=SV1*SK1*4.
    B(M)=2.*SK1
101 CONTINUE
SK2=(T/N2)*0.5
SV2=(D*(D-T)/L2)*0.5
DO 201 J=1,N2
  II=J-1
  DO 201 I=1,L2
    M=I+L2-1*(N1*L1)
    UO(M)=BB*(2*J-1)*SK2
    VO(M)=BB*(D*(2*I-1)*SV2)
    AO(M)=SV2*SK2*4.
    B(M)=2.*SV2
201 CONTINUE
NUMP=L1*N1+L2*N2
ECCX0=ECCX0
ECCY0=ECCY0
RETURN
END
-----
SUBROUTINE ROMA(XA,YA,ZA,ROZA,XB,YB,ZB,ROZB,VX0,VY0,VZ0,WX0,
WY0,WZ0,ISEG,X,P,BETA,TL,NSEGG)
DIMENSION R(12,12),PR(NSEGG,9),BETA(NSEGG),TL(NSEGG)
TL(1:ISEG)=SQRT((XB-XA)**2+(YB-YA)**2+(ZB-ZA)**2)
VX=(XB-XA)/TL(1:ISEG)
VY=(YB-YA)/TL(1:ISEG)
WZ=(ZB-ZA)/TL(1:ISEG)
ROMA=BA-BETA(1:ISEG)*0.5*(ROZA-ROZB)
S=SIN(ROMA)
C=COS(ROMA)
VBAX=VX0*C+(VY0*WZ0-VZ0*WY0)*S
VBAY=VY0*C+(VZ0*WZ0-VX0*WY0)*S
VBZ=VZ0*C+(VX0*WY0-VY0*WX0)*S
TV=VBAX*WX+VBAY*WY+VBZ*WZ
TLV=SQRT((VBAX-TV*WX)**2+(VBAY-TV*WY)**2+(VBZ-TV*WZ)**2)
V1=(VBAX-TV*WX)/TLV
V2=(VBAY-TV*WY)/TLV
V3=(VBZ-TV*WZ)/TLV
UX=VY*WZ-VZ*WY
UY=VZ*WX-VX*WZ
UZ=VX*WY-VY*WX
C FIND THE ROTATION MATRIX AND THE SECTION PROPERTIES
DO 52 I=1,12
  DO 52 J=1,12
    R(I,J)=0.
52 CONTINUE
I=1
72 R(I,I)=UX
R(I,I+1)=UY
R(I,I+2)=UZ
R(I+1,I)=VX
R(I+1,I+1)=VY
R(I+1,I+2)=VZ
R(I+2,I)=WX
R(I+2,I+1)=WY
R(I+2,I+2)=WZ
I=I+3
IF(I.EQ.13) GO TO 62
GO TO 72
62 PR(1:ISEG,1)=UX
PR(1:ISEG,2)=VY
PR(1:ISEG,3)=UZ
PR(1:ISEG,4)=VX
PR(1:ISEG,5)=VY
PR(1:ISEG,6)=VZ
PR(1:ISEG,7)=WX
PR(1:ISEG,8)=WY
PR(1:ISEG,9)=WZ
RETURN
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C CALCULATE PRESENT SECTIONAL ELEMENT COORDINATE
C ACCORDING TO INSTANTANEOUS AXES
C
DO 52 J=1,NUMP
  U(ISEG,J)= COS(BETA(ISEG))*UO(J)-SIN(BETA(ISEG))*VO(J)-VCO
  V(ISEG,J)= SIN(BETA(ISEG))*UO(J)+COS(BETA(ISEG))*VO(J)+VCO
52 CONTINUE
RETURN
END

C
SUBROUTINE JUDEL0(NSEG,NUMP,ISEG,EM,EY,RATIO3,
  & DE,E,E1,E2,TE,STR)
-----
ELASTO-PLASTIC STRESS-STRAIN HYSTERESIS MODEL
-----
DIMENSION DE(NSEG,NUMP),E(NUMP),E1(NSEG,NUMP)
DIMENSION E2(NSEG,NUMP),TE(NUMP),STR(NSEG,NUMP)

DO 75 I=1,NUMP
  IF (E(I).LT.E1(ISEG,I).AND.E(I).GT.E2(ISEG,I)) THEN
    TE(I)=EM
    STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
  ENDIF
  -----
  -- TENSION STRAIN CONTROL --
  -----
  IF (E(I).GE.E1(ISEG,I).AND.DE(ISEG,I).GE.0.) THEN
    E1(ISEG,I)=E(I)
    E2(ISEG,I)=E(I)+2.*EY
    TE(I)=RATIO3*EM
    STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
  ENDIF
  IF (E(I).GE.E1(ISEG,I).AND.DE(ISEG,I).LT.0.) THEN
    TE(I)=EM
    STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
  ENDIF
  -----
  -- COMPRESSION STRAIN CONTROL --
  -----
  IF (E(I).LE.E2(ISEG,I).AND.DE(ISEG,I).LE.0.) THEN
    E2(ISEG,I)=E(I)
    E1(ISEG,I)=E(I)+2.*EY
    TE(I)=RATIO3*EM
    STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
  ENDIF
  IF (E(I).LE.E2(ISEG,I).AND.DE(ISEG,I).GT.0.) THEN
    TE(I)=EM
    STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
  ENDIF
75 CONTINUE
RETURN
END

SUBROUTINE JUDEL3(NSEG,NUMP,ISEG,EM,EY,RATIO3,YS,
  & DE,E,E1,E2,TE,STR)
-----
MODIFIED BILINEAR STRESS-STRAIN HYSTERESIS MODEL
-----
DIMENSION DE(NSEG,NUMP),E(NUMP),E1(NSEG,NUMP)
DIMENSION E2(NSEG,NUMP),TE(NUMP),STR(NSEG,NUMP)

DO 75 I=1,NUMP
  IF (E(I).LT.E1(ISEG,I).AND.E(I).GT.E2(ISEG,I)) THEN
    TE(I)=EM
    STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
  ENDIF
  -----
  -- TENSION STRAIN CONTROL --
  -----
  ELSE IF (E(I).GE.E1(ISEG,I).AND.DE(ISEG,I).GE.0.) THEN
    IF (ABS(STR(ISEG,I)).GE.YS) THEN
      E1(ISEG,I)=E(I)
      E2(ISEG,I)=E(I)+2.*EY
      TE(I)=RATIO3*EM
      STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
    ELSE IF (ABS(STR(ISEG,I)).LT.YS) THEN
      TE(I)=EM
      STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
    ENDIF
  ELSE IF (ABS(STR(ISEG,I)).LT.YS) THEN
    TE(I)=EM
    STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
  ENDIF
  IF (E(I).LE.E2(ISEG,I).AND.DE(ISEG,I).LT.0.) THEN
    STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
  ENDIF
  -----
  -- COMPRESSION STRAIN CONTROL --
  -----
  ELSE IF (E(I).LE.E2(ISEG,I).AND.DE(ISEG,I).LE.0.) THEN
    IF (ABS(STR(ISEG,I)).GE.YS) THEN
      E2(ISEG,I)=E(I)
      E1(ISEG,I)=E(I)+2.*EY
      TE(I)=RATIO3*EM
      STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
    ELSE IF (ABS(STR(ISEG,I)).LT.YS) THEN
      TE(I)=EM
      STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
    ENDIF
  ELSE IF (ABS(STR(ISEG,I)).LT.YS) THEN
    TE(I)=EM
    STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
  ENDIF
75 CONTINUE
RETURN
END

SUBROUTINE JUDEL3(NSEG,NUMP,ISEG,EM,IR,YS,DE,E,TE,
  & DEP,EOP,STRP,STR,TEP,EPEP,SREP,MOPT)
-----
RAMBERG-OSGOOD STRESS-STRAIN HYSTERESIS MODEL
-----
DIMENSION DE(NSEG,NUMP),E(NUMP),TE(NUMP),STR(NSEG,NUMP)
DIMENSION DEP(NSEG,NUMP),EOP(NSEG,NUMP),STRP(NSEG,NUMP)
& TEP(NSEG,NUMP),EPEP(NSEG,NUMP),SREP(NSEG,NUMP)
-----
-- FOR INITIAL CONDITION --
-----
DO 75 I=1,NUMP
  IF (MOPT.EQ.2) THEN
    TE(I)=EM
    STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
  ENDIF
  DEP(ISEG,I)=DE(ISEG,I)
  EOP(ISEG,I)=E(I)
  STRP(ISEG,I)=STR(ISEG,I)
  -----
  -- CHECK REVERSE POINT --
  -----
  ELSE IF (DEP(ISEG,I)*DE(ISEG,I).LT.0) THEN
    EPEP(ISEG,I)=EOP(ISEG,I)
    SREP(ISEG,I)=STRP(ISEG,I)
    TE(I)=EM
    STR(ISEG,I)=STR(ISEG,I)+TE(I)*DE(ISEG,I)
  ENDIF
  DEP(ISEG,I)=DE(ISEG,I)
  EOP(ISEG,I)=E(I)
  STRP(ISEG,I)=STR(ISEG,I)
75 CONTINUE
RETURN
END

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C -----
C -- FOR SKELETON AND BRANCH REGION --
C
ELSE IF (DEP(ISEG,I)*DE(ISEG,I).GE.0) THEN
  STR(ISEG,I)=STR(ISEG,I)+TEP(ISEG,I)*DE(ISEG,I)
C
C ..... FOR FIRST SKELETON CURVE
C IF (EPEP(ISEG,I).EQ.0 AND SREP(ISEG,I).EQ.0) THEN
  IF (ABS(STR(ISEG,I)/YS).LT.0.2) THEN
    TEMPI=1/EM
    ELSE IF (STR(ISEG,I).GE.0) THEN
      TEMPI=(1+IR*ABS(STR(ISEG,I)/YS)**(IR-1))/EM
    ELSE IF (STR(ISEG,I).LT.0) THEN
      TEMPI=(1+IR*ABS(STR(ISEG,I)/YS)**(IR-1))/EM
    ENDIF
    TE(I)=1/TEMPI
    DEP(ISEG,I)=DE(ISEG,I)
    EOP(ISEG,I)=E(I)
    STRP(ISEG,I)=STR(ISEG,I)
  ENDIF
C
C ..... AFTER FIRST SKELETON CURVE
C ELSE IF (EPEP(ISEG,I).NE.0 OR SREP(ISEG,I).NE.0) THEN
  TEMPI=STR(ISEG,I)+SREP(ISEG,I)
  IF (ABS(TEMPI/(2*YS)).LT.0.2) THEN
    TEMPI=1/EM
    ELSE IF (TEMPI.GE.0) THEN
      TEMPI=(1+IR*ABS(TEMPI/(2*YS)**(IR-1))/EM
    ELSE IF (TEMPI.LT.0) THEN
      TEMPI=(1+IR*ABS(TEMPI/(2*YS)**(IR-1))/EM
    ENDIF
    TE(I)=1/TEMPI
    DEP(ISEG,I)=DE(ISEG,I)
    EOP(ISEG,I)=E(I)
    STRP(ISEG,I)=STR(ISEG,I)
  ENDIF
75 CONTINUE
C
C RETURN
C END
C -----
C SUBROUTINE BOXLOC(PJL,STR,NSEG,NUMP,ISEG,INEB,INEM,
  & TE,ST,EM,UO,VO,FLP)
C
C JUDGE LOCAL BUCKLING STRESS FOR BOX SECTION
C *****
C NOTE FLP=0 : NO LOCAL BUCKLING OCCURRED
C FLP=1-5 : LOCAL BUCKLING OCCURRED
C
REAL INEB,INEM
DIMENSION STR(NSEG,NUMP),UO(NUMP),VO(NUMP),PJL(NUMP),TE(NUMP)
C
DO 60 I=1,NUMP
  PJL(I)=0
  IPT1=1
  INEQ=INEB+INEM
  IPT2=INEQ-1
  IPT3=2*INEQ-1
  IPT4=3*INEQ-1
C
C CALCULATE SECTION ELEMENT LENGTH
C
ELM=ABS(VO(2)-VO(3))
ELB=ABS(UO(INEB-1)-UO(INEM-1))
*****
-- JUDGE LOCAL BUCKLING INDEX FJL --
*****
AVOID SECTION REACH TO FULLY YIELD COMPRESSION STRESS
C
IF (STR(ISEG,IPT1).LT.0 AND STR(ISEG,IPT3).LT.0 AND
  & STR(ISEG,IPT4).LT.0 AND STR(ISEG,IPT2).LT.0) THEN
  IF (TE(IPT1).NE.EM AND TE(IPT3).NE.EM AND
    & TE(IPT4).NE.EM AND TE(IPT2).NE.EM) THEN
    DO 260 I=1,NUMP
      FJL(I)=1
      FLP=5
      RETURN
    ENDIF
  ENDIF
  IF (STR(ISEG,IPT1).LT.0 AND STR(ISEG,IPT3).LT.0) THEN
    IF (TE(IPT1).NE.EM AND TE(IPT3).NE.EM) THEN
      DO 70 I=INEB+1,INEQ
        FJL(I)=1
      DO 72 I=2*INEQ+1,3*INEQ
        FJL(I)=1
      L1=2
      L2=INEM
      L3=INEQ-2
      L4=INEQ-INEH
      CALL SUBL11(1,FLP,L1,L2,L3,L4,ELM,EM,ST,STR,PJL,ISEG,
        & NSEG,NUMP)
      L1=2*INEQ-2
      L2=2*INEQ-INEH
      L3=3*INEQ-2
      L4=3*INEQ-INEH
      CALL SUBL11(1,FLP,L1,L2,L3,L4,ELM,EM,ST,STR,PJL,ISEG,
        & NSEG,NUMP)
      L1=2*INEQ-2
      L2=2*INEQ-INEH
      L3=3*INEQ-2
      L4=3*INEQ-INEH
      CALL SUBL11(2,FLP,L1,L2,L3,L4,ELM,EM,ST,STR,PJL,ISEG,
        & NSEG,NUMP)
      L1=2*INEQ-2
      L2=2*INEQ-INEH
      L3=3*INEQ-2
      L4=3*INEQ-INEH
      CALL SUBL11(3,FLP,L1,L2,L3,L4,ELM,EM,ST,STR,PJL,ISEG,
        & NSEG,NUMP)
      L1=INEH-1
      L2=INEQ
C
C IF (TE(IPT1).NE.EM AND TE(IPT3).NE.EM) THEN
  DO 20 I=1,INEH
    FJL(I)=1
  DO 22 I=INEQ+1,INEQ-INEH
    FJL(I)=1
  L1=INEQ-INEH+1
  L2=2*INEQ
  L3=3*INEQ-INEH-1
  L4=4*INEQ
  CALL SUBL11(3,FLP,L1,L2,L3,L4,ELM,EM,ST,STR,PJL,ISEG,
    & NSEG,NUMP)
  L1=INEH-1
  L2=INEQ

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JUDE0370
JUDE0380
JUDE0390
JUDE0400
JUDE0410
JUDE0420
JUDE0430
JUDE0440
JUDE0450
JUDE0460
JUDE0470
JUDE0480
JUDE0490
JUDE0500
JUDE0510
JUDE0520
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JUDE0600
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JUDE0690
JUDE0700
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BOXK0970
BOXK0980
BOXK0990
BOXK1000

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L4=2*INEQ*INEH*1
L4=3*INEQ
CALL SUB11(3,FLP,L1,L2,L3,L4,ELH,EM,ST,STR,FJL,ISEG,
NSEQ,NUMP)
4 ENDIF
ENDIF
C
IF (STR(ISEG,IP73) .LT. 0 .AND. STR(ISEG,IP74) .LT. 0) THEN
IP7E(IP73) .NE. EM .AND. TE(IP74) .NE. EM) THEN
DO 30 I=2*INEQ+1,2*INEQ*INEH
30 FJL(I)=1
DO 32 I=3*INEQ+1,3*INEQ*INEH
32 FJL(I)=1
C
L1=INEH*1
L2=INEQ
L3=2*INEQ*INEH*1
L4=3*INEQ
CALL SUB11(4,FLP,L1,L2,L3,L4,ELH,EM,ST,STR,FJL,ISEG,
NSEQ,NUMP)
4 ENDIF
ENDIF
C
L1=INEQ*INEH*1
L2=2*INEQ
L3=3*INEQ*INEH*1
L4=4*INEQ
CALL SUB11(4,FLP,L1,L2,L3,L4,ELH,EM,ST,STR,FJL,ISEG,
NSEQ,NUMP)
4 ENDIF
ENDIF
C
RETURN
END
SUBROUTINE SUB11(KK,FLP,L1,L2,L3,L4,ELH,EM,ST,STR,FJL,ISEG,
NSEQ,NUMP)
4 DIMENSION STR(NSEQ,NUMP),FJL(NUMP)
W=0
TOTST=0
J=0
DO 74 I=L1,L3
IF (STR(ISEG,I) .LT. 0) THEN
W=W+ELH
TOTST=TOTST+STR(ISEG,I)
74 CONTINUE
DO 76 I=L3,L4
IF (STR(ISEG,I) .LT. 0) THEN
J=J+1
TOTST=TOTST+STR(ISEG,I)
76 CONTINUE
C
PIND AVERAGE STRESS AND CRITICAL STRESS SCR
IF (J .NE. 0) THEN
SAVE=ABS(TOTST/J)
SCR=0.425*(3.14159**3) 14159)*EM/(12*0.91*(W/ST)**2)
C
IF (SAVE .GT. SCR) THEN
DO 78 I=L1,L2
IF (STR(ISEG,I) .LT. 0 .AND. FJL(I) .NE. 1) FJL(I)=1
DO 80 I=L3,L4
IF (STR(ISEG,I) .LT. 0 .AND. FJL(I) .NE. 1) FJL(I)=1
FLP=PK
ENDIF
80 CONTINUE
RETURN
END
C
SUBROUTINE SKY(ASAT,STIFF,ISTCR,NOP,MDIAG,IND)
C-- CONVERT STIFFNESS MATRIX TO HALF STORAGE AND BANDED MODE --
C DIMENSION ASAT(NOP,NOP),STIFF(ISTCR),MDIAG(NOP*1)
C CONVERT STIFFNESS MATRIX TO HALF STORAGE BANDED MODE - STIFF
C THE MAIN DIAGONAL ADDRESS ARE STORED IN MDIAG ARRAY - MDIAG
C
L=0
DO 70 J=1,NOP
DO 80 I=1,J
IF (ASAT(I,J) .EQ. 0 .AND. I .NE. J) GO TO 80
IF (ASAT(I,J) .EQ. 0 .AND. I .EQ. J) THEN
WRITE(106,*) 'THE STIFFNESS 'I','TH DIAGONAL TERM ARE ZERO'
4 RETURN
ENDIF
N=I
GO TO 100
80 CONTINUE
100 DO 70 I=1,M-1
L=L+1
IF (I .EQ. J) MDIAG(J)=L
IF (ASAT(I,J) .EQ. 0 .AND. I .EQ. J) THEN
WRITE(106,*) 'THE STIFFNESS 'I','TH DIAGONAL TERM ARE ZERO'
4 RETURN
ENDIF
ENDIF
STIFF(L)=ASAT(I,J)
IPT=L
70 CONTINUE
MDIAG(NOP*1)=IPT*1
MDIAG(MDIAG(NOP*1)-1)
C
RETURN
END
C
SUBROUTINE SECRIG(LIEN,ST,TOTA,BJ,LI,N1,L2,N2,B,NUMP)
PIND SECTION RIGIDITY
NOTE:
THE TORSIONAL RIGITY IS ASSUMED ELASTIC CURRENTLY
DIMENSION B(NUMP)
BJ=0.
BJ=0.
IF (LIEN.LE.2) THEN
C FOR SECT 1,2 ELEMENT
DO 48 I=1,NUMP
BJ=BJ+B(I)
48 CONTINUE
BJ=4.*TOTA*TOTA*ST/BJJ
ELSE IF (LIEN.EQ.3) THEN
C FOR SECTION 3 ELEMENT
STT1=1-(B(2)**4/(12*B(1)**4))
BJ=(B(1)*B(2)**3)*(5.3333-(3.36*B(2)/(B(1))))*STT1
ELSE IF (LIEN.EQ.4 OR LIEN.EQ.5) THEN
C FOR SECT 4 ELEMENT
BJ=B(1)
ELSE IF (LIEN.EQ.6) THEN
C FOR SECT 5 ELEMENT
NNUMP=NUMP/2
DO 58 I=1,NNUMP
BJ=BJ+(B(I)*B(1+NNUMP)/2.)
58 CONTINUE
BJ=4.*TOTA*TOTA*ST/BJJ
ELSE IF (LIEN.EQ.7 OR LIEN.EQ.8) THEN
C FOR SECT 6 ELEMENT
STT1=ST/NI
DO 50 I=1,L1
STT1=J.
DO 60 J=1,N1
K=I+(J-1)*L1
60 CONTINUE
BJ=BJ+((B(I)*STT1**3)/3.)
50 CONTINUE
STT2=ST/NI
DO 70 I=(L1*N1)-1,(L1*N1)-L2
STT2=0.
DO 80 J=1,N2
K=I+(J-1)*L2
80 CONTINUE
BJ=BJ+((B(I)*STT2**3)/3.)
70 CONTINUE
ENDIF
RETURN
END
SUBROUTINE ECCENT(NSEQ,UCC,VCC,ISEG,RO2A,RO2B,ECCX,ECCY,ECCC)
CALCULATE MEMBER END ECCENTRICITIES
DIMENSION ECCX(2),ECCY(2),ECCX(2),ECCY(2)
ECCENTRICITIES ABOUT SECTION REFERENCE COORDINATES
IF (ISEG .EQ. 1) THEN
ECCX=UCC-ECC(1)
ECCY=VCC-ECC(1)
ELSE IF (ISEG .EQ. NSEQ) THEN
ECCX=UCC-ECC(2)
ECCY=VCC-ECC(2)
ENDIF
C NOTE: ECCX AND ECCY ARE VECTORS FROM LOAD APPLIED POINT TO CENTROID
IN SECTION REFERENCE COORDINATES DIRECTION
ECC IS THE DISTANCE FROM REFERENCE COORDINATE ORIGIN TO
LOAD APPLIED LOCATION
C ECCENTRICITIES ABOUT MEMBER COORDINATES
IF (ISEG .EQ. 1) THEN
ECCX(1)=-COS(RO2A)*ECCX - SIN(RO2A)*ECCY
ECCY(1)=-SIN(RO2A)*ECCX + COS(RO2A)*ECCY
ELSE IF (ISEG .EQ. NSEQ) THEN
ECCX(2)=-COS(RO2B)*ECCX - SIN(RO2B)*ECCY
ECCY(2)=-SIN(RO2B)*ECCX + COS(RO2B)*ECCY
ENDIF
C NOTE: ECCX AND ECCY ARE VECTORS FROM LOAD APPLIED POINT TO CENTROID
IN MEMBER COORDINATES DIRECTION
RETURN
END
SUBROUTINE ELESTI(NSEQ,ISEG,ISTIP,EA,EIU,EIV,GKT,EL,FLOC,PSK,SK)
CALCULATE STABILITY ELEMENT STIFFNESS
DIMENSION EL(NSEQ),FLOC(NSEQ,12),PSK(NSEQ,12,12),SK(12,12)
C INITIALIZE THE SEGMENT STIFFNESS
AUN=100.
DO 430 I=1,12
DO 430 J=1,12
SK(I,J)=0.
430 CONTINUE
C FORMULATE THE ELEMENT STIFFNESS MATRIX
P=FLOC(ISEG,3)
COP=9/16.*EL(ISEG)
IF (ISTIP .EQ. 1) THEN
SK(1,1)=12.*EIV/(EL(ISEG)**3).COP*12.
SK(2,2)=12.*EIV/(EL(ISEG)**3).COP*12.
SK(3,3)=EA/EL(ISEG)
SK(4,4)=4.*EIU/EL(ISEG).COP*(4.*EL(ISEG)*EL(ISEG)/3.)
SK(5,5)=4.*EIV/EL(ISEG).COP*(4.*EL(ISEG)*EL(ISEG)/3.)
SK(6,6)=GKT/EL(ISEG)
SK(5,6)=4.*EIV/EL(ISEG)**2).COP*EL(ISEG)
SK(2,4)=-6.*EIU/(EL(ISEG)**2).COP*(EL(ISEG))
SK(7,7)=SK(1,1)
SK(8,8)=SK(2,2)
SK(9,9)=SK(3,3)
SK(10,10)=SK(4,4)
SK(11,11)=SK(5,5)
SK(12,12)=SK(6,6)
SK(7,11)=SK(11,7)
SK(8,12)=SK(12,8)
SK(9,11)=SK(11,9)
SK(10,12)=SK(12,10)
SK(4,10)=0.5*SK(4,4).COP*(EL(ISEG)*EL(ISEG)/3.)
SK(5,11)=0.5*SK(5,5).COP*(EL(ISEG)*EL(ISEG)/3.)
SK(6,12)=SK(6,6)
SK(1,11)=SK(11,1)
SK(2,10)=SK(10,2)
SK(1,7)=SK(7,1)
SK(2,8)=SK(8,2)
SK(3,9)=SK(9,3)
SK(4,10)=0.5*SK(4,4).COP*(EL(ISEG)*EL(ISEG)/3.)
SK(5,11)=0.5*SK(5,5).COP*(EL(ISEG)*EL(ISEG)/3.)
SK(6,12)=SK(6,6)
SK(1,11)=SK(11,1)
SK(2,10)=SK(10,2)
SK(4,8)=SK(8,4)
SK(5,9)=SK(9,5)
SK(1,7)=SK(7,1)
ELSE IF (IP .GT. 1) THEN
SKU=SQRT(P/EIU)
CU=COS(SKU*EL(ISEG))*AUN
SU=SIN(SKU*EL(ISEG))*AUN
SKV=SKU*P/EIU
CV=COS(SKV*EL(ISEG))*AUN
SV=SIN(SKV*EL(ISEG))*AUN

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SK(1,1)-EIV*((SKV**3)*SV/(2.*AUN-2.*CV*SKV*EL(ISEGG)*SV))
SK(2,2)-EIV*((SKU**3)*SU/(2.*AUN-2.*CV*SKU*EL(ISEGG)*SU))
SK(3,3)-EA/EL(ISEGG)
SK(4,4)-EIV*SKU*((SU*SKU*EL(ISEGG)*CU)/
  (2.*AUN-2.*CV*SKU*EL(ISEGG)*SU))
SK(5,5)-EIV*SKV*((SV*SKV*EL(ISEGG)*CV)/
  (2.*AUN-2.*CV*SKV*EL(ISEGG)*SV))
SK(6,6)-GKT/EL(ISEGG)
SK(1,5)-EIV*((SKV**3)*SV/(2.*AUN-2.*CV*SKV*EL(ISEGG)*SV))
SK(5,1)-EIV*((SKU**3)*SU/(2.*AUN-2.*CV*SKU*EL(ISEGG)*SU))
SK(7,7)-SK(1,1)
SK(8,8)-SK(2,2)
SK(9,9)-SK(3,3)
SK(10,10)-SK(4,4)
SK(11,11)-SK(5,5)
SK(12,12)-SK(6,6)
SK(7,11)-SK(1,5)
SK(8,10)-SK(2,4)
SK(1,7)-SK(1,1)
SK(2,8)-SK(2,2)
SK(3,9)-SK(3,3)
SK(4,10)-EIV*SKU*((SU*SKU*EL(ISEGG)*AUN)/
  (2.*AUN-2.*CV*SKU*EL(ISEGG)*SU))
SK(5,11)-EIV*SKV*((SV*SKV*EL(ISEGG)*AUN)/
  (2.*AUN-2.*CV*SKV*EL(ISEGG)*SV))
SK(6,12)-SK(6,6)
SK(1,11)-SK(1,5)
SK(2,10)-SK(2,4)
SK(4,8)-SK(2,10)
SK(5,7)-SK(1,11)
ELSE IF (LT .1.) THEN
  SKU=SQRT(-E/EL(I))
  CU=COSH(SKU*EL(ISEGG))*AUN
  SU=SINH(SKU*EL(ISEGG))*AUN
  SKV=SQRT(-P/E/IV)
  CV=COSH(SKV*EL(ISEGG))*AUN
  SV=SINH(SKV*EL(ISEGG))*AUN
  SK(1,1)-EIV*((SKV**3)*SV/(2.*AUN-2.*CV*SKV*EL(ISEGG)*SV))
  SK(2,2)-EIV*((SKU**3)*SU/(2.*AUN-2.*CV*SKU*EL(ISEGG)*SU))
  SK(3,3)-EA/EL(ISEGG)
  SK(4,4)-EIV*SKU*((SU*SKU*EL(ISEGG)*CU)/
    (2.*AUN-2.*CV*SKU*EL(ISEGG)*SU))
  SK(5,5)-EIV*SKV*((SV*SKV*EL(ISEGG)*CV)/
    (2.*AUN-2.*CV*SKV*EL(ISEGG)*SV))
  SK(6,6)-GKT/EL(ISEGG)
  SK(1,5)-EIV*((SKV**3)*SV/(2.*AUN-2.*CV*SKV*EL(ISEGG)*SV))
  SK(5,1)-EIV*((SKU**3)*SU/(2.*AUN-2.*CV*SKU*EL(ISEGG)*SU))
  SK(7,7)-SK(1,1)
  SK(8,8)-SK(2,2)
  SK(9,9)-SK(3,3)
  SK(10,10)-SK(4,4)
  SK(11,11)-SK(5,5)
  SK(12,12)-SK(6,6)
  SK(7,11)-SK(1,5)
  SK(8,10)-SK(2,4)
  SK(1,7)-SK(1,1)
  SK(2,8)-SK(2,2)
  SK(3,9)-SK(3,3)
  SK(4,10)-EIV*SKU*((SU*SKU*EL(ISEGG)*AUN)/
    (2.*AUN-2.*CV*SKU*EL(ISEGG)*SU))
  SK(5,11)-EIV*SKV*((SV*SKV*EL(ISEGG)*AUN)/
    (2.*AUN-2.*CV*SKV*EL(ISEGG)*SV))
  SK(6,12)-SK(6,6)
  SK(1,11)-SK(1,5)
  SK(2,10)-SK(2,4)
  SK(4,8)-SK(2,10)
  SK(5,7)-SK(1,11)
ENDIF
DO 90 I=1,12
  I1=I-1
  DO 90 J=I,12
    SK(J,I)-SK(I,J)
  90 CONTINUE
MEMORIZE THE ELEMENT STIFFNESS FOR BEING USED IN SUBROUTINE
C CLEAR IN THE NEXT STEP
DO 190 II=1,12
  DO 190 JJ=1,12
    PFK(ISEGG,II,JJ)-SK(II,JJ)
190 CONTINUE
RETURN
END
SUBROUTINE FRANK (PFASAT, ECCX, ECCY, TPJT, ASATP)
TRANSFER MEMBER STIFFNESS MATRIX INTO ECCENTRICITY LOAD
APPLY DIRECTIO
DIMENSION ECCX(2), ECCY(2), TPJT(12,12), PFASAT(12,12), ASATP(12,12)
DO 250 I=1,12
  DO 250 J=I,12
    TPJT(I,J)=0.
    IF (I.EQ.J) TPJT(I,J)=1.
250 CONTINUE
TPJT(1,4)=ECCY(1)
TPJT(3,5)=ECCX(1)
TPJT(1,6)=ECCY(1)
TPJT(12,6)=ECCX(1)
TPJT(9,10)=ECCY(2)
TPJT(9,11)=ECCX(2)
TPJT(7,12)=ECCY(2)
TPJT(8,12)=ECCX(2)
EASATP=TPJT
CALL MULTM(0,12,ASATP,PFASAT,TPJT)
CALL MULTM(1,12,PFASAT,TPJT,ASATP)
RETURN
END
SUBROUTINE SKYLIN (JFLAG, IOPT, NDOF, IBUG, TITLE, MDIAG,
  IELDOF, LM, SE, MD, NAME)
DIMENSION MDIAG(NDOF+1), SE(1:IELDOF+1), LM(1:IELDOF)
LOGICAL ITEST, JT
CHARACTER*(*) NAME, TITLE(2)
CHARACTER*1 CX(100)
IF (IOPT.EQ.1) THEN
  DO 10 I=1, NDOF+1
    MDIAG(I)=I
    IF (ISTEST(1BUG,12)) THEN
      WRITE(108,30) I, MDIAG(I), I=1, NDOF
    ENDIF
  10 CONTINUE
ELSE IF (IOPT.EQ.2) THEN
  ***** DETERMINE THE SKYLINE OF THIS ELEMENT, AND MODIFY THE
  ***** GLOBAL SKYLINE.
  ***** I = THE ROW OF THE MEMBER STIFFNESS MATRIX
  ***** J = THE COLUMN OF THE MEMBER STIFFNESS MATRIX
  ***** I1 = THE ADDRESS OF THE MEMBER STIFFNESS TERM I,J
  ***** LMJ = THE ROW OF THE GLOBAL STIFFNESS MATRIX CORRESPONDING
  ***** TO ROW I OF THE MEMBER STIFFNESS MATRIX
  ***** LMJ = THE COLUMN OF THE GLOBAL STIFFNESS MATRIX CORRESPONDING
  ***** TO COLUMN J OF THE MEMBER STIFFNESS MATRIX
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MDIAG(JJ) = THE JJ'TH ELEMENT OF THE GLOBAL SKYLINE MATRIX.
AT THIS POINT THE GLOBAL SKYLINE MATRIX CONTAINS THE
LOWEST ROW NUMBER THAT CONTAINS A NON-ZERO STIFFNESS
TERM.
DO 250 I=1, IELDOF
  LMJ=LM(I)
  IF (LMJ.LE.0) GO TO 250
  DO 240 J=1, IELDOF
    LMJ=LM(J)
    IF (LMJ.LE.0) GO TO 240
    IJ=MD(J)+J-1
    IF (SE(I1).EQ.0 AND JFLAG.EQ.0) GO TO 240
    IF (SE(I1).EQ.0) GO TO 240
    IF (LMJ.LE.LMJ) THEN
      MDIAG(LMJ)=MIN(MDIAG(LMJ), LMJ)
    ELSE
      MDIAG(LMJ)=MIN(MDIAG(LMJ), LMJ)
    ENDIF
  240 CONTINUE
  250 CONTINUE
ELSE IF (IOPT.EQ.3) THEN
  ***** CALCULATE THE INDICES OF THE MAIN DIAGONAL TERMS.
  IF (BTEST(1BUG,12)) THEN
    WRITE(108,*) 'AFTER COLUMN HEIGHTS'
    WRITE(108,320) (I, MDIAG(I), I=1, NDOF)
  ENDIF
  MDIAG(1)=1
  ICOLMT=1
  DO 310 IROW=2, (NDOF+1)
    ILOCN=MDIAG(IROW)-MDIAG(IROW-1)
    ICOLMT=IROW-MDIAG(IROW-1)
  310 MDIAG(IROW)=ILOCN
  IF (BTEST(1BUG,12)) THEN
    WRITE(108,*) 'ADDRESS OF MAIN DIAGONAL ELEMENTS'
    WRITE(108,320) (I, MDIAG(I), I=1, NDOF)
  320 FORMAT (' MDIAG(', I3, ')=', I5)
  321 FORMAT (216)
  IF (.NOT.(BTEST(1BUG,12) OR BTEST(1BUG,9))) GO TO 370
  ***** PRINT A MAP OF THE STIFFNESS ARRAY
  NP=NDOF/100
  NR=MD(NDOF,100)
  IF (NR.GT.0) NP=NP+1
  DO 270 IP=1, NP
    JCI=(IP-1)*100
    JCI2=MIN(IP*100, NDOF)
    JI=100
    IF (IP.EQ.NP) JI=NR
    WRITE(108,290) TITLE(1), TITLE(2), NAME, NL, (I, I=JCI, JCI2, 10)
    DO 270 I=1, NDOF
      PT=FALSE
      DO 260 J=I-1, JI
        J=J-1
        IJ=MDIAG(J)+J-1
        IF (IJ.LT.MDIAG(J-1) AND .I.LT.J) THEN
          CX(JI)=I
          PT=TRUE
        ELSE IF (I.EQ.J) THEN
          CX(JI)=D
          PT=TRUE
        ELSE
          CX(JI)=*
        ENDIF
      260 CONTINUE
    270 IF (PT) WRITE(108,395) I, (CX(J), J=1, JI)
  290 FORMAT (' STRUCTURE ..... A, //
    & SOLUTION ..... A, //
    & SKYLINE OF THE A, // MATRIX ', //
    & ' THE MATRIX REQUIRES', I6, ' STORAGE LOCATIONS', //
    & '/X, I2(I4, I1, I1, I1)')
  295 FORMAT (16, I32A1)
ENDIF
RETURN
END
SUBROUTINE NDDOP (NDOF, NCON, NPRE, NPOST, MAXNOD, NNODE, NCOB,
  SCALE, NSUP, NCONST, ID, IDOP, INDEX, COORD, COSINE, CONST,
  JTLG, JTCCB, BUG, MSTRUT)
DIMENSION COORD (MAXNOD, 3), ID (MAXNOD), INDEX (MAXNOD), IPM(6), JPM(6),
  IDOP (MAXNOD, 6), COSINE (3, 3, NCOB), CONST (MAXNOD, 6),
  & JTLG (MAXNOD), JTCCB (MAXNOD), MSTRUT (MAXNOD, 6)
DIMENSION VN(3), VY(3), VZ(3)
LOGICAL FLAG, PR, BUG, TEST, BTEST
CHARACTER*15 TYPE
CHARACTER*2 CX(6)
C VARIABLE TABLE
C COORD = JOINT COORDINATES X,Y,Z
C ID = JOINT NUMBER
C IDOP = GLOBAL DEGREES OF FREEDOM ASSOC. WITH JOINT FX, FY, FZ, MX, MY, MZ
C JTCCB = ROT. IN COSINE MATRIX ASSOC WITH JOINT
C COSINE = COSINE MATRIX
C CONST = GLOBAL CONSTRAINT MATRIX FOR EACH NODE
C JTLG = CONTAINS JOINT FLAG'S -- MUST BE 24 BIT'S LONG
C BIT'S 6 TO 8 RESTRAIN DOP FX, FY, FZ, MX, MY, MZ
C BIT'S 9 TO 11 CONDENSE DOP FX, FY, FZ, MX, MY, MZ
C BIT'S 12 TO 17 CONSTRAIN DOP FX, FY, FZ, MX, MY, MZ
C BIT'S 18 TO 23 ELIMINATE DOP FX, FY, FZ, MX, MY, MZ
C MSTRUT = INTERNAL MASTER JT # OF CONSTRAINED JOINTS.
INPUT NODES
IF (BUG) WRITE(108,1000) 'INPUT NODES'
I=0
DO I=1, I
  READ(105,*) ID(I), (COORD(I,J), J=1,3), JTCCB(I), IGEN
  IF (ID(I).LE.0) THEN
    NNODE=I-1
    GO TO 26
  ENDIF
  IF (BUG) WRITE(108,30) I, ID(I), (COORD(I,J), J=1,3), JTCCB(I)
  IF (IGEN.GT.0) THEN
    READ(105,*) IDINC, COORD1, COORD2, COORD3
    DO 25 K=1, IGEN
      I1=I
      ID(I)=ID(I-1)+IDINC
      IF (ID(I).LE.0) THEN
        NNODE=I-1
        GO TO 26
      ENDIF
      COORD(1,1)=COORD(I-1,1)+COORD1
      COORD(1,2)=COORD(I-1,2)+COORD2
      COORD(1,3)=COORD(I-1,3)+COORD3
      JTCCB(I)=JTCCB(I-1)
    25 CONTINUE
  26 CONTINUE
SORT NODES
CALL ISORT (ID, INDEX, NNODE)
MOVE NODE INFO. TO TEMPORARY STORAGE
DO 40 I=1, NNODE
  J=INDEX(I)
  IDOP(I,1)=ID(J)
  NDDO010
  NDDO020
  NDDO030
  NDDO040
  NDDO050
  NDDO060
  NDDO070
  NDDO080
  NDDO090
  NDDO100
  NDDO110
  NDDO120
  NDDO130
  NDDO140
  NDDO150
  NDDO160
  NDDO170
  NDDO180
  NDDO190
  NDDO200
  NDDO210
  NDDO220
  NDDO230
  NDDO240
  NDDO250
  NDDO260
  NDDO270
  NDDO280
  NDDO290
  NDDO300
  NDDO310
  NDDO320
  NDDO330
  NDDO340
  NDDO350
  NDDO360
  NDDO370
  NDDO380
  NDDO390
  NDDO400
  NDDO410
  NDDO420
  NDDO430
  NDDO440
  NDDO450
  NDDO460
  NDDO470
  NDDO480
  NDDO490
  NDDO500
  NDDO510
  NDDO520
  NDDO530
  NDDO540
  NDDO550
  NDDO560
  NDDO570
  NDDO580
  NDDO590
  NDDO600
  NDDO610
  NDDO620
  NDDO630
  NDDO640
  NDDO650
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C----- IDOP(I,2)-JTCOS(I)
C--- CORRECT BY GEN. JENCO-PW JTCOS(I) REPLACED BY JTCOS(J) ---
C----- IDOP(I,2)-JTCOS(J)
DO 40 K=1,3
40 CONST(I,K)-COORD(J,K)*SCALE
C
C----- MOVE NODES INPO BACK TO MAIN STORAGE, DELETE DUPLICATE INFO.
NNOCD-NNODE
I=0
J=0
43 I=1
J=1
ID(I)-IDOP(J,1)
IF (I,GE,2) THEN
  IF (ID(I).EQ.ID(I-1)) THEN
    I=I-1
    NNODE-NNODE-1
  ENDIF
ENDIF
NNOCD-NNODE
44 COORD(I,K)-CONST(J,K)
IF (I.LT.NNODE) GO TO 43
C
30 FORMAT(' NODE ',I3,',',I4,',',I5,' X:',F10.4,' Y:',F10.4,' Z:',F10.4,
  ',',I5,' COS=',I3)
C----- GLOBAL JOINT COSINE MATRIX
IF (BUG) WRITE(108,1000) 'DIRECTION COSINES ... '
DO 55 ICOS=1,NCOS
  READ(105,*) VX,VY
  CALL CROSS(VZ,VX,VY,0)
  CALL CROSS(VY,VZ,VX,1)
  DO 50 J=1,3
    COSINE(I,J,ICOS)=VX(J)
    COSINE(I,J,ICOS)=VY(J)
    COSINE(I,J,ICOS)=VZ(J)
50 IF (BUG) WRITE(108,56) ICOS, 'X:',VX,' Y:',VY,' Z:',VZ
55 CONTINUE
56 FORMAT(' COS=',I3,',',
  ' A:',F9.5,' B:',F9.5,' C:',F9.5,' K ',I,8S)
C----- INPUT RESTRAINTS
ZERO IDOP MATRIX WHICH WILL HOLD RESTRAINT, CONSTRAINT AND
ULTIMATELY DOP INFORMATION
DO 59 I=1,NNODE
59 JPLG(I)=0
I=0
FLAG=TRUE
60 I=1
IF (I.LE.NSUPT) THEN
  IF (FLAG.AND.BUG) WRITE(108,1000) ' NODE RESTRAINTS '
  FLAG=FALSE
  READ(105,*) NODE,IPM,IGEN,NCODINC
  IF (NODE.EQ.0) THEN
    DO 70 J=1,NNODE
      IF (IPM(I),NE.0) JPLG(J)+IBSET(JPLG(J),I-1)
      IF (IPM(I),EQ.2) JPLG(J)+IBSET(JPLG(J),I+1)
    CONTINUE
68 IF (BUG) WRITE(108,80) ID(J),IPM
70 ELSE
  J=IQUICK(NODE,ID,NNODE)
  IF (J.EQ.0) THEN
    IF (BUG) WRITE(108,75) NODE
  ELSE
    DO 71 II=1,6
      IF (IPM(II),NE.0) JPLG(J)+IBSET(JPLG(J),II-1)
      IF (IPM(II),EQ.2) JPLG(J)+IBSET(JPLG(J),II+1)
    CONTINUE
71 IF (BUG) WRITE(108,80) ID(J),IPM
  ENDF
  IF (IGEN.GT.0) THEN
    NCDE=NODE+NCODINC
    IGEN=IGEN-1
    GO TO 65
  ENDF
  ENDF
  GO TO 60
75 FORMAT(' NODE ',I5,' NOT FOUND, RESTRAINT IGNORED...')
80 FORMAT(' NODE ',I4,',',I5,' PX:',I2,' PY:',I2,' PZ:',I2,
  ',',I5,' MX:',I2,' MY:',I2,' MZ:',I2)
C----- INPUT NODE DOP'S TO BE CONDENSED OUT...
I=0
FLAG=TRUE
90 I=1
IF (I.LE.NCOND) THEN
  IF (FLAG.AND.BUG)
    WRITE(108,1000) ' NODE DOP'S TO BE CONDENSED OUT '
  FLAG=FALSE
  READ(105,*) NODE,IPM,IGEN,NCODINC
  IF (NODE.EQ.0) THEN
    DO 100 J=1,NNODE
      IF (IPM(II),NE.0.AND. NOT.BTEST(JPLG(J),II-1) THEN
        JPLG(J)+IBSET(JPLG(J),II+5)
        JPM(II)=IPM(II)
      ELSE
        JPM(II)=0
      ENDF
    CONTINUE
99 IF (PT.AND.BUG) WRITE(108,80) ID(J),JPM
100 ELSE
  J=IQUICK(NODE,ID,NNODE)
  IF (J.EQ.0) THEN
    IF (BUG) WRITE(108,110) NODE
  ELSE
    DO 101 II=1,6
      IF (IPM(II),NE.0.AND. NOT.BTEST(JPLG(J),II-1) THEN
        JPLG(J)+IBSET(JPLG(J),II-5)
        JPM(II)=IPM(II)
      ELSE
        JPM(II)=0
      ENDF
    CONTINUE
101 IF (PT.AND.BUG) WRITE(108,80) ID(J),JPM
  ENDF
  IF (IGEN.GT.0) THEN
    NCDE=NODE+NCODINC
    IGEN=IGEN-1
    GO TO 95
  ENDF
  ENDF
  GO TO 90
110 FORMAT(' NODE ',I5,' NOT FOUND, DOP ARE NOT CONDENSED...')
C
C----- INPUT CONSTRAINTS...
I=0
FLAG=TRUE
400 I=1
IF (I.LE.NCONST) THEN
  IF (FLAG) WRITE(108,1000) ' NODE CONSTRAINTS '
  FLAG=FALSE
  READ(105,*) ITYPE,MASTER,NODE,IGEN,NCODINC
  J=IQUICK(NODE,ID,NNODE)
  IF (J.EQ.0) THEN
    WRITE(108,420) NODE
    NODD0660
    NODD0670
    NODD0680
    NODD0690
    NODD0700
    NODD0710
    NODD0720
    NODD0730
    NODD0740
    NODD0750
    NODD0760
    NODD0770
    NODD0780
    NODD0790
    NODD0800
    NODD0810
    NODD0820
    NODD0830
    NODD0840
    NODD0850
    NODD0860
    NODD0870
    NODD0880
    NODD0890
    NODD0900
    NODD0910
    NODD0920
    NODD0930
    NODD0940
    NODD0950
    NODD0960
    NODD0970
    NODD0980
    NODD0990
    NODD1000
    NODD1010
    NODD1020
    NODD1030
    NODD1040
    NODD1050
    NODD1060
    NODD1070
    NODD1080
    NODD1090
    NODD1100
    NODD1110
    NODD1120
    NODD1130
    NODD1140
    NODD1150
    NODD1160
    NODD1170
    NODD1180
    NODD1190
    NODD1200
    NODD1210
    NODD1220
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    NODD1470
    NODD1480
    NODD1490
    NODD1500
    NODD1510
    NODD1520
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    NODD1540
    NODD1550
    NODD1560
    NODD1570
    NODD1580
    NODD1590
    NODD1600
    NODD1610
    NODD1620
    NODD1630
    NODD1640
    NODD1650
    NODD1660
    NODD1670
    NODD1680
    NODD1690
    NODD1700
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    NODD1760
    NODD1770
    NODD1780
    NODD1790
    NODD1800
    NODD1810
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    NODD1880
    NODD1890
    NODD1900
    NODD1910
    NODD1920
    NODD1930
    NODD1940
    NODD1950
    NODD1960
    NODD1970
    NODD1980
    NODD1990
    NODD2000
    NODD2010
    NODD2020
    NODD2030
    NODD2040
    NODD2050
    NODD2060
    NODD2070
    NODD2080
    NODD2090
    NODD2100
    NODD2110
    NODD2120
    ELSE
      IF (ITYPE.EQ.0) THEN
        TYPE='3D RIGID BODY'
        DO 415 K=1,6
          IF (.NOT.BTEST(JPLG(J),K-1)) THEN
            JPLG(J)+IBSET(JPLG(J),K-1)
            MSTRJT(J,K)=IQUICK(MASTER,ID,NNODE)
          ELSE
            WRITE(108,430) NODE,K
          ENDF
        CONTINUE
        ELSE IF (ITYPE.EQ.1) THEN
          TYPE='XY-PLANE'
          K=1
          IF (.NOT.BTEST(JPLG(J),K-1)) THEN
            JPLG(J)+IBSET(JPLG(J),K-1)
            MSTRJT(J,K)=IQUICK(MASTER,ID,NNODE)
          ELSE
            WRITE(108,430) NODE,K
          ENDF
          K=2
          IF (.NOT.BTEST(JPLG(J),K-1)) THEN
            JPLG(J)+IBSET(JPLG(J),K-1)
            MSTRJT(J,K)=IQUICK(MASTER,ID,NNODE)
          ELSE
            WRITE(108,430) NODE,K
          ENDF
          K=6
          IF (.NOT.BTEST(JPLG(J),K-1)) THEN
            JPLG(J)+IBSET(JPLG(J),K-1)
            MSTRJT(J,K)=IQUICK(MASTER,ID,NNODE)
          ELSE
            WRITE(108,430) NODE,K
          ENDF
          ELSE
            WRITE(108,440) ITYPE,MASTER,NODE
          ENDF
          WRITE(108,450) TYPE,MASTER,NODE
          ENDF
          IF (IGEN.GT.0) THEN
            NCDE=NODE+NCODINC
            IGEN=IGEN-1
            GO TO 410
          ENDF
          GO TO 400
        ENDF
        415 FORMAT(' NODE ',I6,' IS NOT FOUND, CONSTRAINT IS IGNORED')
        420 FORMAT(' NODE ',I6,' IS RESTRAINED, CONSTRAINT ',I2,
          '&' ' IS IGNORED...')
        440 FORMAT(' INVALID CONSTRAINT TYPE ',I3,
          '&' ' MASTER ',I6,' SLAVE ',I6)
        450 FORMAT('X,A,'CONSTRAINT,','
          '&' ' MASTER ',I6,' SLAVE ',I6)
      C
      IF (BUG) WRITE(108,1000) 'JOINT RESTRAINT CODES...'
      IF (BUG) CALL PBRIT(NNODE,JPLG,IGEN)
    C----- DETERMINE THE DEGREES OF FREEDOM...
    C----- ZERO CONSTRAINT MATRIX WHICH WILL HOLD THE MOMENT
    C----- TRANSFORMATION FOR CONSTRAINED DOP
    DO 499 J=1,6
      499 CONST(I,J)=0
    NDOF=0
    C----- DEGREES OF FREEDOM TO BE CONDENSED OUT
    DO 500 I=1,NNODE
      DO 500 J=1,6
        IF (BTEST(JT,J,5) .AND. NOT.BTEST(JT,J,11)) THEN
          NDOF=NDOF+1
          IDOP(I,J)=NDOF
        ENDF
      500 CONTINUE
      NCOND=NDOF
    C----- UNCONSTRAINED AND UNRESTRAINED DEGREES OF FREEDOM
    DO 510 I=1,NNODE
      DO 510 J=1,6
        NDE=IDOP(I,J)
        IF (.NOT.BTEST(JPLG(I),J-1) .OR. BTEST(JPLG(I),J+5) .OR.
          & BTEST(JPLG(I),J+11)) THEN
          C
          IF (IDOP(I,J).EQ.0) THEN
            NDOF=NDOF+1
            IDOP(I,J)=NDOF
          ENDF
          510 CONTINUE
          NFREE=NDOF-NCOND
        C----- RESTRAINED DEGREES OF FREEDOM
        DO 530 I=1,NNODE
          IF (BTEST(JPLG(I),J-1) .AND. NOT.BTEST(JPLG(I),J+1) THEN
            NDOF=NDOF+1
            IDOP(I,J)=NDOF
          ENDF
          530 CONTINUE
          NREST=NDOF-NFREE-NCOND
        C----- UNCONSTRAINED DEGREES OF FREEDOM
        DO 535 I=1,NNODE
          IF (BTEST(JTPLG(I),J-1) .AND. NOT.BTEST(JTPLG(I),J+1) THEN
            NDOF=NDOF+1
            IDOP(I,J)=NDOF
          ENDF
          535 CONTINUE
          NREST=NDOF-NFREE-NCOND
        C----- UNCONSTRAINED DEGREES OF FREEDOM
        I=NODE ID OF SLAVE NODE
        II=NODE ID OF MASTER NODE
        DO 520 I=1,NNODE
          DO 520 J=1,6
            ... CHECK DOP FOR CONSTRAINT ...
            IF (BTEST(JTPLG(I),J-1)) THEN
              ... DETERMINE MASTER NODE AND DOP # ...
              I=I+1
              NDE=ID(II)
              II=MSTRJT(II,J)
              IF (II.EQ.0) THEN
                WRITE(108,525) NODE,II,I,J
                IDOP(I,J)=0
                GO TO 520
              ENDF
              IF (BTEST(JTPLG(II),J-1)) GO TO 515
              IDOP(II,J)=IDOP(I,J)
            C----- CHECK FOR DIFFERENT JOINT COORDINATE SYSTEMS ...
            IF (JTCOS(I) NE JTCOS(II)) THEN
              WRITE(108,411) ID(I),ID(II)
              FORMAT(' CONFLICTING COSINE ID NUMBERS, MASTER ',
                I6,' SLAVE ',I6,' CORRECT INPUT & RETURN...')
              ENDF
            ... CALCULATE THE CONSTRAINT MATRIX ...
            IF (J.GE.4) THEN
              XG=COORD(I,1)-COORD(II,1)
              YG=COORD(I,2)-COORD(II,2)
              ZG=COORD(I,3)-COORD(II,3)
              N=JTCOS(I)
              XNS=COSINE(1,1,N)*XG-COSINE(1,2,N)*YG-COSINE(1,3,N)*ZG
            NODD2130
            NODD2140
            NODD2150
            NODD2160
            NODD2170
            NODD2180
            NODD2190
            NODD2200
            NODD2210
            NODD2220
            NODD2230
            NODD2240
            NODD2250
            NODD2260
            NODD2270
            NODD2280
            NODD2290
            NODD2300
            NODD2310
            NODD2320
            NODD2330
            NODD2340
            NODD2350
            NODD2360
            NODD2370
            NODD2380
            NODD2390
            NODD2400
            NODD2410
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            NODD2430
            NODD2440
            NODD2450
            NODD2460
            NODD2470
            NODD2480
            NODD2490
            NODD2500
            NODD2510
            NODD2520
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            NODD2770
            NODD2780
            NODD2790
            NODD2800
            NODD2810
            NODD2820
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            NODD2930
            NODD2940
            NODD2950
            NODD2960
            NODD2970
            NODD2980
            NODD2990
            NODD3000
            NODD3010
            NODD3020
            NODD3030
            NODD3040
            NODD3050
            NODD3060
            NODD3070
            NODD3080
            NODD3090
            NODD3100
            NODD3110
            NODD3120
            NODD3130
            NODD3140
            NODD3150
            NODD3160
            NODD3170
            NODD3180
            NODD3190
            NODD3200
            NODD3210
            NODD3220
            NODD3230
            NODD3240
            NODD3250
            NODD3260
            NODD3270
            NODD3280
            NODD3290
            NODD3300
            NODD3310
            NODD3320
            NODD3330
            NODD3340
            NODD3350
            NODD3360
            NODD3370
            NODD3380
            NODD3390
            NODD3400
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            NODD3440
            NODD3450
            NODD3460
            NODD3470
            NODD3480
            NODD3490
            NODD3500
            NODD3510
            NODD3520
            NODD3530
            NODD3540
            NODD3550
            NODD3560
            NODD3570
```



```

VMS-COSINE(2,1,N)*XG-COSINE(2,2,N)*YG-COSINE(2,3,N)*ZG
ZMS-COSINE(3,1,N)*XG-COSINE(3,2,N)*YG-COSINE(3,3,N)*ZG
ENDIF
IF (J.EQ.4) THEN
  CONST(1,1) = ZMS
  CONST(1,2) = VMS
ELSE IF (J.EQ.5) THEN
  CONST(1,3) = ZMS
  CONST(1,4) = VMS
ELSE IF (J.EQ.6) THEN
  CONST(1,5) = VMS
  CONST(1,6) = XMS
ENDIF
520 CONTINUE
525 FORMAT (' NODE#',I6,' NOT FOUND WHILE ASSIGNING CONSTRAINED',
  ' A * DROP TO NODE#',I6,' DOF#',I2,' ... DOF IS DELETED ')
C
PRINT OUT ORDERED NODE INFO. AND DEGREES OF FREEDOM
WRITE(108,1000) 'NODE COORDINATES AND DEGREES OF FREEDOM'
WRITE(108,600) 'DOF, NCOND, NFREE, NREST'
600 FORMAT
  & (4X,'TOTAL NUMBER OF DEGREES OF FREEDOM',I6,/,
  & 4X,'NUMBER OF DEGREES OF FREEDOM CONDENSED OUT',I6,/,
  & 4X,'NUMBER OF FREE DEGREES OF FREEDOM',I6,/,
  & 4X,'NUMBER OF CONSTRAINED DEGREES OF FREEDOM',I6,/,
  & 4X,' NODE COOR',4X,'X COORD',8X,'Y COORD',8X,'Z COORD',4X,/,
  & 4X,'FX',6X,'FY',6X,'FZ',6X,'MX',6X,'MY',6X,'MZ')
DO 610 I=1,NNODE
  DO 605 J=1,6
    IF (BTST(JTFLG(I),J-1)) THEN
      CX(J) = 'R'
    ELSE IF (BTST(JTFLG(I),J-1)) THEN
      CX(J) = 'C'
    ELSE
      CX(J) = ' '
    ENDIF
605 CONTINUE
610 WRITE(108,620) ID(I),JTCS(I),COORD(I,J),J-1,3,
  & (IDOP(I,X),CX(X),K-1,6)
620 PCMAT(4X,I6,I6,15,3)G15 9,CP,6(I6,A2)
WRITE(108,525)
625 FORMAT(10X,'NOTE: R - RESTRAINED DEGREE OF FREEDOM',
  10X,' C - CONSTRAINED DEGREE OF FREEDOM')
C
PRINT OUT CONSTRAINT MATRIX
IF (BUG) THEN
  FLAG = TRUE
  DO 540 I=1,NNODE
    TEST = FALSE
    DO 536 J=1,6
      TEST = TEST .OR. BTST(JTFLG(I),J-1)
    IF (TEST) THEN
      IF (FLAG) WRITE(108,1000) 'NODE CONSTRAINT MATRIX'
      FLAG = FALSE
      WRITE(108,550) ID(I),CONST(I,J),J-1,6)
    ENDIF
540 CONTINUE
550 FORMAT(4X,I6,I6,/,
  & ' T12:',G12.5,' T13:',G12.5,/,
  & ' T21:',G12.5,' T23:',G12.5,/,
  & ' T31:',G12.5,' T32:',G12.5)
ENDIF
C
PRINT OUT DIRECTION COSINES
WRITE(108,1000) 'DIRECTION COSINES ...'
DO 560 ICOS=1,NCOS
  WRITE(108,56) ICOS,' VX:',(COSINE(1,J,ICOS),J=1,3),/,
  & ' VY:',(COSINE(2,J,ICOS),J=1,3),/,
  & ' VZ:',(COSINE(3,J,ICOS),J=1,3)
560 CONTINUE
RETURN
1000 PCMAT (//,5X,A)
END
-----
SUBROUTINE PBIT(NNODE,JTFLG,ID)
DIMENSION JTFLG(NNODE),ID(NNODE)
LOGICAL FLAG(0:31),BTST
WRITE(108,40)
40 FORMAT (4X,' RESTRAINT', ' CONDENSE', ' CONSTRAIN', ' ELIMINATE',/,
  & 14X,'(4X,'PPPPM')',(4X,'XYXZY2'))
DO 20 I=1,NNODE
  DO 10 J=0,31
    FLAG(J) = BTST(JTFLG(I),J)
10 WRITE(108,30) ID(I),FLAG(K),K=0,31,JTFLG(I)
30 PCMAT (' NODE',I4,')',4(4X,6L1),110)
RETURN
END
-----
SUBROUTINE ISORT(ID,INDEX,N)
INTEGER ID(N),INDEX(N)
LOGICAL FLAG
DO 10 I=1,N
  INDEX(I) = I
10 CONTINUE
DO 30 I=N,2,-1
  FLAG = TRUE
  DO 20 J=2,I
    IF (ID(INDEX(J)) .LT. ID(INDEX(J-1))) THEN
      INDEX(J) = INDEX(J-1)
      INDEX(J-1) = I
      FLAG = FALSE
20 CONTINUE
IF (FLAG) RETURN
30 CONTINUE
RETURN
END
-----
SUBROUTINE DMPDAT(OPT,WRITE,TO,DT)
LOGICAL TEST,OPEN,FALSE,AXIAL,FIRST
CHARACTER*40 OPTION
CHARACTER*80 NAME,DIR*2
CHARACTER*112 TITL(2)
CHARACTER*12 NAME
DIMENSION RINFUT(100)
SAVE FIRST
$INCLUDE 'ZCOMM'
C
GO TO (100,200,300,400) IOPT
-----
C
READ DATA ID TO BE SAVED, AND INITIALIZE FILES ---
100 FIRST = .TRUE.
I2DUMP = I2
I2 = I2-1
NWRITE = 0
WRITE(108,109)
109 FORMAT (//,5X,' DATA WRITTEN TO FILES ',/
  & 5X,'-----')
101 READ (105,*) OPTION,IDOP,IUNIT,IGEN,IIDOP,IUNIT
102 CONTINUE
C
SET UP DOF DATA
IF (TEST(OPTION,'DOF',FALSE)) THEN
  WRITE(108,110) 'DEGREE OF FREEDOM',IDOP,IUNIT
C
CHECK FILE STATUS
INQUIRE (UNIT=IUNIT,OPENED=OPEND)
IF (.NOT.OPEND) OPEN (UNIT=IUNIT)
INITIALIZE DATA AND FILE
NWRITE=NWRITE+1
N2(I2)=3
N2(I2-1)=0
N2(I2+1)=0
N2(I2-2)=0
N2(I2+2)=0
N2(I2-3)=0
N2(I2+3)=0
WRITE(IUNIT,120) TITLE(1)(1:80),TITLE(1)(81:
  WRITE(IUNIT,130) 1, IDOP, IWRITE, TO, DT
C
SET UP JOINT
ELSE IF (TEST(OPTION,'JOINT',FALSE)) THEN
  JOINT=IDOP
  JOINT=ICQUICK(IDOP,N2(I2ID),NMODE)-1
  IF (TEST(OPTION,'FK',FALSE)) THEN
    IDOP=N2(I2IDOP+JOINT)
    DIR='FK'
  ELSE IF (TEST(OPTION,'FY',FALSE)) THEN
    IDOP=N2(I2IDOP+JOINT+MAXNCB+1)
    DIR='FY'
  ELSE IF (TEST(OPTION,'FZ',FALSE)) THEN
    IDOP=N2(I2IDOP+JOINT+MAXNCB+2)
    DIR='FZ'
  ELSE IF (TEST(OPTION,'MX',FALSE)) THEN
    IDOP=N2(I2IDOP+JOINT+MAXNCB+3)
    DIR='MX'
  ELSE IF (TEST(OPTION,'MY',FALSE)) THEN
    IDOP=N2(I2IDOP+JOINT+MAXNCB+4)
    DIR='MY'
  ELSE IF (TEST(OPTION,'MZ',FALSE)) THEN
    IDOP=N2(I2IDOP+JOINT+MAXNCB+5)
    DIR='MZ'
  ENDIF
C
WRITE(108,110) 'DEGREE OF FREEDOM',IDOP,IUNIT,JOINT,DIR
C
CHECK FILE STATUS
INQUIRE (UNIT=IUNIT,OPENED=OPEND)
IF (.NOT.OPEND) OPEN (UNIT=IUNIT)
INITIALIZE DATA AND FILE
NWRITE=NWRITE+1
N2(I2)=1
N2(I2-1)=IDOPJ
N2(I2+1)=IUNIT
I2=I2-3
WRITE(IUNIT,120) TITLE(1)(1:80),TITLE(1)(81:
  WRITE(IUNIT,130) 2, IDOP, IWRITE, TO, DT, JOINT, DIR
C
SET UP ELEMENT
ELSE IF (TEST(OPTION,'ELE',FALSE)) THEN
  WRITE(108,110) 'ELEMENT',IDOP,IUNIT
C
CHECK FILE STATUS
INQUIRE (UNIT=IUNIT,OPENED=OPEND)
IF (.NOT.OPEND) OPEN (UNIT=IUNIT)
INITIALIZE DATA AND FILE
NWRITE=NWRITE+1
N2(I2)=2
N2(I2+1)=IDOP
I2=I2-3
WRITE(IUNIT,120) TITLE(1)(1:80),TITLE(1)(81:
  WRITE(IUNIT,130) 3, IDOP, IWRITE, TO, DT, IELTYP, 'ELEMENT'
C
SET UP ENERGY
ELSE IF (TEST(OPTION,'ENERGY',FALSE)) THEN
  WRITE(108,111) 'ENERGY BALANCE',IUNIT
C
CHECK FILE STATUS
INQUIRE (UNIT=IUNIT,OPENED=OPEND)
IF (.NOT.OPEND) OPEN (UNIT=IUNIT)
INITIALIZE DATA AND FILE
NWRITE=NWRITE+1
N2(I2)=3
N2(I2-1)=0
N2(I2+1)=IUNIT
I2=I2-3
WRITE(IUNIT,120) TITLE(1)(1:80),TITLE(1)(81:
  WRITE(IUNIT,130) 3, IDOP, IWRITE, TO, DT, 'ENERGY'
C
SET UP SUM OF REACTIONS
ELSE IF (TEST(OPTION,'SUMCT',FALSE)) THEN
  WRITE(108,111) 'SUMMATION OF REACTIONS',IUNIT
C
CHECK FILE STATUS
INQUIRE (UNIT=IUNIT,OPENED=OPEND)
IF (.NOT.OPEND) OPEN (UNIT=IUNIT)
INITIALIZE DATA AND FILE
NWRITE=NWRITE+1
N2(I2)=4
N2(I2+1)=0
N2(I2-2)=IUNIT
I2=I2-3
WRITE(IUNIT,120) TITLE(1)(1:80),TITLE(1)(81:
  WRITE(IUNIT,130) 4, IDOP, IWRITE, TO, DT, 'SUMCT'
C
SET UP SUM OF REACTIONS
ELSE IF (TEST(OPTION,'GROUND',FALSE)) THEN
  WRITE(108,111) 'GROUND MOTION',IUNIT
C
CHECK FILE STATUS
INQUIRE (UNIT=IUNIT,OPENED=OPEND)
IF (.NOT.OPEND) OPEN (UNIT=IUNIT)
INITIALIZE DATA AND FILE
NWRITE=NWRITE+1
N2(I2)=5
N2(I2-1)=0
N2(I2+1)=0
N2(I2-2)=IUNIT
I2=I2-3
WRITE(IUNIT,120) TITLE(1)(1:80),TITLE(1)(81:
  WRITE(IUNIT,130) 5, IDOP, IWRITE, TO, DT, 'GROUND'
C
SET UP SUM OF REACTIONS
ELSE IF (TEST(OPTION,'END',FALSE)) THEN
  N2(I2DUMP)=NWRITE
  RETURN
ELSE
  WRITE(108,140) OPTION
  GO TO 101
ENDIF
C
GENERATE LOADS
IF (IGEN.NE.0) THEN
  IGEN=IGEN+1
  IDOP = IDOP + IIDOP
  IUNIT = IUNIT + IUNIT
  GO TO 102
ENDIF
GO TO 101

```

```

C
105 FORMAT (5X,'FILE IS ALREADY OPENED, WRITE IS ABORTED',10(' '), DMFD1780
DMFD1790
110 FORMAT (5X,A,' #,15, ' IS WRITTEN TO UNIT #,15, ' JOINT:',16 DMFD1800
& DIRECTION: ',A) DMFD1810
111 FORMAT (5X,A,' IS WRITTEN TO UNIT #,15 ) DMFD1820
120 FORMAT (A) DMFD1830
130 FORMAT (3110,1P,2G15.5,OP,1X,16,A) DMFD1840
140 FORMAT (5X,'INVALID INPUT: ',A, ', INPUT IS SKIPPED...') DMFD1850
C
DMFD1860
DMFD1870
C WRITE DATA TO FILES ---
DMFD1880
DMFD1890
DMFD1900
C
200 NUMB=N2(1ZDUMP) DMFD1910
DO 250 I=1,NUMB DMFD1920
  I1=1ZDUMP-3*I-1) *1 DMFD1930
  ITYPE=N2(I1) DMFD1940
  IDOP =N2(I1-1) DMFD1950
  IUNIT=N2(I1-2) DMFD1960
  IF (ITYPE.EQ.1) THEN DMFD1970
    J=IDOP-1 DMFD1980
    IF (1ZVEL.EQ.0 OR 1ZACC.EQ.0) THEN DMFD1990
      WRITE(IUNIT,210) 2(1ZLOAD-J),2(1ZDISP-J),0.,0. DMFD2000
    ELSE DMFD2010
      WRITE(IUNIT,210) 2(1ZLOAD-J),2(1ZDISP-J), DMFD2020
      2(1ZVEL-J),2(1ZACC-J) DMFD2030
    ENDIF DMFD2040
  ELSE IF (ITYPE.EQ.2) THEN DMFD2050
    I7=7 DMFD2060
    IELNO=IDOP DMFD2070
    LASTYP=0 DMFD2080
    FALSE=.FALSE. DMFD2090
    CALL EL2LIB DMFD2100
    (I7,LSTTYP,PIRST,IREL,PIRST,NAME,ESE,EPSE,DAMAGE,DUCFAG DMFD2110
    IELNO,1ZDOP,KDOP,PGEOM,RINPUT,AXIAL,1ZDDSP,1ZDLGA,MSTOR) DMFD2120
  ELSE IF (ITYPE.EQ.3) THEN DMFD2130
    DIFF=E1E-E2E-E3E-PSE-EDD DMFD2140
    IF (E1E.NE.0) THEN DMFD2150
      RE=DIFF/E1E * 100 DMFD2160
    ELSE DMFD2170
      IF (DIFF.NE.0) RE=100.00 DMFD2180
      IF (DIFF.EQ.0) RE= 0.00 DMFD2190
    ENDIF DMFD2200
    WRITE(IUNIT,210) E1E, E2E, E3E, PSE, EDD, RE DMFD2210
  ELSE IF (ITYPE.EQ.4) THEN DMFD2220
    WRITE(IUNIT,210) SURMCT DMFD2230
  ELSE IF (ITYPE.EQ.5) THEN DMFD2240
    WRITE(IUNIT,210) GROUND DMFD2250
  ENDIF DMFD2260
250 CONTINUE DMFD2270
310 FORMAT(1P,6G13.5) DMFD2280
PIRST=.FALSE. DMFD2290
RETURN DMFD2300
C
DMFD2310
DMFD2320
C READ DATA FROM FILE AND WRITE TO PT06 ---
DMFD2330
C
400 REWIND(UNIT=IUNIT) DMFD2340
READ(IUNIT,120,END=499) TITL(1)(1:80),TITL(1)(81:) DMFD2350
READ(IUNIT,120,END=499) TITL(2)(1:80),TITL(2)(81:) DMFD2360
WRITE(109,405) TITL(1),TITL(2) DMFD2370
405 FORMAT (1) STRUCTURE 'A,/' SOLUTION..... 'A,/' DMFD2380
READ(IUNIT,131,END=499) ITYPE,IDOP,IWRITE,TO,DT,ITEMP,INAME DMFD2390
131 FORMAT (3110,OP,2G15.5,OP,1X,16,A) DMFD2400
C
DMFD2410
DMFD2420
DMFD2430
DMFD2440
DMFD2450
DMFD2460
DMFD2470
DMFD2480
DMFD2490
DMFD2500
DMFD2510
DMFD2520
DMFD2530
DMFD2540
DMFD2550
DMFD2560
DMFD2570
DMFD2580
DMFD2590
DMFD2600
DMFD2610
DMFD2620
DMFD2630
DMFD2640
DMFD2650
DMFD2660
DMFD2670
DMFD2680
DMFD2690
DMFD2700
DMFD2710
DMFD2720
DMFD2730
DMFD2740
DMFD2750
DMFD2760
DMFD2770
DMFD2780
DMFD2790
DMFD2800
DMFD2810
DMFD2820
DMFD2830
DMFD2840
DMFD2850
DMFD2860
DMFD2870
DMFD2880
DMFD2890
DMFD2900
DMFD2910
DMFD2920
DMFD2930
DMFD2940
DMFD2950
DMFD2960
DMFD2970
DMFD2980
DMFD2990
DMFD3000
DMFD3010
DMFD3020
DMFD3030
DMFD3040
DMFD3050
DMFD3060
DMFD3070
DMFD3080
DMFD3090
DMFD3100
DMFD3110
DMFD3120
DMFD3130
DMFD3140
DMFD3150
DMFD3160
DMFD3170
DMFD3180
DMFD3190
DMFD3200
DMFD3210
DMFD3220
DMFD3230
436 READ(IUNIT,'END=440) GROUND DMFD3240
ISTEP=ISTEP+1WRITE DMFD3250
T=TO+DT*ISTEP DMFD3260
IF (MOD(ISTEP,INC).EQ.0) WRITE(108,461) ISTEP,T, GROUND DMFD3270
GO TO 436 DMFD3280
ENDIF DMFD3290
C 440 RETURN DMFD3300
DMFD3310
DMFD3320
DMFD3330
DMFD3340
DMFD3350
DMFD3360
DMFD3370
DMFD3380
DMFD3390
DMFD3400
DMFD3410
DMFD3420
DMFD3430
DMFD3440
DMFD3450
DMFD3460
DMFD3470
DMFD3480
DMFD3490
DMFD3500
DMFD3510
DMFD3520
DMFD3530
DMFD3540
DMFD3550
DMFD3560
DMFD3570
DMFD3580
DMFD3590
DMFD3600
DMFD3610
DMFD3620
DMFD3630
DMFD3640
DMFD3650
DMFD3660
DMFD3670
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DMFD3690
DMFD3700
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DMFD3750
DMFD3760
DMFD3770
DMFD3780
DMFD3790
DMFD3800
DMFD3810
DMFD3820
DMFD3830
DMFD3840
DMFD3850
DMFD3860
DMFD3870
DMFD3880
DMFD3890
DMFD3900
DMFD3910
DMFD3920
DMFD3930
DMFD3940
DMFD3950
DMFD3960
DMFD3970
DMFD3980
DMFD3990
DMFD4000
DMFD4010
DMFD4020
DMFD4030
DMFD4040
DMFD4050
DMFD4060
DMFD4070
DMFD4080
DMFD4090
DMFD4100
DMFD4110
DMFD4120
DMFD4130
DMFD4140
DMFD4150
DMFD4160
DMFD4170
DMFD4180
DMFD4190
DMFD4200
DMFD4210
DMFD4220
DMFD4230
DMFD4240
DMFD4250
DMFD4260
DMFD4270
DMFD4280
DMFD4290
DMFD4300
DMFD4310
DMFD4320
DMFD4330
DMFD4340
DMFD4350
DMFD4360
DMFD4370
DMFD4380
DMFD4390
DMFD4400
DMFD4410
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DMFD4430
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DMFD4450
DMFD4460
DMFD4470
DMFD4480
DMFD4490
DMFD4500
DMFD4510
DMFD4520
DMFD4530
DMFD4540
DMFD4550
DMFD4560
DMFD4570
DMFD4580
DMFD4590
DMFD4600
DMFD4610
DMFD4620
DMFD4630
DMFD4640
DMFD4650
DMFD4660
DMFD4670
DMFD4680
DMFD4690
DMFD4700
DMFD4710
DMFD4720
DMFD4730
DMFD4740
DMFD4750
DMFD4760
DMFD4770
DMFD4780
DMFD4790
DMFD4800
DMFD4810
DMFD4820
DMFD4830
DMFD4840
DMFD4850
DMFD4860
DMFD4870
DMFD4880
DMFD4890
DMFD4900
DMFD4910
DMFD4920
DMFD4930
DMFD4940
DMFD4950
DMFD4960
DMFD4970
DMFD4980
DMFD4990
DMFD5000
436 READ(IUNIT,'END=440) GROUND DMFD5010
ISTEP=ISTEP+1WRITE DMFD5020
T=TO+DT*ISTEP DMFD5030
IF (MOD(ISTEP,INC).EQ.0) WRITE(108,461) ISTEP,T, GROUND DMFD5040
GO TO 436 DMFD5050
ENDIF DMFD5060
C 440 RETURN DMFD5070
DMFD5080
DMFD5090
DMFD5100
DMFD5110
DMFD5120
DMFD5130
DMFD5140
DMFD5150
DMFD5160
DMFD5170
DMFD5180
DMFD5190
DMFD5200
DMFD5210
DMFD5220
DMFD5230
DMFD5240
DMFD5250
DMFD5260
DMFD5270
DMFD5280
DMFD5290
DMFD5300
DMFD5310
DMFD5320
DMFD5330
DMFD5340
DMFD5350
DMFD5360
DMFD5370
DMFD5380
DMFD5390
DMFD5400
DMFD5410
DMFD5420
DMFD5430
DMFD5440
DMFD5450
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DMFD5580
DMFD5590
DMFD5600
DMFD5610
DMFD5620
DMFD5630
DMFD5640
DMFD5650
DMFD5660
DMFD5670
DMFD5680
DMFD5690
DMFD5700
DMFD5710
DMFD5720
DMFD5730
DMFD5740
DMFD5750
DMFD5760
DMFD5770
DMFD5780
DMFD5790
DMFD5800
DMFD5810
DMFD5820
DMFD5830
DMFD5840
DMFD5850
DMFD5860
DMFD5870
DMFD5880
DMFD5890
DMFD5900
DMFD5910
DMFD5920
DMFD5930
DMFD5940
DMFD5950
DMFD5960
DMFD5970
DMFD5980
DMFD5990
DMFD6000
300 RETURN DMFD6010
END DMFD6020
C
C SUBROUTINE GETCHR DETERMINES THE VALUE OF THE FIRST CHARACTER
AFTER AN OPTIONAL (LWORD=.TRUE.) KEYWORD (WORD).
AND REMOVES THE STRING (WORD)//CHAR TP
MODIFY=.TRUE.
C
SUBROUTINE GETCHR(VERB,WORD,CHAR,LWORD,MODIFY)
C
CHARACTER(*) VERB,WORD,CHAR
LOGICAL MODIFY,LWORD
999 CHAR=(*) THEN
  IF (LWORD) THEN
    I=START-INDEX(VERB,WORD)
    IF (I=START) RETURN
    I=I-START+LEN(WORD)
  ELSE
    I=1
    I=START-1
    CALL FIRST(VERB)
  ENDIF
  IF (VERB(I:I).EQ.'') THEN
    I=I-1
    IF (I=1) THEN
      J=INDEX(VERB(I:J),'')-J + J
      IF (VERB(I:J-1:J-1).EQ.'') THEN
        I=J-1
        GO TO 10
      ENDIF
      IF (J.EQ.0) THEN
        CHAR=VERB(I:1)
        GO TO 10
      ELSE
        CHAR=VERB(I:J)
      ENDIF
    ELSE
      J=INDEX(VERB(I:1),'') + I - 1
      IF (J.GE.1) CHAR=VERB(I:J)
    ENDIF
  IF (MODIFY) THEN
    IF (VERB(J-1:J-1).EQ.'') J=J-1
    IF (VERB(J:J).EQ.'') VERB(J:J)
    CALL FIRST(VERB)
  ENDIF
RETURN
END
C
STEPS PRECEDING BLANK SPACES OFF A CHARACTER STRING
SUBROUTINE FIRST(WORD)
CHARACTER(*) WORD
IF (INDEX(WORD,' ') .EQ. 0) RETURN
DO 10 I=1,(LEN(WORD))
  J=INDEX(WORD(I:1),' ')
  IF (J.EQ.1) GO TO 10
RETURN
10 CONTINUE
20 RETURN
END
C
SUBROUTINE GETINT DETERMINES THE VALUE OF THE FIRST INTEGER (INTEG)
AFTER AN OPTIONAL (LWORD=.TRUE.) KEYWORD (WORD)
AND REMOVES THE STRING (WORD)//INTEGER IF
MODIFY=.TRUE. INTEG CAN BE ANY LENGTH REPRESENTABLE
BY SINGLE PRECISION.
C
SUBROUTINE GETINT(VERB,WORD,INTEG,DEPULT,LWORD,MODIFY)
C
CHARACTER(*) VERB,WORD
CHARACTER*10 NUMB
LOGICAL MODIFY,LWORD,HIT
INTEGER DEPULT
DATA NUMB/'0123456789'/
C
INTEG=DEPULT
HIT=.TRUE.
IF (LWORD) THEN
  I=START-INDEX(VERB,WORD)
  IF (I=START) RETURN
  I=I-START+LEN(WORD)
ELSE
  I=1
  I=START-1
ENDIF
ISIGN = 1
L=LEN(WORD)
5 IF (I.GT.L) RETURN
IF (VERB(I:I).EQ.'') THEN
  ISIGN = -1
  I = I - 1
  GO TO 5
ENDIF
GETIO010
GETIO020
GETIO030
GETIO040
GETIO050
GETIO060
GETIO070
GETIO080
GETIO090
GETIO100
GETIO110
GETIO120
GETIO130
GETIO140
GETIO150
GETIO160
GETIO170
GETIO180
GETIO190
GETIO200
GETIO210
GETIO220
GETIO230
GETIO240
GETIO250
GETIO260
GETIO270
GETIO280
GETIO290
GETIO300
GETIO310
GETIO320
GETIO330

```



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      PE = W*L/2
      FIC = W*L/2
      CALL FEMREL(F4 , F10, B5 , B3 , 1, IELNO)
      ELSE
        WRITE(108,905) IELNO, ' UNIFORM MY OR UNIFORM MZ '
      ENDIF
C ..... USER INPUT FIXED END FORCE
      ELSE IF (KGRP.EQ.400) THEN
        DO 400 I=1,12
          FORCE(I) = ELD(I,N)
400 ..... INVALID LOAD GROUP
      ELSE
        WRITE(108,900) IELNO, ' INVALID LOAD GROUP '
      ENDIF
C ..... TRANSFER BEAM LOADS TO FIXED END FORCES ...
      DO 800 I=1,12
        FEM(I,LOAD) = FEM(I,LOAD) + FORCE(I)
800 ..... CONTINUE
900 FORMAT(' ER', 'RCR CALCULATING FIXED END FORCES FOR ELEMENT N',
          & I6, 'A', ' IS NOT AVAILABLE')
      RETURN
      END
-----
SUBROUTINE ROTXYZ(VX,VY,COSINE,CT,EX,EY,EZ,CTXYZ,GLOXYZ,NROW)
DIMENSION VX(3),VY(3),VZ(3),COSINE(3,3),CT(3,3),
          & XYZ(3,3),CTXYZ(3,3),CE(3,3),GLOXYZ(NROW,6)
-----
THIS SUBROUTINE PERFORMS SEVERAL FUNCTIONS:
1) CALCULATE LOCAL COORDINATE SYS COSINE MATRIX.
2) CALCULATE ROTATION MATRIX BETWEEN LOCAL AND JOINT COORD (CT)
3) CALCULATE TRANSFORMATION FOR GLOBAL CONSTRAINTS AND MEMBER
   END DEPTH - STORE IN (CTXYZ)
-----
INPUT VARIABLES:
COSINE = INPUT JOINT COORDINATE DIRECTION COSINE
VX = INPUT X-DIRECTION OF ELEMENT COORDINATE SYS
VY = INPUT VECTOR IN THE ELEMENT COORDINATE SYS XY PLANE.
GLOXYZ = GLOBAL CONSTRAINT TRANSFORMATION COEFFICIENTS
EX = MEMBER END ECCENTRICITY IN THE LOCAL X AXIS.
EY = MEMBER END ECCENTRICITY IN THE LOCAL Y AXIS.
EZ = MEMBER END ECCENTRICITY IN THE LOCAL Z AXIS
-----
OUTPUT VARIABLES:
VX = UNIT VECTOR, X-DIRECTION OF ELEMENT COORDINATE SYS
VY = UNIT VECTOR, Y-DIRECTION OF ELEMENT COORDINATE SYS
VZ = UNIT VECTOR, Z-DIRECTION OF ELEMENT COORDINATE SYS
CT = ROTATED COORDINATE TRANSFORMATION
CTXYZ = CONSTRAINT AND MEMBER END DEPTH TRANSFORMATION
-----
      FX      PY      PZ      CT      0      PZ
      MY      MZ      CTXYZ      CT      MY      MZ
-----
      GLOBAL      ELEMENT COORD.
-----
      CROSSI(A1,B1,C1,A2,B2,C2) = B1*Y2 - B2*Y1
      CROSSJ(A1,B1,C1,A2,B2,C2) = A1*Z2 - A2*Z1
      CROSSK(A1,B1,C1,A2,B2,C2) = A1*B2 - A2*B1
      VALUE(A1,A2,A3) = SQRT(A1**2 + A2**2 + A3**2)
      DOT(A1,B1,C1,A2,B2,C2) = A1*A2 + B1*B2 + C1*C2
-----
C ..... CHECK ANGLE BETWEEN VX AND VY
      CL=VCOS(VX,VY)
      IF(ABS(C1).GT.999) WRITE(108,*) ' VY IS PARALLEL TO VX, REINPUT VY '
C ..... CHECK VY, IF VY1.EQ.0, SET VY IN GLOBAL YZ PLANE..
      AVY=VALUE(VY(1),VY(2),VY(3))
      IF(AVY.EQ.0) THEN
        VZ(1) = 0
        VZ(2) = 0
        VZ(3) = 0
      ELSE
        CALCULATE VZ, IF VY WAS CORRECTLY INPUT
        VZ(1) = CROSSI(VX(1),VX(2),VX(3),VY(1),VY(2),VY(3))
        VZ(2) = CROSSJ(VX(1),VX(2),VX(3),VY(1),VY(2),VY(3))
        VZ(3) = CROSSK(VX(1),VX(2),VX(3),VY(1),VY(2),VY(3))
      ENDIF
C ..... RECALCULATE VY TO INSURE VX,VY AND VZ ARE ORTHOGONAL...
      VY(1) = CROSSI(VZ(1),VZ(2),VZ(3),VX(1),VX(2),VX(3))
      VY(2) = CROSSJ(VZ(1),VZ(2),VZ(3),VX(1),VX(2),VX(3))
      VY(3) = CROSSK(VZ(1),VZ(2),VZ(3),VX(1),VX(2),VX(3))
C ..... CHECK ANGLE BETWEEN VX AND VY
      CL=VCOS(VX,VY)
      IF(ABS(C1).GT.999) WRITE(108,*) ' VY IS PARALLEL TO VX, REINPUT VY '
C ..... NORMALIZE VX,VY,VZ
      AVX=VALUE(VX(1),VX(2),VX(3))
      AVY=VALUE(VY(1),VY(2),VY(3))
      AVZ=VALUE(VZ(1),VZ(2),VZ(3))
      DO 15 I=1,3
        VX(I) = VX(I)/AVX
        VY(I) = VY(I)/AVY
        VZ(I) = VZ(I)/AVZ
        CE(I,1) = VX(I)
        CE(I,2) = VY(I)
        CE(I,3) = VZ(I)
15 ..... CALC THE DIRECTION COSINE MATRIX
      DO 15 I=1,3
        DO 15 J=1,3
          CT(I,J) = 0
          DO 15 K=1,3
            CT(I,J) = CT(I,J) + COSINE(I,K)*CE(K,J)
15 ..... REMOVE THE FUZZ FROM THE COSINE MATRIX
      DO 20 I=1,3
        DO 20 J=1,3
          IF(ABS(CT(I,J)) LT .0001) CT(I,J) = 0.
20 ..... SET UP THE GLOBAL CONSTRAINT MATRIX
      XYZ(1,1) = 0
      XYZ(1,2) = GLOXYZ(1,1)
      XYZ(1,3) = GLOXYZ(1,2)
      XYZ(2,1) = GLOXYZ(2,1)
      XYZ(2,2) = 0
      XYZ(2,3) = GLOXYZ(2,4)
      XYZ(3,1) = GLOXYZ(3,5)
      XYZ(3,2) = GLOXYZ(3,6)
      XYZ(3,3) = 0
      DO 25 I=1,3
        DO 25 J=1,3
          DO 25 K=1,3
            CTXYZ(I,J) = CTXYZ(I,J) + XYZ(I,K)*CT(K,J)
25 ..... SKIP THE MEMBER END ECCENTRICITY MATRIX IF EX=EY=EZ=0
      IF(EX.EQ.0 .AND. EY.EQ.0 .AND. EZ.EQ.0) RETURN
C ..... CALC THE MEMBER END ECCENTRICITY MATRIX

```

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-----STRU0010          BACKSPACE (105)          STRU1470
SUBROUTINE STRUCT      STRU0020          CALL MATLIB (IOPT,LTSTYP,HEAD,EBSE,EPSE,DUCT,EXCR, STRU1480
DIMENSION RINPUT(100),GINPUT(100),DUCT(3),EXCR(3)          STRU0030          & P,PL,D,DL,V,VL,LELEM,LMAT,MAT,MATYP,SE,1,DAMAGE) STRU1490
CHARACTER*80 NAME,TYPE          STRU0040          C          RESERVE STORAGE          STRU1500
DIMENSION SE(100)          STRU0050          C          12-12:LMAT-1          STRU1510
LOGICAL FIRST,PT,HEAD,TST,AXIAL,HEAD2,STEST          STRU0060          C          IF (MAT .LT. NMAT) GO TO 110          STRU1520
INCLUDE '2CCON'          STRU0070          C          120 CONTINUE          STRU1530
WRITE(108,5) TITLE(1),TITLE(2)          STRU0080          C          -----          STRU1540
5 FORMAT ('1 STRUCTURE .....',A,          STRU0090          C          INPUT GEOMETRIC STIFFNESS DATA ---          STRU1550
& ' SOLUTION .....',A,          STRU0100          C          -----          STRU1560
/)          STRU0110          C          -----          STRU1570
WRITE(108,6)          STRU0120          C          INPUT GEOMETRIC STIFFNESS DATA ---          STRU1580
6 FORMAT (' *** PROGRAM FEM ***          STRU0130          C          -----          STRU1590
DOUBLE PRECISION VERSION'//)          STRU0140          C          INPUT # OF NODES, # OF SUPTS, # OF CONST., AND SCALE FACTOR          STRU1600
READ(105,*) NNODE,NCOS,NSUPT,NCOND,NCONST,SCALE          STRU0150          C          Z(1ZKGD1-1) - VERTICAL GROUND ACCELERATION          STRU1610
MAINCD=NNODE          STRU0160          C          NZ(1ZKGD1-1) - TYPE OF AXIAL LOAD USED          STRU1620
          STRU0170          C          0, GEOMETRIC STIFFNESS IS NOT CONSIDERED          STRU1630
          STRU0180          C          1, LOAD - P(INPUT)*1.0 - ACCEL          STRU1640
          STRU0190          C          2, PREVIOUS LOAD STEP'S AXIAL LOAD IS USED          STRU1650
          STRU0200          C          NZ(1ZKGD1-2) - TYPE OF ELEMENT GEOMETRIC STIFFNESS TO BE USED          STRU1660
          STRU0210          C          0, GEOMETRIC STIFFNESS IS NOT CONSIDERED          STRU1670
          STRU0220          C          1, LUMPED FORMULATION          STRU1680
          STRU0230          C          2, CONSISTENT FORMULATION          STRU1690
          STRU0240          C          NZ(1ZKGD1-3) - FORM OF SOLUTION          STRU1700
          STRU0250          C          0, GEOMETRIC STIFFNESS IS NOT CONSIDERED          STRU1710
          STRU0260          C          1, KG IS SUBTRACTED FROM ELEMENT STIFFNESS          STRU1720
          STRU0270          C          2, SEPARATE GLOBAL KG IS FORMED          STRU1730
          STRU0280          C          KGCCOND - LOGICAL FLAG, TRUE IF KG IS TO BE CONDENSED          STRU1740
          STRU0290          C          IZKGD1-12          STRU1750
          STRU0300          C          IZ - IZ *4          STRU1760
          STRU0310          C          READ(105,*) (NZ(1ZKGD1-I),I=1,3),XGCOND          STRU1770
          STRU0320          C          I=IZKGD1          STRU1780
          STRU0330          C          IF (NZ(1-1).EQ.0 .OR. NZ(1-2).EQ.0 .OR. NZ(1-3).EQ.0) THEN          STRU1790
          STRU0340          C          NZ(1-1)-0          STRU1800
          STRU0350          C          NZ(1-2)-0          STRU1810
          STRU0360          C          NZ(1-3)-0          STRU1820
          STRU0370          C          ELSE          STRU1830
          STRU0380          C          WRITE(108,150) TITLE(1),TITLE(2)          STRU1840
          STRU0390          C          IP (NZ(1-1).EQ.1) WRITE(108,160)          STRU1850
          STRU0400          C          & 'KGDATA(1)- 1, LOAD - P(INPUT)*1.00-ACCEL',          STRU1860
          STRU0410          C          & ' P(INPUT) IS POSITIVE IN COMPRESSION',          STRU1870
          STRU0420          C          & ' ACCEL IS POSITIVE GLOBAL Z-AXIS ACCELERATION'          STRU1880
          STRU0430          C          & IP (NZ(1-1).EQ.2) WRITE(108,160)          STRU1890
          STRU0440          C          & 'KGDATA(1)- 2, PREVIOUS LOAD STEP'S AXIAL LOAD IS USED'          STRU1900
          STRU0450          C          -----          STRU1910
          STRU0460          C          IF (NZ(1-2).EQ.1) WRITE(108,160)          STRU1920
          STRU0470          C          & 'KGDATA(2)- 1, LUMPED FORMULATION'          STRU1930
          STRU0480          C          & IP (NZ(1-2).EQ.2) WRITE(108,160)          STRU1940
          STRU0490          C          & 'KGDATA(2)- 2, CONSISTENT FORMULATION'          STRU1950
          STRU0500          C          -----          STRU1960
          STRU0510          C          -----          STRU1970
          STRU0520          C          IF (NZ(1-3).EQ.1) WRITE(108,160)          STRU1980
          STRU0530          C          & 'KGDATA(3)- 1, KG IS SUBTRACTED FROM ELEMENT STIFFNESS'          STRU1990
          STRU0540          C          & IP (NZ(1-3).EQ.2) WRITE(108,160)          STRU2000
          STRU0550          C          & 'KGDATA(3)- 2, SEPARATE GLOBAL KG IS FORMED'          STRU2010
          STRU0560          C          -----          STRU2020
          STRU0570          C          WRITE(108,160)          STRU2030
          STRU0580          C          IF (KGCCOND) WRITE(108,160)          STRU2040
          STRU0590          C          & ' THE GEOMETRIC STIFFNESS MATRIX IS CONDENSED'//          STRU2050
          STRU0600          C          & ' WITH THE STRUCTURAL STIFFNESS MATRIX.'          STRU2060
          STRU0610          C          & IF (NOT KGCCOND) WRITE(108,160)          STRU2070
          STRU0620          C          & ' THE GEOMETRIC STIFFNESS MATRIX IS NOT CONDENSED'//          STRU2080
          STRU0630          C          & ' WITH THE STRUCTURAL STIFFNESS MATRIX.'          STRU2090
          STRU0640          C          ENDP          STRU2100
          STRU0650          C          -----          STRU2110
          STRU0660          C          150 FORMAT ('1 STRUCTURE .....',A,          STRU2120
          STRU0670          C          & ' SOLUTION .....',A,          STRU2130
          STRU0680          C          & ' 5X, GEOMETRIC STIFFNESS DATA '//          STRU2140
          STRU0690          C          & ' ',A,          STRU2150
          STRU0700          C          160 FORMAT(5X,A)          STRU2160
          STRU0710          C          -----          STRU2170
          STRU0720          C          -----          STRU2180
          STRU0730          C          -----          STRU2190
          STRU0740          C          SET UP STORAGE FOR GEOMETRIC STIFFNESS DATA ---          STRU2200
          STRU0750          C          -----          STRU2210
          STRU0760          C          GLOBAL STIFFNESS STORAGE          STRU2220
          STRU0770          C          NZ(1ZKGD) - INDICES OF THE MAIN DIAGONAL TERMS          STRU2230
          STRU0780          C          Z(1ZKG) - GLOBAL GEOMETRIC STIFFNESS MATRIX          STRU2240
          STRU0790          C          IF (NZ(1ZKGD-3).EQ.2) THEN          STRU2250
          STRU0800          C          IZMKG=IZ          STRU2260
          STRU0810          C          IZ - IZ -NDOP + 1          STRU2270
          STRU0820          C          CALL SKYLN(0,1,NDOP,IBUG,TITLE,NZ(1ZMKG),          STRU2280
          STRU0830          C          & KGDOP,NZ(1ZLMKG),Z(1ZKERG),MD,'GLOBAL GEOMETRIC STIFFNESS')          STRU2290
          STRU0840          C          ELSE          STRU2300
          STRU0850          C          DUMMY LOCATION FOR KG TO AVOID ADDRESSING EXCEPTIONS          STRU2310
          STRU0860          C          KG DATA IS NOT STORED IN THESE LOCATIONS...          STRU2320
          STRU0870          C          IZMKG=IZKGD          STRU2330
          STRU0880          C          IZKG =IZKGD          STRU2340
          STRU0890          C          ENDP          STRU2350
          STRU0900          C          -----          STRU2360
          STRU0910          C          -----          STRU2370
          STRU0920          C          INPUT ELEMENT DATA ---          STRU2380
          STRU0930          C          -----          STRU2390
          STRU0940          C          READ(105,*) NELMT          STRU2400
          STRU0950          C          -----          STRU2410
          STRU0960          C          ELEMENT STORAGE          STRU2420
          STRU0970          C          NZ(1ZELE) - # OF MATERIAL PROPERTIES (NELMT)          STRU2430
          STRU0980          C          NZ(1ZELE+ 1) - ABSOLUTE ADDRESS OF ELEMENT #1          STRU2440
          STRU0990          C          NZ(1ZELE+ 2) - ABSOLUTE ADDRESS OF ELEMENT #2          STRU2450
          STRU1000          C          NZ(1ZELE+ 3) - ABSOLUTE ADDRESS OF ELEMENT #3          STRU2460
          STRU1010          C          -----          STRU2470
          STRU1020          C          -----          STRU2480
          STRU1030          C          NZ(1ZMAT-NELMT) - ABSOLUTE ADDRESS FOR ELEM # NELMT          STRU2490
          STRU1040          C          2(NZ(1ZELE+2))-          STRU2500
          STRU1050          C          MATERIAL DATA FOR ELEMENT #1          STRU2510
          STRU1060          C          -----          STRU2520
          STRU1070          C          2(NZ(1ZELE+2))-          STRU2530
          STRU1080          C          MATERIAL DATA FOR ELEMENT #1          STRU2540
          STRU1090          C          -----          STRU2550
          STRU1100          C          -----          STRU2560
          STRU1110          C          -----          STRU2570
          STRU1120          C          -----          STRU2580
          STRU1130          C          -----          STRU2590
          STRU1140          C          -----          STRU2600
          STRU1150          C          -----          STRU2610
          STRU1160          C          -----          STRU2620
          STRU1170          C          -----          STRU2630
          STRU1180          C          -----          STRU2640
          STRU1190          C          -----          STRU2650
          STRU1200          C          -----          STRU2660
          STRU1210          C          -----          STRU2670
          STRU1220          C          -----          STRU2680
          STRU1230          C          -----          STRU2690
          STRU1240          C          -----          STRU2700
          STRU1250          C          -----          STRU2710
          STRU1260          C          -----          STRU2720
          STRU1270          C          -----          STRU2730
          STRU1280          C          -----          STRU2740
          STRU1290          C          -----          STRU2750
          STRU1300          C          -----          STRU2760
          STRU1310          C          -----          STRU2770
          STRU1320          C          -----          STRU2780
          STRU1330          C          -----          STRU2790
          STRU1340          C          -----          STRU2800
          STRU1350          C          -----          STRU2810
          STRU1360          C          -----          STRU2820
          STRU1370          C          -----          STRU2830
          STRU1380          C          -----          STRU2840
          STRU1390          C          -----          STRU2850
          STRU1400          C          -----          STRU2860
          STRU1410          C          -----          STRU2870
          STRU1420          C          -----          STRU2880
          STRU1430          C          -----          STRU2890
          STRU1440          C          -----          STRU2900
          STRU1450          C          -----          STRU2910
          STRU1460          C          -----          STRU2920

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ELSE IF (TEST(TYPE,'I3DBRAM',.FALSE.)) THEN
  NINPUT= 14
  NINZ= 614
  NELTY= 9
C
ELSE IF (TEST(TYPE,'STABILITY',.FALSE.)) THEN
  NINPUT= 8
  NINZ= 618
  NELTY= 12
ELSE IF (TEST(TYPE,'SHBAR_OPEN',.FALSE.)) THEN
  NINPUT= 9
  NINZ= 775
  NELTY= 14
C
TO ADD ADDITIONAL ELEMENTS, COPY THE NEXT FOUR LINES
ELSE IF (TEST(TYPE,'(1)',.FALSE.)) THEN
  NINPUT= (2)
  NINZ= (3)
  NELTY= (4)
C
CHANGE THE FOLLOWING VALUES
(1) YOUR ELEMENT NAME
(2) # OF VALUES TO BE INPUT
(3) # OF STORAGE LOCATIONS READ FOR ELEMENT
(4) ELEMENT ID NUMBER - *K
ADD CALL ELEXX TO SUBPROGRAM ELE_LIB
ADD SUBPROGRAM ELEXX
C
ELSE IF (TEST(TYPE,'END',.FALSE.)) THEN
  NEMT=IELNO-1
  NZ(IZELE)=NEMT
  GO TO 300
ELSE
  WRITE(106,*) 'INVALID ELEMENT TYPE ',HEAD
  GO TO 210
ENDIF
BACKSPACE(105)
READ(105,*) TYPE,NAME,( RINPUT(1),I-1,NINPUT),IGEN
IF (IGEN.GT.0) READ(105,*) (GINPUT(1),I-1,NINPUT)
ELSE
  IGEN=IGEN-1
  NAME='GENERATED'
  DO 220 I=1,NINPUT
    RINPUT(I)=RINPUT(I)-GINPUT(I)
220
ENDIF
RESERVE STORAGE
NZ(IZELE)=IELNO+IZ
IC=IZ
IZWK=IZ
C
STORE ELEMENT TYPE
NZ(IC)=NELTY
C
CALL ELEMENT LIBRARY TO DETERMINE THE STORAGE FOR MATL DATA
IOPT=9
HEAD=.FALSE.
LSTYP=0
CALL ELELIB
& (IOPT,LSTYP,.TRUE.,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,DUCTPAG,
& IELNO,IELDOP,KGDOP,PGEOM,RINPUT,AXIAL,IZDISP,IZLOAD,HSTOR)
C
RESERVE STORAGE
IZ=IZ-NINZ-MSTOR
IZWK=IZ
C
DU MP ELEMENT DATA IF DESU_GING
IF (BTST(IBUG,8)) THEN
  WRITE(108,*) 'IZLM:',IZLM,' IZKE:',IZKE
  JJ=1
  DO 7100 I=IC,(IZWK-1)-IREL
    JJ=JJ+1
7100 IF (NZ(I).NE.0) WRITE(106,7010) IELNO,JJ,I,NZ(I),2(I)
ENDIF
C
CALL ELEMENT LIBRARY TO INITIALIZE ELEMENT DATA AND RETURN
INFO ON ELEMENT DOP
IOPT=1
CALL ELELIB
& (IOPT,LSTYP,.TRUE.,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,DUCTPAG,
& IELNO,IELDOP,KGDOP,PGEOM,RINPUT,AXIAL,IZDISP,IZLOAD,HSTOR)
C
RELEASE UNUSED STORAGE
IZ=IZ-IREL
C
DU MP ELEMENT DATA IF DESU_GING
IF (BTST(IBUG,8)) THEN
  WRITE(108,*) 'IZLM:',IZLM,' IZKE:',IZKE
  JJ=1
  DO 7000 I=IC,(IZWK-1)-IREL
    JJ=JJ+1
7000 IF (NZ(I).NE.0) WRITE(106,7010) IELNO,JJ,I,NZ(I),2(I)
7010 FORMAT (' IELNO:',I3,' #',I6,' 2',I6,' ',I10,I9,'G1.8')
ENDIF
C
RESERVE SPACE IN THE SKYLINE FOR THE STIFFNESS
JFLAG=0
IF (TEST(TYPE,'STABILITY',.FALSE.)) JFLAG=1
CALL SKYLN(JFLAG,2,NDOP,IBUG,TITLE,NZ(IZMD),
& IELDOP,NZ(IZLM),2(IZKE),MD,'GLOBAL STIFFNESS')
C
RESERVE SPACE IN THE SKYLINE FOR THE GEOMETRIC STIFFNESS
IF (NZ(IZKDT-3).EQ.1 .AND. AXIAL) THEN
  CALL SKYLN(0,2,NDOP,IBUG,TITLE,NZ(IZMD),
& KGDOP,NZ(IZLMKG),2(IZKKG),MD,'GLOBAL GEOMETRIC STIFFNESS')
ELSE IF (NZ(IZKDT-3).EQ.2 .AND. AXIAL) THEN
  CALL SKYLN(0,2,NDOP,IBUG,TITLE,NZ(IZMDKG),
& KGDOP,NZ(IZLMKG),2(IZKKG),MD,'GLOBAL GEOMETRIC STIFFNESS')
ENDIF
C
IF (IELNO.LT.NELMT) GO TO 210
300 CONTINUE
C
-----
C- MODIFY GEOMETRIC STIFFNESS MATRIX FOR CONDENSATION ---
C- THE CONDENSATION PROCESS MAY EXPAND THE GEOMETRIC STIFFNESS MATRIX'S
C- SKYLINE. THE MAXIMUM SKYLINE OF BOTH THE GEOMETRIC STIFFNESS AND
C- THE STIFFNESS MATRIX IS USED.
IF (NZ(IZKDT-3).EQ.2 .AND. KCOND .AND. NCOND.GT.0) THEN
  DO 310 I=0,NDOP
    J=I-1
    MDXG1=NZ(IZMDKG-1)
    NZ(IZMDKG-1)=MAX( NZ(IZMDKG-1),NZ(IZMD-1) )
    NZ(IZMDKG-1)=MIN( NZ(IZMDKG-1),NZ(IZMD-1) )
310 CONTINUE
ENDIF
C
SET UP GLOBAL STIFFNESS STORAGE ---
C-----
NZ(IZMD) = INDICES OF THE MAIN DIAGONAL TERMS
2(IZSTIP) = GLOBAL STIFFNESS MATRIX
C
CALL SKYLN(0,3,NDOP,IBUG,TITLE,NZ(IZMD),
& IELDOP,NZ(IZLM),2(IZKE),MD,'GLOBAL STIFFNESS')
IZSTIP=IZ
IZ = IZ-NZ(IZMD-NDOP)+1
C
SET UP STORAGE FOR GEOMETRIC STIFFNESS DATA ---
C-----
NZ(IZMDKG) = INDICES OF THE MAIN DIAGONAL TERMS
2(IZKG) = GLOBAL GEOMETRIC STIFFNESS MATRIX
IF (NZ(IZKDT-3).EQ.2) THEN
  CALL SKYLN(0,3,NDOP,IBUG,TITLE,NZ(IZMDKG),
& KGDOP,NZ(IZLMKG),2(IZKKG),MD,'GLOBAL GEOMETRIC STIFFNESS')

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C SE(23)-EISE(2) MAT10830
C SE(24)-EPSE MAT10831
C SE(25)-PEBOLD MAT10840
C SE(26)-CSE MAT10850
C SE(27)-DMAX ULTIMATE DISPLACEMENT MAT10860
C SE(28)-D MAX MAXIMUM ELEMENT DISPLACEMENT FOR DAMAGE INDEX MAT10870
C SE(29)-P MAX ELEMENT LOAD CORRESPONDING TO D MAX MAT10880
C SE(30)-D MAX CURRENT MAXIMUM AXIAL DISPLACEMENT FOR DUCTILITIES MAT10890
C SE(31)-INITIAL STIFFNESS MAT10900
-----
DO 210 I=1,31 MAT10910
210 SE(I)=0 MAT10920
SE(20)-SMAT(1) MAT10940
SE(5)=1 MAT10950
MOPT=IOPT MAT10960
CALL HYST11(MOPT,SE(1),C.,0.,0.,C.,SE(3),SE(4),SE(5),SE(6),SE(7), MAT10970
& SE(8),SE(9),SMAT(1),SMAT(2),SMAT(3),SE(11),SE(13),SMAT(4)) MAT10980
SE(26)-SE(20)*SMAT(2)/4 MAT10990
SE(27)-SMAT(1)*SMAT(4) MAT11000
SE(28)=0 MAT11010
SE(29)=0 MAT11020
SE(31)-SE(1) MAT11030
C RETURN MAT11040
-----
C --- GET STIFFNESS TERMS -----
300 CONTINUE MAT11060
SE(21)=P*PL MAT11070
SE(3)=D*DL MAT11080
C CALL HYST11 TO CALC NEW STIFFNESS STIP MAT11090
MOPT=IOPT MAT11100
CALL HYST11(MOPT,SE(1),P,D,PL,DL,SE(3),SE(4),SE(5),SE(6),SE(7), MAT11110
& SE(8),SE(9),SMAT(1),SMAT(2),SMAT(3),SE(11),SE(13),SMAT(4)) MAT11120
C STORE MAXIMUM DISPLACEMENT AND CORRESPONDING LOAD MAT11130
IF(ABS(D).GT.ABS(SE(28))) THEN MAT11140
SE(28)=D MAT11150
SE(29)=P MAT11160
ENDIF MAT11170
-----
C --- GET ENERGIES, DUCTILITY AND EXCURSION FOR BOTH IOPT=3 AND 4 ---
400 CONTINUE MAT11220
C --- TRANSFER ENERGIES FOR BOTH IOPT=3 AND 4. MAT11230
EISE=SE(21) MAT11240
EPSE=SE(24) MAT11250
C --- TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT=3 AND 4. MAT11260
DO 410 I=1,3 MAT11270
410 DUCT(I)=SE(10+I) MAT11280
EXCR(I)=SE(13+I) MAT11290
EXCR(I)=SE(13+I) MAT11300
C RETURN MAT11310
-----
C --- DAMAGE INDEX -----
500 CONTINUE MAT11360
C --- DAMAGE INDEX IS NOT AVAILABLE MAT11370
DAMAGE=0 MAT11380
C RETURN MAT11390
ENDIF MAT11400
-----
C --- SUBROUTINE TOWER SHORT DIRECTION GIRDER MOMENT-ROTATION MAT11410
HYSTERESIS MODEL BASED ON HYSTERESIS LOOP OF TIA GIRDER MAT11420
SUBROUTINE HYST11(MOPT,STIP,P,X,PLA,XLA,DX,RV,PLD,PTOP,DTOP, MAT11430
& PBOT,DBOT,HA,VA,RATIO, DUC,DEMAX,DUCHMAX) MAT11440
LOGICAL HIGH MAT11450
-----
C --- ELEMENT DUCTILITY RATE MAT11500
C --- ELEMENT EXCURSION RATE MAT11510
C --- ELASTIC STRAIN RATE MAT11520
C --- ELEMENT PLASTIC STRAIN RATE MAT11530
C --- ELEMENT PLASTIC STRAIN RATE AT LAST ZERO CROSSING MAT11540
C --- ELEMENT CONSTANT STRAIN ENERGY MAT11550
C --- STRAIN HARDEN RATIO TO ELASTIC STIFFNESS MAT11560
C --- ELEMENT STIFFNESS MAT11570
C --- CURRENT ELEMENT INTERNAL FORCE MAT11580
C --- CURRENT ELEMENT INTERNAL DISPLACEMENT MAT11590
C --- PREVIOUS ELEMENT INTERNAL FORCE MAT11600
C --- PREVIOUS ELEMENT INTERNAL DISPLACEMENT MAT11610
C --- ELEMENT INTERNAL FORCE INCREMENT MAT11620
C --- ELEMENT INTERNAL DISPLACEMENT INCREMENT MAT11630
C --- LOGICAL INDEX, IF P.GT.0 THEN HIGH=.TRUE. MAT11640
C --- RULE NUMBER MAT11650
C --- REVERSED LOADING POINT AT TOP ENVELOPE MAT11660
C --- REVERSED DISP. POINT AT TOP ENVELOPE MAT11670
C --- REVERSED DISP. POINT AT BOTTOM ENVELOPE MAT11680
C --- FIRST BUCKLING POINT AT TOP ENVELOPE MAT11690
C --- FAILURE DUCTILITY MAT11700
C --- LT. 1 FAILURE DUCTILITY CONSIDERED MAT11710
C --- LT. 1 FAILURE DUCTILITY NOT CONSIDERED MAT11720
-----
C --- CHECK FAILURE DUCTILITY ---
IF( DUC(1).GE.DUCHMAX.AND.DUCHMAX.GE.1) THEN MAT11730
P=0 MAT11740
STIP=0.001 MAT11750
RETURN MAT11760
ENDIF MAT11770
IF(RV.EQ.1) HIGH=.FALSE. MAT11780
IF(RV.EQ.0) HIGH=.TRUE. MAT11790
-----
C --- DEFINE ENVELOPE CONTROL POINTS ---
IF(MOPT.EQ.3) THEN MAT11800
PTOP=VA MAT11810
PBOT=VA MAT11820
DTOP=HA MAT11830
DBOT=HA MAT11840
ENDIF MAT11850
IF(P.GT.0) HIGH=.TRUE. MAT11860
IF(P.LT.0) HIGH=.FALSE. MAT11870
IF(HIGH)RV=1 MAT11880
IF(.NOT.HIGH)RV=1 MAT11890
-----
C --- DEFINE ENVELOPE SLOPE ---
SLOP1=VA/HA MAT11900
SLOP2=RATIO*SLOP1 MAT11910
-----
C --- HYSTERESIS LOOP RULES ---
RULE 1 MAT11920
IF(RULE.EQ.1) THEN MAT11930
IF(P.GE.PTOP.OR.P.LE.PBOT) THEN MAT11940
RULE MAT11950
STIP=SLOP2 MAT11960
*** UNBALANCED FORCE *** MAT11970
IF(P.GE.PTOP) THEN MAT11980
UD=UP1/SLOP1 MAT11990
UP2=UD*SLOP2 MAT12000
P=P+(UP1-UP2) MAT12010
ELSE P.LE.PBOT) THEN MAT12020
UP1=P-PBOT MAT12030

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UD=UP1/SLOP1 MAT10830
UP2=UD*SLOP2 MAT10831
P=P+(UP1-UP2) MAT10832
ENDIF MAT10833
ELSE MAT10834
RULE=1 MAT10835
STIP=SLOP1 MAT10836
ENDIF MAT10837
IF( DMAX.LT.ABS(X)) THEN MAT10838
DMAX=ABS(X) MAT10839
DUC(1)=DMAX/HA MAT10840
ENDIF MAT10841
C ---RULE 2: MAT10842
IF( HIGH.AND.DX.GT.0) THEN MAT10843
RULE=2 MAT10844
STIP=SLOP2 MAT10845
ELSE IF(.NOT.HIGH.AND.DX.LT.0) THEN MAT10846
STIP=SLOP1 MAT10847
ELSE IF( HIGH.AND.DX.LE.0) THEN MAT10848
PTOP=PLA MAT10849
DTOP=XLA MAT10850
RULE=1 MAT10851
STIP=SLOP1 MAT10852
PBOT=PTOP-2*VA MAT10853
DBOT=DTOP-(PTOP-PBOT)/SLOP1 MAT10854
*** UNBALANCED FORCE *** MAT10855
UD=X-DTOP MAT10856
UP=UD+SLOP1 MAT10857
P=PTOP-UP MAT10858
ELSE IF(.NOT.HIGH.AND.DX.GE.0) THEN MAT10859
RULE=1 MAT10860
STIP=SLOP1 MAT10861
PTOP=PTOP-2*VA MAT10862
DTOP=DTOP-(PTOP-PBOT)/SLOP1 MAT10863
*** UNBALANCED FORCE *** MAT10864
UD=X-DTOP MAT10865
UP=UD+SLOP1 MAT10866
P=PTOP-UP MAT10867
ENDIF MAT10868
IF( DMAX.LT.ABS(X)) THEN MAT10869
DMAX=ABS(X) MAT10870
DUC(1)=DMAX/HA MAT10871
ENDIF MAT10872
RETURN MAT10873
END MAT10874
-----
C --- SUBROUTINE MAT13 (MOPT, FIRST, MAT, SE, P, PL, D, DL, LELM, LPMAT, MAT13000
& SMAT, EISE, EPSE, DUCT, EXCR, DAMAGE) MAT13010
LOGICAL FIRST MAT13020
DIMENSION SE(100),SMAT(8),DUCT(3),EXCR(6) MAT13030
CHARACTER*80 TYPE MAT13040
C --- RESERVE STORAGE FOR ELEMENT MAT13050
LELM=39 MAT13060
IF(IOPT.EQ.0) RETURN MAT13070
C --- MAT13080
C --- MAT13090
C --- MAT13100
C --- MAT13110
C --- MAT13120
C --- MAT13130
C --- MAT13140
C --- MAT13150
C --- MAT13160
C --- MAT13170
C --- MAT13180
C --- MAT13190
C --- MAT13200
C --- MAT13210
C --- MAT13220
C --- MAT13230
C --- MAT13240
C --- MAT13250
C --- MAT13260
C --- MAT13270
C --- MAT13280
C --- MAT13290
C --- MAT13300
C --- MAT13310
C --- MAT13320
C --- MAT13330
C --- MAT13340
C --- MAT13350
C --- MAT13360
C --- MAT13370
C --- MAT13380
C --- MAT13390
C --- MAT13400
C --- MAT13410
C --- MAT13420
C --- MAT13430
C --- MAT13440
C --- MAT13450
C --- MAT13460
C --- MAT13470
C --- MAT13480
C --- MAT13490
C --- MAT13500
C --- MAT13510
C --- MAT13520
C --- MAT13530
C --- MAT13540
C --- MAT13550
C --- MAT13560
C --- MAT13570
C --- MAT13580
C --- MAT13590
C --- MAT13600
C --- MAT13610
C --- MAT13620
C --- MAT13630
C --- MAT13640
C --- MAT13650
C --- MAT13660
C --- MAT13670
C --- MAT13680
C --- MAT13690
C --- MAT13700
C --- MAT13710
C --- MAT13720
C --- MAT13730
C --- MAT13740
C --- MAT13750
C --- MAT13760
C --- MAT13770
C --- MAT13780
C --- MAT13790
C --- MAT13800
C --- MAT13810
C --- MAT13820
C --- MAT13830
C --- MAT13840
C --- MAT13850
C --- MAT13860
C --- MAT13870
C --- MAT13880
C --- MAT13890
C --- MAT13900
C --- MAT13910
C --- MAT13920
C --- MAT13930
C --- MAT13940
C --- MAT13950
C --- MAT13960
C --- MAT13970
C --- MAT13980
C --- MAT13990
C --- MAT14000

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C SE(34)= HC CURRENT CONTROL POINT HC MAT10930
C SE(35)= VC CURRENT CONTROL POINT VC MAT10940
C SE(36)= RSN CURRENT RESIDUAL STRAIN VALUE MAT10950
C SE(37)= DMAX CURRENT MAXIMUM AXIAL DISPLACEMENT FOR DUCTILITIES MAT10960
C SE(38)= REGIN CURRENT REGIN ZONE NUMBER MAT10970
C SE(39)= PREGIN PREVIOUS REGIN ZONE NUMBER MAT10980
-----
C SE(1)=1 MAT10990
C SE(2)=1 MAT11010
C SE(3)=1 MAT11020
C SE(4)=1 MAT11030
C DO 210 I=6,28 MAT11040
210 SE(11)=0 MAT11050
C SE(17)=0 MAT11060
C SE(18)=1 MAT11070
C SE(19)=1 MAT11080
C CALL HYST13(0.,0.,0.,0.,SE(5),SE(2),SMAT(1),SMAT(2),SMAT(3),
4 SMAT(4),SMAT(5),SMAT(6),SE(3),SE(4),SE(11),SE(6),SE(7),
4 SE(12),SE(13),SE(14),SE(15),SE(16),SE(17),SE(18),SE(19),
4 SE(8),SE(9),SE(10),SE(11),SE(12),SE(13),SE(16),SE(22),
4 SE(23),SE(26),SE(27),SE(28)) MAT11100
C SE(28)=SE(22)*SMAT(5)/2 MAT11140
C SE(29)=SE(22)*SMAT(7) MAT11150
C SE(30)=0 MAT11160
C SE(31)=0 MAT11170
C RETURN MAT11190
C----- GET STIFFNESS TERMS ----- MAT11200
C 300 CONTINUE MAT11210
C----- CALL HYST13 TO CALC NEW STIFFNESS STIP MAT11230
C CALL HYST13(P,PL,D,DL,SE(5),SE(2),SMAT(1),SMAT(2),SMAT(3),
4 SMAT(4),SMAT(5),SMAT(6),SE(3),SE(4),SE(11),SE(6),SE(7),
4 SE(12),SE(13),SE(14),SE(15),SE(16),SE(17),SE(18),SE(19),
4 SE(8),SE(9),SE(10),SE(11),SE(12),SE(13),SE(16),SE(22),
4 SE(23),SE(26),SE(27),SE(28)) MAT11240
C STORE MAXIMUM DISPLACEMENT AND CORRESPONDING LOAD MAT11300
C IP(ABS(D).GT.ABS(SE(10))) THEN MAT11310
C SE(10)=D MAT11320
C SE(11)=P MAT11340
C ENDIF MAT11350
C----- GET ENERGIES, DUCTILITY AND EXCURSION FOR BOTH IOPT=3 AND 4 ----- MAT11360
C 400 CONTINUE MAT11370
C----- TRANSFER ENERGIES FOR BOTH IOPT=3 AND 4 MAT11390
C ESE=SE(23) MAT11400
C EPSE=SE(26) MAT11410
C----- TRANSFER DUCTILITIES AND EXCURSION RATIOS FOR BOTH IOPT=3 AND 4. MAT11420
C DO 410 I=1,3 MAT11430
C DUCT(I)=SE(12*I) MAT11440
C EXCR(I+3)=SE(15*I+3) MAT11450
C EXCR(I)=SE(15-I) MAT11460
410 MAT11470
C RETURN MAT11480
C----- DAMAGE INDEX ----- MAT11490
C 500 CONTINUE MAT11500
C ESE=SE(23) MAT11510
C EPSE=SE(26) MAT11520
C DMAX=SE(29) MAT11530
C D=SE(30) MAT11540
C P=SE(31) MAT11550
C PY=SMAT(5) MAT11560
C BDAM=SMAT(8) MAT11570
C DAMA=ABS(D)/DMAX + BDAM / (PY*DMAX) * EPSE MAT11580
C RETURN MAT11590
C END MAT11600
C----- GOEL TRUSS HYSTERESIS MODEL FOR SIMULATED LOCAL BUCKLING MAT11620
C OF BOX SECTION MAT11630
C NOTE: IT IS ASSUMED THAT OVERALL BUCKLING WILL NOT OCCURRED MAT11640
C----- SUBROUTINE HYST13(P,PLA,DE,DELA,EL,SLK,EM,AREA,RADG,YS,PY,
4 CC,CYCLE,RULE,STIP,IRV,IDUC,HA,VA,HC,VC,RSN,DMAX,
4 REGIN,PREGIN,PBOT,PDOT,PTOP,DTOP,EXI,DOC,
4 EXCR,DELY,ESE,PSE,PSEOLD,CSE) MAT11650
C DIMENSION DUC(3), EXCR(6), ESE(3) MAT11660
C LOGICAL REVRSC MAT11670
C REAL*4 IRV, IDUC MAT11680
C IF( IRV.EQ.0.) REVRSC=FALSE. MAT11690
C IF( IRV.NE.1.) REVRSC=TRUE. MAT11700
C PI=3.1415926 MAT11710
C ES=AREA*EM/EL MAT11720
C DELV=PY/ES MAT11730
C DEL=DE/DELY MAT11740
C PL=P/PY MAT11750
C ESR=SLK*EL/RADG MAT11760
C SLOP=STIP/ES MAT11770
C DA=ABS(D) MAT11780
C DP=P*PLA MAT11790
C DDE=DE-DELA MAT11800
C DDEL=DDE/DELY MAT11810
C----- CALCULATE EXCURSION RATE AT EVERY ZERO CROSSING ----- MAT11820
C IF (P*PLA.LE.0.) CALL DNE(D,DOC,EXCR,DELY,DA,ESE,PSE,PSEOLD,CSE) MAT11830
C-----
C DUC ---- ELEMENT DUCTILITY RATE MAT11840
C EXCR ---- ELEMENT EXCURSION RATE MAT11850
C DELV ---- ELEMENT YIELDING DISPLACEMENT MAT11860
C ESE ---- ELEMENT ELASTIC STRAIN ENERGY MAT11870
C PSE ---- ELEMENT PLASTIC STRAIN ENERGY MAT11880
C PSEOLD ---- ELEMENT PLASTIC STRAIN ENERGY AT LAST ZERO CROSSING MAT11890
C CSE ---- ELEMENT CONSTANT STRAIN ENERGY MAT11900
C DP ---- ELEMENT AXIAL LOAD INCREMENT (KIPS) MAT11910
C ESR ---- EFFECTIVE SLENDER RATIO (KL/R) MAT11920
C YS ---- YIELDING STRESS MAT11930
C AREA ---- CROSS SECTION AREA (A) MAT11940
C EM ---- ELASTIC MODULUS (E) MAT11950
C DP ---- ELEMENT AXIAL LOAD INCREMENT (KIPS) MAT11960
C P ---- CURRENT AXIAL LOAD (KIPS) MAT11970
C PL ---- P / PY RATIO (P/PY) MAT11980
C PLA ---- LAST AXIAL LOAD MAT11990
C DDEL ---- DISPLACEMENT INCREMENT (IN) MAT12000
C DE ---- CURRENT AXIAL DISPLACEMENT (IN) MAT12010
C DMAX ---- MAXIMUM AXIAL DISPLACEMENT (IN) MAT12020
C DA ---- CURRENT ABSOLUTE AXIAL DISPLACEMENT (IN) MAT12030
C DELA ---- LAST AXIAL DISPLACEMENT MAT12040
C DDE ---- ELEMENT DISPLACEMENT INCREMENT MAT12050
C DEL ---- DEL-DE/DELY MAT12060
C DELV ---- CURRENT DISPLACEMENT MAT12070
C V ---- YIELDING VELOCITY MAT12080
C ES ---- ELEMENT ELASTIC STIFFNESS MAT12090
C EXI ---- UNLOADING EQUIVALENT STIFFNESS FOR CALCULATING ESE MAT12100
C PY ---- TENSION YIELDING LOAD MAT12110
C E ---- ELEMENT LENGTH (IN) MAT12120
C CC ---- C SUB. C CONSTANT MAT12130
C PMAK ---- BUCKLING LOAD (KIPS) MAT12140
C DELMAX ---- BUCKLING DISPLACEMENT (IN) MAT12150
C HA,HB,HC,HD,HE,HEP ---- P/PY DE/DELY PLOT'S DE/DELY COORDINATE MAT12160
C VA,VB,VC,VD,VE ---- P/PY DE/DELY PLOT'S P/PY COORDINATE MAT12170
C RSN ---- RESIDUAL STRAIN MAT12180
C RULE ---- RULE NUMBER MAT12190
C CYCLE ---- NO OF CYCLE MAT12200
C STIP ---- ELEMENT STIFFNESS MAT12210
C REVRSC ---- LOGICAL INDEX FOR REVERSING CURVE OCCURRED MAT12220
C IDUC ---- 1: HC LESS THAN 1, 0: HC=12 MAT12230
C REGIN ---- REGIN 1: DE HA MAT12240
C 2: DE = (HH, HA) MAT12250
C 3: DE = (HB, HH) MAT12260

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C 4. DE = (HC, HB) MAT12270
C PREGIN ---- PREVIOUS STEP REGIN ID NUMBER MAT12280
-----
C INITIAL BUCKLING LOAD MAT12290
C IF( ESR.LT.CC.AND.CYCLE.EQ.1) THEN MAT12300
C PHAX=PI*AREA MAT12310
C ELSE IF( ESR.GE.CC.AND.CYCLE.EQ.1) THEN MAT12320
C PHAX=PI*PI*AREA*EM/(ESR**2) MAT12330
C ENDIF MAT12340
C RESIDUAL ELONGATION FACTOR RFAC MAT12350
C RFAC=1.4 MAT12360
C GENERATE CONTROL POINTS COORDINATE MAT12370
C (1) POINT A MAT12380
C IF( CYCLE.EQ.1) THEN MAT12390
C VA=PHAX/PY MAT12400
C HA=PHAX/(ES*DELY) MAT12410
C ELSE IF( CYCLE.EQ.2) THEN MAT12420
C IF( ESR.LE.40) THEN MAT12430
C PHAX=-0.2*PY MAT12440
C ELSE IF( ESR.GT.40) THEN MAT12450
C PHAX=-0.2*PY MAT12460
C ENDIF MAT12470
C ELSE IF( CYCLE.GT.2) THEN MAT12480
C IF( ESR.LE.40) THEN MAT12490
C PHAX=-0.2*PY MAT12500
C ELSE IF( ESR.GT.40) THEN MAT12510
C PHAX=-0.2*PY MAT12520
C ENDIF MAT12530
C (2) POINT B MAT12540
C HB=-5 MAT12550
C IF( ESR.LE.40) THEN MAT12560
C VB=-0.15 MAT12570
C ELSE IF( ESR.GT.40) THEN MAT12580
C VB=-0.15 MAT12590
C ENDIF MAT12600
C (3) POINT C MAT12610
C IF( IDUC.EQ.0.) THEN MAT12620
C HC=-12 MAT12630
C IF( ESR.LE.40) THEN MAT12640
C VC=0.1 MAT12650
C ELSE IF( ESR.GT.40) THEN MAT12660
C VC=0.1 MAT12670
C ENDIF MAT12680
C (4) POINT E MAT12690
C HE=1 - (RSN*EL/DELY) MAT12700
C VE=1. MAT12710
C (5) POINT D1 MAT12720
C SLOPE=(VE-VC)/(HE-HC) MAT12730
C HD1=(SLOPE*HC-VC)/(SLOPE-0.267949) MAT12740
C VD1=-0.267949/HD1 MAT12750
C (6) POINT D MAT12760
C IF( ESR.LE.40) THEN MAT12770
C HD=HD1*31./40 MAT12780
C VD=VD1*31./40 MAT12790
C ELSE IF( ESR.GT.40) THEN MAT12800
C HD=HD1*20./40 MAT12810
C VD=VD1*20./40 MAT12820
C ENDIF MAT12830
C DEFINE ENVELOPE SLOPE MAT12840
C SLOP1=1. MAT12850
C SLOP2=(VB-VA)/(HB-HA) MAT12860
C SLOP3=(VC-VB)/(HC-HB) MAT12870
C SLOP4=(VD-VC)/(HD-HC) MAT12880
C SLOP5=(VE-VD)/(HE-HD) MAT12890
C SLOP6=0.0001 MAT12900
C DEFINE ENVELOPE INTERMEDIATE POINTS MAT12910
C (1) POINT H MAT12920
C VH=(VB-SLOP2*(-VD-HD-HB))/(1.-SLOP2) MAT12930
C HV=VH+VD+HD MAT12940
C (2) POINT C1 MAT12950
C VC1=0. MAT12960
C HC1=HC-(VC/SLOP4) MAT12970
C (3) POINT (PREP,DREP) : CALCULATE IN RULE NO. 4 MAT12980
C (4) POINT (HREF,VREF) : CALCULATE IN RULE NO. 5 MAT12990
C----- DEFINE CURRENT REGIN ZONE NUMBER MAT13000
C PREGIN=REGIN MAT13010
C IF( DEL.GE.HA) REGIN=1 MAT13020
C IF( DEL.GE.HH.AND.DEL.LT.HA) REGIN=2 MAT13030
C IF( DEL.GE.HB.AND.DEL.LT.HH) REGIN=3 MAT13040
C IF( DEL.GE.HC.AND.DEL.LT.HB) REGIN=4 MAT13050
C----- GOEL'S HYSTERESIS LOOP RULES MAT13060
C (1) RULE 1 MAT13070
C IF( RULE.EQ.1) THEN MAT13080
C IF( P.GE.PHAX.AND.P.LT.PY) THEN MAT13090
C STIP=SLOP1*ES MAT13100
C EXI=SLOP1*ES MAT13110
C IF( DMAX.LT.0.AND.DE.LE.DMAX) THEN MAT13120
C RSN=((0.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC MAT13130
C ELSE IF( P.GE.PY) THEN MAT13140
C STIP=SLOP2*ES MAT13150
C EXI=SLOP1*ES MAT13160
C ELSE IF( P.LT.PHAX) THEN MAT13170
C HA=DEL MAT13180
C VA=DE MAT13190
C SLOP2=(VB-VA)/(HB-HA) MAT13200
C STIP=SLOP2*ES MAT13210
C RULE=2 MAT13220
C IF( DMAX.LT.0.AND.DE.LE.DMAX) THEN MAT13230
C RSN=((0.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC MAT13240
C ENDIF MAT13250
C EXI=SLOP1*ES MAT13260
C ENDIF MAT13270
C CALL STRENG(ESE,PSE,PSEOLD,P,PLA,DE,DELA,EXI) MAT13280
C CALL DNE(1,DOC,EXCR,DELY,DA,ESE,PSE,PSEOLD,CSE) MAT13290
C RETURN MAT13300
C (2) RULE 2 MAT13310

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IF( RULE EQ 2.1 ) THEN
  IF( DEL GE. HH ) THEN
    IF( DDE LE 0 ) THEN
      IF( PREGIN .NE. REGIN ) REVRSC= FALSE.
      STIP-SLOP2*ES
      IF( DEMAX LT. 0 .AND. DE LE. DEMAX ) THEN
        RSN=(10.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC
      ELSE IF( DEMAX GT. 0 ) THEN
        RSN=(10.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC
      ENDIF
    ELSE IF( NOT REVRSC ) THEN
      EKI-SLOP1*ES
    ELSE IF( REVRSC ) THEN
      EKI-ES*((PTOP-PL)/(DTOP-DEL))
    ENDIF
  ELSE IF( DDE GT. 0 ) THEN
    IF( NOT REVRSC ) THEN
      STIP-SLOP1*ES
      PLA-PL-SLOP2*DDEL
      DLA-DEL-DDEL
      DBOT-DLA
      PBOT-PLA
      PL-PLA-SLOP1*DDEL
      P-PL*PY
      PTOP-VD*(SLOP5*(PBOT-VE-SLOP1*DBOT
        +SLOP1*HB)/(SLOP5-SLOP1))
      DTOP-(PBOT-VE-SLOP5*HB-SLOP1*DBOT)
        / (SLOP5-SLOP1)
      EKI-ES-SLOP1
    ELSE IF( REVRSC ) THEN
      RULE-12
      DBOT-DEL
      PBOT-P/PY
      SLOP7-(PTOP-PBOT)/(DTOP-DBOT)
      STIP-SLOP7*ES
      EKI-ES-SLOP7
    ENDIF
  ELSE IF( DEL LT. HH .AND. DEL GE. HB ) THEN
    IF( DDE LE 0 ) THEN
      IF( DEMAX LT. 0 .AND. DE LE. DEMAX ) THEN
        RSN=(10.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC
      ELSE IF( DEMAX GT. 0 ) THEN
        RSN=(10.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC
      ENDIF
    ELSE IF( NOT REVRSC ) THEN
      EKI-SLOP1*ES
    ELSE IF( REVRSC ) THEN
      EKI-ES*((PTOP-PL)/(DTOP-DEL))
    ENDIF
  ELSE IF( DDE GT. 0 ) THEN
    IF( NOT REVRSC ) THEN
      RULE-8
      STIP-SLOP1*ES
      PLA-PL-SLOP3*DDEL
      DLA-DEL-DDEL
      DBOT-DLA
      PBOT-PLA
      PL-PLA-SLOP1*DDEL
      P-PL*PY
      PTOP-VD*(SLOP4*(PBOT-VD-SLOP1*DBOT
        +SLOP1*HD)/(SLOP4-SLOP1))
      DTOP-(PBOT-VD-SLOP4*HD-SLOP1*DBOT)
        / (SLOP4-SLOP1)
      EKI-ES-SLOP1
    ELSE IF( REVRSC ) THEN
      RULE-11
      DBOT-DEL
      PBOT-P/PY
      PTOP-VD
      DTOP-HD
      SLOP7-(PTOP-PBOT)/(DTOP-DBOT)
      STIP-SLOP7*ES
      EKI-ES-SLOP7
    ENDIF
  ELSE IF( DEL LT. HB ) THEN
    IF( PREGIN .NE. REGIN ) REVRSC= FALSE.
    RULE-3
    EKI-ES-SLOP1
  ENDIF
  IF( REVRSC ) IRV=1
  IF ( NOT REVRSC ) IRV=0
  CALL STRENG(ESR,PSE,PSEOLD,P,PLA,DE,DELA,EKI)
  CALL DNE(1, DUC, EKCR, DELY, DA, ESE, PSE, PSEOLD, CSE )
  DEMAX-DE
  ENDIF
RETURN
ENDIF
(3) RULE 3
IF( RULE EQ 3.1 ) THEN
  IF( DEL GT. HC ) THEN
    IF( DDE LE 0 ) THEN
      IF( PREGIN .NE. REGIN ) REVRSC= FALSE.
      STIP-SLOP3*ES
      IF( DEMAX LT. 0 .AND. DE LE. DEMAX ) THEN
        RSN=(10.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC
      ELSE IF( DEMAX GT. 0 ) THEN
        RSN=(10.55*ABS(DE)/40)+0.0002*ABS(DE)**2)*RFAC
      ENDIF
    ELSE IF( NOT REVRSC ) THEN
      EKI-SLOP1*ES
    ELSE IF( REVRSC ) THEN
      EKI-ES*((PTOP-PL)/(DTOP-DEL))
    ENDIF
  ELSE IF( DDE GT. 0 ) THEN
    IF( NOT REVRSC ) THEN
      STIP-SLOP1*ES
      PLA-PL-SLOP3*DDEL
      DLA-DEL-DDEL
      DBOT-DLA
      PBOT-PLA
      PL-PLA-SLOP1*DDEL
      P-PL*PY
      PTOP-VD*(SLOP4*(PBOT-VD-SLOP1*DBOT
        +SLOP1*HD)/(SLOP4-SLOP1))
      DTOP-(PBOT-VD-SLOP4*HD-SLOP1*DBOT)
        / (SLOP4-SLOP1)
      EKI-ES-SLOP1
    ELSE IF( REVRSC ) THEN
      RULE-10
      DBOT-DEL
      PBOT-P/PY
      SLOP7-(PTOP-PBOT)/(DTOP-DBOT)
      STIP-SLOP7*ES
      EKI-ES-SLOP7
    ENDIF
  ELSE IF( DEL LE. -12 ) THEN
    REVRSC= FALSE.
    RULE-4
    STIP-SLOP4*ES
    EKI-ES-SLOP4
  ENDIF
ENDIF
ENDIF
ELSE IF( DEL LE. HC ) THEN
  ENDIF
ELSE IF( DEL LT. 0 ) THEN
  RULE-3
  HC-DEL
  VC-PL
  IDUC-1
  STIP-SLOP6*ES
  EKI-ES*((VD-VC)/(HD-HC))
  IF( DEMAX LT. 0 .AND. DE LE. DEMAX ) THEN
    RSN=(10.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC
  ELSE IF( DEMAX GT. 0 ) THEN
    RSN=(10.55*ABS(DE)/ESR)+0.0002*ABS(DE)**2)*RFAC
  ENDIF
  ELSE IF( DDE GT. 0 ) THEN
    RULE-4
    STIP-SLOP4*ES
    EKI-ES-SLOP4
  ENDIF
  IF( REVRSC ) IRV=1.
  IF ( NOT REVRSC ) IRV=0
  CALL STRENG(ESR,PSE,PSEOLD,P,PLA,DE,DELA,EKI)
  CALL DNE(1, DUC, EKCR, DELY, DA, ESE, PSE, PSEOLD, CSE )
  DEMAX-DE
  ENDIF
RETURN
ENDIF
(4) RULE 7
IF( RULE EQ 7.1 ) THEN
  IF( DEL GT. DBOT .AND. DEL LT. DTOP .AND. P GT. PMAX ) THEN
    STIP-SLOP1*ES
    EKI-ES-SLOP1
  ELSE IF( DEL LT. DBOT OR. P LE. PMAX ) THEN
    IF( P LE. PMAX ) THEN
      REVRSC= FALSE.
      PTOP-C
      DTOP-C
      PBOT-PL
      DBOT-DEL
      HA-DEL
      VA-PL
      SLOP2*(VB-VA)/(HB-HA)
    ENDIF
    STIP-SLOP2*ES
    RULE-2
    PL-PBOT-(DBOT-DEL)*SLOP2
    P-PL*PY
    EKI-ES*((PTOP-PL)/(DTOP-DEL))
  ELSE IF( DEL GT. DTOP .AND. PL LT. VE ) THEN
    STIP-SLOP5*ES
    RULE-5
    EKI-ES-SLOP1
  ELSE IF( DEL GT. DTOP .AND. PL GE. VE ) THEN
    STIP-SLOP6*ES
    RULE-6
    EKI-ES-SLOP1
  ENDIF
  IF( REVRSC ) IRV=1.
  IF ( NOT REVRSC ) IRV=0
  CALL STRENG(ESR,PSE,PSEOLD,P,PLA,DE,DELA,EKI)
  RETURN
ENDIF
(5) RULE 8
IF( RULE EQ 8.1 ) THEN
  IF( DEL GT. DBOT .AND. DEL LT. DTOP ) THEN
    STIP-SLOP1*ES
    EKI-ES-SLOP1
  ELSE IF( DEL LT. DBOT ) THEN
    STIP-SLOP2*ES
    RULE-2
    PL-PBOT-(DBOT-DEL)*SLOP2
    P-PL*PY
    EKI-ES*((PTOP-PL)/(DTOP-DEL))
  ELSE IF( DEL GT. DTOP ) THEN
    STIP-SLOP4*ES
    RULE-4
    EKI-ES-SLOP1
  ENDIF
  IF( REVRSC ) IRV=1.
  IF ( NOT REVRSC ) IRV=0
  CALL STRENG(ESR,PSE,PSEOLD,P,PLA,DE,DELA,EKI)
  RETURN
ENDIF
(6) RULE 9
IF( RULE EQ 9.1 ) THEN
  IF( DEL GT. DBOT .AND. DEL LT. DTOP ) THEN
    STIP-SLOP1*ES
    EKI-ES-SLOP1
  ELSE IF( DEL LT. DBOT ) THEN
    STIP-SLOP1*ES
    RULE-3
    PL-PBOT-(DBOT-DEL)*SLOP3
    P-PL*PY
    EKI-ES*((PTOP-PL)/(DTOP-DEL))
  ELSE IF( DEL GT. DTOP ) THEN
    STIP-SLOP4*ES
    RULE-4
    EKI-ES-SLOP1
  ENDIF
  IF( REVRSC ) IRV=1.
  IF ( NOT REVRSC ) IRV=0
  CALL STRENG(ESR,PSE,PSEOLD,P,PLA,DE,DELA,EKI)
  RETURN
ENDIF
(7) RULE 4
IF( RULE EQ 4.1 ) THEN
  IF( DEL GE. HC1 .AND. DEL LT. HD ) THEN
    IF( DDE LE 0 ) THEN
      STIP-SLOP4*ES
      EKI-ES-SLOP1
    ELSE IF( DDE LT. 0 ) THEN
      PREP-VD*(SLOP4*(VB-VD-SLOP1*HB
        +SLOP1*HD)/(SLOP4-SLOP1))
      DREP-(VB-VD-SLOP4*HD-SLOP1*HB)/(SLOP4-SLOP1)
      IF( DEL LT. DREP ) THEN
        PTOP-P/PY
        DTOP-DEL
        DBOT*(SLOP3*HB-VB+PTOP-SLOP1*DEL)/(SLOP3-SLOP1)
        PBOT-VB-SLOP3*(DBOT-HB)
        STIP-SLOP1*ES
        RULE-9
        REVRSC= TRUE.
        EKI-ES-SLOP1
      ELSE IF( DEL GE. DREP ) THEN
        DTOP-DEL
        DBOT*(SLOP3*HA-VA+PTOP-SLOP1*DEL)/(SLOP3-SLOP1)
        PBOT-VA-SLOP2*(DBOT-HA)
        STIP-SLOP1*ES
        RULE-8
        REVRSC= TRUE.
        EKI-ES-SLOP1
      ENDIF
    ENDIF
  ELSE IF( DEL GT. HD .AND. DDE GT. 0 ) THEN
    STIP-SLOP5*ES
  ENDIF
ENDIF

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      RULE-5
      EXI-ES*SLOP1
    ELSE IF ( DEL .LT. HCL .AND. DEL .GE. HC ) THEN
      REVRSC= .FALSE.
      RULE-4
      STIP-SLOP4*ES
      EXI-ES*SLOP4
    ELSE IF ( DEL .LT. HC ) THEN
      REVRSC= .FALSE.
      RULE-3
      HC-DEL
      VC-PL
      IDUC=1.
      STIP-SLOP5*ES
      EXI-ES*((VD-VC)/(HD-HC))
    ENDIF
    IF ( REVRSC ) IRV=1.
    IF ( .NOT. REVRSC ) IRV=0.
    CALL STRENG(ESSE,PSE,PSEOLD,P,PLA,DE,DELA,EXI)
    IF ( ABS(DPMAX) .LT. ABS(DE) ) THEN
      CALL DNE(1, DUC, EXCR, DELY, DA, ESE, PSE, PSEOLD, CSE )
      DPMAX=DE
    ENDIF
  RETURN
ENDIF

(8) RULE 5
  IF ( RULE .EQ. 5 ) THEN
    IF ( DEL .LT. HR ) THEN
      IF ( DDE .GE. G ) THEN
        STIP-SLOP5*ES
        EXI-ES*SLOP1
      ELSE IF ( DDE .LT. G ) THEN
        BREF=VA-SLOP1*HA
        HREF=(VD-SLOP5*HD-BREF)/(SLOP1-SLOP5)
        IF ( DEL .GE. HREF ) CYCLE-CYCLE*1
        PTOP=P/PPY
        DTOP=DEL
        DBOT=(SLOP2*HA-VA-PTOP-SLOP1*DEL)/(SLOP2-SLOP1)
        PBOT=VA-SLOP2*(DBOT+HA)
        STIP-SLOP1*ES
        RULE-7
        REVRSC= .TRUE.
        EXI-ES*SLOP1
      ENDIF
    ELSE IF ( DEL .GE. HCL .AND. DDE .GT. G ) THEN
      STIP-SLOP6
      RULE-6
      EXI-ES*SLOP1
    ENDIF
    IF ( REVRSC ) IRV=1.
    IF ( .NOT. REVRSC ) IRV=0.
    CALL STRENG(ESSE,PSE,PSEOLD,P,PLA,DE,DELA,EXI)
    IF ( ABS(DPMAX) .LT. ABS(DE) ) THEN
      CALL DNE(1, DUC, EXCR, DELY, DA, ESE, PSE, PSEOLD, CSE )
      DPMAX=DE
    ENDIF
  RETURN
ENDIF

(9) RULE 6
  IF ( RULE .EQ. 6 ) THEN
    IF ( DDE .GE. G ) THEN
      STIP-SLOP6
      EXI-ES*SLOP1
    ELSE IF ( DDE .LT. G ) THEN
      STIP-SLOP1*ES
      RULE-1
      CYCLE-CYCLE*1.
      REVRSC= .FALSE.
      EXI-ES*SLOP1
    ENDIF
    IF ( REVRSC ) IRV=1.
    IF ( .NOT. REVRSC ) IRV=0.
    CALL STRENG(ESSE,PSE,PSEOLD,P,PLA,DE,DELA,EXI)
    IF ( ABS(DPMAX) .LT. ABS(DE) ) THEN
      CALL DNE(1, DUC, EXCR, DELY, DA, ESE, PSE, PSEOLD, CSE )
      DPMAX=DE
    ENDIF
  RETURN
ENDIF

(10) RULE 10
  IF ( RULE .EQ. 10 ) THEN
    IF ( DEL .GT. DBOT .AND. DEL .LT. DTOP ) THEN
      STIP-SLOP7*ES
      EXI-ES*SLOP7
    ELSE IF ( DEL .LT. DBOT ) THEN
      STIP-SLOP3*ES
      REVRSC= .FALSE.
      PTOP=0
      DTOP=0
      DBOT=0
      EXI-ES*((PTOP-PL)/(HD-DEL))
    ELSE IF ( DEL .GT. DTOP ) THEN
      STIP-SLOP4*ES
      RULE-4
      EXI-ES*SLOP1
    ENDIF
    IF ( REVRSC ) IRV=1.
    IF ( .NOT. REVRSC ) IRV=0.
    CALL STRENG(ESSE,PSE,PSEOLD,P,PLA,DE,DELA,EXI)
  RETURN
ENDIF

(11) RULE 11
  IF ( RULE .EQ. 11 ) THEN
    IF ( DEL .GT. DBOT .AND. DEL .LT. DTOP ) THEN
      STIP-SLOP7*ES
      EXI-ES*SLOP7
    ELSE IF ( DEL .LT. DBOT ) THEN
      STIP-SLOP2*ES
      RULE-2
      REVRSC= .FALSE.
      PTOP=0
      DTOP=0
      DBOT=0
      EXI-ES*((VD-PL)/(HD-DEL))
    ELSE IF ( DEL .GT. DTOP ) THEN
      STIP-SLOP4*ES
      RULE-4
      EXI-ES*SLOP1
    ENDIF
    IF ( REVRSC ) IRV=1.
    IF ( .NOT. REVRSC ) IRV=0.
    CALL STRENG(ESSE,PSE,PSEOLD,P,PLA,DE,DELA,EXI)
  RETURN
ENDIF

(12) RULE 12
  IF ( RULE .EQ. 12 ) THEN
    IF ( DEL .GT. DBOT .AND. DEL .LT. DTOP ) THEN
      STIP-SLOP7*ES
      EXI-ES*SLOP7
    ELSE IF ( DEL .LT. DBOT ) THEN
      STIP-SLOP2*ES
      RULE-2
      REVRSC= .FALSE.

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      HYST5150
      HYST5160
      HYST5170
      HYST5180
      HYST5190
      HYST5200
      HYST5210
      HYST5220
      HYST5230
      HYST5240
      HYST5250
      HYST5260
      HYST5270
      HYST5280
      HYST5290
      HYST5300
      HYST5310
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      HYST5370
      HYST5380
      HYST5390
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      HYST5990
      HYST6000
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      HYST6080
      HYST6090
      HYST6100
      HYST6110
      HYST6120
      HYST6130
      HYST6140
      HYST6150
      HYST6160
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      HYST6190
      HYST6200
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      HYST6530
      HYST6540
      HYST6550
      HYST6560
      HYST6570
      HYST6580
      HYST6590
      HYST6600
      PTOP=0
      DTOP=0
      PBOT=0
      DBOT=0
      EXI-ES*((PTOP-PL)/(HD-DEL))
    ELSE IF ( DEL .GT. DTOP ) THEN
      STIP-SLOP5*ES
      RULE-5
      EXI-ES*SLOP1
    ENDIF
  ENDIF
  IF ( REVRSC ) IRV=1.
  IF ( .NOT. REVRSC ) IRV=0.
  CALL STRENG(ESSE,PSE,PSEOLD,P,PLA,DE,DELA,EXI)
  RETURN
ENDIF

-----
SUBROUTINE SOL01
  INCLUDE 'ZCONM'
  READ(105,*) NLOAD,MAXELD
  WRITE(106,1) TITLE(1),TITLE(2),NLOAD
  1 FORMAT(1 STRUCTURE , , 'A,' SOLUTION , , 'A,/'
    & 1X, ' SOLUTION #1, STATIC ELASTIC ANALYSIS ' /
    & 1X, '-----' /
    & 1X, ' NUMBER OF LOAD CASES .....(IS)' )
  IF (N2(12XNDT-3) .EQ. 2) WRITE(106,2)
  2 FORMAT(1X, ' GEOMETRIC STIFFNESS IS NOT INCLUDED. '
    & 1X, ' USE KCDATA(3)-1 TO INCLUDE GEOMETRIC STIFFNESS. )
  ----- INITIALIZE STORAGE FOR STIFFNESS, LOAD AND DISPL. -----
  ELSTIC= .TRUE.
  ----- INITIALIZE STORAGE FOR STIFFNESS, LOAD AND DISPL. -----
  2(I2STIP) = LOCATION OF THE STIFFNESS MATRIX
  2(I2LOAD) = LOCATION OF THE LOAD MATRIX
  2(I2LOAD) = LOCATION OF THE DISPLACEMENT MATRIX
  IF (I2LOAD .EQ. 0) THEN
    I2LOAD = I2
    I2=NDOP*NLOAD
  ENDIF
  IF (I2DISP .EQ. 0) THEN
    I2DISP = I2
    I2=I2+NDOP*NLOAD
  ENDIF
  IF (I2VEL .EQ. 0) THEN
    I2VEL = I2
    I2=I2+NDOP
  ENDIF
  IF (I2VEL-1) .EQ. 0
    DO 3 I=1,NDOP
      3 2(I-1)VEL=I
  ENDIF
  IF (I2VEL-1) .EQ. 0 THEN
    I2=I2+NDOP
  ENDIF
  DO 4 I=1,NDOP
    4 2(I-1)VEL=I-1
  ENDIF
  IF (I2PUB .EQ. 0) THEN
    I2PUB = I2
    I2=I2+NDOP
  ENDIF
  DO 5 I=1,NDOP
    5 2(I-1)PUB=I
  ENDIF
  ----- SET UP CONSTANTS -----
  L1=1
  L1-NCND=NDOP
  L1-NCND=NDOP+1
  L1-NDOP
  IF (MAXELD .GT. 0) THEN
    I2ELD=I2
    I2=I2+MAXELD*5
    I2ELD=I2
    I2=I2+MAXELD*12
  ENDIF
  I2=I2-1
  I2=NDOP+L1
  CALL SOL01A
  & (L1,L2,L3,L4,IMD,NLOAD,MAXELD,NELD,NKND,NCOS,NEHMT,
  & I2DISP,I2LOAD,IBUG,MAXNOD,TITLE,STEPID,ESSE,
  & STIP,LOAD,DISP,IELD,ELD,
  & WORK,MD,ID,ICOP,CONST,
  & TITLE,COSID,COSINE,IJCOS,SUBRECT)
  CHARACTER*(*) TITLE(2),STEPID
  LOGICAL PRINT,DBPFLG,HEAD,AXIAL,BTEST,BUG,BUG3
  CHARACTER*80 NAME
  REAL L1,L2,L3,L4,NLOAD,MAXELD,NELD,NKND,NCOS,NEHMT,
  DIMENSION INPUT(100),SUBRECT(6)
  DIMENSION STIP(IMD),LOAD(L4,NLOAD)
  DIMENSION IELD(MAXELD,5),ELD(MAXELD,12),WORK(1),MD(L4,1)
  DIMENSION COORD(COSINE),IJCOS(SUBRECT),
  DIMENSION IDOF(MAXNOD,6),COSINE(3,3),NCOS(1),CONST(MAXNOD,6)
  & BUG-BTEST(1BUG,5)
  & BUG3-BTEST(1BUG,3)
  RETURN
  END
-----
SUBROUTINE SOL01A
  & (L1,L2,L3,L4,IMD,NLOAD,MAXELD,NELD,NKND,NCOS,NEHMT,
  & I2DISP,I2LOAD,IBUG,MAXNOD,TITLE,STEPID,ESSE,
  & STIP,LOAD,DISP,IELD,ELD,
  & WORK,MD,ID,ICOP,CONST,
  & TITLE,COSID,COSINE,IJCOS,SUBRECT)
  CHARACTER*(*) TITLE(2),STEPID
  LOGICAL PRINT,DBPFLG,HEAD,AXIAL,BTEST,BUG,BUG3
  CHARACTER*80 NAME
  REAL L1,L2,L3,L4,NLOAD,MAXELD,NELD,NKND,NCOS,NEHMT,
  DIMENSION INPUT(100),SUBRECT(6)
  DIMENSION STIP(IMD),LOAD(L4,NLOAD)
  DIMENSION IELD(MAXELD,5),ELD(MAXELD,12),WORK(1),MD(L4,1)
  DIMENSION COORD(COSINE),IJCOS(SUBRECT),
  DIMENSION IDOF(MAXNOD,6),COSINE(3,3),NCOS(1),CONST(MAXNOD,6)
  & BUG-BTEST(1BUG,5)
  & BUG3-BTEST(1BUG,3)
  C-----
  C INPUT LOADING DATA --
  C-----
  C ..... ZERO LOAD AND DISPLACEMENT MATRIX .....
  DO 6 I=1,L4
    DO 6 J=1,NLOAD
      LOAD(I,J)=0
    6 DISP(I,J)=0
  C-----
  C ..... INPUT JOINT LOADS .....
  C JOINT LOADS AND SUPPORT DISPLACEMENTS ARE STORED IN THE
  C DISPLACEMENT MATRIX
  CALL JOILQA(BUG,MAXNOD,L4,NKND,NLOAD,L2,ID,DOF,CONST,DISP,
  & DBPFLG,IJPLG,TITLE, .FALSE )
  C-----
  C ..... INPUT ELEMENT LOADS .....
  C ELEMENT LOADS ARE STORED IN THE LOAD MATRIX
  IF (MAXELD .GT. 0) THEN
    CALL ELQA(BUG,L3,NEHMT,MAXELD,NELD,L4,NLOAD,
    & LOAD,IELD,ELD,I2DISP,I2LOAD,NAME,TITLE, .FALSE )
  C-----
  C ..... ADD ELEMENT LOADS ON FREE JOINTS TO THE JOINT LOADS ON FREE
  C JOINTS, STORE IN THE DISPLACEMENT MATRIX
  C ..... SAVE ELEMENT LOADS ON ALL JOINTS IN LOAD MATRIX, USED LATER
  C ..... FOR CALCULATING THE REACTIONS.
  DO 10 I=L1,L3
    DO 10 J=1,NLOAD
      DISP(I,J)=DISP(I,J)+LOAD(I,J)
    10
  C-----
  C ..... SNAP THE SIGN OF THE FEJ AT REACTIONS
  DO 15 I=L3,L4
    DO 15 J=1,NLOAD
      LOAD(I,J)=-LOAD(I,J)
    15
  C-----
  ENDIF

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C- FORMULATE STIFFNESS ----
CALL FORM(1)
C
C- MODIFY LOADS FOR RESTRAINT DISPLACEMENTS ---
IF (DSPLG) THEN
  IOPT=1
  CALL REACTN(IOPT,BUG,NNODE,L1,L2,L3,L4,DISP,DISP,
    & MD,STIP,IND,L4,NLOAD,1.0
    & MAXNOD,IDOP,COORD,ID,TITLE,STEPID, .FALSE.,
    & NCOS,COSINE,JTCOS,JTFLG,SUMRCT)
  ENDIF
C-----
C- SOLVE FOR FREE DISPL ----
C
IF (BTEST(1BUG,5)) THEN
  CALL LMATRX(STIP,MD,L4,IND,'GLOBAL STIFFNESS DUMP-7')
  CALL WMATRX(DISP,L4,NLOAD,'LOAD MATRIX DUMP-7')
  ENDIF
C
DO 12 IOPT=1,3,2
  CALL MSOLL(IOPT,BTEST(1BUG,6),L2,IND,L4,NLOAD,L1,L3,MD,STIP,
    & DISP,WORK, .FALSE.,WORK,WORK,L4, .FALSE.,WORK,WORK,L4)
  CONTINUE
12 C
IF (BTEST(1BUG,5))
  & CALL WMATRX(DISP,L4,NLOAD,'DISPLACEMENT DUMP-7')
C-----
C- SOLVE FOR REACTIONS ---
IOPT=2
CALL REACTN(IOPT,BUG,NNODE,L1,L2,L3,L4,LOAD,DISP,
  & MD,STIP,IND,L4,NLOAD,1.0
  & MAXNOD,IDOP,COORD,ID,TITLE,STEPID, .FALSE.,
  & NCOS,COSINE,JTCOS,JTFLG,SUMRCT)
IF (BTEST(1BUG,5))
  & CALL WMATRX(LOAD,L4,NLOAD,'FINAL LOADS AND REACTIONS')
C-----
C- PRINT GLOBAL DISPLACEMENTS ---
CALL JOIDSP(2,MAXNOD,L4,NNODE,NLOAD,L2,IND,IDOP,CONST,DISP,
  & TITLE,'DISPLACEMENTS',STEPID, .FALSE.,
  & NCOS,COSINE,JTCOS)
C-----
C- PRINT GLOBAL REACTIONS ---
IOPT=3
CALL REACTN(IOPT, .TRUE.,NNODE,L1,L2,L3,L4,LOAD,DISP,
  & MD,STIP,IND,L4,NLOAD,1.0
  & MAXNOD,IDOP,COORD,ID,TITLE,STEPID, .TRUE.,
  & NCOS,COSINE,JTCOS,JTFLG,SUMRCT)
C-----
C- CALCULATE AND PRINT MEMBER FORCES ---
IOPT=4
LSTYP=C
PRINT= .TRUE.
HEAD= .TRUE.
DO 300 IELNO=1,NELMT
  CALL ELEM18
  & (IOPT,LSTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,DUCPAG,
  & IELNO,IELDOP,KDOP,PQCOM,RINPUT,AXIAL,I2DISP,I2LOAD,MSTOR)
C-----
C- ADD LAST LOAD CASE TO TOTAL MEMBER LOADS ---
C THIS SETS THE TOTAL LOAD IN THE ELEMENT SUBROUTINE EQUAL TO THE
C SUM OF THE TOTAL LOAD AND THE LAST LOAD CASE
C TWO MAJOR BENEFITS ARE DERIVED FROM THIS:
C 1) TOTAL ELEMENT LOADS ARE USED AS INITIAL VALUES FOR BUCKLING AND
C DYNAMIC LOADINGS
C 2) TOTAL ELEMENT LOADS ARE USED IN NONLINEAR ANALYSIS
C-----
IOPT=5
LSTYP=C
PRINT= .FALSE.
HEAD= .FALSE.
DO 310 IELNO=1,NELMT
  CALL ELEM18
  & (IOPT,LSTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,DUCPAG,
  & IELNO,IELDOP,KDOP,PQCOM,RINPUT,AXIAL,I2DISP,I2LOAD,MSTOR)
C-----
C- OBTAIN TOTAL STRAIN ENERGY ---
ESE=0
DO 1312 IELNO=1,NELMT
  CALL ELEM18
  & (10,LSTYP, .FALSE.,IREL, .FALSE.,NAME,ESE,EPSE,DAMAGE,DUCPAG,
  & IELNO,IELDOP,KDOP,PQCOM,RINPUT,AXIAL,I2DISP,I2LOAD,MSTOR)
  ESE=ESE+ESE
1312 WRITE(108,1313) ESE
1313 FORMAT(1X,1X,'THE STATICS STRAIN ENERGY OF SYSTEM ','G12.4//')
WRITE(108,*)
C-----
IF (NLOAD.EQ.1) RETURN
C-----
WRITE(108,1000) TITLE(1),TITLE(2)
1000 FORMAT('1) STRUCTURE ..... 'A/' SOLUTION ..... 'A, //
  & 1X, ' MAXIMUM VALUES FOR ALL LOAD CASES ' /
  & 1X, '-----')
C-----
C- GET AND PRINT MAXIMUM DISPL ---
DO 1010 I=1,L4
  WORK(1)=DISP(I,1)
  DO 1010 J=2,NLOAD
    IF (ABS(DISP(I,J)) .GT. ABS(WORK(1))) WORK(1)=DISP(I,J)
  1010 CONTINUE
  CALL JOIDSP(2,MAXNOD,L4,NNODE,L1,L3,IND,IDOP,CONST,WORK,
    & TITLE,'MAXIMUM DISPLACEMENTS',STEPID, .FALSE.,
    & NCOS,COSINE,JTCOS)
C-----
C- GET AND PRINT MAXIMUM REACTIONS ---
WRITE(108,1000) TITLE(1),TITLE(2)
DO 1020 I=1,L4
  WORK(1)=LOAD(I,1)
  DO 1020 J=2,NLOAD
    IF (ABS(LOAD(I,J)) .GT. ABS(WORK(1))) WORK(1)=LOAD(I,J)
  1020 CONTINUE
  DO 1030 I=1,6
  1030 SUMRCT(I)=0
  CALL REACTN(1, .TRUE.,NNODE,L1,L2,L3,L4,WORK,DISP,
    & MD,STIP,IND,L4,1.0
    & MAXNOD,IDOP,COORD,ID,TITLE,STEPID, .FALSE.,
    & NCOS,COSINE,JTCOS,JTFLG,SUMRCT)
C-----
C- PRINT MAXIMUM ELEMENT FORCES ---
C-----
C-----
```

```
WRITE(108,1000) TITLE(1),TITLE(2)
LSTYP=0
PRINT= .TRUE.
HEAD= .FALSE.
DO 1310 IELNO=1,NELMT
  CALL ELEM18
  & (12,LSTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,DUCPAG,
  & IELNO,IELDOP,KDOP,PQCOM,RINPUT,AXIAL,I2DISP,I2LOAD,MSTOR)
  C
  220 FORMAT(5X,15,1P,6G15.6)
  200 FORMAT('1) GLOBAL 'A, ' LOADING #'15//4X,'NODE'7X,'DK',
  & 13X,'DY'13X,'DZ'13X,'RX'13X,'RY'13X,'RZ'1//)
  C
  RETURN
  END
C-----
C- SUBROUTINE JOILOA(BUG,MAXNOD,NDOF,NNODE,NLOAD,NP,IND,IDOP,CONST,
  & FORCE,DSPLG,JTFLG,TITLE,HEAD)
  CHARACTER*(*) TITLE(2)
  CHARACTER DIR*4,SUPT*25,DOP*18
  CHARACTER*1 CX(6)
  CHARACTER*14 P(6)
  LOGICAL DSPLG,FIRST,BUG,HEAD,BTEST
  DIMENSION IDOP(MAXNOD,6),CONST(MAXNOD,6),FORCE(NDOF,NLOAD)
  DIMENSION ID(MAXNOD),JTFLG(MAXNOD)
  C
  FIRST= .TRUE.
  DSPLG= .FALSE.
  5 FORMAT('1) STRUCTURE ..... 'A/' SOLUTION ..... 'A, //
  6 FORMAT('1) APPLIED JOINT LOADS'//)
  C-----
  C- INPUT LOADING DATA ----
  10 READ(105,*) LOAD,JOINT,JTGEN,JTINC,DIR,VALUE
  IF (INDEX(DIR,'END').NE.0) GO TO 100
  15 DOP=
  IF (FIRST) THEN
    IF (HEAD) WRITE(108,5) TITLE(1),TITLE(2)
    WRITE(108,6)
    FIRST= .FALSE.
  ENDIF
  C-----
  C- CHECK FOR A VALID JOINT NUMBER ----
  IF (JOINT.LT.ID(1) .OR. JOINT.GT.ID(NNODE)) THEN
    WRITE(108,21) LOAD
    GO TO 10
  ENDIF
  C-----
  J=IQUICK(JOINT,IND,NNODE)
  SUPT=
  C-----
  PUT A LOAD ON A JOINT
  C NOTES: 1X, 1Y, 1Z, MX, MY AND MZ ARE THE DOP THAT THE LOADS ARE
  & APPLIED TO
  & THE MOMENTS RESULTING FROM PX, PY AND PZ ARE DUE TO
  & CONSTRAINTS. IF A NODE IS NOT CONSTRAINED, THEN THE VALUE
  & OF CONST IS ZERO.
  & LOADS APPLIED TO RESTRAINED DOP ARE INTERPRETED AS
  & SUPPORT DISPLACEMENTS IE: SUPPORT SETTLEMENT OR
  & DISPLACEMENT CONTROL. ....
  C-----
  IF (INDEX(DIR,'PK').NE.0) THEN
    IDOP(J,1)
    FORCE(IX,LOAD)+FORCE(IX,LOAD)+VALUE
    IF (BTEST(JTFLG(J),13)) THEN
      MY=IDOP(J,5)
      FORCE(MY,LOAD)+FORCE(MY,LOAD)+VALUE*CONST(J,3)
      MZ=IDOP(J,6)
      FORCE(MZ,LOAD)+FORCE(MZ,LOAD)+VALUE*CONST(J,5)
    WRITE(DOP,35) 1X,MX,MZ
    ELSE
      WRITE(DOP,35) 1X
    ENDIF
    IF (JY.GT.NP) THEN
      DSPLG= .TRUE.
      SUPT= .JOINT DISPLACEMENT ...
    ENDIF
    ELSE IF (INDEX(DIR,'PY').NE.0) THEN
      IY=IDOP(J,2)
      FORCE(IY,LOAD)+FORCE(IY,LOAD)+VALUE
      IF (BTEST(JTFLG(J),13)) THEN
        MX=IDOP(J,4)
        FORCE(MX,LOAD)+FORCE(MX,LOAD)+VALUE*CONST(J,1)
        MZ=IDOP(J,6)
        FORCE(MZ,LOAD)+FORCE(MZ,LOAD)+VALUE*CONST(J,6)
      WRITE(DOP,35) 1Y,MX,MZ
    ELSE
      WRITE(DOP,35) 1Y
    ENDIF
    IF (JZ.GT.NP) THEN
      DSPLG= .TRUE.
      SUPT= .JOINT DISPLACEMENT ...
    ENDIF
    ELSE IF (INDEX(DIR,'PZ').NE.0) THEN
      JZ=IDOP(J,3)
      FORCE(JZ,LOAD)+FORCE(JZ,LOAD)+VALUE
      IF (BTEST(JTFLG(J),14)) THEN
        MX=IDOP(J,4)
        FORCE(MX,LOAD)+FORCE(MX,LOAD)+VALUE*CONST(J,2)
        MY=IDOP(J,5)
        FORCE(MY,LOAD)+FORCE(MY,LOAD)+VALUE*CONST(J,4)
      WRITE(DOP,35) JZ,MX,MY
    ELSE
      WRITE(DOP,35) JZ
    ENDIF
    IF (JX.GT.NP) THEN
      DSPLG= .TRUE.
      SUPT= .JOINT DISPLACEMENT ...
    ENDIF
    ELSE IF (INDEX(DIR,'PX').NE.0) THEN
      MX=IDOP(J,4)
      FORCE(MX,LOAD)+FORCE(MX,LOAD)+VALUE
      WRITE(DOP,35) MX
      IF (MX.GT.NP) THEN
        DSPLG= .TRUE.
        SUPT= .JOINT DISPLACEMENT ...
      ENDIF
    ELSE IF (INDEX(DIR,'MY').NE.0) THEN
      MY=IDOP(J,5)
      FORCE(MY,LOAD)+FORCE(MY,LOAD)+VALUE
      WRITE(DOP,35) MY
      IF (MY.GT.NP) THEN
        DSPLG= .TRUE.
        SUPT= .JOINT DISPLACEMENT ...
      ENDIF
    ELSE IF (INDEX(DIR,'MZ').NE.0) THEN
      MZ=IDOP(J,6)
      FORCE(MZ,LOAD)+FORCE(MZ,LOAD)+VALUE
      IF (MZ.GT.NP) THEN
        DSPLG= .TRUE.
        SUPT= .JOINT DISPLACEMENT ...
      ENDIF
    ELSE
      WRITE(108,20) JOINT,DIR,VALUE
    ENDIF
    WRITE(108,36) LOAD,JOINT,DIR,DOP,VALUE,SUPT
    IF (JTGEN.GT.0) THEN
      JTGEN=JTGEN-1
      JOINT=JOINT+JTINC
      GO TO 15
    ENDIF
```

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SOLO0600
SOLO0610
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SOLO1000
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SOLO1080
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SOLO1100
SOLO1110
SOLO1120
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SOLO2110
SOLO2120
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SOLO2160
SOLO2170
SOLO2180
SOLO2190
SOLO2200
SOLO2210
SOLO2220
SOLO2230
SOLO2240
SOLO2250
SOLO2260
SOLO2270
SOLO2280
SOLO2290
SOLO2300
SOLO2310
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C      GO TO 10
20 FORMAT (' INVALID JOINT LOAD... ' ,I6,2X,A,2X,IP,G14.6)
21 FORMAT (' INVALID LOAD CASE... ' ,I6,... ' LOAD IS SKIPPED')
30 FORMAT (' LOAD CASE: ',I3, ' JOINT: ',I6, ' DIRECTION ',A,
4 ' DOP(S) ',A, ' MAGNITUDE: ',IP,G14.6,2X,A)
35 FORMAT (' ',I5,... ' ',I5,... ' ')
C      RETURN IF NO JOINT LOADS WERE INPUT
100 IF (FIRST) RETURN
C      IF (.NOT. BUG) RETURN
C      PRINT JOINT LOADS ON EACH DOP
NS=NDOP*5
DO 120 I=1,NLOAD
IF (HEAD) WRITE(108,5) TITLE(1),TITLE(2)
WRITE(109,130) L
DO 120 I=1,NS,5
15=MIN(I-4,NDOP)
DO 110 J=1,I5
K=J-1
CX(K)=1
IF (J.GT.NP) CX(K)=1
110 WRITE(108,140) (J,FORCE(J,L),CX(J-I+1),J-1,15)
120 CONTINUE
C      130 FORMAT (' APPLIED JOINT LOADS, LOADING #',I5,10X,
4 ' NOTE: * DENOTES SUPPORT DISPLACEMENT' /
4 ' -----' /
4 ' 5X,5(' DOP',7X,LOAD ') /
140 FORMAT (5X,5(OP,15,IP,G14.6,A) )
DO 220 L=1,NLOAD
IF (HEAD) WRITE(108,5) TITLE(1),TITLE(2)
WRITE(109,200) L
DO 220 I=1,NMODE
DO 210 J=1,6
CX(J)=1
IF (J.EQ.1) THEN
P(J)=1
ELSE
K=IDOP(I,J)
WRITE(P(J),240) FORCE(K,L)
IF (K.GT.NP) CX(J)=1
ENDIF
CONTINUE
WRITE(108,230) ID(I),(P(J),CX(J),J=1,6)
220 CONTINUE
200 FORMAT (' APPLIED JOINT LOADS, LOADING #',I5,10X,
4 ' NOTE: * DENOTES SUPPORT DISPLACEMENT' /
4 ' -----' /
4 ' 13X,'PY',13X,'PZ',13X,'MX',13X,'MY',13X,'MZ' /
230 FORMAT (5X,15,12A)
240 FORMAT (1P,G14.6)
RETURN
END
-----
SUBROUTINE JOIDSP(OPT,MAXNOD,NDOP,NMODE,NLOAD,NP,IP,IO,
4 CONST,DISP,TITLE,NAME,STEPID,HEAD,NCOS,COSINE,JTCOS)
CHARACTER*(*) TITLE(2),STEPID,NAME
CHARACTER*15 CD(6),EQUAL*60
LOGICAL HEAD
DIMENSION IDOP(MAXNOD,6),CONST(MAXNOD,6)
DIMENSION DISP(NDOP,NLOAD),IP(MAXNOD)
DIMENSION D(6),G(6),COSINE(1,3),NCOS(1,3),JTCOS(MAXNOD)
DATA EQUAL
-----
NLEN=MIN(LEN(NAME)+1,60)
NC=1
-----
C      LOOP FOR EACH LOAD CASE ---
C      DO 300 L=1,NLOAD
NDSPLS=0
IF (.NOT.(NCOS.EQ.1 .AND. COSINE(1,1),EQ.1 .AND.
4 COSINE(2,2),EQ.1 .AND. COSINE(3,3),EQ.1)
5 .AND. IOPT.EQ.2) GO TO 210
C      PRINT GLOBAL DISPLACEMENTS ---
C      NOTE: DISP MAY CONTAIN DISPLACEMENTS, VELOCITIES, ACCELERATIONS OR
EIGENVALUES...
ALL OF WHICH ARE PRINTED OUT AT EACH NODE...
IF (HEAD) WRITE(108,310) TITLE(1),TITLE(2)
IF (IOPT.NE.2) WRITE(108,315) NAME,L,STEPID,EQUAL(1:NLEN)
IF (IOPT.EQ.2) WRITE(108,415) NAME, EQUAL(1:NLEN)
DO 100 I=1,NMODE
C      TRANSFER DISPL TO LOCAL VECTOR
DO 30 J=1,6
IJ=IDOP(I,J)
D(J)=DISP(IJ,L)
C      ADD ROTATIONS TO DISPL FOR CONSTRAINTS
IF UNCONSTRAINED, CONST(I,J)=0
D(1) = D(1) + D(5)*CONST(I,3) + D(6)*CONST(I,5)
D(2) = D(2) + D(4)*CONST(I,1) + D(6)*CONST(I,6)
D(3) = D(3) + D(4)*CONST(I,2) + D(5)*CONST(I,4)
C      TRANSFER TO GLOBAL DISPLACEMENTS
JCOS=JTCOS(I)
DO 25 I1=1,3
I2=1-I1
G(I1)=0
G(I2)=0
DO 25 J1=1,3
J2=J1-3
G(I1)+G(I2)*COSINE(J1,I1,JCOS)*D(J1)
G(I2)-G(I2)*COSINE(J1,I1,JCOS)*D(J2)
25 WRITE DISPL
DO 30 J=1,6
30 WRITE(CD(J),9) G(J)
9 FORMAT(1P,G14.6)
WRITE(108,320) ID(I),(CD(K),K=1,6)
NDSPLS=NDSPLS+1
100 CONTINUE
IF (NCOS.EQ.1 .AND. COSINE(1,1),EQ.1 .AND. COSINE(2,2),EQ.1
4 .AND. COSINE(3,3),EQ.1) GO TO 300
-----
C      PRINT JOINT DISPLACEMENTS ---
C      NOTE: DISP MAY CONTAIN DISPLACEMENTS, VELOCITIES, ACCELERATIONS OR
EIGENVALUES...
ALL OF WHICH ARE PRINTED OUT AT EACH NODE...
210 IF (HEAD) NDSPLS.GT.30 WRITE(108,310) TITLE(1),TITLE(2)
IF (IOPT.NE.2) WRITE(108,315) NAME,L,STEPID,EQUAL(1:NLEN)
IF (IOPT.EQ.2) WRITE(108,415) NAME, EQUAL(1:NLEN)
DO 200 I=1,NMODE
C      TRANSFER GLOBAL DISPL TO LOCAL VECTOR
DO 210 J=1,6

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IF (BUG) THEN
  NS=NDOP+5
  DO 120 I=1,NLOAD
    IF (HEAD) WRITE(108,5) TITLE(1),TITLE(2)
    WRITE(109,130) L
    DO 120 I=1,NS,5
      IS=MIN(12,4)*NDOP
      DO 110 J=1,IS
        K=J-1
        CK(K)=
110      WRITE(108,140) (J,FORCE(J,L),CK(J-1-1),J-1,IS)
120      CONTINUE
ENDIF
C
130 FORMAT (' GLOBAL JOINT FORCES DUE TO ELEMENT LOADS ',3X,
4 ' LOADING #',15,10X,
4 ' NOTE ' DENOTES SUPPORT DISPLACEMENT',
4 '-----',
4 ' 5X,5(' DOP',7X,'LOAD ')/)
140 FORMAT (5X,5(0P,15,1P,14.6,A) )
RETURN
END
C-----
SUBROUTINE CKRNG(V,A,B,NAME)
CHARACTER(*) NAME
IF (A LT V .AND. V LT B) RETURN
WRITE(108,10) NAME,A,V,B
10 FORMAT('-----',A,' IS OUT OF RANGE, SET ',1P,G13.5,' LE INPUT',
4 G13.5,' LE ',G13.5)
V=MIN(V,B)
V=MAX(V,A)
RETURN
END
C-----
SUBROUTINE SOLG2
LOGICAL SLMASS,TEST,BTEST,UNBAL
CHARACTER INTEG*20
$INCLUDE 'ZCOMN'
C
C-----
C PRINT HEADER ---
WRITE(108,10) TITLE(1),TITLE(2)
10 FORMAT ('1 STRUCTURE.....',A,' SOLUTION.....',A,/)
C-----
C INPUT LOADING DATA --
READ(105,*) INTEG,THETA,ELSTIC,UNBAL
READ(105,*) IPRNT,WRITE,MAXACC,SLMASS
READ(105,*) TO,DT,TF,GRAY
IPRNT=MAX(IPRNT,1)
C
IF (TEST(INTEG,'LINEAR') .FALSE.) THEN
  INTEG=1
  WRITE(108,11) 'LINEAR ACCELERATION METHOD'
  &
  ELSE IF (TEST(INTEG,'AVERAGE') .FALSE.) THEN
    INTEG=2
    WRITE(108,11) 'AVERAGE ACCELERATION METHOD'
    &
  ELSE IF (TEST(INTEG,'WILSON') .FALSE.) THEN
    INTEG=3
    WRITE(108,11) 'WILSON THETA METHOD'
    &
  ELSE IF (TEST(INTEG,'WILSON') .FALSE.) THEN
    INTEG=4
    WRITE(108,*) 'INVALID INTEGRATION METHOD'
    RETURN
  ENDIF
C-----
CHECK FOR INVALID INTEGRATION WITH PROPORTIONAL DAMPING
IF (INTEG.EQ.1 .OR. INTEG.EQ.2) .AND. FPAMP.NE.1) THEN
  WRITE(108,19)
19 FORMAT (5X,'INTEGRATION METHOD IS ONLY VALID FOR ',
4 ' PROPORTIONAL DAMPING'//5X,'SOLN IS ABORTED')
RETURN
ENDIF
C-----
WRITE(108,13) TO,DT,TF,GRAY,IPRNT
IF (ELSTIC) WRITE(108,14)
C
11 FORMAT(1X,' SOLUTION #2, ',A,' OF NUMERICAL INTEGRATION'//
4 1X,'-----',A,/)
12 FORMAT(1X,' THETA .....',1P,G15.6)
13 FORMAT(1X,' INITIAL TIME .....',1P,G15.6/
4 1X,' TIME STEP .....',1P,G15.6/
4 1X,' FINAL TIME .....',1P,G15.6/
4 1X,' ACCELERATION DUE TO GRAVITY .....',1P,G15.6/
4 1X,' STEP INTERVAL FOR PRINTING .....',0P,15)
14 FORMAT(1X,' THE STRUCTURE IS ASSUMED TO BEHAVE ELASTICALLY')
C-----
C SET GEOMETRIC STIFFNESS FLAGS ---
KGLoad=NZ(12KGD+3)
KTYPE=NZ(12KGT+3)
KFORM=NZ(12KGD+3)
NEWKQ=.TRUE.
IF (KGLoad.EQ.0 .OR. KTYPE.EQ.0 .OR. KFORM.EQ.0) NEWKQ=.FALSE.
C-----
SET UP CONSTANTS -----
L1=NCND+1
L2=NCND+NFREE
C-----
INITIALIZE STORAGE FOR LOAD, DISPL, VELOC, ACCEL
2(12LOAD) = LOCATION OF THE TOTAL LOAD MATRIX
2(12DISP) = LOCATION OF THE TOTAL DISPLACEMENT MATRIX
2(12VEL) = LOCATION OF THE TOTAL VELOCITY MATRIX
2(12ACC) = LOCATION OF THE TOTAL ACCELERATION MATRIX
2(12DLOA) = LOCATION OF THE INCREMENTAL LOAD MATRIX
2(12DOSP) = LOCATION OF THE INCREMENTAL DISPLACEMENT MATRIX
2(12DVEL) = LOCATION OF THE INCREMENTAL VELOCITY MATRIX
2(12DACC) = LOCATION OF THE INCREMENTAL ACCELERATION MATRIX
C
IF (12LOAD.EQ.0) THEN
  12LOAD = 12
  12 = 12+NDOP
  CALL CKSTOR(0,0)
  DO 1 I=1,NDOP
    2(I-12LOAD-1) = 0
  ENDF
C
IF (12DISP.EQ.0) THEN
  12DISP = 12
  12 = 12+NDOP
  CALL CKSTOR(0,0)
  DO 2 I=1,NDOP
    2(I-12DISP-1) = 0
  ENDF
C
IF (12VEL.EQ.0) THEN
  12VEL = 12
  12 = 12+NDOP
  CALL CKSTOR(0,0)
  DO 3 I=1,NDOP
    2(I-12VEL-1) = 0
  ENDF
C
IF (12ACC.EQ.0) THEN
  12ACC = 12
  12 = 12+NDOP
  CALL CKSTOR(0,0)
  DO 4 I=1,NDOP
    2(I-12ACC-1) = 0
  ENDF
C-----
EL11060
EL11070
EL11080
EL11090
EL11100
EL11110
EL11120
EL11130
EL11140
EL11150
EL11160
EL11170
EL11180
EL11190
EL11200
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EL11300
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EL11370
EL11380
EL11390
EL11400
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EL12500
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EL12550

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I2RMX=I2
I2 = I2-NDOP *6
DO 28 I=0,NDOP
  2(I2RMX+I)=0
  2(I2RMX-1)=0
  2(I2RMX+1)=0
  28 DO 29 I=1,6
  29 2(I2RMX+1-NDOP+I)=0
  DO 30 I=1,6
  30 SUBCT(I)=0
C-----
C INPUT FILE OUTPUT CONTROLL DATA ---
C-----
IF (IWRITE GT 0) THEN
  CALL IOPDAT(IOPF, IWRITE, TO, DT)
ENDIF
C-----
C----- SET UP WORK STORAGE AREA
IW = I2 - 1
C-----
C----- CHECK STORAGE REQUIREMENTS
CALL CKSTOR(NDOP, IW)
C-----
C----- DETERMINE THE LENGTH OF THE STIFFNESS MATRIX
IMD=N2(I2MD+NDOP)
N2(I2MD) = 1
C-----
C----- DETERMINE THE LENGTH OF THE MASS MATRIX
IMDS=N2(I2MDS+NDOP)
N2(I2MDS) = 1
C-----
C----- DETERMINE THE LENGTH OF THE GEOMETRIC STIFFNESS MATRIX
IF (N2(I2KGD+1) EQ 2) THEN
  IMDKG = N2(I2MDKG+NDOP)
  N2(I2MDKG) = 1
ELSE
  IMDKG = 1
ENDIF
C-----
C----- CALL SOL2A TO CALC DATA
CALL SOL2A(IMD, IMDS, IMDKG, IMDSL, IMDSM, IMDSM, IMDSM, IMDSM,
  & NNODE, NCOS, NEMT, TBUG, MAXMCD,
  & TITLE, STIPID, NIMP, NDOP, NFREE,
  & KGFORM, KGLoad, KGTYPE, NCOND, INTEGO,
  & NEWKG, MCOND, KCOND, UNBAL, NEWK,
  & THETA, ELSTIC, IPRINT, IWRITE, GROUND(1),
  & MAXACC, SLMASS, TO, DT, TF, GROUND(4),
  & GRAV, FDAMP, I2DDSP, I2DLOA, GROUND(7),
  & I2DISP, I2ZVEL, I2ZACC, I2ZLOA, I2ZLN, I2ZMAX,
  & I2DISP, I2ZVEL, I2ZACC, I2ZLOA, I2ZLN, I2ZMAX,
  & N2(I2MD), N2(I2MDS), N2(I2MDKG), N2(I2MDS), I2ZMAX,
  & I2ZAMP, I2ZPBAR, I2ZMCS, I2ZAGTY, I2ZLOA,
  & I2ZAGTY, I2ZAGTY, I2ZAMP, I2ZPUB, I2ZM, SUMCT,
  & N2(I2D), N2(I2DOP), I2ZCNET, N2(I2ZFC), I2ZCORD, I2ZDOP,
  & I2ZCOS, N2(I2ZCOS), I2ZDFG, EIB, ESE, PSE, EKE, EDD,
  & I2ZZERO, I2ZFC, I2ZDFG)
C-----
RETURN
END
C-----
SUBROUTINE AVEACC(IOPF, DT, THETA, NEWDY, L2, L3, L4,
  & IMDMAS, IMD, IMDS, IMDKG, NDOP, NFREE, KGLoad, KGFORM,
  & A, V, DA, DV, DD, DP,
  & WORK, MASS, MDHMAS, DAMP, STIFF, MD,
  & S, Q, MDKG, ACC, S, MDS, Q, R, PBAR,
  & FDAMP, ALPHA, BETA)
C-----
INTEGER FDAMP
REAL MASS, KG
LOGICAL NEWDY
DIMENSION A(NDOP), V(NDOP), DA(NDOP), DV(NDOP), DD(NDOP)
DIMENSION DP(NDOP), WORK(2*NDOP), KG(IMDKG), MDKG(NDOP+1)
DIMENSION PBAR(L3, L4), Q(L3, L4), R(L3, L4), MD(NDOP+1), STIFF(IMD)
DIMENSION MDHMAS(NDOP+1), MASS(IMDMAS), MDS(NDOP+1), S(IMDS), DAMP(3)
C-----
C----- DETERMINE THE RESPONSE AT TIME DT*
TWO=DT**2
THETA=DT**2
CALL LINACC(IOPF, TWO, THETA, NEWDY, L2, L3, L4,
  & IMDMAS, IMD, IMDS, IMDKG, NDOP, NFREE, KGLoad, KGFORM,
  & A, V, DA, DV, DD, DP,
  & WORK, MASS, MDHMAS, DAMP, STIFF, MD,
  & S, Q, MDKG, ACC, S, MDS, Q, R, PBAR,
  & FDAMP, ALPHA, BETA)
C-----
IF (IOPF.NE 2) RETURN
C-----
C----- DETERMINE ACCELERATION, VELOCITY AND DISPLACEMENT AT TIME DT
DA = INCREMENTAL ACCELERATION BETWEEN TIMES T AND T+DT
DV = INCREMENTAL VELOCITY BETWEEN TIMES T AND T+DT
DD = INCREMENTAL DISPLACEMENT BETWEEN TIMES T AND T+DT
DO 270 I=L3, L4
  DP(I)=DA(I)
  VEL2DT=DV(I)
  ACC2DT=DA(I)
  DA(I)=ACC2DT/2.0
  DV(I)=(4*DA(I)-ACC2DT)*DT/4.
  DD(I)=V(I)*DT+(6.00*A(I)+ACC2DT)*DT**2/12.
C-----
RETURN
END
C-----
SUBROUTINE WILSON(IOPF, DT, THETA, KG, MDKG, ACC,
  & L2, L3, L4, IMDMAS, IMD, IMDS, IMDKG, NDOP, NFREE,
  & KGLoad, KGFORM, A, V, D, P,
  & DA, DV, DD, DP, WORK,
  & MASS, MDHMAS, DAMP, STIFF, MD,
  & S, Q, MDKG, ACC, S, MDS, Q, R, PBAR,
  & FDAMP, ALPHA, BETA)
C-----
INTEGER FDAMP
REAL MASS, KG
DIMENSION A(NDOP), V(NDOP), D(NDOP), DA(NDOP), DV(NDOP), DD(NDOP)
DIMENSION DP(NDOP), DP(NDOP), DUMMY(1), IDUMMY(1), WORK(2*NDOP)
DIMENSION MDHMAS(NDOP+1), MASS(IMDMAS), S(IMDS), STIFF(IMD)
DIMENSION MDKG(NDOP+1), KG(IMDKG), MDS(NDOP+1), MD(NDOP+1)
DIMENSION PBAR(L3, L4), Q(L3, L4), R(L3, L4), DAMP(3)
C-----
GO TO (100, 200, 300) IOPF
C-----
100 CONTINUE
C1=6/(THETA*DT)**2
C2=3/(THETA*DT)
C3=6/(THETA*DT)
C4=THETA*DT/2
C5=DT/2
C6=DT**2/2
CT=DT**6
ALPHA=DAMP(1)
BETA=-DAMP(2)
C10=1+BETA*C2
C11=C1+ALPHA*C2
C-----
DEVELOP SKYLINE OF DYNAMIC STIFFNESS S
C-----
LSTIFF = # OF STIFFNESS TERMS IN COLUMN J
C-----
LMASS = # OF MASS TERMS IN COLUMN J
C-----
LKG = # OF KG TERMS IN COLUMN J
C-----
RETURN

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DO 270 I=L3,L4
DA(I)=C1*PBAR(I)-DA(I)/THETA
DV(I)=A(I)*DT+CS*DA(I)
DD(I)=V(I)*DT+C6*A(I)+C7*DA(I)
C
RETURN
C-----
C CALC TOTAL DISPLACEMENTS, VELOCITIES AND ACCELERATIONS --
C-----
300 CONTINUE
DO 310 I=1,NDOP
A(I)=-DA(I)-A(I)
V(I)=DV(I)-V(I)
D(I)=DD(I)-D(I)
P(I)=DP(I)-P(I)
DP(I)=0.00
C
RETURN
END
C-----
SUBROUTINE LINACC(OPT,DT,THETA,NEWMDY,L3,L4,
& IMMAS,IMD,IMDS,IMDK,NDOP,NFREE,KGLOAD,KGFORM,
& A,DA,DV,DD,DP,
& WORK,MASS,MDAMP,STIFF,MD,
& KG,MDKG,ACC,S,MDS,Q,R,PBAR,
& PDAMP,ALPHA,BETA)
C
INTEGER PDAMP
REAL MASS,KG
LOGICAL NEWMDY
DIMENSION A(NDOP),V(NDOP),DA(NDOP),DV(NDOP),DD(NDOP)
DIMENSION DP(NDOP),DUMMY(1),IDUMMY(1),WORK(2*NDOP)
DIMENSION PBAR(L3:L4),Q(L3:L4),R(L3:L4),MD(NDOP),STIFF(IMD)
DIMENSION MDMASS(NDOP),MASS(IMD),MDKG(NDOP),KG(IMDK)
DIMENSION MDS(NDOP),STIFFS(1),DAMP(3)
C
GO TO (100,200,300) OPT
C-----
C INITIALIZE VALUES ---
C-----
100 CONTINUE
ALPHA=DAMP(1)
BETA=DAMP(2)
CO=3*ALPHA/DT+.6/(DT**2)
C1=.07*(1+.3*BETA/DT)
C2=C*CO
C3=ALPHA-C2*BETA
C4=.6/DT
C5=DT/2.
C6=.0
C7=.3/DT
C8=.6/(DT**2)
C-----
C DEVELOP SKELINE OF DYNAMIC STIFFNESS (S)
C-----
MDS(I) = ADDRESS OF DYNAMIC STIFF. MAIN DIAGONAL TERM ROW I
MD(I) = ADDRESS OF STIFFNESS MAIN DIAGONAL TERM ROW I
MDMAS(I) = ADDRESS OF MASS MAIN DIAGONAL TERM ROW I
MDKG(I) = ADDRESS OF GEOMETRIC STIFF. MAIN DIAGONAL TERM ROW I
LSTIFF = # OF STIFFNESS TERMS IN COLUMN J
LMASS = # OF MASS TERMS IN COLUMN J
LIG = # OF KG TERMS IN COLUMN J
L = # OF S TERMS IN COLUMN J
C-----
CHECK FOR PROPORTIONAL DAMPING ----
IF (PDAMP.NE.1) THEN
WRITE(108,*) '*****'
WRITE(108,*) ' PROPORTIONAL DAMPING WAS NOT SPECIFIED '
WRITE(108,*) ' SOLUTION IS ABORTED. '
WRITE(108,*) '*****'
STOP
ENDIF
RETURN
C-----
C SOLVE FOR INCREMENTAL DISPLACEMENTS, VELOCITIES AND ACCELERATIONS --
C-----
200 CONTINUE
C-----
FORM DYNAMIC STIFFNESS
SOME LOCAL VARIABLES
C J = COLUMN NUMBER FOR MASS AND STIFF
C I = ROW NUMBER FOR MASS AND STIFF
C J1 = COLUMN NUMBER FOR S
C I1 = ROW NUMBER FOR S
C IJSTIP = ADDRESS OF ELEMENT I,J IN MATRIX STIFF
C IJMS = ADDRESS OF ELEMENT I,J IN MATRIX MASS
C IJJK = ADDRESS OF ELEMENT I,J IN GEOMETRIC STIFFNESS MATRIX
C IJS = ADDRESS OF ELEMENT I,J IN MATRIX S
C LSTIFF = ROW # CONTAINING THE FIRST NONZERO TERM IN COLUMN J OF STIFF
C LMAS = ROW # CONTAINING THE FIRST NONZERO TERM IN COLUMN J OF MASS
C LKG = ROW # CONTAINING THE FIRST NONZERO TERM IN COLUMN J OF KG
C LS = ROW # CONTAINING THE FIRST NONZERO TERM IN COLUMN J OF S
C-----
FORM DYNAMIC STIFFNESS IF NEWMDY IS TRUE
IF (NEWMDY) THEN
C
Determine the factor for KG
IF (KGLOAD.EQ.1) THEN
CKG=(1+ACC)
ELSE IF (KGLOAD.EQ.2) THEN
CKG=1.00
ENDIF
C
K(BAR) = STIFFNESS
DO 225 J=L3,L4
J1=J-L3+1
MDJ1P=MD(J-1)
MDJ=MD(J)
LSTIFF=J-(MD(J-1)-MD(J)-1)
MDSJ1P=MDS(J1-1)
MDSJ=MDS(J)
LS=-1-(MDS(J1-1)-MDS(J)-1)-L3
L=MAX(LS,LSTIFF)
DO 210 I=J,L-1
I1=I-L3+1
IJS=(MDS(J1)+J1)-I1
IJSTIP=(MD(J)+J)-I
S(IJS)=STIFF(IJSTIP)
IF (LS.LT.LSTIFF) THEN
DO 215 I=L-1,LS,-1
IJS=(MDS(J1)+J1)-I1
S(IJS)=0
ENDIF
C
K(BAR) = K(BAR) + C2*MASS
&MASS(J)-MDMASS(J)-1)
L=MAX(LS,LMAS)
DO 220 I=J,L-1
I1=I-L3+1
IJS=(MDS(J1)+J1)-I1
&MASS(J)-1)
S(IJS)=MASS(IMAS(J)+C2 + S(IJS)
IF (KGFORM.EQ.2) THEN
C
NOTE THE GEOMETRIC STIFFNESS WAS DERIVED WITH COMPRESSION
BEING POSITIVE, KG HAS POSITIVE TERMS ON THE MAIN DIAGONAL
REPRESENTING COMPRESSION, THUS KG IS SUBTRACTED FROM S
CKG IS AN INCREASE FOR VERT ACCEL

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WRITE(108,23) (I,DACC(I),DVEL(I),DDISP(I),DLOAD(I),I-1,L6) ENER0660
WRITE(108,24) (I,FG(I),DPG(I),FDAMP(I),DPDAMP(I),I-1,L6) ENER0665
ELSE
WRITE(108,22) (I,ACC(I),VEL(I),DISP(I),LOAD(I),I-1,L6) ENER0670
WRITE(108,23) (I,DACC(I),DVEL(I),DDISP(I),DLOAD(I),I-1,L6) ENER0675
WRITE(108,24) (I,FG(I),DPG(I),FDAMP(I),DPDAMP(I),I-1,L6) ENER0680
22 ENDP
FORMAT(/T10,'TOTAL RELATIVE RESPONSE FOR PREVIOUS TIME STEP'/ ENER0700
(T10,'DOP',I6,'ACC',I1P,G13.5,'VEL',I1P,G13.5, ENER0710
'DSP',I1P,G13.5,'LOAD',I1P,G13.5)) ENER0720
23 FORMAT(/T10,'INCREMENTAL RELATIVE RESPONSE'/ ENER0740
(T10,'DOP',I6,'ACC',I1P,G13.5,'VEL',I1P,G13.5, ENER0750
'DSP',I1P,G13.5,'LOAD',I1P,G13.5)) ENER0760
24 FORMAT(/T10,'SPECIAL FORCES (/T10,'DOP',I6,'FD',I1P,G13.5, ENER0770
'DFD',I1P,G13.5,'FDAMP',I1P,G13.5,'DPDAMP',I1P,G13.5)) ENER0780
ENDIF ENER0790
C----- CALCULATE INPUT EARTHQUAKE ENERGY ----- ENER0800
C DEIE=0 ENER0810
C----- CALCULATES THE INCR INPUT ENERGY DUE TO GRND MOTION ENER0820
C IF (DYN) THEN ENER0830
C IF (PT2) THEN ENER0840
WRITE(108,*) 'SUMRC',SUMRC ENER0850
WRITE(108,*) 'DSUMRC',DSUMRC ENER0860
WRITE(108,*) 'SUMPD',SUMPD ENER0870
WRITE(108,*) 'DSUMPD',DSUMPD ENER0880
WRITE(108,*) 'SUMUB',SUMUB ENER0890
WRITE(108,*) 'DSUMUB',DSUMUB ENER0900
ENDIF ENER0910
DO 48 I=1,3 ENER0920
FORCEI=SUMRC(I)+0.5*DSUMRC(I)+SUMPD(I)+0.5*DSUMPD(I) ENER0930
+SUMUB(I)+REACFI(I) ENER0940
ENER=GD(1)*FORCEI ENER0950
IF (PT2) WRITE(108,*) I,'FORCEI',FORCEI, ENER0960
+GD(I),GD(I),ENER,ENER ENER0970
48 DEIE=DEIE+ENER ENER0980
ENDIF ENER0990
C----- CALCULATE THE INCR INPUT ENERGY DUE TO APPLIED LOADS ----- ENER1000
C N2=L6 ENER1010
IF (DYN) N2=L4 ENER1020
DO 50 I=L3,N2 ENER1030
FORCEI=LOAD(I)*DLOAD(I)/2 ENER1040
ENER=FORCEI*DDISP(I) ENER1050
IF (PT2) WRITE(108,*) I, ENER1060
'FORCEI',FORCEI,'DDISP',DDISP(I),ENER,ENER ENER1070
50 DEIE=DEIE+ENER ENER1080
ENDIF ENER1090
C----- CALCULATE THE ENERGY DUE TO GEOMETRIC STIFFNESS ----- ENER1100
C IF (NFORM EQ 2) THEN ENER1110
C GET THE INCREMENTAL DISPL AND STORE IN VECTOR WORK2 ENER1120
C SAVE INCREMENTAL IN VECTOR WORK2 ENER1130
DO 51 I=L3,N2 ENER1140
WORK(I)=DDISP(I) ENER1150
51 ADD GROUND RESPONSE IN JOINT COORDINATES TO RELATIVE RESP ENER1160
MULTIPLY GROUND DISPL * DIRECTION COSINE ENER1170
C----- OMIT ALL DOP THAT ARE CONSTRAINED TO A MASTER DOP ----- ENER1180
C IF (GD(1).NE.0 .OR. GD(2).NE.0 .OR. GD(3).NE.0) THEN ENER1190
DO 52 NODE=1,NNGDE ENER1200
ICOS=JTCOS(NODE) ENER1210
DO 52 K=1,3 ENER1220
IF (.NOT. BTEST(JTFLG(NODE),K+1)) THEN ENER1230
II=IDOP(NODE,K) ENER1240
IF (II.LE.L4 .AND. II.GE.L3) ENER1250
WORK(II)=WORK(II)-GD(1)*COSINE(1,K,ICOS) ENER1260
+GD(2)*COSINE(2,K,ICOS)+GD(3)*COSINE(3,K,ICOS) ENER1270
52 ENDP ENER1280
CONTINUE ENER1290
ENDIF ENER1300
C----- CALCULATE THE INCREMENTAL INPUT ENERGY DUE TO KG ----- ENER1310
C DIKG=0 ENER1320
DO 53 I=L3,L4 ENER1330
ENER=FG(I)*DPG(I)/2*WORK(I) ENER1340
IF (PT2) WRITE(108,*) I, ENER1350
'FG',DPG,'PG(I),DPG(I),' DDISP',WORK(II),ENER,ENER ENER1360
53 DIKG=DIKG+ENER ENER1370
DEIE=DEIE+DIKG ENER1380
ENDIF ENER1390
C----- IF (ISP1 EQ 1) THEN ----- ENER1400
C DO 61 I=L3,L4 ENER1410
WORK(II)=DDISP(I) ENER1420
61 ADD GROUND RESPONSE IN JOINT COORDINATES TO RELATIVE RESP ENER1430
MULTIPLY GROUND DISPL * DIRECTION COSINE ENER1440
C----- OMIT ALL DOP THAT ARE CONSTRAINED TO A MASTER DOP ----- ENER1450
C IF (GD(1).NE.0 .OR. GD(2).NE.0 .OR. GD(3).NE.0) THEN ENER1460
DO 62 NODE=1,NNGDE ENER1470
ICOS=JTCOS(NODE) ENER1480
DO 62 K=1,3 ENER1490
IF (.NOT. BTEST(JTFLG(NODE),K+1)) THEN ENER1500
II=IDOP(NODE,K) ENER1510
IF (II.LE.L4 .AND. II.GE.L3) ENER1520
WORK(II)=WORK(II)+GD(1)*COSINE(1,K,ICOS) ENER1530
+GD(2)*COSINE(2,K,ICOS)+GD(3)*COSINE(3,K,ICOS) ENER1540
62 ENDP ENER1550
CONTINUE ENER1560
DEINT=0 ENER1570
DO 57 I=L3,L4 ENER1580
ENER=ELG(I)*WORK(II) ENER1590
57 DEINT=DEINT+ENER ENER1600
DEIE=DEIE+DEINT ENER1610
ENDIF ENER1620
C----- CALCULATE THE TOTAL INPUT ENERGY ----- ENER1630
C EIE=EIE+DEIE ENER1640
C----- CALCULATE TOTAL STRAIN ENERGY ----- ENER1650
C----- CALCULATE THE TOTAL STRAIN ENERGY ----- ENER1660
C DESE=ESE ENER1670
DPSE=PSE ENER1680
ESE=0 ENER1690
PSE=0 ENER1700
ENR1710
C----- GET ELASTIC AND PLASTIC STRAIN ENERGY FOR EACH ELEMENT ----- ENER1720
C LSTYP=0 ENER1730
DO 60 IELNO=1,NELMT ENER1740
CALL ELIIB ENER1750
4 (LD,LSTYP,FALSE,IREL,FALSE,NAME,ESE,EPSE,DAMAGE,DUCFAG, ENER1760
+IELNO,IELDOP,KDOP,PGCOM,RINPOT,AXIAL,IDLDP,ILOAD,MSTOR) ENER1770
IF (PT2) WRITE(108,55) IELNO,ESE,EPSE ENER1780
55 FORMAT(' ELEMENT#',I5,' ELASTIC SE',I1P,G13.4, ENER1790
'E',ESE+EPSE, ENER1800
'PSE',PSE+EPSE) ENER1810
60 ENR1820
C----- CALCULATE THE INCREMENTAL STRAIN ENERGY ----- ENER1830
C DESE=ESE+DESE ENER1840
DPSE=PSE+DPSE ENER1850
C----- CALCULATE KINETIC ENERGY ----- ENER1860
C IF (DYN) THEN ENER1870
C GET THE ABSOLUTE VELOCITY AND STORE IN VECTOR WORK2 ENER1880
C SAVE RELATIVE RESPONSE IN VECTOR WORK2 ENER1890
DO 15 I=1,L6 ENER1900
WORK2(I)=VEL(I)*DVEL(I) ENER1910
15 ADD GROUND RESPONSE IN JOINT COORDINATES TO RELATIVE RESP ENER1920
MULTIPLY GROUND VELOCITY * DIRECTION COSINE ENER1930

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T2-DT
IP (N LE 1) RETURN
SUMX=A(1)
ADBL=A(1)
SUMXSQ=ADBL**2
DO 40 I=2,N
ADBL=A(I)
SUMX = SUMX + ADBL
SUMXSQ = SUMXSQ + ADBL**2
IF (YMX LT A(I)) THEN
  YMX=A(I)
  T1=T
ELSE IF (YMN GT A(I)) THEN
  YMN=A(I)
  T2=T
ENDIF
T-T-DT
40 CONTINUE
AVE=SUMX/N
IF ((N*SUMXSQ-SUMX**2)/GT.0)
  STDEV=SQRT((N*SUMXSQ-SUMX**2)/(N*N-1))
RMS=SQRT(SUMXSQ/N)
RETURN
END
-----
SUBROUTINE SOLG0A1MD IMDS, IMDKG, IMDS, IMDSLSL,
  & NNODE, MCOSS, VELMT, IZUB, MAXNOD,
  & TITLE, STEPID, NAINP, NDOP, NFREE,
  & KGFORM, KGLDAD, KGTYPF, NCND, INTEGO,
  & NEWKG, MCOND, KGCND, UNBAL, NEWK,
  & THETA, ELTIC, FFRNT, IWRITE,
  & MAXACC, SLMASS, DT, TF, GVT,
  & GRAV, PDAMP, IZDLOA, IZDLOA, GDT,
  & DISP, VEL, ACC, LOAD, AN, DSPMAX,
  & DDISP, DVEL, DACC, DLOAD, BN, VELMAX,
  & STIFF, MASS, MASSSL, SBAR, ACCMAX,
  & MD, MDMS, MDMSLSL, MDKG, MDS, RCTMAX,
  & DAMP, PBAR, MCOSS, AGTX, DLOAD2,
  & AGTY, AGTZ, AINP, WORK, SUNRC,
  & ID, IDOP, CNST, JTFLG, COORD, TFDAMP,
  & COSINE, JTCCS, DFPAMP, ETE, ESE, PSE, EKE, EDD,
  & ZERO, PG, DFG )
COMMON /ISPEC/ISP1,SGD(500,2),REACP(6)
COMMON /IDSTEP/ ISTEP
CHARACTER*(1) TITLE(1),STEPID
CHARACTER NAME*90
LOGICAL PRINT,HEAD,REPEAT,SLMASS,AXIAL,NEWKDY,BTEST
LOGICAL NEWKG,NEWK,ELTIC,MCND,KGCND,UNBAL,MCND2,KGCND2,WRITO
LOGICAL BUGS,BUG6,BUG11,DUCLOG
REAL LOAD,MASS,MASSSL,KG,MCOSS
INTEGER PDAMP
DIMENSION ZERO(NDOP),DLOAD2(NDOP),PG(NDOP),DFG(NDOP),KG(IMDKG)
DIMENSION SUMRC(6),DSUMRC(6),SUMPF(6),DSUMPF(6),SUMJB(6)
DIMENSION TFDAMP(NDOP),PDAMP(NDOP),RINPUT(100),MASSLSL(IMDSLSL)
DIMENSION DSPMAX(NDOP),VELMAX(NDOP),ACCMAX(NDOP),RCTMAX(NDOP*6)
DIMENSION GD(3),GV(3),GA(3),GDT(3),GVT(3),GAT(3),TSM(6)
DIMENSION STIFF(MD),MASS(MDMS),SBAR(MDS),MDMSLSL(NDOP*1)
DIMENSION MD(NDOP*1),MDKG(NDOP*1),MDS(NDOP*1)
DIMENSION LOAD(NDOP),DISP(NDOP),VEL(NDOP),ACC(NDOP),AN(NDOP)
DIMENSION DLOAD(NDOP),DDISP(NDOP),DVEL(NDOP),DACC(NDOP),BN(NDOP)
DIMENSION PUB(NDOP),PBAR(NDOP),MCOSS(NDOP*3),DAMP(3),WORK(2*NDOP)
DIMENSION ACTX(MAXACC),ACTY(MAXACC),ACTZ(MAXACC),AINP(NAINP)
DIMENSION COORD(MAXNOD,3),ID(MAXNOD),JTFLG(MAXNOD),JTCCS(MAXNOD),
  IDOP(MAXNOD,6),COSINE(3,3),MCOSS(3,3),CNST(MAXNOD,6)
DO 5 I=1,3
  SUMJB(I)=0
  SUMRC(I)=0
  SUMPF(I)=0
  DSUMRC(I)=0
  DSUMPF(I)=0
  DUCLOG=.FALSE.
  DUCFAG=-1
  L1=1
  L2=NCND
  L3=L2+1
  L4=NCND+NFREE
  L5=L4+1
  L6=NDOP
-----
BUGS=STEST(BUG,5)
BUG6=STEST(BUG,6)
BUG11=STEST(BUG,11)
-----
IF (BUGS) CALL LMATRIX(MASS,MDMS,NDOP,IMDS,'MASS MATRIX DUMP-1')
C-----
C- INITIALIZE GROUND ACCELERATION CONSTANTS ---
C-----
DO 91 I=1,3
  GV(I)=0
  GA(I)=0
  GDT(I)=0
  GVT(I)=0
  GAT(I)=0
  ACCOLD=0
  YGACC=0
  ZGACC=0
C-----
C- INITIALIZE ENERGY CONSTANTS ---
C-----
PSE=0
EKE=0
EDD=0
BEM=0
PEM=0
PSEM=0
EKEM=0
EDDM=0
DO 92 I=1,5
  SUMFD(I)=0
C-----
C- SET GEOMETRIC STIFFNESS FLAGS ---
C-----
NEWKG=.TRUE.
IF (KGLDAD.EQ.0 .OR. KGTYPF.EQ.0 .OR. KGFORM.EQ.0) NEWKG=.FALSE.
C-----
C- SET STIFFNESS FLAGS ---
C-----
NEWK=.TRUE.
KSAME=0
C-----
INPUT INITIAL VALUES FOR DISPL, VELOC, ACCEL
CALL INTDVA(NDOP,MAXNOD,NNODE,STEPID,NDOP,
  ID,JTFLG,DISP,VEL,ACC,LOAD)
C-----
IF (ISP1.EQ.1) THEN
  EIE=ESE
ELSE IF (ISP1.EQ.0) THEN
  ESE=0
  DO 96 I=1,6
    REACP(I)=0
96 ENDP
C-----
INITILIZE TIME TO CALC INITIAL GROUND ACCELERATION
T-T-DT
TL-T-DT
C-----
READ AND INITIALIZE DYNAMIC GROUND ACCELERATIONS
IP (SLMASS) THEN
  CALL DYLOA(1,MAXACC,NDOP,NAINP,T,TL,
  & ACCEL2,TOACC,DTACC,GRAY,
  & MAXNOD,NNODE,COSINE,IDOP,JTFLG,JTCCS,MCOSS,
  & AGTX,AGTY,AGTZ,MASS,MDMS,IMDSLSL,
  & AINP,MCOSS,DLOAD,GA,GAT)
ELSE
  CALL DYLOA(1,MAXACC,NDOP,NAINP,T,TL,
  & ACCEL2,TOACC,DTACC,GRAY,
  & MAXNOD,NNODE,COSINE,IDOP,JTFLG,JTCCS,MCOSS,
  & AGTX,AGTY,AGTZ,MASS,MDMS,IMDS,
  & AINP,MCOSS,DLOAD,GA,GAT)
ENDIF
C-----
GET INITIAL DYNAMIC LOAD
IP (SLMASS) THEN
  CALL DYLOA(2,MAXACC,NDOP,NAINP,T,TL,
  & ACCEL2,TOACC,DTACC,GRAY,
  & MAXNOD,NNODE,COSINE,IDOP,JTFLG,JTCCS,MCOSS,
  & AGTX,AGTY,AGTZ,MASS,MDMS,IMDSLSL,
  & AINP,MCOSS,LOAD,GA,GAT)
ELSE
  CALL DYLOA(2,MAXACC,NDOP,NAINP,T,TL,
  & ACCEL2,TOACC,DTACC,GRAY,
  & MAXNOD,NNODE,COSINE,IDOP,JTFLG,JTCCS,MCOSS,
  & AGTX,AGTY,AGTZ,MASS,MDMS,IMDS,
  & AINP,MCOSS,LOAD,GA,GAT)
ENDIF
INITILIZE INTEGRATION ROUTINES
IP (INTEGO.EQ.1) THEN
  CALL LINACC(1,DT,1.00,NEWKDY,L2,L3,L4,
  & IMDS,IMD,IMDS,IMDKG,NDOP,NFREE,KGLDAD,KGFORM,
  & ACC,VEL,DACC,DVEL,DDISP,DLOAD2,
  & WORK,MASS,MDMS,DAMP,STIFF,MD,
  & KG,MDKG,P2,SBAR,MDS,AN,BN,PBAR,
  & PDAMP,ALPHA,BETA)
ELSE IF (INTEGO.EQ.2) THEN
  CALL AVEACC(1,DT,THETA,NEWKDY,L2,L3,L4,
  & IMDS,IMD,IMDS,IMDKG,NDOP,NFREE,KGLDAD,KGFORM,
  & ACC,VEL,DACC,DVEL,DDISP,DLOAD2,
  & WORK,MASS,MDMS,DAMP,STIFF,MD,
  & KG,MDKG,P2,SBAR,MDS,AN,BN,PBAR,
  & PDAMP,ALPHA,BETA)
ELSE IF (INTEGO.EQ.3) THEN
  CALL WILSON(1,DT,THETA,KG,MDKG,P2,
  & L2,L3,L4,IMDS,IMD,IMDS,IMDKG,NDOP,NFREE,
  & KGLDAD,KGFORM,ACC,VEL,DISP,LOAD,
  & DACC,DVEL,DDISP,DLOAD2,WORK,
  & SBAR,MDS,AN,BN,PBAR,PDAMP,
  & ALPHA,BETA)
ENDIF
C-----
C- WRITE INITIAL DATA TO OUTPUT FILE ---
C-----
IP (IWRITE.GT.0) CALL DMPDAT(2,IWRITE,TO,DT)
C-----
C- LOOP FOR EACH TIME STEP, ISTEP=CURRENT STEP # -----
C-----
NSTEP = NUMBER OF TIME STEPS
ISTEP = TIME STEP NUMBER
CURRENT TIME
TL = TIME AT PREVIOUS TIME STEP
TO = INITIAL TIME
DT = TIME INCREMENT
TL = FINAL TIME
MCND2 = MCND
NSTEP = (TP-TO)/DT
DO 500 ISTEP=1,NSTEP
  T=TO+ISTEP*DT
  TL=T-DT
  WRITE(ISTEP,90) ISTEP,T
  FORMAT('STEP:',I6,', TIME:',F10.5)
C-----
C- CALC INCREMENTAL LOADS ---
C-----
CALL DYLOA(2,MAXACC,NDOP,NAINP,T,TL,
  & ACCEL2,TOACC,DTACC,GRAY,
  & MAXNOD,NNODE,COSINE,IDOP,JTFLG,JTCCS,MCOSS,
  & AGTX,AGTY,AGTZ,MASS,MDMS,IMDSLSL,
  & AINP,MCOSS,DLOAD,GA,GAT)
P2=-ACCEL2
C-----
ADD UNBALANCED LOADS TO THE LOAD VECTOR. ... STORE AS DLOAD2
STORE OLD UNBALANCED LOADS AT REACTIONS IN DLOAD
IP (UNBAL) THEN
  DO 195 I=L1,L4
    DLOAD2(I)=DLOAD(I)+FUB(I)
  DO 196 I=L5,L6
    DLOAD2(I)=0
  DO 196 I=L5,L6
    DLOAD2(I)=-FUB(I)
C**** OMIT UNBALANCED LOAD FROM REACTIONS...
C**** UNBALANCED LOADS ADDED TO THE FRAME ARE RESTORED BY
C**** 1) INERTIAL FORCES AND DAMPING FORCES
C**** 2) DAMPING FORCES
C**** 3) MEMBER FORCES
C**** SUBTRACTING UNBALANCED FORCES FROM REACTIONS DUE TO MEMBER FORCES
C**** NEGLECTS THE REACTIONS DUE TO INERTIAL AND DAMPING FORCES
196 FUB(I)=0
ELSE
  DO 197 I=L1,L6
    DLOAD2(I)=DLOAD(I)
197 ENDP
C-----
C- FORMULATE STIFFNESS ---
C-----
FORMULATE THE STIFFNESS IF THE FLAG NEWK IS TRUE, THIS FLAG IS
INITIALIZED ABOVE, AND SET WHEN THE ELEMENT FORCES ARE CALC.
THE STIFFNESS IS ALSO FORMULATED IF KG IS TO BE CALCULATED
AND CONDENSATION IS TO BE PERFORMED. (THE STIFFNESS IS NEEDED
FOR THE CONDENSATION PROCEDURE.)
REPEAT=.TRUE.
KSAME=0
100 IP (NEWK) THEN
  I1=1
  CALL FORM(I1)
  IF (BUGS) CALL LMATRIX
  & ENDP (STIFF,MD,NDOP,IMD,'GLOBAL STIFFNESS DUMP-1')
C-----
C- FORM GEOMETRIC STIFFNESS ---
C-----
FORMULATE THE GEOMETRIC STIFFNESS IF THE FLAG NEWK IS TRUE, THIS
FLAG INITIALIZED ABOVE.
IF KGDATA(1)=1, THE GEOMETRIC STIFFNESS IS FORMED ONCE, AND
MULTIPLIED BY A SCALAR TO ACCOUNT FOR CHANGES
IN THE TOTAL VERTICAL LOAD. THUS THE GEOMETRIC
STIFFNESS ONLY NEEDS TO BE FORMED ONCE
IF KGDATA(1)=2, THE GEOMETRIC STIFFNESS IS BASED ON THE AXIAL
FORCE IN THE ELEMENTS, AND IS RECALCULATED EACH
TIME THE AXIAL FORCE CHANGES.
IF KGDATA(1)=3, THE GEOMETRIC STIFFNESS IS FORMED WITH THE
STIFFNESS, THUS A SEPARATE CALL TO FORM IS
NOT NEEDED.
IF KGDATA(1)=2, A SEPARATE GEOMETRIC STIFFNESS MATRIX IS FORMED

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C IF (NEWKG) THEN SOL02690
C FORMULATE SEPARATE GEOMETRIC STIFFNESS MATRIX SOL02700
C IF (KGFORM.EQ.2) THEN SOL02710
  P2 = 0 SOL02720
  I2=2 SOL02740
  CALL FORM(I2) SOL02750
  KCOND=KCOND SOL02760
  IF (BUGS) CALL LMATRIX SOL02770
  A (KG,MDKG,NDOP,INDXG,KG MATRIX DUMP='1') SOL02780
  ELSE SOL02790
  KCOND= .FALSE. SOL02800
  ENDIF SOL02810
C ENDIF SOL02820
C----- SOL02840
C CONDENSE OUT FREE DISPL.-- SOL02850
C----- SOL02860
C IF NCND2 IS TRUE THE MASS MATRIX IS ALSO REDUCED BY GUYAN REDUCTION SOL02870
C IF NCND2 IS TRUE THEN KG IS ALSO REDUCED SOL02880
  IF (NCND.GT.0 AND NEWK) THEN SOL02890
    CALL MSOLL(1,BUGS,LA,IND,NDOP,1,1,1,1,MD,STIFF,DL0AD2,WORK, SOL02900
    & MCOND,MASS,MDMS,INDMS,KCOND2,KG,MDKG,INDXG) SOL02910
    & MCOND2 = .FALSE. SOL02920
  ELSE IF (NCND.GT.0) THEN SOL02930
    REDUCE THE DYNAMIC LOAD MATRIX IF OLD STIFFNESS IS USED ... SOL02940
    CALL MSOLL(2,BUGS,LA,IND,NDOP,1,1,1,1,MD,STIFF,DL0AD2,WORK, SOL02950
    & MCOND,MASS,MDMS,INDMS,KCOND2,KG,MDKG,INDXG) SOL02960
  ENDIF SOL02970
C----- SOL02980
C DETERMINE IF A NEW DYNAMIC STIFFNESS NEEDS TO BE CALC --- SOL02990
C----- SOL03000
  IF (NEWK OR NEWKG OR (XGLOAD.EQ.1 AND ACCELZ.EQ.ACCLD)) THEN SOL03010
    NEWKY = .TRUE. SOL03020
    ACCOLD=ACCELZ SOL03030
  ELSE SOL03040
    NEWKY = .FALSE. SOL03050
  ENDIF SOL03060
C----- SOL03070
C SOLVE DYNAMIC EQUATION FOR INCREMENTAL DISPL.-- SOL03080
C----- SOL03090
  I2=2 SOL03100
  CALL INTGEO(EQ.1) THEN SOL03110
  CALL LINACC(I2,DT,THETA,NEWKY,L1,L3,LA, SOL03120
  & INDMS,IND,INDS,INDKG,NDOP,NFREE,XGLOAD,KGFORM, SOL03130
  & ACC,VEL,DACC,DVEL,DACC,DDISP,DDISP,DL0AD2, SOL03140
  & WORK,MASS,MDMS,DAMP,STIFF,MD, SOL03150
  & KG,MDKG,P2,SBAR,MDS,AN,BN,PBAR, SOL03160
  & FDAMP,ALPHA,BETA) SOL03170
  ELSE IF (INTGEO.EQ.2) THEN SOL03180
  CALL AVSACC(I2,DT,THETA,NEWKY,L1,L3,LA, SOL03190
  & INDMS,IND,INDS,INDKG,NDOP,NFREE,XGLOAD,KGFORM, SOL03200
  & ACC,VEL,DACC,DVEL,DACC,DDISP,DL0AD2, SOL03210
  & WORK,MASS,MDMS,DAMP,STIFF,MD, SOL03220
  & KG,MDKG,P2,SBAR,MDS,AN,BN,PBAR, SOL03230
  & FDAMP,ALPHA,BETA) SOL03240
  ELSE IF (INTGEO.EQ.3) THEN SOL03250
  CALL WILSON(I2,DT,THETA,KG,MDKG,P2, SOL03260
  & L1,L3,LA,INDMS,IND,INDS,INDKG,NDOP,NFREE, SOL03270
  & XGLOAD,KGFORM,ACC,VEL,DISP,LOAD, SOL03280
  & DACC,DVEL,DDISP,DL0AD2,WORK, SOL03290
  & MASS,MDMS,DAMP,STIFF,MD, SOL03300
  & KG,MDKG,P2,SBAR,MDS,AN,BN,PBAR,FDAMP, SOL03310
  & ALPHA,BETA) SOL03320
  ENDIF SOL03330
C----- SOL03340
C BACK SUBSTITUTION TO SOLVE FOR NON-DYNAMIC DOP --- SOL03350
C----- SOL03360
  DISP_C = T + DISP_F SOL03370
  DISP_C = INV(K_CCT) * (K_CCT * DISP_C) SOL03380
  SUBRTN PREA_CTN (IOPT=1) SOL03390
  PERFORMS THE MULTIPLICATION DISP_C = (K_CCT) * DISP_C SOL03400
  SUBRTN MSOLL (IOPT=3) SOL03410
  PERFORMS THE BACK SUBSTITUTION TO EQUIVALENT TO SOL03420
  DISP_C = INV(K_CCT) * DISP_C SOL03430
  ONE=1.00 SOL03440
  IF (NCND.GT.0) THEN SOL03450
    I3=3 SOL03460
    I1=1 SOL03470
  C----- SOL03480
  C DISPLACEMENTS SOL03490
  IF (BUGS) WRITE(108,*) 'CALC DISPL' SOL03500
  CALL REACTN(1,BUGS,NNODE,L1,L3,LA,DDISP,DDISP, SOL03510
  & MD,STIFF,MD(NDOP),NDOP,1,ONE, SOL03520
  & MAXNO,IDO,COORD,1D,TITLE,STEPID, .FALSE., SOL03530
  & NCOB,COSINE,JTCOS,JTFLG,DSUMRC) SOL03540
  CALL MSOLL(1,BUGS,LA,IND,NDOP,1,1,1,1,MD,STIFF,DDISP, SOL03550
  & WORK, .FALSE.,MASS,MDMS,1, .FALSE.,KG,MDKG,1) SOL03560
  C----- SOL03570
  C VELOCITIES SOL03580
  IF (BUGS) WRITE(108,*) 'CALC VEL' SOL03590
  CALL REACTN(1,BUGS,NNODE,L1,L3,LA,DVEL,DVEL, SOL03600
  & MD,STIFF,MD(NDOP),NDOP,1,ONE, SOL03610
  & MAXNO,IDO,COORD,1D,TITLE,STEPID, .FALSE., SOL03620
  & NCOB,COSINE,JTCOS,JTFLG,DSUMRC) SOL03630
  CALL MSOLL(1,BUGS,LA,IND,NDOP,1,1,1,1,MD,STIFF,DVEL, SOL03640
  & WORK, .FALSE.,MASS,MDMS,1, .FALSE.,KG,MDKG,1) SOL03650
  C----- SOL03660
  C ACCELERATION SOL03670
  IF (BUGS) WRITE(108,*) 'CALC ACCEL' SOL03680
  CALL REACTN(1,BUGS,NNODE,L1,L3,LA,DACC,DACC, SOL03690
  & MD,STIFF,MD(NDOP),NDOP,1,ONE, SOL03700
  & MAXNO,IDO,COORD,1D,TITLE,STEPID, .FALSE., SOL03710
  & NCOB,COSINE,JTCOS,JTFLG,DSUMRC) SOL03720
  CALL MSOLL(1,BUGS,LA,IND,NDOP,1,1,1,1,MD,STIFF,DACC, SOL03730
  & WORK, .FALSE.,MASS,MDMS,1, .FALSE.,KG,MDKG,1) SOL03740
  ENDIF SOL03750
C----- SOL03760
C CALCULATE INCREMENTAL MEMBER FORCES --- SOL03770
C----- SOL03780
C ELEMIS CALLS THE INDIVIDUAL ELEMENT LIBRARIES, EACH OF WHICH DETERMINES SOL03790
C 1) DID THE STIFFNESS CHANGE? IF SO, NEWK = TRUE. SOL03800
C 2) SHOULD THE INCREMENTAL DISPLACEMENT BE RECALCULATED? SOL03810
C IF SO, KSAME=KSAME (A VALUE DEPENDING ON ELEMENT TYPE) SOL03820
  LSTYP=0 SOL03830
  PRINT = .FALSE. SOL03840
  HEAD = .FALSE. SOL03850
  NEWK = .FALSE. SOL03860
  NEMT = NEMT SOL03870
  DO 150 I=NO-1,NEMT SOL03880
    CALL ELEMIS SOL03890
    & (LSTYP,PRINT,I,REL,HEAD,NAME,ESSE,EPSE,DAMAGE,DUCFAG, SOL03900
    & IELNO,IELDOP,XDOP,POBEM,RINPUT,AXIAL,I2DDSP,I2DL0A,MSTOR) SOL03910
  C----- SOL03920
  C ZERO STIFFNESS CHANGE FLAGS IF ELASTIC --- SOL03930
  C----- SOL03940
  IF (ELSTIC) THEN SOL03950
    NEWK = .FALSE. SOL03960
    KSAME=0 SOL03970
  ENDIF SOL03980
C----- SOL04000
C DETERMINE IF KG IS TO BE UPDATED BASED ON --- SOL04010
C----- SOL04020
C CHANGING ELEMENT AXIAL FORCE --- SOL04030
C----- SOL04040
  IF (XGLOAD.EQ.2) THEN SOL04050
    NEWKG = .TRUE. SOL04060
  ELSE SOL04070
    NEWKG = .FALSE. SOL04080
  ENDIF SOL04090
  IF (NCND.GT.0) NEWK = .TRUE. SOL04100
  ELSE SOL04110
    NEWK = .FALSE. SOL04120
  ENDIF SOL04130
  ENDIF SOL04140
C----- SOL04150
C REPEAT STEP IF NECESSARY --- SOL04160
C----- SOL04170
C IF AN ELEMENT STIFFNESS CHANGE DICTATED THAT THE STEP BE REANALYZED, SOL04180
C REFORMULATE THE STIFFNESS AND REANALYZE SOL04190
C THE VARIABLES REPEAT IS SET UP TO SUPPRESS THE REANALYZING OF A STEP SOL04200
C CURRENTLY, EACH STEP MAY ONLY BE REANALYZED ONCE, THIS PREVENTS SOL04210
C AN ENDLESS LOOP. SOL04220
C IF (KSAME.NE.0 AND REPEAT) THEN SOL04230
  REPEAT = .FALSE. SOL04240
  GO TO 100 SOL04250
ENDIF SOL04260
C----- SOL04270
C SOLVE FOR REACTIONS --- SOL04280
C----- SOL04290
C REACTIONS ARE DUE TO INTERNAL MEMBER FORCES SOL04300
  IF (BUGS) WRITE(108,*) 'CALC REACT-K*X' SOL04310
  CALL REACTN(2,BUGS,NNODE,L1,LA,LS,LS,DL0AD,DDISP, SOL04320
  & MD,STIFF,MD(NDOP),NDOP,1,1,0, SOL04330
  & MAXNO,IDO,COORD,1D,TITLE,STEPID, .FALSE., SOL04340
  & NCOB,COSINE,JTCOS,JTFLG,DSUMRC) SOL04350
  C----- SOL04360
  C GET SUMMATION OF REACTIONS SOL04370
  CALL REACTN(3,BUGS,NNODE,L1,LA,LS,LS,DL0AD,DDISP, SOL04380
  & MDKG,KG,MDKG(NDOP),NDOP,1,1,0, SOL04390
  & MAXNO,IDO,COORD,1D,TITLE,STEPID, .FALSE., SOL04400
  & NCOB,COSINE,JTCOS,JTFLG,DSUMRC) SOL04410
  C----- SOL04420
  C GET SUMMATION OF REACTIONS DUE TO UNBALANCED LOAD SOL04430
  IF (.NOT.ELSTIC) SOL04440
    IF (BUGS) WRITE(108,*) 'CALC REACT-N*X' SOL04450
    CALL REACTN(4,BUGS,NNODE,L1,LA,LS,LS,DL0AD,DDISP, SOL04460
    & MDKG,KG,MDKG(NDOP),NDOP,1,1,0, SOL04470
    & MAXNO,IDO,COORD,1D,TITLE,STEPID, .FALSE., SOL04480
    & NCOB,COSINE,JTCOS,JTFLG,DSUMRC) SOL04490
  C----- SOL04500
  C SOLVE FOR INCREMENTAL GEOMETRIC STIFFNESS FORCE, DPG --- SOL04510
  C----- SOL04520
  C----- SOL04530
  C----- SOL04540
  C----- SOL04550
  C----- SOL04560
  C----- SOL04570
  C----- SOL04580
  C----- SOL04590
  C----- SOL04600
  IF (KGFORM.EQ.2) THEN SOL04610
    IF (BUGS) WRITE(108,*) 'CALC INCREMENTAL FORCE DUE TO KG' SOL04620
    CALL TPLY(BUGS,TRUE,IND,L1,LA,LS,LA, SOL04630
    & MDKG,KG,DPG,DDISP) SOL04640
  C----- SOL04650
  C SCALE FORCE TO INCLUDE GROUND ACCELERATION SOL04660
  159 DPG(I) = (1-P2)*DPG(I) SOL04670
  ENDIF SOL04680
C----- SOL04690
C SOLVE FOR INCREMENTAL DAMPING FORCE, DPDAMP --- SOL04700
C----- SOL04710
  IF (BETA.NE.0) THEN SOL04720
    C----- SOL04730
    C INCREMENTAL DAMPING FORCE DUE TO STIFFNESS SOL04740
    IF (BUGS) WRITE(108,*) 'CALC DAMPING FORCE/REACT DUE TO STIFFNESS' SOL04750
    CALL TPLY(BUGS,TRUE,IND,L1,LA,LS,LA, SOL04760
    & MD,STIFF,DPDAMP,DVEL) SOL04770
    CALL TPLY(BUGS,TRUE,IND,L1,LA,LS,LA, SOL04780
    & MD,STIFF,DPDAMP,DVEL) SOL04790
  C----- SOL04800
  C INCREMENTAL DAMPING FORCE DUE TO KG SOL04810
  IF (KGFORM.EQ.2) THEN SOL04820
    IF (BUGS) WRITE(108,*) 'CALC DAMPING FORCE/REACT DUE TO KG' SOL04830
    CALL TPLY(BUGS,TRUE,IND,L1,LA,LS,LA, SOL04840
    & MDKG,KG,DPG,DDISP) SOL04850
    CALL TPLY(BUGS,TRUE,IND,L1,LA,LS,LA, SOL04860
    & MDKG,KG,DPG,DDISP) SOL04870
    DO 158 I=L1,L3 SOL04880
      DPG(I) = (1-P2)*DPG(I) SOL04890
    ENDIF SOL04900
  C----- SOL04910
  C CALC SUM OF STIFFNESS PROPORTIONAL DAMPING REACTIONS SOL04920
  CALL REACTN(5,BUGS,NNODE,L1,LA,LS,LS,DL0AD,DPDAMP,DVEL, SOL04930
  & MDMS,MASS,MDMS(NDOP),NDOP,1,1,0, SOL04940
  & MAXNO,IDO,COORD,1D,TITLE,STEPID, .FALSE., SOL04950
  & NCOB,COSINE,JTCOS,JTFLG,DSUMRC) SOL04960
  C----- SOL04970
  C----- SOL04980
  C----- SOL04990
  C----- SOL05000
  C----- SOL05010
  C----- SOL05020
  C----- SOL05030
  C----- SOL05040
  C----- SOL05050
  C----- SOL05060
  C----- SOL05070
  C----- SOL05080
  C----- SOL05090
  C----- SOL05100
  C----- SOL05110
  C----- SOL05120
  C----- SOL05130
  C----- SOL05140
  C----- SOL05150
  C----- SOL05160
  C----- SOL05170
  C----- SOL05180
  C----- SOL05190
  C----- SOL05200
  C----- SOL05210
  C----- SOL05220
  C----- SOL05230
  C----- SOL05240
  C----- SOL05250
  C----- SOL05260
  C----- SOL05270
  C----- SOL05280
  C----- SOL05290
  C----- SOL05300
  C----- SOL05310
  C----- SOL05320
  C----- SOL05330
  C----- SOL05340
  C----- SOL05350
  C----- SOL05360
  C----- SOL05370
  C----- SOL05380
  C----- SOL05390
  C----- SOL05400
  C----- SOL05410
  C----- SOL05420
  C----- SOL05430
  C----- SOL05440
  C----- SOL05450
  C----- SOL05460
  C----- SOL05470
  C----- SOL05480
  C----- SOL05490
  C----- SOL05500
  C----- SOL05510
  C----- SOL05520
  C----- SOL05530
  C----- SOL05540
  C----- SOL05550
  C----- SOL05560
  C----- SOL05570
  C----- SOL05580
  C----- SOL05590
  C----- SOL05600
  CALL ENERGY(L1,L2,L3,LA,LA,MAXNO,NCOB,BUGI, SOL05610
  & ALPHA,BETA,KGFORM,NEMT,NNODE,DT,SUMB,SUMRC, SOL05620
  & DSUMRC,SUMPD,DSUMPD,TPDAMP,DPDAMP,2ERO,OTV,CD,2ERO, SOL05630
  & OPT,DT,WORK,WORK(L6=1),FG,DFG,DACC,DVEL,DDISP,ACC, SOL05640
  & VEL,DISP,LOAD,DL0AD,IDO,JTFLG,COSINE,JTCOS) SOL05650
  ELSE SOL05660
    C----- SOL05670
    C ENERGY FORMULATION WITH/O SPECIAL LOADING MASS --- SOL05680
    C----- SOL05690
    CALL ENERGY(L1,L2,L3,LA,LA,MAXNO,NCOB,BUGI, SOL05700
    & ALPHA,BETA,KGFORM,NEMT,NNODE,DT,SUMB,SUMRC, SOL05710
    & DSUMRC,SUMPD,DSUMPD,TPDAMP,DPDAMP,2ERO,OTV,CD,2ERO, SOL05720
    & OPT,DT,WORK,WORK(L6=1),FG,DFG,DACC,DVEL,DDISP,ACC, SOL05730
    & VEL,DISP,LOAD,DL0AD,IDO,JTFLG,COSINE,JTCOS) SOL05740
  ENDIF SOL05750

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1      IMDS,MDMS , TRUE ,WRITD,MASS,EIE,ESE,PSE,EKE,EDD,      SOLO5610
4      ALPH, BETA, KGFGRM, NEMLT, NNCD, DT, SUMUB, SUNRC,      SOLO5620
4      DSUNRC, SUNFP, DSUNFP, TPDAMP, DFDAMP, GA, GV, GD, GAT,      SOLO5630
4      GVT, GVT, WBRK, WBRN(1,6), PG, PFG, DACC, DVEL, DDISP, ACC,      SOLO5640
4      VEL, DISP, ZERO, ZERO, IDOP, JTFGL, COSINE, JTCOS)      SOLO5650
ENDIF
ELEM -AMAXI(SNGL(EIE),ELEM)      SOLO5670
ELEM -AMAXI(SNGL(ESE),ELEM)      SOLO5680
ELEM -AMAXI(SNGL(PSE),ELEM)      SOLO5690
ELEM -AMAXI(SNGL(EKE),ELEM)      SOLO5700
ELEM -AMAXI(SNGL(EDD),ELEM)      SOLO5710
C-----
C- GET TOTAL GLOBAL LOAD, DISPL, VELOC AND ACCEL ---
C-----
DO 310 I=1,NDOP
ACC(I) = DACC(I) + ACC(I)      SOLO5750
VEL(I) = DVEL(I) + VEL(I)      SOLO5760
DISP(I) = DDISP(I) + DISP(I)      SOLO5770
LOAD(I) = DLOAD(I) + LOAD(I)      SOLO5780
TFDAMP(I) = DFDAMP(I) + TFDAMP(I)      SOLO5810
PFG(I) = DPG(I) + PFG(I)      SOLO5820
IF (ABS(ACC(I)) .GT. ABS(ACCMAX(I))) ACCMAX(I) = ACC(I)      SOLO5830
IF (ABS(VEL(I)) .GT. ABS(VELMAX(I))) VELMAX(I) = VEL(I)      SOLO5840
IF (ABS(DISP(I)) .GT. ABS(DSPMAX(I))) DSPMAX(I) = DISP(I)      SOLO5850
IF (ABS(LOAD(I)) .GT. ABS(RCTMAX(I))) RCTMAX(I) = LOAD(I)      SOLO5860
DACC(I) = 0.00      SOLO5870
DVEL(I) = 0.00      SOLO5880
DDISP(I) = 0.00      SOLO5890
DPDAMP(I) = 0.00      SOLO5900
DLOAD(I) = 0.00      SOLO5910
C-----
310 DO 309 I=1,6
SUNRC(I) = DSUNRC(I) + SUNRC(I)      SOLO5940
SUNFP(I) = DSUNFP(I) + SUNFP(I)      SOLO5950
SUNRCM-SORT(SUNRC(1)**2+SUNRC(2)**2+SUNRC(3)**2)      SOLO5960
SUNRCM-SORT(SUNRC(4)**2+SUNRC(5)**2+SUNRC(6)**2)      SOLO5970
SUMMFO=MAX(SUNRCM,SUMMFO)      SOLO5980
SUMMFO=MAX(SUNRCM,SUMMFO)      SOLO5990
C-----
C- PRINT TOTAL GLOBAL DISPLACEMENTS, VELOCITIES, ACCELERATIONS ---
C-----
IF (WRITD) THEN
CALL JOIDSP(1,MAXNOD,NDOP,NNODE,1,L4,LD,IDOP,CNST,DISP,      SOLO6050
TITLE,'DISPLACEMENTS',STEPID,TRUE,
NCOS,COSINE,JTCOS)      SOLO6060
CALL JOIDSP(1,MAXNOD,NDOP,NNODE,1,L4,LD,IDOP,CNST,VEL,      SOLO6070
TITLE,'VELOCITIES',STEPID,TRUE,
NCOS,COSINE,JTCOS)      SOLO6080
CALL JOIDSP(1,MAXNOD,NDOP,NNODE,1,L4,LD,IDOP,CNST,ACC,      SOLO6090
TITLE,'ACCELERATIONS',STEPID,TRUE,
NCOS,COSINE,JTCOS)      SOLO6100
ENDIF
C-----
C- PRINT TOTAL GLOBAL REACTIONS ---
C-----
CALL REACTN(3,WRITD,NNODE,1,L4,L5,L6,LOAD,ACC,      SOLO6180
MDMS,MASS,MDMS(NDOP),NDOP,1,ONE,
MAXNOD,IDOP,COORD,LD,TITLE,STEPID,TRUE,      SOLO6190
NCOS,COSINE,JTCOS,JTFGL,SUNRC)      SOLO6200
IF (WRITD) WRITE(108,313) SUNMPO,SUMMFO      SOLO6210
313 FORMAT (/4X,'RESULTANT OF REACTIONS, FORCE='/,P,G12.4,
MOMENT='/,G12.4)      SOLO6220
DO 311 I=1,6
J=NDOP+I
IF (ABS(SUNRC(I)).GT.ABS(RCTMAX(J))) RCTMAX(J)=SUNRC(I)      SOLO6270
CONTINUE      SOLO6280
C-----
C- CALCULATE AND PRINT TOTAL MEMBER FORCES ---
C-----
LSTTYP=0
IF (WRITD) THEN
PRINT=TRUE
HEAD=TRUE
ELSE
PRINT=FALSE
HEAD=FALSE
ENDIF
DO 300 IELNO=1,NELMT
CALL ELELIB      SOLO6430
(1,LSTTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,DUCPAG,      SOLO6440
IELNO,IELDOP,KGDOF,PGECM,RINPUT,AXIAL,I2DDSP,I2DLQA,MSTOR)
IF (ESE-PSE).EQ.0) RETURN      SOLO6450
DAMAGE=DAMAGE/(ESE-PSE)      SOLO6460
WRITE(109,533B) DAMAGE      SOLO6470
330 FORMAT (/OX,'STRUCTURE DAMAGE INDEX='/,P,G15.5)
ENDIF
C-----
C- PRINT PEAK ENERGY INFO ---
C-----
WRITE(108,251) TITLE(1),TITLE(2)
5340 FORMAT (' PEAK ENERGY VALUES / IP,
MAX INPUT ENERGY ...../G12.5/
MAX ELASTIC STRAIN ENERGY ...../G12.5/
MAX PLASTIC STRAIN ENERGY ...../G12.5/
MAX KINETIC ENERGY ...../G12.5/
MAX DAMPING ENERGY ...../G12.5/)
C-----
250 FORMAT ('1 STRUCTURE.....',A,' SOLUTION.....',A,/,
2X,A,/, ' LOADING ',IS,SK,A)
RETURN
END
C-----
SUBROUTINE JOIDSP(IOPT,MAXNOD,NDOP,NNODE,NLOAD,NF,LD,IDOP,
CNST,DISP,TITLE,NAME,STEPID,HEAD,NCOS,COSINE,JTCOS)
CHARACTER(*) TITLE(2),STEPID,NAME
CHARACTER*15 CD(6),EQUAL*60
LOGICAL HEAD
DIMENSION IDOP(MAXNOD,6),CNST(MAXNOD,6)
DIMENSION DISP(NDOP,NLOAD),ID(MAXNOD)
DIMENSION D(6),G(6),COSINE(3,3),NCOS(1,JTCOS(MAXNOD))
DATA EQUAL
/-----
JOID010
JOID020
JOID030
JOID040
JOID050
JOID060
JOID070
JOID080
JOID090
JOID100
JOID110
JOID120
JOID130
JOID140
JOID150
JOID160
JOID170
JOID180
JOID190
JOID200
DO 300 I=1,NLOAD
NDSP(I)=0
IF (.NOT.(NCOS.EQ.1.AND.COSINE(1,1).EQ.1.AND.
COSINE(2,2).EQ.1.AND.COSINE(3,3).EQ.1))
AND.IOPT.EQ.2) GO TO 310
C-----
C- PRINT GLOBAL DISPLACEMENTS ---
C-----
C- NOTE: DISPL MAY CONTAIN DISPLACEMENTS, VELOCITIES, ACCELERATIONS OR
EIGENVALUES
C- ALL OF WHICH ARE PRINTED OUT AT EACH NODE.
IF (HEAD) WRITE(108,310) TITLE(1),TITLE(2)
IF (IOPT.NE.2) WRITE(108,315) NAME,L,STEPID,EQUAL(1:NLEN)
IF (IOPT.EQ.2) WRITE(108,415) NAME, EQUAL(1:NLEN)
DO 100 I=1,NNODE
C-----
TRANSFER DISPL TO LOCAL VECTOR :
DO 20 J=1,6
IJ=IDOP(I,J)
D(I) = D(I) + D(J)*CNST(I,J) + D(6)*CNST(I,5)

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D(2) = D(2) + D(4)*CONST(I,1) + D(6)*CONST(I,6)
D(3) = D(3) + D(4)*CONST(I,2) + D(5)*CONST(I,4)
C----- TRANSFER TO GLOBAL DISPLACEMENTS ...
JCOB=JTCOS(I)
DO 25 I=1,3
  J2=J1-3
  G(I)=0
  G(I2)=0
  DO 25 J1=1,3
    J2=J1-3
    G(I1)=G(I1)-COSINE(J1,I,JCOB)*D(J1)
    G(I2)=G(I2)-COSINE(J1,I,JCOB)*D(J2)
  25
C----- WRITE DISPL
DO 30 J=1,6
  WRITE(CD(J),9) G(J)
  30
  9
  FORMAT(I9,G14.5,' ')
  WRITE(I08,220) ID(I),(CD(X),K=1,6)
C----- NDSPLS=NDSPLS-1
100 CONTINUE
C----- IP (NCOB.EQ.1 .AND. COSINE(1,1,1) EQ.1 .AND. COSINE(2,2,1) EQ.1
      .AND. COSINE(3,3,1) EQ.1 ) GO TO 300
C-----
C----- PRINT JOINT DISPLACEMENTS ---
C-----
C----- NOTE DISP MAY CONTAIN DISPLACEMENTS, VELOCITIES, ACCELERATIONS OR
C----- EIGENVALUES...
C----- ALL OF WHICH ARE PRINTED OUT AT EACH NODE...
210 IP (HEAD AND NDSPLS GT. 30) WRITE(I08,310) TITLE(1),TITLE(2)
  IP (IOPT.NE.2) WRITE(I08,316) NAME,L,STEPID,EQUAL(1,NLEN)
  IP (IOPT.EQ.2) WRITE(I08,416) NAME,          EQUAL(1,NLEN)
  DO 200 I=1,NNODE
C----- TRANSFER GLOBAL DISPL TO LOCAL VECTOR ...
DO 219 J=1,6
  I3=IDOP(I,J)
  D(J)=DISP(I3,I)
C----- ADD ROTATIONS TO DISPL FOR CONSTRAINTS
C----- IF UNCONSTRAINED, CONST(I,J)=0
D(1) = D(1) + D(5)*CONST(I,3) + D(6)*CONST(I,5)
D(2) = D(2) + D(4)*CONST(I,1) + D(6)*CONST(I,5)
D(3) = D(3) + D(4)*CONST(I,2) + D(5)*CONST(I,4)
C----- WRITE DISPL
DO 230 J=1,6
  WRITE(CD(J),9) D(J)
  I3=IDOP(I,J)
  IP (I3 GT. NP) CD(J)(15:15)='*'
  230
  WRITE(I08,221) ID(I),(CD(X),K=1,6),JTCOS(I)
C-----
200 CONTINUE
WRITE(I08,320)
300 CONTINUE
10 FORMAT (I9,G14.5)
220 FORMAT (5X,15,6A)
221 FORMAT (5X,15,6A,I10)
310 FORMAT ('1) STRUCTURE ..... /A/' SOLUTION ..... /A)
315 FORMAT (/ /2X, 'GCS',A, ' LOADING #',I5,5X,A, /
  & '1) 1X, '...',A//6X, 'NODE',7X, 'DX',13X, 'DY',13X, 'DZ',
  & '1X, 'RX',13X, 'RY',13X, 'RZ'/)
316 FORMAT (/ /2X, 'JCS',A, ' LOADING #',I5,5X,A, /1X, '...',A
  & '1) 1X, '...',A//6X, 'NODE',7X, 'DX',13X, 'DY',13X, 'DZ',13X, 'RX',
  & '1X, 'RY',13X, 'RZ',6X, '...',A//6X, 'COSINE #'/)
415 FORMAT (/ /2X, 'GCS',A//6X, '...',A//25X,
  & 'NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY//6X,
  & 'NOTE: 7X, 'DX',13X, 'DY',13X, 'DZ',13X, 'RX',13X, 'RY',13X, 'RZ'/)
416 FORMAT (/ /2X, 'JCS',A//6X, '...',A//6X,
  & 'NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY//6X,
  & 'NOTE: 7X, 'DX',13X, 'DY',13X, 'DZ',13X, 'RX',13X, 'RY',13X, 'RZ',
  & '6X, 'COSINE #'/)
320 FORMAT (4X, 'NOTE: * DENOTES A RESTRAINED DEGREE OF FREEDOM')
RETURN
END
C-----
C----- TRIANGULAR MATRIX MULTIPLICATION ...
C----- MATRIX A IS AN UPPER TRIANGULAR MATRIX, WITH MAIN DIAGONAL ADDRESS
C----- MATRIX MD, K IS A VECTOR, P=AX
C----- THE MULTIPLICATION IS CARRIED OUT FROM ROW IR1 TO IR2 AND FROM
C----- COLUMN JC1 TO JC2.
SUBROUTINE TMPLY(PRINT,ZERO,IMD,N,IR1,IR2,JC1,JC2,MD,A,P,X)
  DIMENSION A(IMD),X(N),P(N),MD(N-1)
  LOGICAL PRINT,ZERO
C-----
  IJ1(I,J)=MD(J)+J-1
  IF (PRINT) THEN
    WRITE(I08,500) 'TRIANGULAR MULTIPLICATION',IR1,IR2,JC1,JC2,N
    CALL LMATRIX(A,MD,N,MD(N-1),MATRIX A')
  ENDIF
C----- ZERO P ...
  IP (ZERO) THEN
    DO 10 J=IR1,IR2
      P(J)=0
    10
  ENDIF
C----- MULTIPLY TERMS ABOVE AND INCLUDING THE MAIN DIAGONAL
  DO 30 J=JC1,JC2
    IRC=J-MD(J-1)-MD(J)+1
    MDJ=MD(J)+J
    I1=MAX(IIRC,IR1)
    I2=MIN(J,IR2)
    IF (I2 GE. I1) THEN
      DO 10 I=I1,I2
        P(I)=P(I)+A(MDJ-I)*X(J)
      10
    ENDIF
  20 CONTINUE
C----- MULTIPLY TERMS BELOW THE MAIN DIAGONAL
  DO 30 I=IR1,IR2
    IRC=I-MD(I-1)-MD(I)+1
    MDI=MD(I)+I
    J1=MAX(IIRC,JC1)
    J2=MIN(I-1,JC2)
    IF (J2 GE. J1) THEN
      DO 40 J=J1,J2
        P(I)=P(I)+A(MDI-J)*X(J)
      40
    ENDIF
  30 CONTINUE
  500 FORMAT (/2X,A//2X, ' IR1:',I5, ' IR2:',I5,
    & ' JC1:',I5, ' JC2:',I5, ' N ',I5)
  IF (PRINT) THEN
    DO 515 I=IR1,IR2
      WRITE(I08,520) I,X(I),P(I)
    515
    WRITE(I08,*)
  520 FORMAT (2X,'DOP',I6,' X(DOP)',I9,G14.5,' A*X=P(DOP)',I9,G14.5)
  RETURN
END

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J01D0480 C X IS A VECTOR
J01D0490 C P IS A DIMEN INPUT VECTOR
J01D0500
J01D0510
J01D0520 SUBROUTINE ETMPLY(PRINT,L1,L2,L3,L4,L5,L6,IMD,MD,A,P,X,U,E)
J01D0530 DIMENSION A(IMD),X(L6),P(L6),MD(L6-1),U(L6)
J01D0540 LOGICAL PRINT,T,F
J01D0550 IJ1(I,J)=MD(J)+J-1
J01D0560
J01D0570 T=.TRUE
J01D0580 F=.FALSE
C-----
C----- IF (PRINT) THEN
J01D0590
J01D0600 WRITE(I08,500) 'CALC ENERGY FOR PARTIALLY REDUCED A MATRIX',
J01D0610 & ' L1,L2,L3,L4,L5,L6
J01D0620 & CALL LMATRIX(A,MD,L6,IMD,'MATRIX A ')
J01D0630 DO 515 I=L1,L6
J01D0640 WRITE(I08,520) I,X(I),U(I)
J01D0650
J01D0660
J01D0670
J01D0680
J01D0690
J01D0700 C----- CALC PF = KPF**X1
J01D0710 CALL TMPLY(P,T,IMD,L6,L3,L4,L3,L4,MD,A,P,X)
J01D0720
J01D0730 E=0
J01D0740 DO 10 L=L3,L4
J01D0750 E=E + U(L)*P(L)
J01D0760
J01D0770
J01D0780
J01D0790 C----- CALC PF = KFR**X
J01D0800 CALL TMPLY(P,T,IMD,L6,L1,L4,L5,L6,MD,A,P,X)
J01D0810
J01D0820 C----- CALC PF = KFR**X
J01D0830 CALL TMPLY(P,T,IMD,L6,L5,L6,L1,L6,MD,A,P,X)
J01D0840
J01D0850 C----- CALC E2 = E3 + E4 + XP * PF + KR * PR
J01D0860 DO 20 L=L1,L6
J01D0870 E2 = E + U(L)*P(L)
J01D0880
J01D0890
J01D0900 PRINT ANSWER
J01D0910 IF (PRINT) WRITE(I08,510) E,E1
J01D0920
J01D0930
J01D0940
J01D0950 500 FORMAT (/2X,A//2X,'L1:',I5,' L2:',I5,' L3:',I5,' L4:',I5,
J01D0960 & ' L5:',I5,' L6:',I5)
J01D0970
J01D0980 510 FORMAT (/2X, ' TOTAL ENERGY:',I9,G14.5,' E1:',G14.5)
J01D0990 520 FORMAT (2X,'DOP',I6,' X(DOP)',I9,G14.5,' U(DOP)',I9,G14.5)
J01D1000 RETURN
J01D1010
J01D1020
J01D1030
J01D1040
J01D1050
J01D1060
J01D1070
J01D1080
J01D1090
J01D1100
J01D1110
J01D1120
J01D1130
J01D1140
J01D1150
J01D1160
J01D1170
J01D1180
J01D1190
J01D1200
J01D1210
J01D1220
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J01D1830
J01D1840
J01D1850
J01D1860
J01D1870
J01D1880
J01D1890
J01D1900
J01D1910
J01D1920
J01D1930
J01D1940
J01D1950
J01D1960
J01D1970
J01D1980
J01D1990

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C ..... GENERATE VALUES AT OTHER NODES
C IP (IGEN DT, 0) THEN
C IGEN=IGEN-1
C NODE=NODE-INC
C GO TO 20
C ENDDIF
C ..... LOOP TO READ ADDITIONAL VALUES
C GO TO 10
C ENDDIF
C 500 CONTINUE
C ISPI=0
C ..... PRINT OUT NON ZERO INITIAL VALUES
C FLAG=TRUE
C DO 510 I=1,NDOP
C IF (DISP(I) NE 0 OR VEL(I) NE 0 OR ACC(I) NE 0 OR
C & FORCE(I) NE 0) THEN
C IF (FLAG) WRITE(108,510)
C & FLAG=.FALSE.
C ENDDIF
C 520 CONTINUE
C 510 FORMAT(/5X,'INITIAL NON ZERO VALUES'/TX,'DOP',
C & TX,'FORCE',10X,'DISP',11X,'VEL',11X,'ACC')
C 530 FORMAT(110,4G15.6)
C RETURN
C END
C ..... SUBROUTINE DYLOA(IOP,T,N,NDOP,NAIN,T,TL,ACCEL2,TO,DT,GRAV,
C & MAXMOD,NMODE,COSINE,IDOP,JTFLG,JTCOS,NCOS,
C & TX,TY,TZ,MASS,MD,IMD,A,MC,P,GA,GAT)
C LOGICAL FMT,ECHO,PRINT,REWIND,BTEST
C CHARACTER*80 FORMAT(1),ATITLE(2)
C REAL MASS(IMD),MC(NDOP,3)
C DIMENSION COSINE(3,3),NCOS(1),IDOP(MAXMOD,6),JTCOS(MAXMOD)
C DIMENSION TX(N),TY(N),TZ(N),A(NAIN),P(NDOP),JTFLG(MAXMOD)
C DIMENSION V1(3),V2(3),V3(3),MD(NDOP+1),GA(3),GAT(3)
C ACCEL2=0
C GO TO (100,200) IOPT
C ..... ZERO GLOBAL BASE ACCELERATIONS
C DO 101 I=1,N
C TX(I)=0
C TY(I)=0
C TZ(I)=0
C 101 CONTINUE
C ..... INPUT NUMBER OF ACCEL, AMP FACTOR SCALE, AND TIME INCREMENT
C READ(105,*) NA,ASCALE,TO,DT,PRINT
C ..... INPUT DIRECTION VECTORS V1 AND V2
C READ(105,*) V1,V2
C ..... CALCULATE V3 = V1 X V2
C CALL CROSS(V3,V1,V2,0)
C ..... CALCULATE V2 = V3 X V1, AND NORMALIZE ALL VECTORS...
C CALL CROSS(V2,V3,V1,1)
C ..... INPUT INDIVIDUAL ACCELEROGRAMS
C DO 30 I=1,NA
C READ(105,*) IN,NPTS,IDIR,FMT,ECHO,REWIND
C NPTS=MIN(NPTS,N)
C IF (REWIND.AND.IN.NE.105) REWIND(IN)
C IF (FMT) THEN
C READ(105,91) FORMAT(1),FORMAT(2)
C READ(IN,FORMAT(1)) ATITLE(1),ATITLE(2)
C ELSE
C READ(IN,91) ATITLE(1),ATITLE(2)
C ENDDIF
C IF (IDIR.EQ.1.AND.IDIR.LE.3) THEN
C WRITE(108,10) I,IN,ATITLE(1),ATITLE(2),NPTS,TO,DT
C FORMAT(/5X,'GROUND ACCELERATION RECORD '//
C & 5X,'-----'//
C & 5X,'INPUTTING TRANSLATIONAL ACCELERATION RECORD ',
C & 12,' FROM DIR ',14,'/5X,A/5X,A/
C & 5X,'THE RECORD CONTAINS ',16,' POINTS, ',
C & ' BEGINNING AT TIME: ',1P,10G15.6,
C & ' WITH A TIME INCREMENT OF: ',1P,10G15.6)
C ELSE
C WRITE(108,*) 'INVALID DIRECTION: ',IDIR,' SET 0 < IDIR < 4'
C GO TO 30
C ENDDIF
C IF (IDIR.EQ.1) THEN
C CX=V1(1)
C CY=V1(2)
C CZ=V1(3)
C WRITE(108,92) V1
C ELSE IF (IDIR.EQ.2) THEN
C CX=V2(1)
C CY=V2(2)
C CZ=V2(3)
C WRITE(108,92) V2
C ELSE IF (IDIR.EQ.3) THEN
C CX=V3(1)
C CY=V3(2)
C CZ=V3(3)
C WRITE(108,92) V3
C ENDDIF
C IF (FMT) THEN
C READ(IN,FORMAT(2),END=25) (A(J),J=1,NPTS)
C ELSE
C READ(IN,*,END=25) (A(J),J=1,NPTS)
C ENDDIF
C GO TO 27
C ..... END OF FILE WAS ENCOUNTERED BEFORE ALL INPUT WAS READ
C 25 NPTS=J
C WRITE(108,26) J
C FORMAT(5X,'ONLY ',IS,
C & ' POINTS WERE READ BEFORE END OF FILE WAS ENCOUNTERED')
C 27 IF (ECHO) WRITE(108,94) (A(J),J=1,NPTS)
C WRITE(108,9280)
C CALL SMAX (NPTS,DT,A,YMX,YMN,T1,T2,NPTS,AVE,STDEV,RMS)
C T1=T1-TO
C T2=T2-TO
C WRITE(108,9290) 'INPUT ACCELERATION ',
C & YMX,T1,YMN,T2,AVE,STDEV,RMS
C DO 20 J=1,NPTS
C ACC=A(J)*ASCALE
C TX(J)=TX(J)+ACC*CX
C TY(J)=TY(J)+ACC*CY
C TZ(J)=TZ(J)+ACC*CZ
C 20 CONTINUE
C 30 CONTINUE
C IF (PRINT) THEN
C WRITE(108,31) 'X'
C INTD1000 WRITE(108,93) (TX(J),J=1,N)
C INTD1010 WRITE(108,31) 'Y'
C INTD1020 WRITE(108,93) (TY(J),J=1,N)
C INTD1030 WRITE(108,31) 'Z'
C INTD1040 WRITE(108,93) (TZ(J),J=1,N)
C INTD1050 31 FORMAT(/5X,'GLOBAL BASE ACCELERATION IN THE 'A,
C & ' DIRECTION (g''s) ')
C ENDDIF
C WRITE(108,9280)
C CALL SMAX (N,DT,TX,YMX,YMN,T1,T2,N,AVE,STDEV,RMS)
C T1=T1-TO
C T2=T2-TO
C WRITE(108,9290) 'X-AXIS ACCELERATION ',YMX,T1,YMN,T2,AVE,STDEV,RMS
C CALL SMAX (N,DT,TY,YMX,YMN,T1,T2,N,AVE,STDEV,RMS)
C T1=T1-TO
C T2=T2-TO
C WRITE(108,9290) 'Y-AXIS ACCELERATION ',YMX,T1,YMN,T2,AVE,STDEV,RMS
C CALL SMAX (N,DT,TZ,YMX,YMN,T1,T2,N,AVE,STDEV,RMS)
C T1=T1-TO
C T2=T2-TO
C WRITE(108,9290) 'Z-AXIS ACCELERATION ',YMX,T1,YMN,T2,AVE,STDEV,RMS
C ..... CALCULATE MASS * COSINE MATRIX FOR EACH DOP...
C ..... LOOP FOR DIRECTION JDIR
C DO 90 JDIR=1,3
C ..... ZERO MC MATRIX AND TEMP COSINE MATRIX FOR JDIR
C DO 40 I=1,NDOP
C MC(I,JDIR)=0
C A(I)=0
C ..... CONSTRUCT COSINE MATRIX FOR JDIR
C OMIT ALL DOP THAT ARE CONSTRAINED TO A MASTER DOP...
C DO 50 NMODE,NMODE
C ICOS=JTCOS(NMODE)
C DO 50 K=1,3
C IF (.NOT. BTEST(JTFLG(NMODE),K-1)) THEN
C A(I)=A(I)-COSINE(JDIR,K,ICOS)
C ENDDIF
C 50 CONTINUE
C ..... MULTIPLY MASS * COSINE MATRIX FOR JDIR
C STORE IN MC(____,JDIR)
C DO 60 J=1,NDOP
C MD=MD(I)+J
C M=J-(MD(J)-1)*(MD(J)+1)
C DO 60 I=M,J
C MC(I,JDIR)+MC(I,JDIR)+MASS(MDJ-I)*A(J)
C DO 70 I=2,NDOP
C MDI=MD(I)+I
C M=I-(MDI(I)-1)*(MDI(I)+1)
C DO 70 J=M,I-1
C MC(I,JDIR)+MC(I,JDIR)+MASS(MDI-J)*A(J)
C 90 CONTINUE
C 91 FORMAT (A)
C 92 FORMAT(5X,'DIRECTION OF ACCELERATION: ',
C & 4X,SP,PP,5,' I',SP,PP,5,' J',PP,5,' K ',SS)
C 93 FORMAT (1X,1P,10G12.4)
C 94 FORMAT (/5X,'INPUT ACCELERATION: ',
C & '(g''s) '(1X,1P,10G12.4))
C ..... INITIAL GROUND ACCELERATION ---
C IF (T.GE.TO.AND.T.LE.N*DT) THEN
C I=(T-TO)/DT+1
C J=I-1
C R=(I*DT-TO)/DT
C GAT(1)=TX(I)*R
C GAT(2)=TY(I)*R
C GAT(3)=TZ(I)*R
C IF (J.LE.N.AND.R.NE.1) THEN
C GAT(1)=GAT(1)+(1-R)*TX(J)
C GAT(2)=GAT(2)+(1-R)*TY(J)
C GAT(3)=GAT(3)+(1-R)*TZ(J)
C ELSE
C GAT(1)=0
C GAT(2)=0
C GAT(3)=0
C ENDDIF
C GAT(1)=GAT(1)+GRAV
C GAT(2)=GAT(2)+GRAV
C GAT(3)=GAT(3)+GRAV
C RETURN
C ..... DETERMIN INCREMENTAL ACCELERATIONS ---
C 200 CONTINUE
C ..... DETERMINE THE INCREMENTAL ACCELERATION BETWEEN TIME T AND TL
C DO 201 I=1,3
C 201 GA(I)=0
C T=TL
C DT=DT
C ..... CURRENT ACCELERATION
C IF (T.GE.TO.AND.T.LE.N*DT) THEN
C I=(T-TO)/DT+1
C J=I-1
C R=(I*DT-TO)/DT
C GA(1)=TX(I)*R
C GA(2)=TY(I)*R
C GA(3)=TZ(I)*R
C IF (J.LE.N.AND.R.NE.1) THEN
C GA(1)=GA(1)+(1-R)*TX(J)
C GA(2)=GA(2)+(1-R)*TY(J)
C GA(3)=GA(3)+(1-R)*TZ(J)
C ENDDIF
C TXI=TX(I)
C TXJ=TX(J)
C ENDDIF
C ..... PREVIOUS ACCELERATION
C IF (TL.GE.TO.AND.TL.LE.N*DT) THEN
C I=(TL-TO)/DT+1
C J=I-1
C R=(I*DT-TO)/DT
C GA(1)=GA(1)-TX(I)*R
C GA(2)=GA(2)-TY(I)*R
C GA(3)=GA(3)-TZ(I)*R
C IF (J.LE.N.AND.R.NE.1) THEN
C GA(1)=GA(1)-(1-R)*TX(J)
C GA(2)=GA(2)-(1-R)*TY(J)
C GA(3)=GA(3)-(1-R)*TZ(J)
C ENDDIF
C TXI=TX(I)
C TXJ=TX(J)
C ENDDIF
C ..... MULTIPLY B BY GRAV.
C DO 215 K=1,3
C 215 GA(K)=GA(K)+GRAV
C ..... CALC ACCELERATION VECTOR = MASS * COSINE * GROUND ACCELERATION
C DO 220 I=1,NDOP
C DO 220 K=1,3
C 220 P(I)=P(I)-MC(I,K)*GA(K)
C 9280 FORMAT(/3X,20X,' MAXIMUM AT TIME ',1X,
C & ' MINIMUM AT TIME ',1X,
C & ' RMS ')

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9290 FORMAT (3X,A,1P,7G15.5)

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RETURN
END
SUBROUTINE SOL3A
LOGICAL BTST
INCLUDE 'ZCOMN'
C-----
C----- SET UP CONSTANTS -----
L1=1
L2=NCND
L3=L2+1
L4=NCND-NFREE
L5=L4+1
L6=NDOP
ELSTIC=TRUE
C-----
C----- INITIALIZE STORAGE
Z(IZVAL)=LOCATION OF THE EIGENVALUES
Z(IZVEC)=LOCATION OF THE EIGENVECTORS
Z(IZA)=LOCATION OF THE SYMMETRIC STIFFNESS
Z(IZB)=LOCATION OF THE SYMMETRIC MASS OR KG
C-----
IZVAL=IZ
IZ=IZ-NFREE
IZVEC=IZ
IZ=IZ-NFREE-NFREE
IZA=IZ
IZ=IZ-NFREE-NFREE
IZB=IZ
IZ=IZ-NFREE-NFREE
IZTMP=IZ
IZ=IZ-NFREE
IZTMP2=IZ
IZ=IZ-NDOP
IWB=IZ
C-----
CALL CKSTOR(NDOP,IWB)
C-----
IMD=NZ(IZMD-NDOP)
IMDMS=NZ(IZMDMS-NDOP)
IMDKG=NZ(IZMDKG-NDOP)
C-----
FORM MASS MATRIX -----
DO 400 I=1,IMDMS
Z(1+I*ZMAB-1)=0.
C-----
CALL MFORM(2,IMASS,NMASS,Z(IZMASS),NZ(IZZMASS),MD,NDOP,IMDMS,
& NNODE,MAXNOD,BTEST(IBUG,5),IBUG,NZ(IZIDOP),NZ(IZIDOP),
& Z(IZCNST),NZ(IZJCOS),Z(IZZMAT),Z(IWB),NZ(IZW-21),Z(IZW-27),
& Z(IZW-63),Z(IZW-99),TITLE)
C-----
CALL SOL3A
& (L1,L2,L3,L4,L5,L6,IMD,IMDMS,IMDKG,
& KCOND,MCND, Z(IZKCDT),NZ(IZKCDT)*3,
& NDOP,NCND,NFREE,TITLE,STEPID,
& Z(IZSTIP),Z(IZMASS),Z(IZKG),Z(IZA),Z(IZB),
& NZ(IZMD),NZ(IZMDMS),NZ(IZMDKG),Z(IWB),Z(IZTMP),
& NNODE,NCOS,MAXNOD,IBUG,Z(IZTMP2),
& NZ(IZID),NZ(IZIDOP),Z(IZCNST),NZ(IZJCOS),Z(IZZCORD),
& Z(IZCOS),NZ(IZJCOS),Z(IZZVAL),Z(IZZVEC))
RETURN
END
SUBROUTINE JOIDSP(IOPT,MAXNOD,NDOP,NNODE,NLOAD,NP,ID,IDOP,
& CONST,DISP,TITLE,NAME,L,STEPID,EQUAL,NLEN,
& CHARACTER*(*) CD(6),EQUAL*9)
LOGICAL HEAD
DIMENSION IDOP(MAXNOD,6),CONST(MAXNOD,6)
DIMENSION DISP(MAXNOD,NLOAD),ID(MAXNOD)
DIMENSION D(6),G(6),COSINE(3,3),NCOS,JTCOS(MAXNOD)
DATA EQUAL
& /-----
& NLEN=MIN(LEN(NAME)+1,60)
& NC=1
C-----
C----- LOOP FOR EACH LOAD CASE ---
DO 300 I=1,NLOAD
NDSPLS=1
IF (.NOT.(NCOS.EQ.1 .AND. COSINE(1,1,1).EQ.1 .AND.
& COSINE(2,2,1).EQ.1 .AND. COSINE(3,3,1).EQ.1)
& .AND. IOPT.EQ.2) GO TO 210
C-----
PRINT GLOBAL DISPLACEMENTS ---
C-----
NOTE:DISP MAY CONTAIN DISPLACEMENTS, VELOCITIES, ACCELERATIONS OR
EIGENVALUES.
ALL OF WHICH ARE PRINTED OUT AT EACH NODE.
IF (HEAD) WRITE(108,310) TITLE(1),TITLE(2)
IF (IOPT.NE.2) WRITE(108,315) NAME,L,STEPID,EQUAL(1,NLEN)
IF (IOPT.EQ.2) WRITE(108,415) NAME,
& EQUAL(1,NLEN)
DO 100 I=1,NNODE
C-----
TRANSFER DISPL TO LOCAL VECTOR
DO 30 J=1,6
J3=IDOP(I,J)
D(J)=DISP(I,J,L)
C-----
ADD ROTATIONS TO DISPL FOR CONSTRAINTS
IF UNCONSTRAINED, CONST(I,J)=0
D(1)=D(1)+D(5)*CONST(I,3)+D(6)*CONST(I,5)
D(2)=D(2)+D(4)*CONST(I,1)+D(6)*CONST(I,6)
D(3)=D(3)+D(4)*CONST(I,2)+D(5)*CONST(I,4)
C-----
TRANSFER TO GLOBAL DISPLACEMENTS
JCOS=JTCOS(I)
DO 25 I1=1,3
I2=I1+1
G(I1)=0
G(I2)=0
DO 25 J1=1,3
J2=J1+1
G(I1)+G(I2)*COSINE(J1,I1,JCOS)*D(J1)
G(I2)+G(I1)*COSINE(J1,I1,JCOS)*D(J2)
C-----
WRITE DISPL
DO 30 J=1,6
DO 30 J1=1,3
WRITE(CD(J),9) G(J)
9 FORMAT(1P,G14.6,' ')
WRITE(108,220) ID(I),CD(K),K=1,6)
NDSPLS=NDSPLS+1
100 CONTINUE
IF (NCOS.EQ.1 .AND. COSINE(1,1,1).EQ.1 .AND. COSINE(2,2,1).EQ.1
& .AND. COSINE(3,3,1).EQ.1) GO TO 300
C-----
PRINT JOINT DISPLACEMENTS ---
C-----
NOTE:DISP MAY CONTAIN DISPLACEMENTS, VELOCITIES, ACCELERATIONS OR
EIGENVALUES
ALL OF WHICH ARE PRINTED OUT AT EACH NODE.

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SOL09380
SOL09390
SOL09400
SOL09410
SOL09420
SOL09430
SOL09440
SOL09450
SOL09460
SOL09470
SOL09480
SOL09490
SOL09500
SOL09510
SOL09520
SOL09530
SOL09540
SOL09550
SOL09560
SOL09570
SOL09580
SOL09590
SOL09600
SOL09610
SOL09620
SOL09630
SOL09640
SOL09650
SOL09660
SOL09670
SOL09680
SOL09690
SOL09700
SOL09710
SOL09720
SOL09730
SOL09740
SOL09750
SOL09760
SOL09770
SOL09780
SOL09790
SOL09800
SOL09810
SOL09820
SOL09830
SOL09840
SOL09850
SOL09860
SOL09870
SOL09880
SOL09890
SOL09900
SOL09910
SOL09920
SOL09930
SOL09940
SOL09950
SOL09960
SOL09970
SOL09980
SOL09990

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```

330 IP (I1-J) 320,330,330
340 DO 140 K=1,I1
340 X=X-A(K,J)*B(I,K)
320 J1=J-1
340 IP (J1) 350,350,360
360 DO 170 K=1,J1
370 X=X-A(J,K)*B(I,K)
350 A(I,J)=X/DL(I)
310 CONTINUE
400 RETURN
END
C
SUBROUTINE REBAC1(N,B,DL,2)
DIMENSION B(N,N),DL(N),Z(N,N)
DO 100 J=1,N
P=1
DO 40 I=N,1,-1
X=Z(I,J)
I1=I-1
IF (I1-N) GT 0) GOTO 50
DO 40 K=I1,N
X=X-B(K,I)*Z(K,J)
40 Z(I,J)=X/DL(I)
IF (ABS(Z(I,J)) .GT. P) P=ABS(Z(I,J))
60 CONTINUE
DO 80 I=1,N
Z(I,J)=Z(I,J)/P
100 CONTINUE
RETURN
END
C
SUBROUTINE JACOBI(N,A,D,V,B,Z)
DIMENSION A(N,N),D(N),V(N,N),B(N),Z(N)
20 DO 21 I=1,N
DO 22 J=1,N
22 V(I,J)=0.0
21 V(I,J)=1.0
10 DO 30 K=1,N
D(K)=A(K,K)
E(K)=D(K)
Z(K)=0.0
30 CONTINUE
DO 40 I=1,500
SM=0.0
N1=N-1
DO 50 IP=1,N1
IP1=IP-1
DO 50 IQ=IP1,N
SM=SM+ABS(A(IP,IQ))
IF (SM) 60,70,60
60 IF (I-4) 80,90,90
80 TRESH=0.5*SM/PLCAT(N)**2
90 GOTO 100
100 TRESH=0.0
DO 110 IP=1,N1
IP1=IP-1
DO 110 IQ=IP1,N
G=100.0*ABS(A(IP,IQ))
IF (G LE 1.E-20) G=0.0
IF (I 4) 120,130,130
130 IF (ABS(D(IP))-G) ABS(D(IP)) 120,140,120
140 IF (ABS(D(IQ))-G) ABS(D(IQ)) 120,150,120
150 A(IP,IQ)=0.0
GOTO 110
120 IF (ABS(A(IP,IQ))-TRESH) 110,110,160
160 H=D(IQ)-D(IP)
IF (ABS(H)-G) ABS(H) 170,180,170
180 T=A(IP,IQ)/H
GOTO 190
170 THETA=0.5*H/A(IP,IQ)
T=SIGN(1.0,THETA)/ABS(THETA)*SQRT(1.+THETA*THETA)
190 G=1.0
TAU=S/(1.-C)
H=T*A(IP,IQ)
Z(IP)=Z(IP)+H
Z(IQ)=Z(IQ)+H
D(IP)=D(IP)+H
D(IQ)=D(IQ)+H
A(IP,IQ)=0.0
IF2=IP-1
IF (IP2) 210,210,201
DO 205 K=1,IP2
G=A(K,IP)
H=A(K,IQ)
A(K,IP)=G-S*(H+G*TAU)
A(K,IQ)=H-S*(G-H*TAU)
CONTINUE
IF1=IQ-1
IF (IP1) 211,211,220
DO 215 K=IP1,IQ1
G=A(IP,K)
H=A(IQ,K)
A(IP,K)=G-S*(H+G*TAU)
A(IQ,K)=H-S*(G-H*TAU)
CONTINUE
IF1=IQ1-1
IF (IQ1) 221,221,230
DO 225 K=IQ1,N
G=A(IP,K)
H=A(IQ,K)
A(IP,K)=G-S*(H+G*TAU)
A(IQ,K)=H-S*(G-H*TAU)
CONTINUE
DO 235 K=1,N
G=V(K,IP)
H=V(K,IQ)
V(K,IP)=G-S*(H+G*TAU)
V(K,IQ)=H-S*(G-H*TAU)
235 CONTINUE
110 DO 240 K=1,N
B(K)=B(K)+Z(K)
D(K)=B(K)
Z(K)=0.0
CONTINUE
40 CONTINUE
70 RETURN
END
SUBROUTINE SOLD4
LOGICAL BTEST,UNBAL
INCLUDE 'SOLCOM'
C
C- SET GEOMETRIC STIFFNESS FLAGS ---
Z(12KGD1)=0.0
KGLOAD=NZ(12KGD1)
KGTYP=NZ(12KGD2)
KGFORM=NZ(12KGD3)
C
C- READ INFO AND SET OPTIONS ---
READ (IGS,*) MAXELD,IPRINT,IWRITE,UNBAL
NLOAD=1
IPRINT=MAX(IPRINT,1)
C
C- PRINT HEADER ---

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REDO0330
REDO0340
REDO0350
REDO0360
REDO0370
REDO0380
REDO0390
REDO0400
REDO0410
REDO0420
REDO0430
REBA0010
REBA0020
REBA0030
REBA0040
REBA0050
REBA0060
REBA0070
REBA0080
REBA0090
REBA0100
REBA0110
REBA0120
REBA0130
REBA0140
REBA0150
REBA0160
REBA0170
REBA0180
REBA0190
REBA0200
REBA0210
JAC00010
JAC00020
JAC00030
JAC00040
JAC00050
JAC00060
JAC00070
JAC00080
JAC00090
JAC00100
JAC00110
JAC00120
JAC00130
JAC00140
JAC00150
JAC00160
JAC00170
JAC00180
JAC00190
JAC00200
JAC00210
JAC00220
JAC00230
JAC00240
JAC00250
JAC00260
JAC00270
JAC00280
JAC00290
JAC00300
JAC00310
JAC00320
JAC00330
JAC00340
JAC00350
JAC00360
JAC00370
JAC00380
JAC00390
JAC00400
JAC00410
JAC00420
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JAC00480
JAC00490
JAC00500
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JAC00580
JAC00590
JAC00600
JAC00610
JAC00620
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JAC00650
JAC00660
JAC00670
JAC00680
JAC00690
JAC00700
JAC00710
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JAC00790
JAC00800
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JAC00870
JAC00880
JAC00890
JAC00900
JAC00910
JAC00920
SOL00010
SOL00020
SOL00030
SOL00040
SOL00050
SOL00060
SOL00070
SOL00080
SOL00090
SOL00100
SOL00110
SOL00120
SOL00130
SOL00140
SOL00150
SOL00160
SOL00170
SOL00180
SOL00190
SOL00200
SOL00210
SOL00220

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WRITE(108,101) TITLE(1),TITLE(2),IPRINT,IWRITE
101 FORMAT ('1 STRUCTURE',A,' SOLUTION',A,/,
1X,' SOLUTION #4, STATIC NONLINEAR SOLUTION',/,
4 1X,' INTERVAL FOR PRINTING DATA',A,15/,
4 1X,' INTERVAL FOR WRITING DATA TO FILE',A,15/)
C
IF (KGFORM.EQ.2) WRITE(108,11)
11 FORMAT (1X,' GEOMETRIC STIFFNESS IS NOT INCLUDED, ',
1X,' USE KGDATA(3)=1 TO INCLUDE GEOMETRIC STIFFNESS. ')
IF ( UNBAL) WRITE(108,13) ' ARE '
IF ( NOT UNBAL) WRITE(108,13) ' ARE NOT '
13 FORMAT(' UNBALANCED JOINT FORCES',A,' ADDED TO THE NEXT CYCLE')
C
C-----
C-INPUT FILE OUTPUT CONTROL DATA ---
C-----
IF (IWRITE.GT.0) THEN
IOPT=1
TO=0
DT=0
CALL DMFDDAT(IOPT,IWRITE,TO,DT)
ENDIF
C
C-----
C-INITIALIZE STIFFNESS FOR THIS SOLN ---
C-----
Z(12LOAD) = LOCATION OF THE TOTAL LOAD MATRIX
Z(12DISP) = LOCATION OF THE TOTAL DISPLACEMENT MATRIX
Z(12VEL) = LOCATION OF THE TOTAL VELOCITY MATRIX
Z(12DLGA) = LOCATION OF THE INCREMENTAL LOAD MATRIX
Z(12DSDP) = LOCATION OF THE INCREMENTAL DISPLACEMENT MATRIX
Z(12DVEL) = LOCATION OF THE INCREMENTAL VELOCITY MATRIX
Z(12JO) = LOCATION OF THE JOINT LOADS
Z(12EL) = LOCATION OF THE ELEMENT LOADS
C
IF (12LOAD.EQ.0) THEN
12LOAD = 12
DO 1 I=1,NDOP
2(1-12LOAD-1) = 0
ENDIF
C
IF (12DISP.EQ.0) THEN
12DISP = 12
DO 2 I=1,NDOP
2(I-12DISP-1) = 0
ENDIF
C
IF (12VEL.EQ.0) THEN
12VEL = 12
DO 3 I=1,NDOP
2(I-12VEL-1) = 0
ENDIF
C
IF (12DLGA.EQ.0) THEN
12DLGA = 12
DO 5 I=1,NDOP
2(I-12DLGA-1) = 0
ENDIF
C
IF (12DSDP.EQ.0) THEN
12DSDP = 12
DO 6 I=1,NDOP
2(I-12DSDP-1) = 0
ENDIF
C
IF (12DVEL.EQ.0) THEN
12DVEL = 12
DO 7 I=1,NDOP
2(I-12DVEL-1) = 0
ENDIF
C
12Q = 12
DO 8 I=1,NDOP
2(I-12Q-1) = 0
ENDIF
C
12R = 12
DO 9 I=1,NDOP
2(I-12R-1) = 0
ENDIF
C
IF (12PUB.EQ.0) THEN
12PUB = 12
DO 10 I=1,NDOP
2(I-12PUB-1) = 0
ENDIF
C
12UBLD = 12
DO 12 I=1,NDOP
2(I-12UBLD-1) = 0
ENDIF
C
12UBDP = 12
DO 12 I=1,NDOP
2(I-12UBDP-1) = 0
ENDIF
C
14=12+1
C
DETERMINE THE LENGTH OF THE STIFFNESS MATRIX
IND=NZ(12MD+NDOP)* NZ(12MD)+1
C
SET UP CONSTANTS -----
L1=1
L2=NCORD
L3=L2+1
L4=NCORD+NFREE
L5=L4+1
L6=NDOP
C
NLOAD=1
C
ELASTIC=FALSE.
C
IF (MAXELD.GT.0) THEN
12IELD=12
12=12+MAXELD*5
12ELD=12
12=12+MAXELD*12
ENDIF
C
12PG = 12
12 = 12+NDOP
12DPG = 12
12 = 12+NDOP
12DMAN = 12
12 = 12+NDOP
12DMANJ = 12
12 = 12+NDOP
12DRLO = 12
12 = 12+NDOP
12 = 12+NDOP
12LCA = 12
12 = 12+NDOP
DO 27 I=0,NDOP-1
2(12PG+I)=0
2(12DPG+I)=0
2(12DMAN+I)=0
2(12DMANJ+I)=0
2(12LCA+I)=0

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SOL00230
SOL00240
SOL00250
SOL00260
SOL00270
SOL00280
SOL00290
SOL00300
SOL00310
SOL00320
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SOL00340
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SOL00360
SOL00370
SOL00380
SOL00390
SOL00400
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SOL00500
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SOL00600
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SOL00690
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SOL00970
SOL00980
SOL00990
SOL01000
SOL01010
SOL01020
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SOL01070
SOL01080
SOL01090
SOL01100
SOL01110
SOL01120
SOL01130
SOL01140
SOL01150
SOL01160
SOL01170
SOL01180
SOL01190
SOL01200
SOL01210
SOL01220
SOL01230
SOL01240
SOL01250
SOL01260
SOL01270
SOL01280
SOL01290
SOL01300
SOL01310
SOL01320
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SOL01370
SOL01380
SOL01390
SOL01400
SOL01410
SOL01420
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SOL01460
SOL01470
SOL01480
SOL01490
SOL01500
SOL01510
SOL01520
SOL01530
SOL01540
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SOL01570
SOL01580
SOL01590
SOL01600
SOL01610
SOL01620
SOL01630
SOL01640
SOL01650
SOL01660
SOL01680

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27 Z(IZDRLO-I)*0
-----
C
C----- CHECK MEMORY
CALL CXSTOR(NDOF,IM)
C
C----- SET UP CONSTANTS
IMD =N2(IZMD +NDOF)
IMMS =N2(IZMMS+NDOF)
IMDKG =N2(IZMDKG+NDOF)
IMMSL=1
IMDS =1
C-----
C----- ZERO GROUND MOTION ---
DO 18 I=1,6
S0URCT(I)=0
C-----
C----- ZERO GROUND MOTION ---
DO 19 I=1,9
GROUND(I)=0.
C-----
C-----
19 GROUND(I)=0.
C-----
C-----
C----- CALC PRINTING INFO ---
C-----
C-----
IPDISP=6,NNODE
IPRCTN=7
DO 20 I=1,NNODE
DO 15 J=0,5
IF (STEST(N2(IZPLG-I+1),J)) THEN
IPRCTN=IPRCTN-1
GO TO 20
ENDIF
20 CONTINUE
IPELEM=NELMT
LSTTYP=0
DO 26 I=1,NELMT
I=N2(IZEL-I)
IF (N2(J) NE.LSTTYP) IPELEM=IPELEM+4
26 LSTTYP=N2(J)
111 FORMAT (0P, 'ZMATRIX(',I6, ','),I15,1P,020,10)
C
CALL SOL04A(L1,L2,L3),L4,L5,L6,IMD,IMMS,IMDKG,
4 MAXVEL, IPRMT, IWRITE, UNBAL,
4 MAXDOP, NDOF, NCOSS, NELMT,
4 ZEDDSP, IZDLOA, IBUG, MAXMND,
4 TITLE, STEPID, NDOF, Z(IZRLOA),
4 KGFPCM, XGLDAD, KGTYP2, Z(IZRLOA),
4 NEWRC, NEWK, SUBRCT,
4 IPDISP, IPRCTN, IPELEM, Z(IZMAX), Z(IZRMAX),
4 Z(IZDISP), Z(IZVEL), Z(IZLOAD), Z(IZQ), Z(IZPUB),
4 Z(IZDISP), Z(IZVEL), Z(IZDLOA), Z(IZR), Z(IZW),
4 Z(IZSTEP), Z(IZMAS), Z(IZKG), Z(IZELD), Z(IZBLD),
4 N2(IZMD), N2(IZMMS), N2(IZMDKG), N2(IZELD), Z(IZBDFP),
4 N2(IZID), N2(IZIDOP), Z(IZONST), N2(IZPLG), Z(IZCORD),
4 Z(IZCOS), N2(IZCOS), EIR, ESE, PSE, EKE, EDD,
4 Z(IZFG), Z(IZDPO)
C
RETURN
END
-----
C
C-----
SUBROUTINE JOILOA(BUG,MXNOD,NDOF,NNODE,NLOAD,NF,ID,IDO,CONST,
FORCE,DSFFLG,JTFLG,TITLE,HEAD)
CHARACTER*(*) TITLE(2)
CHARACTER DIR*4,SUP*25,DOF*18
CHARACTER*1 CD(6)
CHARACTER*14 F(6)
LOGICAL DSPFLG,FIRST,BUG,HEAD,BTEST
DIMENSION IDO(MXNOD,6),CONST(MXNOD,6),FORCE(NDOF,NLOAD)
DIMENSION ID(MXNOD),JTFL(MXNOD)
FIRST=.TRUE.
DSFFLG=.FALSE.
5 FORMAT('1. STRUCTURE...',A, '/' SOLUTION...',A, '/')
6 FORMAT('2. APPLIED JOINT LOADS', '/' .....)
C-----
C-----
C----- INPUT LOADING
DO 220 I=1,NLOAD
READ(105,*) LOAD,JOINT,JTGEN,JTINC,DIR,VALUE
IF (INDEX(DIR,'END').NE.0) GO TO 100
15 DOF=
IF (FIRST) THEN
IF (HEAD) WRITE(106,5) TITLE(1),TITLE(2)
WRITE(108,6)
FIRST=.FALSE.
ENDIF
C----- CHECK FOR A VALID JOINT NUMBER.
IF (JOINT LT 1) OR (JOINT GT ID(NNODE)) THEN
WRITE(108,21) LOAD
GO TO 10
ENDIF
J=IQUICK(JOINT, ID,NNODE)
SUP* =
C-----
C----- PUT A LOAD ON A JOINT
C NOTES:
1 IX, IY, IZ, MX, MY AND Z ARE THE DOF THAT THE LOADS ARE
APPLIED TO.
2 THE MOMENTS RESULTING FROM PX, PY AND PZ ARE DUE TO
CONSTRAINTS. IF A NODE IS NOT CONSTRAINED, THEN THE VALUE
OF CONST IS ZERO.
3 LOADS APPLIED TO RESTRAINED DOF ARE INTERPRETED AS
SUPPORT DISPLACEMENTS IF SUPPORT SETTLEMENT OR
C DISPLACEMENT CONTROLL...
C-----
IF (INDEX(DIR,'FM').NE.0) THEN
IX=IDOP(J,1)
FORCE(IX,LOAD)+FORCE(IX,LOAD)+VALUE
IF (BTEST(JTFLG(J),12)) THEN
MY=IDOP(J,5)
FORCE(MY,LOAD)+FORCE(MY,LOAD)+VALUE*CONST(J,3)
MZ=IDOP(J,6)
FORCE(MZ,LOAD)+FORCE(MZ,LOAD)+VALUE*CONST(J,5)
WRITE(DOF,35) IX,MX,MZ
ELSE
WRITE(DOF,35) IX
ENDIF
IF (K.GT.NF) THEN
DSFFLG=.TRUE.
SUPT* = JOINT DISPLACEMENT ...
ENDIF
ELSE IF (INDEX(DIR,'FY') NE.0) THEN
IY=IDOP(J,2)
FORCE(IY,LOAD)+FORCE(IY,LOAD)+VALUE
IF (BTEST(JTFLG(J),13)) THEN
MX=IDOP(J,4)
FORCE(MX,LOAD)+FORCE(MX,LOAD)+VALUE*CONST(J,1)
M2=IDOP(J,6)
FORCE(M2,LOAD)+FORCE(M2,LOAD)+VALUE*CONST(J,6)
WRITE(DOF,35) IY,MX,M2
ELSE
WRITE(DOF,35) IY
ENDIF
IF (K.GT.NF) THEN
DSFFLG=.TRUE.
SUPT* = JOINT DISPLACEMENT ...
ENDIF
ELSE IF (INDEX(DIR,'MZ') NE.0) THEN
IY=IDOP(J,5)
FORCE(IY,LOAD)+FORCE(IY,LOAD)+VALUE
IF (BTEST(JTFLG(J),14)) THEN
MX=IDOP(J,4)
FORCE(MX,LOAD)+FORCE(MX,LOAD)+VALUE*CONST(J,2)
MY=IDOP(J,6)
FORCE(MY,LOAD)+FORCE(MY,LOAD)+VALUE*CONST(J,4)
WRITE(DOF,35) JZ,MX,MY
ELSE
WRITE(DOF,35) JZ
ENDIF
IF (JZ.GT.NF) THEN
DSFFLG=.TRUE.
SUPT* = JOINT DISPLACEMENT ...
ENDIF
ELSE IF (INDEX(DIR,'MY') NE.0) THEN
MY=IDOP(J,5)
FORCE(MY,LOAD)+FORCE(MY,LOAD)+VALUE
IF (BTEST(JTFLG(J),15)) THEN
M2=IDOP(J,6)
FORCE(M2,LOAD)+FORCE(M2,LOAD)+VALUE*CONST(J,6)
WRITE(DOF,35) M2
ENDIF
SUPT* = JOINT DISPLACEMENT ...
ENDIF
ELSE IF (INDEX(DIR,'M2') NE.0) THEN
M2=IDOP(J,6)
FORCE(M2,LOAD)+FORCE(M2,LOAD)+VALUE
IF (BTEST(JTFLG(J),16)) THEN
M1=IDOP(J,5)
FORCE(M1,LOAD)+FORCE(M1,LOAD)+VALUE*CONST(J,5)
WRITE(DOF,35) M1,M2
ENDIF
SUPT* = JOINT DISPLACEMENT ...
ENDIF
ELSE
WRITE(108,20) JOINT,DIR,VALUE
ENDIF
WRITE(108,30) LOAD,JOINT,DIR,DOF,VALUE,SUPT
IF (JTGEN.GT.0) THEN
JTGEN=JTGEN-1
JOINT=JOINT+JTINC
GO TO 15
ENDIF
GO TO 10
200 FORMAT (' INVALID JOINT LOAD...',I6,2X,A,2X,1P,014,6,
2 ' ... LOAD IS SKIPPED')
21 FORMAT (' INVALID LOAD CASE...',I6, ' ... LOAD IS SKIPPED')
30 FORMAT (' LOAD CASE:',I3, ' JOINT:',I6, ' DIRECTION:',A,
4 ' DOP(S):',A, ' MAGNITUDE:',1P,014,6,2X,A)
35 FORMAT (15, ',', ',',15, ',', ',')
C----- RETURN IF NO JOINT LOADS WERE INPUT
100 IF (FIRST) RETURN
IF (.NOT.BUG) RETURN
PRINT JOINT LOADS ON EACH DOF
NS=NDOF-5
DO 130 L=1,NLOAD
IF (HEAD) WRITE(108,5) TITLE(1),TITLE(2)
WRITE(108,130) L
DO 120 I=1,NS+5
I5=MIN(I+4,NDOF)
DO 110 J=I,15
K=J-1
IF (J.GT.NF) CX(K)=
WRITE(108,140) (J,FORCE(J,L),CX(I+J-1),J,I,15)
120 CONTINUE
C-----
130 FORMAT (' APPLIED JOINT LOADS, LOADING #',I5,10X,
4 ' NOTE: * DENOTES SUPPORT DISPLACEMENT',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *')
140 FORMAT (5X,5(0P,15,1P,014,6,A))
C-----
DO 220 L=1,NLOAD
IF (HEAD) WRITE(108,5) TITLE(1),TITLE(2)
WRITE(108,200) L
DO 220 I=1,NNODE
DO 210 J=1,6
IF (BTEST(JTFLG(I),J+1)) THEN
F(J)=
ELSE
F(J)=IDOP(I,J)
WRITE(F(J),240) FORCE(K,L)
IF (K.GT.NF) CX(J)=
ENDIF
CONTINUE
WRITE(108,230) ID(I),(F(J),CX(J),J+1,6)
220 CONTINUE
C-----
200 FORMAT (' APPLIED JOINT LOADS, LOADING #',I5,10X,
4 ' NOTE: * DENOTES SUPPORT DISPLACEMENT',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *',
4 ' * * * * *')
230 FORMAT (5X,15,13X,'P2',13X,'MX',13X,'MY',13X,'M2')
240 FORMAT (1P,014,6)
C-----
RETURN
END
C-----
SUBROUTINE JOIDSP(IOPT,MXNOD,NDOF,NNODE,NLOAD,NF,ID,IDO,
CONST,DISP,TITLE,NAME,STEPID,HEAD,NCOS,COSINE,JTCOS)
CHARACTER*(*) TITLE(2),STEPID,NAME
LOGICAL EQUAL*60
DIMENSION IDO(MXNOD,6),CONST(MXNOD,6)
DIMENSION DISP(NDOF,NLOAD),ID(MXNOD)
DIMENSION D(6),G(6),COSINE(3,3),NCOS(JTCOS,MXNOD)
DATA EQUAL
C-----
NLEN=MIN(LEN(NAME)-1,60)
NC=1
C-----
C-----
C----- LOOP FOR EACH LOAD CASE
C-----
DO 300 I=1,NLOAD
IF (.NOT.(NCOS EQ 1 AND COSINE(1,1,1) EQ 1 AND
4 COSINE(2,2,1) EQ 1 AND COSINE(3,3,1) EQ 1)
4 AND. IOPT.EQ.2) GO TO 210
C-----
C----- PRINT GLOBAL DISPLACEMENTS ---
C-----
NOTE: DISP MAY CONTAIN DISPLACEMENTS, VELOCITIES, ACCELERATIONS OR
EIGENVALUES ...
ALL OF WHICH ARE PRINTED OUT AT EACH NNODE ...
IF (HEAD) WRITE(108,310) TITLE(1),TITLE(2)
IF (IOPT.NE.2) WRITE(108,315) NAME,L,STEPID,EQUAL(1:NLEN)

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IP (IOPT EQ 2) WRITE(108,415) NAME, EQUAL(1,NLEN)
DO 100 I=1,NNODE
C
C ..... TRANSFER DISPL TO LOCAL VECTOR .....
DO 20 J=1,6
IJ=IDOF(I,J)
D(J)=DISP(IJ,L)
C
C ..... ADD ROTATIONS TO DISPL FOR CONSTRAINTS
IF UNCONSTRAINED, CONST(I,J)=0
D(1) = D(1) + D(5)*CONST(I,3) + D(6)*CONST(I,5)
D(2) = D(2) + D(4)*CONST(I,1) + D(6)*CONST(I,6)
D(3) = D(3) + D(4)*CONST(I,2) + D(5)*CONST(I,4)
C
C ..... TRANSFER TO GLOBAL DISPLACEMENTS .....
JCOS=JTCOS(I)
DO 25 I1=1,3
I2=I1-3
G(I1)=0
G(I2)=0
DO 25 J1=1,3
J2=J1-3
G(I1)+G(I1)*COSINE(J1,I1,JCOS)+D(J1)
G(I2)+G(I2)*COSINE(J1,I1,JCOS)+D(J2)
C
C ..... WRITE DISPL
DO 30 J=1,6
WRITE(CD(J),9) G(J)
9 FORMAT(1P,G14.6,' ')
WRITE(108,220) ID(I), (CD(K),K=1,6)
C
C ..... NDSPLS=NDSPLS+1
100 CONTINUE
C
C ..... IF (NCOS.EQ.1 .AND. COSINE(1,1,1).EQ.1 .AND. COSINE(2,2,1).EQ.1
& .AND. COSINE(3,3,1).EQ.1) GO TO 300
C
C ..... PRINT JOINT DISPLACEMENTS .....
C NOTE DISP MAY CONTAIN DISPLACEMENTS, VELOCITIES, ACCELERATIONS OR
EIGENVALUES
C ALL OF WHICH ARE PRINTED OUT AT EACH NODE
C
C ..... TRANSFER GLOBAL DISPL TO LOCAL VECTOR .....
DO 219 J=1,6
IJ=IDOF(I,J)
D(J)=DISP(IJ,L)
C
C ..... ADD ROTATIONS TO DISPL FOR CONSTRAINTS
IF UNCONSTRAINED, CONST(I,J)=0
D(1) = D(1) + D(5)*CONST(I,3) + D(6)*CONST(I,5)
D(2) = D(2) + D(4)*CONST(I,1) + D(6)*CONST(I,6)
D(3) = D(3) + D(4)*CONST(I,2) + D(5)*CONST(I,4)
C
C ..... WRITE DISPL
DO 230 J=1,6
WRITE(CD(J),9) D(J)
IJ=IDOF(I,J)
230 IF (I3.GT.NP) CD(IJ)(15:15)=''
WRITE(108,221) ID(I), (CD(K),K=1,6),JTCOS(I)
C
200 CONTINUE
WRITE(108,320)
300 CONTINUE
10 FORMAT (1P,G15.7)
200 FORMAT (5X,15,6A)
221 FORMAT (5X,15,6A,110)
310 FORMAT ('1) STRUCTURE ..... A, SOLUTION ..... A)
315 FORMAT (1//2X,'GCS',A,/, 'LOADING #',15,5X,A,/,
& 1X,'-----',A//6X,'NODE',7X,'DX',13X,'DY',13X,'DZ',
& 13X,'RX',13X,'RY',13X,'RZ')
316 FORMAT (1//2X,'A',A,/, 'LOADING #',15,5X,A,/,1X,'-----',A
& //6X,'NODE',7X,'DX',13X,'DY',13X,'DZ',13X,'RX',
& 13X,'RY',13X,'RZ',6X,' COSINE #')
415 FORMAT (1//2X,'GCS',A//1X,'-----',A/25X,
& 'NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY//6X,
& 'NODE',7X,'DX',13X,'DY',13X,'DZ',13X,'RX',13X,'RY',13X,'RZ')
416 FORMAT (1//2X,'JCS',A//1X,'-----',A/25X,
& 'NOTE: MAXIMUM VALUES MAY NOT OCCUR SIMULTANEOUSLY//6X,
& 'NODE',7X,'DX',13X,'DY',13X,'DZ',13X,'RX',13X,'RY',13X,'RZ',
& 6X,' COSINE #')
320 FORMAT (4X,'NOTE: * DENOTES A RESTRAINED DEGREE OF FREEDOM')
RETURN
END
C
C ..... SUBROUTINE ELELOA (BUG,NF,NELMT,MAXELD,NELD,NDOP,NLOAD,FORCE,
& IELD,ELD,DISP,LOAD,NAME,TITLE,HEAD)
C
C NOTE ON ARGUMENTS. THE LOAD MATRIX FORCE IS PASSED FROM THE CALLING
ROUTINE. FORCE IS EQUIVALENT TO THE Z MATRIX BY THE CALLING ROUTINE
C IE. 2*(LOAD+FORCE(1,1)), WHERE LOAD IS THE OFFSET IN THE Z MATRIX.
C BOTH THE CALLING ROUTINE AND SUBROUTINE ELE_LIB USE THE Z MATRIX
C
C LOGICAL FIRST,HEAD,FAISE,AXIAL,BUG
CHARACTER*(*) TITLE(2),NAME
CHARACTER*20 TYPE,DIRI,DIR*40
CHARACTER*1 CX(6)
DIMENSION FORCE(NDOP,NLOAD), IELD(5,MAXELD), ELD(12,MAXELD)
DIMENSION RINPUT(100)
C
C FIRST=.TRUE.
5 FORMAT ('1) STRUCTURE ..... A, SOLUTION ..... A,')
6 FORMAT ('7) ELEMENT LOADS ..... A, VALUES ..... A,')
& 1X,'LOAD ELEM GEN INC GROUP & DIRECTION VALUES ..... A,')
C
C INPUT LOADING DATA --
C
10 N=N+1
IF (N.GT.MAXELD) GO TO 100
20 READ(105,*) (IELD(I,N),I=1,4),TYPE,DIRI,ELD(I,N)
DIR=TYPE//DIRI
IF (INDEX(DIR,'END') NE 0) GO TO 100
NELD=N
IF (FIRST) THEN
IF (HEAD) WRITE(108,5) TITLE(1),TITLE(2)
WRITE(108,6)
FIRST=.FALSE.
ENDIF
C
C ..... PROCESS LOAD GROUPS AND READ ADDITIONAL INFO
IF (INDEX(DIR,'CONC') NE 0) THEN
KODE=100
EELD(4)=0
EELD(5)=0
BACKSPACE(105)
J=2
BACKSPACE(105)
READ(105,*) (IELD(I,N),I=1,4),TYPE,DIRI,(ELD(I,N),I=1,J)
DIR=TYPE//DIRI
CALL CERN(ELD(2,N),0,1,'DISTANCE')
ELSE IF (INDEX(DIR,'UNIP') NE 0) THEN
J=1
KODE=200
ELSE IF (INDEX(DIR,'VARR') NE 0) THEN
KODE=300
J=4
BACKSPACE(105)
READ(105,*) (IELD(I,N),I=1,4),TYPE,DIRI,(ELD(I,N),I=1,J)
DIR=TYPE//DIRI
CALL CERN(ELD(2,N),0,1,'DISTANCE')
ELSE IF (INDEX(DIR,'FEM') NE 0) THEN
KODE=400
J=12
BACKSPACE(105)
READ(105,*) (IELD(I,N),I=1,4),TYPE,DIRI,(ELD(I,N),I=1,J)
DIR=TYPE//DIRI
ELSE
WRITE(108,30) 'INVALID LOAD TYPE',
& (IELD(I,N),I=1,4),DIR,ELD(I,N)
GO TO 20
ENDIF
IF (INDEX(DIR,'FX') NE 0) THEN
KODE=KODE+1
ELSE IF (INDEX(DIR,'FY') NE 0) THEN
KODE=KODE+2
ELSE IF (INDEX(DIR,'FZ') NE 0) THEN
KODE=KODE+3
ELSE IF (INDEX(DIR,'MX') NE 0) THEN
KODE=KODE+4
ELSE IF (INDEX(DIR,'MY') NE 0) THEN
KODE=KODE+5
ELSE IF (INDEX(DIR,'MZ') NE 0) THEN
KODE=KODE+6
ELSE IF (KODE.EQ.400) THEN
WRITE(108,30) 'INVALID LOAD DIRECTION',
& (IELD(I,N),I=1,4),DIR,(ELD(I,N),I=1,J)
GO TO 20
ENDIF
30 FORMAT ('1) A, * LOAD IS SKIPPED//
& 1X,4I5,1X,A20,1P,G14.6,/(42X,6G14.6) )
40 FORMAT (1X,4I5,1X,A20,1P,G14.6,/(42X,6G14.6) )
C
C ..... RETURN IF NO ELEMENT LOADS WERE INPUT
100 IF (FIRST) RETURN
C
C ..... LOOP TO GENERATE MEMBER LOADS FOR EACH ELEMENT
IOPT=5
DO 101 IELNO=1,NELMT
FAISE=.FALSE.
CALL ELEM
& (IOPT,LSTYP,FAISE,IREL,FAISE,NAME,EISE,EPSE,DAMAGE,DUCPAC,
& IELNO,IELDOP,KDOP,PGCON,RINPUT,AXIAL,DISP,LOAD,MSTOR)
C
C ..... PRINT LOADS AT EACH DOP
IF (BOG) THEN
NS=NDOP-5
DO 120 L=1,NLOAD
IF (HEAD) WRITE(108,5) TITLE(1),TITLE(2)
WRITE(108,130) L
DO 120 I=1,NS,5
IS=MIN(I+4,NDOP)
DO 110 J=1,15
K=J-I-1
CX(K)=''
IF (J.GT.NP) CX(K)=''
WRITE(108,140) (J,FORCE(J,L),CX(J-I-1),J=1,15)
110 CONTINUE
120 ENDIF
130 FORMAT (' GLOBAL JOINT FORCES DUE TO ELEMENT LOADS ',3X,
& ' LOADING #',15,10X,
& ' * NOTE: * DENOTES SUPPORT DISPLACEMENT'//
& 5X,5(' DOP',7X,'LOAD '))
140 FORMAT (5X,5(OP,15,1P,G14.6,A) )
C
RETURN
END
C
C ..... SUBROUTINE CERN(V,A,B,NAME)
CHARACTER*(*) NAME
IF (A.LT.V .AND. V.LT.B) RETURN
WRITE(108,10) NAME,A,V,B
10 FORMAT ('*****',A, ' IS OUT OF RANGE, SET ',1P,G13.5,' LE INPUT-',
& G13.5,' LE ',G13.5)
V=MIN(V,B)
V=MAX(V,A)
RETURN
END
C
C ..... ENERGY BALANCE
ENER010 ENER010
ENER020 ENER020
ENER030 ENER030
ENER040 ENER040
SUBROUTINE ENERGY(L1,L2,L3,L4,L5,MAXNOD,NCOS,PT2)
DIMENS MDMS,DYN,WRIT0,MASS,EIE,ESE,PEE,EBE,EDD,
& ALPHA,BETA,KGFCRM,NELMT,NGDE,DT,SUMB,SUMRC,
& DSUMRC,SUMPD,DSUMPD,FDAMP,DFDAMP,GA,GV,GD,GAT,
& GVT,GDT,WORK,WORK2,PG,DPG,DACC,DVEL,DDISP,ACC,
& VEL,DISP,LOAD,DLOAD,IDOP,JTPLG,COSINE,JTCOS)
ENER010
ENER020
ENER030
ENER040
REAL MASS,LOAD
CHARACTER*80 NAME
LOGICAL BEST,PT2,AXIAL,DYN,WRIT0
DIMENSION IDOP(MAXNOD,6),COSINE(3,3),NCOS(JTPLG(MAXNOD)
DIMENSION MASS(IMDMS),MDMS(L6-1),JTCOS(MAXNOD)
DIMENSION RINPUT(100),FDAMP(L6),DFDAMP(L6),PG(6),DPG(6)
DIMENSION SUMRC(6),DSUMRC(6),SUMPD(6),DSUMPD(6),SUMB(6)
DIMENSION GD(3),GV(3),GA(3),GDT(3),GVT(3),GAT(3)
DIMENSION DDISP(L6),DVEL(L6),DACC(L6)
DIMENSION DISP(L6),VEL(L6),ACC(L6)
DIMENSION WORK(L6),WORK2(L6),LOAD(L6),DLOAD(L6)
C
C ..... NOMENCLATURE
ENER0250
ENER0260
ENER0270
ENER0280
ENER0290
ENER0300
ENER0310
ENER0320
ENER0330
ENER0340
ENER0350
ENER0360
ENER0370
ENER0380
ENER0390
ENER0400
ENER0410
ENER0420
ENER0430
ENER0440
ENER0450
ENER0460
ENER0470
ENER0480
ENER0490
ENER0500
ENER0510
ENER0520
ENER0530
ENER0540
ENER0550
ENER0560
ENER0570
ENER0580
ENER0590
ENER0600
C
C ..... INTEGRATE GROUND ACCELERATION
IP (DYN) THEN
DO 10 I=1,3
GA(I)=GAI
GV(I)=(2*GAT(I)+GAI)*DT/2
GD(I)=GVT(I)*DT + (3*GAT(I)-GAI)*DT*DT/6
GDT(I)=GAI
GAT(I)=GAI
GVT(I)=GV(I)-GVT(I)
GDT(I)=GD(I)-GDT(I)
10

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ENDIF
IF (PT2) THEN
  IF (DYN) THEN
    WRITE(108,22) (I,ACC(I),VEL(I),DISP(I),LOAD(I),I-1,L4)
    WRITE(108,23) (I,DACC(I),DVEL(I),DDISP(I),DLOAD(I),I-1,L4)
    WRITE(108,24) (I,PG(I),DPG(I),DPAMP(I),DPDAMP(I),I-1,L6)
  ELSE
    WRITE(108,22) (I,ACC(I),VEL(I),DISP(I),LOAD(I),I-1,L6)
    WRITE(108,23) (I,DACC(I),DVEL(I),DDISP(I),DLOAD(I),I-1,L6)
    WRITE(108,24) (I,PG(I),DPG(I),DPAMP(I),DPDAMP(I),I-1,L6)
  ENDIF
22 FORMAT('TIO,' TOTAL RELATIVE RESPONSE FOR PREVIOUS TIME STEP')
   (TIO,'DOP',16,ACC',1P,G13.5,VEL',G13.5,
3 4 DOP',G13.5,LOAD',G13.5))
ENR0710
ENR0720
23 FORMAT('TIO,' INCREMENTAL RELATIVE RESPONSE')
   (TIO,'DOP',16,ACC',1P,G13.5,VEL',G13.5,
4 4 DOP',G13.5,LOAD',G13.5)
ENR0730
ENR0740
ENR0750
ENR0760
ENR0770
ENR0780
ENR0790
ENR0800
ENR0810
ENR0820
ENR0830
ENR0840
ENR0850
ENR0860
ENR0870
ENR0880
ENR0890
ENR0900
ENR0910
ENR0920
ENR0930
ENR0940
ENR0950
ENR0960
ENR0970
ENR0980
ENR0990
ENR1000
ENR1010
ENR1020
ENR1030
ENR1040
ENR1050
ENR1060
ENR1070
ENR1080
ENR1090
ENR1100
ENR1110
ENR1120
ENR1130
ENR1140
ENR1150
ENR1160
ENR1170
ENR1180
ENR1190
ENR1200
ENR1210
ENR1220
ENR1230
ENR1240
ENR1250
ENR1260
ENR1270
ENR1280
ENR1290
ENR1300
ENR1310
ENR1320
ENR1330
ENR1340
ENR1350
ENR1360
ENR1370
ENR1380
ENR1390
ENR1400
ENR1410
ENR1420
ENR1430
ENR1440
ENR1450
ENR1460
ENR1470
ENR1480
ENR1490
ENR1500
ENR1510
ENR1520
ENR1530
ENR1540
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ENR1590
ENR1600
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ENR1910
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ENR1930
ENR1940
ENR1950
ENR1960
ENR1970
ENR1980
ENR1990
ENR2000
ENR2010
ENR2020
ENR2030
ENR2040
ENR2050
ENR2060
ENR2070
ENR2080
ENR2090
ENR2100
ENR2110
ENR2120
ENR2130
ENR2140
ENR2150
ENR2160
ENR2170
ENR2180
ENR2190
ENR2200
ENR2210
ENR2220
ENR2230
ENR2240
ENR2250
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ENR2780
ENR2790
ENR2800
ENR2810
ENR2820
ENR2830
ENR2840
ENR2850
ENR2860
ENR2870
ENR2880
ENR2890
ENR2900
ENR2910
ENR2920
ENR2930
ENR2940
ENR2950
ENR2960
ENR2970
ENR2980
ENR2990
ENR3000

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SUBROUTINE TPLY(PRINT,ZERO,IMD,N,IR1,IR2,JC1,JC2,MD,A,P,X)
DIMENSION A(IMD),X(N),P(N),MD(N*1)
LOGICAL PRINT,ZERO
C
C I(J,I)-MD(J)-J-I
IF (PRINT) THEN
WRITE(108,500) 'TRIANGULAR MULTIPLICATION',IR1,IR2,JC1,JC2,N
CALL LMATRX(A,MD,N,MD(N-1),'MATRIX A ' )
ENDIF
C
C (ZERO) THEN
DO 320 J=IR1,IR2
P(J)=0
320 ENDP
C
C ..... MULTIPLY TERMS ABOVE AND INCLUDING THE MAIN DIAGONAL
DO 30 J=JC1,JC2
IRC=J-MD(J-1)-MD(J)+1
MDI=MD(J)-1
I1=MAX(IRC,IR1)
I2=MIN(J,IR2)
IF (I2.GE.I1) THEN
DO 10 I=I1,I2
P(I)=P(I)+A(MDI-I)*X(I)
10 ENDP
20 CONTINUE
C
C ..... MULTIPLY TERMS BELOW THE MAIN DIAGONAL
DO 30 I=IR1,IR2
IRC=I-MD(I-1)-MD(I)+1
MDI=MD(I)-1
J1=MAX(IRC,JC1)
J2=MIN(I-1,JC2)
IF (J2.GE.J1) THEN
DO 40 J=J1,J2
P(I)=P(I)+A(MDI-J)*X(J)
40 ENDP
30 CONTINUE
500 FORMAT (72X,A/,2X,' IR1',15,' IR2',15,
' JC1',15,' JC2',15,' N',15)
IF (PRINT) THEN
DO 515 I=IR1,IR2
WRITE(108,520) I,X(I),P(I)
515 WRITE(108,*) I,X(I),P(I)
520 FORMAT (2X,'DOP',16,' X(DOP)',1P,G14.5,' A*X-P(DOP)',1P,G14.5)
RETURN
END
C
-----
SUBROUTINE SOLQA
& (L1,L2,L3,L4,L5,L6,IMD,IMDS,SUMKX,
& MAXELD,IPRNT,IWRITE,UNBAL,
& NLOAD,NNODE,NCOS,NELMT,
& IZDLOA,IZDLOA,IBUG,MAXNOD,
& TITLE,STEPID,NDOP,RLoad,
& KGFORM,KGLOAD,KGTYP,DRLOA,
& NEWKG,NEWK,SUMRC,
& IPDISP,IPRCTN,IPELM,DSPMAX,RCRMAX,
& EISP,VEL,Q,PUB,
& DDISP,DVEL,DLOAD,R,WORK,
& STIFF,MASS,KG,ELD,UBLOAD,
& MD,MDS,MKG,IELD,UBDISP,
& ID,IDOP,CNST,UTPLG,COORD,
& COSINE,JTCOS,EIE,ESE,PSE,EKE,EDD,
& PG,DPG)
COMMON /IDSTEP/STEPID
LOGICAL PRINT,DSPPG,HEAD,REPEAT,AXIAL,TEST,BTEST,NEWKAC
LOGICAL NEWKG,NEWK,UNBAL,WRTIO,BUG3,BUG5,BUG6,BUG11
CHARACTER*(*) TITLE(2),STEPID
CHARACTER NAME*90,STEP*10,STEP*10
REAL LOAD,MASS,KG
INTEGER FDAMP
DIMENSION SUMRC(6),DSUMRC(6),SUMPD(6),DSUMPC(6),SUMUB(6)
DIMENSION DSPMAX(L6),RCRMAX(L6),DRLOA(L6),RLoad(L6)
DIMENSION RINPUT(100),WGRK(2*L6),ID(MAXNOD),GAT(3),GVT(3),GDT(3)
DIMENSION STIFF(IMD),MASS(IMDS),KG(IMDKG)
DIMENSION MD(L6-1),MDS(L6-1),MKG(L6-1)
DIMENSION LOAD(L6),DISP(L6),VEL(L6),Q(L6),PG(L6),DPG(L6)
DIMENSION DLOAD(L6),DDISP(L6),DVEL(L6),R(L6)
DIMENSION PUB(L6),UBLOAD(L6),UBDISP(L6)
DIMENSION IELD(1),ELD(1),GA(3),GVT(3),GDT(3)
DIMENSION COORD(MAXNOD,3),UTPLG(MAXNOD),JTCOS(MAXNOD),
& IDOP(MAXNOD,6),COSINE(3,3),NCOS,CNST(MAXNOD,6)
C
DO 5 I=1,L6
SUMUB(I)=0
SUMPC(I)=0
SUMPD(I)=0
SUMRC(I)=0
SUMUB(I)=0
SUMPC(I)=0
SUMPD(I)=0
C
BUG3=TEST(IBUG,3)
BUG5=TEST(IBUG,5)
BUG6=TEST(IBUG,6)
BUG11=TEST(IBUG,11)
C
C
C INITIALIZE ENERGY CONSTANTS ---
EIE=0
ESE=0
PSE=0
EKE=0
EDD=0
C
C SET GEOMETRIC STIFFNESS FLAGS ---
NEWKG=.TRUE.
IF (KGLoad EQ.0 .OR. KGTYP EQ.0 .OR. KGFORM EQ.0) NEWKG=.FALSE.
C
C
C SET STIFFNESS FLAGS ---
NEWK=.TRUE.
KRAMB=0
C
C INPUT LOADING DATA --
C
C ..... MATRICES Q AND R ARE ZEROED IN THE CALLING PGM.
C
C ..... INPUT JOINT LOADS
C
C JOINT LOADS AND SUPPORT DISPLACEMENTS ARE STORED IN Q
CALL JOILOA(BUG3,MAXNOD,NDOP,NNODE,NLOAD,LA,
& ID,IDOP,CNST,Q,DSPPG,UTPLG,TITLE,.FALSE.)
C
C ..... INPUT ELEMENT LOADS
C
C ELEMENT LOADS ARE STORED IN R
IF (MAXELD.GT.0) CALL ELELOA(BUG3,LS,NELMT,MAXELD,NELD,NDOP,
& NLOAD,R,IELD,ELD,IZDLOA,NAME,TITLE,.FALSE.)
C
C WRITE INITIAL DATA TO OUTPUT FILE ---
IF (IWRITE.GT.0) THEN
IOPT=2
CALL DMPDAT(IOPT,IWRITE,TO,DT)
ENDIF
C
C LOOP FOR EACH TIME STEP, ISTEP-CURRENT STEP #
C
C ISTEP = STEP NUMBER

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C-----
C SOLVE FOR DISPL DUE TO UNBALANCED FORCES ---
C-----
C ..... USE FIRST STIFFNESS TO CALCULATE UNBALANCED FORCES.
C IF THE STEP IS REPEATED, DO NOT USE THE NEW STIFFNESS.
C IF (UNBAL AND REPEAT) THEN
C DO 104 I=L1,L4
104 UBDISP(I) = FUB(I)
    UBDISP(I) = -FUB(I)
C DO 105 I=L5,L6
105 UBDISP(I) = -FUB(I)
C IF (BUGS) CALL WMATRX(UBDISP,NDOP,NLOAD,'UNBALANCED LOAD MATRIX')
C CALL MSOLL(2, .FALSE., L4,IND,NDOP,NLOAD,L1,L4,MD,STIFF,UBDISP,
C & WCRK, .FALSE., MASS,MDMS,1, .FALSE.,KG,MDKG,1)
C & CALL MSOLL(3, .FALSE., L4,IND,NDOP,NLOAD,L1,L4,MD,STIFF,UBDISP,
C & WCRK, .FALSE., MASS,MDMS,1, .FALSE.,KG,MDKG,1)
C IF (BUGS)
C & CALL WMATRX(UBDISP,NDOP,NLOAD,'UNBALANCED DISPLACEMENT')
C ENDIF
C ..... ADD UNBALANCED FORCES AND DISPL TO STRUCTURAL LOAD AND DISPL.
C IF (UNBAL) THEN
C DO 107 I=L1,L4
107 DDISP(I)-DDISP(I) = UBDISP(I)
    CONTINUE
C DO 109 I=L5,L6
109 DLOAD(I)-DLOAD(I) = UBDISP(I)
    CONTINUE
C ..... CALCULATE THE NORM OF THE UNBALANCED FORCES
C SUM=0
C DO 108 I=L1,L6
108 SUM=SUM+PUB(I)**2
    PNRM = SQRT(SUM/L6)
C -----
C CALC PSEUDO-VELOCITY ---
C-----
C IF (ISTEP.EQ.1) THEN
C DO 120 I=L1,L6
120 DVEL(I)=Q(I)-R(I)
    PSUVEL-2
    DVEL-2
    ELSE IF (FACTOR.EQ.FACTOR AND
    & NOT ((FACT.LT.FACTOR AND FACTOR.LT.FACT2) .OR.
    & (FACT.GT.FACTOR AND FACTOR.GT.FACT2))) THEN
    PSUVEL = PSUVEL
    DO 121 I=L1,L6
121 DVEL(I) = PSUVEL * (Q(I)-R(I))
    ELSE IF (DVFL.NE.0) THEN
    DO 122 I=L1,L6
122 DVEL(I) = 0
    DVFL = 0
C ENDIF
C-----
C CALCULATE INCREMENTAL MEMBER FORCES ---
C-----
C ELEMENT LIB CALLS THE INDIVIDUAL ELEMENT LIBRARIES, EACH OF WHICH DETERMINES
C 1) DID THE STIFFNESS CHANGE? IF SO, NEWK=TRUE
C 2) SHOULD THE INCREMENTAL DISPLACEMENT BE RECALCULATED?
C IF SO, KSAME=KSAME (A VALUE DEPENDING ON ELEMENT TYPE)
C 3) THE INCREMENTAL ELEMENT FORCES
C 4) THE TOTAL ELEMENT FORCES: STORED IN PUB
C .....
C LSTTYP=0
C PRINT=.FALSE.
C HEAD=.FALSE.
C NEWK=.FALSE.
C DO 150 I=ELNO-1,NELMT
    IOPT=
    CALL ELELIB
    & (IOPT,LSTTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,DUCPAG,
    & IELNO,IELDOP,KGDOP,PGEOM,RINPUT,AXIAL,I2DDSP,I2DLOA,MSTOR)
    IOPT=IOPT
C 150 CONTINUE
C-----
C REPEAT STEP IF NECESSARY ---
C-----
C IF AN ELEMENT STIFFNESS CHANGE DICTATED THAT THE STEP BE REANALYZED,
C REFORMULATES THE STIFFNESS AND REANALYZE
C THE VARIABLE REPEAT IS SET UP TO SUPPRESS THE REANALYZING OF A STEPS
C CURRENTLY, EACH STEP MAY ONLY BE REANALYZED ONCE, THIS PREVENTS
C AN ENDLESS LOOP.
C IF (KSAME.NE.0 .AND. REPEAT) THEN
C REPEAT=.FALSE.
C GO TO 100
C ENDIF
C-----
C SOLVE FOR REACTIONS ---
C-----
C IOPT=2
C CALL REACTN(2,BUGS,NNODE,L1,L4,L5,L6,DLOAD,DDISP,
C & MD,STIFF,IND,NDOP,1,1,0,
C & MAXNO,NDOP,COORD,ITITLE,STEPID,.FALSE.,
C & MCOSS,COSINE,JTCOS,JTFLO,SUMRC)
C-----
C SOLVE FOR INCREMENTAL GEOMETRIC STIFFNESS FORCE, DFG ---
C-----
C ..... INCREMENTAL GEOMETRIC STIFFNESS FORCE
C IF (KGFORN.EQ.2) THEN
C IF (BUGS) WRITE(108,*) '..... INCREMENTAL FORCE DUE TO KG'
C CALL TMPLY(BUGS,TRUE,IND,L4,L3,L4,L3,L4,
C & MDKG,KG,DFG,DDISP)
C SCALE FORCE TO INCLUDE GRAVITATIONAL ACCELERATION ...
C ENDIF
C-----
C PRINT STEP SIZE ---
C-----
C ISTEP=ISTEP
C IPRINT=IPRINT
C WRITE(MOD(ISTEP,IPRINT),EQ.0)
C IF (WRITE) THEN
C WRITE(108,245) TITLE(1),TITLE(2),STEPID,FACTOR,AFAC,DF,FACTL,
C & PMAX,DMAX,FCOEF
C 245 FORMAT ('1) STRUCTURE ... ,A,/' SOLUTION ... ,A,/'
C & //,2X,A/' TARGET FACTOR ... ,IP,G12.4,
C & //,2X,A/' INC FACTOR ... ,IP,G12.4,
C & //,2X,A/' PREV FACTOR ... ,IP,G12.4,/'
C & //,2X,A/' DMAX ... ,IP,G12.4,/'
C & //,2X,A/' NORM OF UNBALANCED FORCE ... ,G12.4,/'
C LINE=6
C ENDIF
C-----
C CALC AND ENERGIES AND DUCTILITIES ---
C-----
C IF (WRITE) LINE=LINE+9
C ..... PULL JOINT DISPL FORCES OUT OF LOAD MATRIX ...
C IF (DSEPL) THEN
C DO 137 I=L1,L4
137 DLOAD(I) = DLOAD(I) - DLOAD(I)
    DLOAD(I) = DLOAD(I) - DLOAD(I)
C-----
C-----
C CALL ENERGY(L1,L2,L3,L4,L5,L6,MAXNO,NCOS,BUG1,
C & MDMS,MDMS,.FALSE.,WRITE,MASS,ESE,EPSE,PSB,EKE,EDD,
C & ALPHA,BETA,KCFORN,NELEMT,NCODE,DT,SUMRC,SUMRC,
C & DSUMRC,SUMFPO,DSUMFPO,PDAMP,DFDAMP,GA,GV,GD,CAT,
C & GVT,GDT,WORK,WCRK(L6,1),ZZERO,ZERO,DACC,DVEL,DDISP,ACC,
C & VEL,DISP,LOAD,DLOAD,IDOP,JTFLO,COSINE,JTCOS)
C-----
C GET TOTAL GLOBAL LOADS AND DISPL ---
C-----
C DO 239 I=L1,L4
239 LOAD(I)-LOAD(I)-DLOAD(I) = RLOAD(I)
    DLOAD(I) = RLOAD(I) - DLOAD(I)
    DISP(I)-DISP(I)-DDISP(I) = DDISP(I)
    DISP(I) = DDISP(I) - DDISP(I)
C CONTINUE
C DO 240 I=L5,L6
240 LOAD(I)-LOAD(I)-DLOAD(I) = RLOAD(I)
    DLOAD(I) = RLOAD(I) - DLOAD(I)
    DISP(I)-DISP(I)-DDISP(I) = DDISP(I)
    DISP(I) = DDISP(I) - DDISP(I)
C CONTINUE
C ..... GET MAXIMUM LOADS AND DISPL
C DO 1007 I=L1,L6
1007 IF (ABS(DISP(I)) .GT. ABS(DSPMAX(I))) DSPMAX(I) = DISP(I)
    IF (ABS(LOAD(I)) .GT. ABS(RCTMAX(I))) RCTMAX(I) = LOAD(I)
    CONTINUE
C 1007 CONTINUE
C-----
C PRINT TOTAL GLOBAL DISPLACEMENTS ---
C-----
C IF (WRITE) THEN
C IF (IPDISP=LINE LE 60) THEN
C LINE=IPDISP+LINE
C ELSE
C LINE=IPDISP+4
C WRITE(108,250) TITLE(1),TITLE(2), ISTEP,STEPID
C ENDIF
C 250 FORMAT ('1) STRUCTURE ... ,A,/' SOLUTION ... ,A,/'
C & //,2X, ' LOADING #',IS,5X,A)
C & CALL JOIDSP(2,MAXNO,NDOP,NNODE,1,L6,ID,IDOP,CNST,DISP,
C & TITLE,'DISPLACEMENTS',STEPID,.FALSE.,
C & MCOSS,COSINE,JTCOS)
C ENDIF
C-----
C PRINT TOTAL GLOBAL REACTIONS ---
C-----
C IF (WRITE) THEN
C IF (IPRCTN=LINE LE 60) THEN
C LINE=IPRCTN+LINE+4
C ELSE
C LINE=IPRCTN+8
C WRITE(108,250) TITLE(1),TITLE(2), ISTEP,STEPID
C ENDIF
C CALL REACTN(3,WRITE,NNODE,L1,L4,L5,L6,LOAD,DISP,
C & MD,STIFF,IND,NDOP,1,1,0,
C & MAXNO,NDOP,COORD,ITITLE,STEPID,.FALSE.,
C & MCOSS,COSINE,JTCOS,JTFLO,SUMRC)
C SUMRC=SQRT(SUMRC(1)**2+SUMRC(2)**2+SUMRC(3)**2)
C SUMMCM=SQRT(SUMRC(4)**2+SUMRC(5)**2+SUMRC(6)**2)
C SUMMFC=MAX(SUMRC(1),SUMRC(2),SUMRC(3))
C SUMMCM=MAX(SUMMCM,SUMMFC)
C IF (WRITE) WRITE(108,313) SUMMFC,SUMMCM
C 313 FORMAT ('/4X, 'RESULTANT OF REACTIONS, FORCE',IP,G12.4,
C & ' MOMENT',IP,G12.4)
C DO 1008 I=1,6
1008 IF (ABS(SUMRC(I)) .GT. ABS(SUMRCM(I))) SUMRCM(I) = SUMRC(I)
    CONTINUE
C ..... CALCULATE AND PRINT TOTAL MEMBER FORCES ---
C-----
C IOPT=5
C LSTTYP=0
C HEAD=.FALSE.
C IF (WRITE) THEN
C PRINT=.TRUE.
C IF (IPELEM=LINE LE 60) THEN
C LINE=IPELEM+LINE
C ELSE
C PRINT=.FALSE.
C DO 300 I=ELNO-1,NELMT
300 CALL ELELIB
    & (IOPT,LSTTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,DUCPAG,
    & IELNO,IELDOP,KGDOP,PGEOM,RINPUT,AXIAL,I2DDSP,I2DLOA,MSTOR)
C-----
C WRITE DATA TO OUTPUT FILES ---
C-----
C IF (IWRITE.GT.0 .AND. MOD(ISTEP,IWRITE).EQ.0) THEN
C IOPT=2
C CALL DMPPAT(IOPT,IWRITE,TO,DT)
C ENDIF
C-----
C EITHER GET A NEW FACTOR, OR KEEP WORKING ON THE CURRENT ONE ---
C-----
C FACTL=FACTL-DF
C IF (NEWFAC) GO TO 1000
C GO TO 1100
C-----
C PRINT MAXIMUM DISPLACEMENTS ---
C-----
C 5000 LINE=TDSPR+4
C WRITE(108,250) TITLE(1),TITLE(2),0,'MAXIMUM DISPLACEMENTS'
C CALL JOIDSP(2,MAXNO,NDOP,NNODE,1,L6,ID,IDOP,CNST,DSPMAX,
C & TITLE,'DISPLACEMENTS','MAXIMUM VALUES',.FALSE.,
C & MCOSS,COSINE,JTCOS)
C-----
C PRINT MAXIMUM GLOBAL REACTIONS ---

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C-----
WRITE(10,250) TITLE(1),TITLE(2),0,'MAXIMUM REACTIONS'
CALL REACTN(4,TRUE,NMODE,LA,LA,LS,LS,ACTMAX,DSPPAX,
& MD,STIFF,IND,NDOF,1,1,0,
& MAXMOD,IDOP,COORD,ID,TITLE,'MAXIMUM VALUES',.FALSE,
& NCOS,COSINE,JTCS,JPLG,SURCRM)
5111 FORMAT (/4X,'MAXIMUM RESULTANT OF REACTIONS, FORCE= ',IP,G12,4,
& MCMENT= ',G12,4)
C-----
C- PRINT MAXIMUM MEMBER FORCES ---
C-----
LSTYP=0
HEAD=.FALSE.
PRINT=.TRUE.
WRITE(108,250) TITLE(1),TITLE(2),0,'PEAK ELEMENT FORCES'
DO 5100 IELNO=1,NELMT
5100 CALL ELELIB
& (12,LSTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,DUCFAG,
& IELNO,IELDOP,KGDOP,PGEOM,RINPOT,AXIAL,IZDOSP,IZDLOA,MSTOR)
C-----
C- PRINT DUCTILITIES AND EXCURSION RATIOS ---
C-----
LSTYP=0
HEAD=.FALSE.
PRINT=.TRUE.
WRITE(108,250) TITLE(1),TITLE(2),0,
& DO 5110 IELNO=1,NELMT
5110 CALL ELELIB
& (11,LSTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,DAMAGE,DUCFAG,
& IELNO,IELDOP,KGDOP,PGEOM,RINPOT,AXIAL,IZDOSP,IZDLOA,MSTOR)
C-----
C- PRINT DAMAGE INDICIES ---
C-----
LSTYP=0
HEAD=.FALSE.
PRINT=.TRUE.
WRITE(108,250) TITLE(1),TITLE(2),0,'DAMAGE INDEX'
DAMAGE=0
DO 5120 IELNO=1,NELMT
CALL ELELIB
& (14,LSTYP,PRINT,IREL,HEAD,NAME,ESE,EPSE,EDAM,DUCFAG,
& IELNO,IELDOP,KGDOP,PGEOM,RINPOT,AXIAL,IZDOSP,IZDLOA,MSTOR)
5120 DAMAGE=DAMAGE+EDAM
IF( (ESE-PSE)/EQ,0 ) RETURN
DAMAGE=DAMAGE+(ESE-PSE)
WRITE(108,5130) DAMAGE
5130 FORMAT (/10X,'STRUCTURE DAMAGE INDEX= ',IP,G15.5)
C-----
RETURN
END
C-----
SUBROUTINE SOL05
LOCAL TEST,TEST
COMMON /RSA/ ICOMN,RSAP(12)
REAL ICOMN
CHARACTER*20 ICOM
$INCLUDE 'ICOM'
C-----
C- PRINT HEADER ---
C-----
OPEN(107,FILE='PEM51',FORM='UNFORMATTED',STATUS='UNKNOWN')
WRITE(108,10) TITLE(1),TITLE(2)
10 FORMAT ('1 STRUCTURE',A,' SOLUTION',A,A)
DO 555 I=1,20
555 WRITE(108,*)
WRITE(108,557)
557 FORMAT (40X,'** RESPONSE SPECTRUM ANALYSIS **',//40X,
& NOTE: MODAL COMBINATION APPROACH CAN BE SRSS ',OR CQC')
C-----
C- INPUT DATA ---
C-----
READ(105,*) ICOM,NMODE,DAMP,NA,NAP,IPRT
READ(105,*) NSTEP,MEIG
C NOTE: NSTEP = 1 - CALCULATE EIGENPAIRS, STORE EIGENPAIRS IN DISK
NSTEP = 2 - READ EIGENPAIR FROM DISK AND DO SPECTRUM ANALYSIS
NSTEP = 3 - CALCULATE EIGENPAIRS AND PERFORM SPECTRUM ANALYSIS,
EIGENPAIRS ARE NOT STORED IN DISK
C-----
WRITE(108,10) TITLE(1),TITLE(2)
IF( TEST(ICOM,'SRSS'),.FALSE. ) THEN
ICOM=1
WRITE(108,11) 'SRSS MODAL COMBINATION METHOD',
&
ELSE IF( TEST(ICOM,'CQC'),.FALSE. ) THEN
ICOM=2
WRITE(108,11) 'CQC MODAL COMBINATION METHOD',
&
ELSE
WRITE(108,*) 'INVALID MODAL COMBINATION METHOD'
WRITE(108,11) ICOM,
RETURN
ENDIF
C-----
WRITE(108,12) NMODE,DAMP,NA,NAP
12 FORMAT (1X,' SOLUTION #5, ',A,1X,/,A,/)
13 FORMAT (1X,' NUMBER OF MODES CONSIDERED ',OP,14,/,
& 1X,' DAMPING RATIO FOR ALL MODES ',OP,G12.3,/,
& 1X,' NO. OF SEISMIC COMPONENTS CONSIDERED ',OP,14,/,
& 1X,' NO. OF INPUT POINTS FOR SPECTRUM ',OP,14)
C-----
IF( NSTEP EQ 1 ) THEN
WRITE(108,13) NSTEP,MEIG
13 FORMAT (1X,' NSTEP ',OP,14,/,
& 1X,' EIGEN-SOLUTION IS STORE IN DISK UNIT ',OP,14)
ELSE IF( NSTEP EQ 2 ) THEN
WRITE(108,14) NSTEP,MEIG
14 FORMAT (1X,' NSTEP ',OP,14,/,
& 1X,' EIGEN-SOLUTION IS READ FROM DISK UNIT ',OP,14)
ENDIF
ELSTIC=.TRUE.
C-----
INITIALIZE STORAGE
2(IZVAL) = LOCATION OF THE EIGENVALUES
2(IZVEC) = LOCATION OF THE EIGENVECTORS
2(IZA) = LOCATION OF THE SYMMETRIC STIFFNESS
2(IZB) = LOCATION OF THE SYMMETRIC MASS
2(IZLOAD) = LOCATION OF THE TOTAL LOAD MATRIX
2(IZDISP) = LOCATION OF THE TOTAL DISPLACEMENT MATRIX
IZVAL = IZ
IZ = IZ-NDFREE
IZVEVC = IZ-NDFREE-NDFREE
IZ = IZ-NDFREE-NDFREE
IZA = IZ
IZ = IZ-NDFREE-NDFREE
IZB = IZ
IZ = IZ-NDFREE-NDFREE
IF( IZLOAD EQ 0 ) THEN
IZLOAD=IZ
IZ = IZ-NDOF
CALL CKXSTOR(0,0)
DO 1 I=1,NDOF
1 2(1:IZLOAD-1)=0
ENDIF
IF( IZDISP EQ 0 ) THEN
IZDISP=IZ
IZ = IZ-NDOF
CALL CKXSTOR(0,0)

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DIMENSION ELEP(NELMT,12,MODE,NA)
C
C----- SET UP CONSTANTS -----
L1=1
L2=NCND
L3=L2+1
L4=NCND+NFREE
L5=L4+1
L6=NDOP
PRINS=BTEST(IBUG,5)
C----- FORMULATE STIFFNESS -----
C----- CALL FORM(1)
IF (PRINS) CALL LMATRX(STIFF,MD,NDOP,IMD,'GLOBAL STIFFNESS MATRIX')
C----- CONDENSE OUT NON-FREE DOP ---
C----- IF MCOND IS TRUE THE MASS MATRIX IS ALSO REDUCED BY GUYAN REDUCTIONS
IF (MCOND.GT.0) THEN
  SET UP ZERO LOAD VECTOR
  DO 28 I=1,NDOP
    TMP(I)=0
  28 CALL MSOLL(1,BTEST(IBUG,6),L4,IMD,NDOP,1,L1,L2,MD,STIFF,TMP,
    & WORK,MCOND,MASS,MDSM,IMDMS,.FALSE.,KG,MDKG,IMDKG)
  ENDP
C----- TRANSFER STIFFNESS INTO A MATRIX ---
C----- NOTE: INMEL ROUTINE GVCSP IS USED TO SOLVE FOR EIGENVALUES,
C----- THIS ROUTINE USES A 'FULL SYMMETRIC MATRIX'.
DO 600 J=L3,L4,+1
  DO 600 I=J,L4,+1
    IJ=MD(J)*J-I
    IF (IJ.LT.MD(J+1) .AND. IJ.GE.MD(J)) THEN
      A(I-L3+1,J-L3+1)=STIFF(IJ)
      SA(I-L3+1,I-L3+1)=A(I-L3+1,J-L3+1)
    ELSE IF (IJ.GE.MD(J+1)) THEN
      A(I-L3+1,J-L3+1)=0
      A(I-L3+1,I-L3+1)=0
    ENDIF
  29 IF (PRINS) WRITE(108,29) I,J,A(I-L3+1,J-L3+1)
  FORMAT(' I ',I5,' J ',J5,' A(I-L3+1,J-L3+1)',IP,G15.5)
C----- CHECK FOR ZERO'S ON MAIN DIAGONAL
IF (I.EQ.J .AND. A(I-L3+1,J-L3+1).LE.0) THEN
  WRITE(108,32) I,A(I-L3+1,J-L3+1)
  STOP
ENDIF
600 CONTINUE
42 FORMAT(' SX, DOP #',I5,' IS LE. 0 IN THE STIFFNESS MATRIX ',
& ' SX,*(I,I)=-1,IP,G12.4.' SOLN IS ABORTED')
C----- TRANSFER MASS INTO B MATRIX ---
DO 700 J=L3,L4,+1
  DO 700 I=J,L4,+1
    IJ=MDMS(J)*J-I
    IF (IJ.LT.MDMS(J+1) .AND. IJ.GE.MDMS(J)) THEN
      B(I-L3+1,J-L3+1)=MASS(IJ)
      BI(L3+1,I-L3+1)=B(I-L3+1,J-L3+1)
    ELSE IF (IJ.GE.MDMS(J+1)) THEN
      B(I-L3+1,J-L3+1)=0
      B(I-L3+1,I-L3+1)=0
    ENDIF
  39 IF (PRINS) WRITE(108,39) I,J,B(I-L3+1,J-L3+1)
  FORMAT(' I ',I5,' J ',J5,' B(I-L3+1,J-L3+1)',IP,G15.5)
C----- CHECK FOR ZERO'S ON MAIN DIAGONAL
IF (I.EQ.J .AND. B(I-L3+1,J-L3+1).LE.0) THEN
  WRITE(108,42) I,B(I-L3+1,J-L3+1)
  STOP
ENDIF
700 CONTINUE
44 FORMAT(' SX, DOP #',I5,' IS LE. 0 IN THE MASS MATRIX ',
& ' SX,*(I,I)=-1,IP,G12.4.' SOLN IS ABORTED')
C----- SOLVE FOR EIGENVALUES AND EIGENVECTORS ---
C----- SU BROUTINE GVCSP
C----- EIGENVALUES AND EIGENVECTORS OF A*X=LAMBDA*B*X=0
C A = STIFFNESS - REAL, FULL SYMMETRIC MATRIX
C B = MASS - REAL, FULL SYMMETRIC MATRIX
C N = NUMBER OF DOP OF A AND B
C EVAL= EIGENVALUES, N BY 1
C EVEC= EIGENVECTORS, N BY N
C-----
IF (NSTEP.EQ.1.OR.NSTEP.EQ.3) THEN
  OPEN(109,FILE='PMS0',FORM='UNFORMATTED',STATUS='UNKNOWN')
  WRITE(109) A
  CALL GVCSP(NFREE,A,B,EVAL,EVEC,WORK,TMP2,TMP)
  REWIND 109
  READ(109) A,B
  DO 15 I=1,NFREE
    DO 15 J=1,NFREE
      A(I,J)=A(I,J)/SQRT(B(I,I)*B(J,J))
  15 CLOSE (109)
C----- NORMALIZE MODE SHAPE TO MAX VALUE OF ONE ---
C----- NOTE THE EIGENVECTOR IN INMEL GVCSP HAS BEEN NORMALIZED
C----- WRITE EIGENVALUES AND EIGENVECTORS TO DISK
WRITE(107) ((EVEC(I,J),J=1,IPRT),I=1,NFREE)
C
ELSE IF (NSTEP.EQ.2) THEN
  READ(107) (EVAL(I),I=1,IPRT)
  READ(107) ((EVEC(I,J),J=1,IPRT),I=1,NFREE)
  ENDP
C----- PRINT EIGENVALUES AND EIGENVECTORS ---
IST = 1
100 IEND=MIN(IST-6,IPRT)
C
WRITE(108,110) (I,I=IST,IENT)
DO 125 I=IST,IENT
  EVAL(I)=SQRT(EVAL(I))
  WRITE(108,126) (EVAL(I),I=IST,IENT)
DO 127 I=IST,IENT
  WRITE(108,127) (EVAL(I),I=IST,IENT)
DO 129 I=IST,IENT
  IF (EVAL(I).LT.1E-10) EVAL(I)=1E-10
  EVAL(I)=1.00/EVAL(I)
  WRITE(108,130) (EVAL(I),I=IST,IENT)
  WRITE(108,135)
  MODE(I)=7*(IIG,5X)
  110 FORMAT(' MODE(I)=7*(IIG,5X)')
  126 FORMAT(' FREQ (RAD/SEC) ',IP,7(G14.5,1X) )
  127 FORMAT(' FREQUENCY (HZ) ',IP,7(G14.5,1X) )
  130 FORMAT(' PERIOD (SEC) ',IP,7(G14.5,1X) )
  135 FORMAT(' EIGENVECTORS: ')
C----- EIGENVECTORS
DO 155 I=1,NFREE
  DO 155 J=1,NFREE
    DO 155 K=1,3
      IF (NOT BTEST(JTFLG(NODE),JDIR+1)) THEN
        I=IDOP(NODE,JDIR)-NCND
        IF (I.GE.1 .AND. I.LE.NFREE) PLU(I,I,K)=COSINE(JDIR,K,ICOS)
      ENDIF
      650 CONTINUE
      554 CONTINUE
      451 CONTINUE
  155 IF (PRINS) CALL WMATRX(PLU,NFREE,3,'INFLUENCE COEF. MATRIX')
  92 FORMAT('X, DIRECTION OF COSINE ',4X,SS,FS,5)
  & ' I ',SP,FS,5,' J ',FS,5,' K ',SS,5)
  93 FORMAT('X,IP,LOG12 4')
  94 FORMAT('I,X, INPUT RESPONSE SPECTRUM VALUES ',/)
  929 FORMAT('X, DIRECTION OF DISPLACEMENT ',4X,SS,FS,5)
  & ' I ',SP,FS,5,' J ',FS,5,' K ',SS,5)
C----- INPUT DIRECTION VECTORS V1 AND V2
READ(105,*) V1,V2
C----- CALCULATE V3 = V1 X V2
CALL CROSS(V3,V1,V2,0)
C----- CALCULATE V2 = V3 X V1, AND NORMALIZE ALL VECTORS ..
CALL CROSS(V2,V3,V1,1)
C----- INPUT RESPONSE SPECTRUM VALUES --
WRITE(108,10) TITLE(1),TITLE(2)
DO 39 I=1,N
  READ(105,*) IDIR
  IF (IDIR.GE.1 .AND. IDIR.LE.3) THEN
    IF (TEST(OPTION,'ACC',.FALSE.)) THEN
      WRITE(108,107) I
      107 FORMAT ('//IX, ** ACCELERATION SPECTRUM VALUES ',
& ' INPUTTING TRANSLATIONAL #',I2,/)
    ELSE IF (TEST(OPTION,'DISP',.FALSE.)) THEN
      WRITE(108,107) I
      1076 FORMAT ('//IX, ** DISPLACEMENT SPECTRUM VALUES ',
& ' INPUTTING TRANSLATIONAL #',I2,/)
    ENDIF
    ELSE
      WRITE(108,*) 'INVALID DIRECTION:',IDIR,' SET 0 < IDIR < 4'
      GO TO 30
    ENDIF
  IF (IDIR.EQ.1) THEN
    CX=V1(1)
    CY=V1(2)
    CZ=V1(3)
    WRITE(108,92) V1
  ELSE IF (IDIR.EQ.2) THEN
    CX=V2(1)
    CY=V2(2)
    CZ=V2(3)
    WRITE(108,92) V2
  ELSE IF (IDIR.EQ.3) THEN
    CX=V3(1)
    CY=V3(2)
    CZ=V3(3)
    WRITE(108,92) V3
  ENDP
  READ(105,*) (RSPN(1,II),RSPN(2,II),II=1,NAP)
  WRITE(108,640)
  640 FORMAT(30X,'PERIOD ',10X,'SPECTRUM VALUE',/
& ' 30X,',10X)
  WRITE(108,907) (RSPN(1,II),RSPN(2,II),II=1,NAP)
  907 FORMAT(30X,G14.6,10X,G14.6)
C
DO 20 J=1,NCNDE
  CALL TINFO(EVAL(J),TMP(3),NAP,RSPN)
  IF (TEST(OPTION,'ACC',.FALSE.)) THEN
    ACC=TMP(J)*ASCALE
    DIS=TMP(J)*ASCALE*((6.2831853/EVAL(J))**2)
  ELSE IF (TEST(OPTION,'DISP',.FALSE.)) THEN
    DIS=TMP(J)*ASCALE
    ACC=TMP(J)*ASCALE*((6.2831853/EVAL(J))**2)
  ENDP
  RA(J,1)=RA(J,1)+ACC*CX
  RA(J,2)=RA(J,2)+ACC*CY
  RA(J,3)=RA(J,3)+ACC*CZ
  RD(J,1)=RD(J,1)+DIS*CX
  RD(J,2)=RD(J,2)+DIS*CY
  RD(J,3)=RD(J,3)+DIS*CZ
  20 CONTINUE
  30 CONTINUE
  IF (PRINS) THEN
    WRITE(108,31) 'X'
    WRITE(108,93) (RA(J,1),J=1,NCNDE)
    WRITE(108,31) 'Y'
    WRITE(108,93) (RA(J,2),J=1,NCNDE)
    WRITE(108,31) 'Z'
    WRITE(108,93) (RA(J,3),J=1,NCNDE)
  31 FORMAT(/IX, 'GLOBAL BASE ACCE. -SPECTRUM VALUES IN THE 'A,
& ' DIRECTION ',/)
    WRITE(108,319) 'X'
    WRITE(108,93) (RD(J,1),J=1,NCNDE)
    WRITE(108,319) 'Y'
    WRITE(108,93) (RD(J,2),J=1,NCNDE)
    WRITE(108,319) 'Z'
    WRITE(108,93) (RD(J,3),J=1,NCNDE)
  319 FORMAT(/IX, 'GLOBAL BASE DISP. -SPECTRUM VALUES IN THE 'A,
& ' DIRECTION ',/)
  ENDP
C----- CALCULATE INFLUENCE COEFFICIENT VECTORS --
DO 440 I=1,NDOP
  DO 440 J=1,3
    440 FLU(I,J)=0
C
DO 451 K=1,3
  OMIT ALL DOP THAT ARE CONSTRAINED TO A MASTER DOP...
  DO 554 NODE=1,NCNDE
    ICOS=ICOS(NODE)
    DO 650 JDIR=1,3
      IF (NOT BTEST(JTFLG(NODE),JDIR+1)) THEN
        I=IDOP(NODE,JDIR)-NCND
        IF (I.GE.1 .AND. I.LE.NFREE) PLU(I,I,K)=COSINE(JDIR,K,ICOS)
      ENDIF
      650 CONTINUE
      554 CONTINUE
      451 CONTINUE
  IF (PRINS) CALL WMATRX(PLU,NFREE,3,'INFLUENCE COEF. MATRIX')
  92 FORMAT('X, DIRECTION OF COSINE ',4X,SS,FS,5)
  & ' I ',SP,FS,5,' J ',FS,5,' K ',SS,5)
  93 FORMAT('X,IP,LOG12 4')
  94 FORMAT('I,X, INPUT RESPONSE SPECTRUM VALUES ',/)
  929 FORMAT('X, DIRECTION OF DISPLACEMENT ',4X,SS,FS,5)
  & ' I ',SP,FS,5,' J ',FS,5,' K ',SS,5)
C----- INPUT DIRECTION VECTORS V1 AND V2
READ(105,*) V1,V2
C----- CALCULATE V3 = V1 X V2
CALL CROSS(V3,V1,V2,0)
C----- CALCULATE V2 = V3 X V1, AND NORMALIZE ALL VECTORS ..
CALL CROSS(V2,V3,V1,1)
C----- INPUT RESPONSE SPECTRUM VALUES --
WRITE(108,10) TITLE(1),TITLE(2)
DO 39 I=1,N
  READ(105,*) IDIR
  IF (IDIR.GE.1 .AND. IDIR.LE.3) THEN
    IF (TEST(OPTION,'ACC',.FALSE.)) THEN
      WRITE(108,107) I
      107 FORMAT ('//IX, ** ACCELERATION SPECTRUM VALUES ',
& ' INPUTTING TRANSLATIONAL #',I2,/)
    ELSE IF (TEST(OPTION,'DISP',.FALSE.)) THEN
      WRITE(108,107) I
      1076 FORMAT ('//IX, ** DISPLACEMENT SPECTRUM VALUES ',
& ' INPUTTING TRANSLATIONAL #',I2,/)
    ENDIF
    ELSE
      WRITE(108,*) 'INVALID DIRECTION:',IDIR,' SET 0 < IDIR < 4'
      GO TO 30
    ENDIF
  IF (IDIR.EQ.1) THEN
    CX=V1(1)
    CY=V1(2)
    CZ=V1(3)
    WRITE(108,92) V1
  ELSE IF (IDIR.EQ.2) THEN
    CX=V2(1)
    CY=V2(2)
    CZ=V2(3)
    WRITE(108,92) V2
  ELSE IF (IDIR.EQ.3) THEN
    CX=V3(1)
    CY=V3(2)
    CZ=V3(3)
    WRITE(108,92) V3
  ENDP
  READ(105,*) (RSPN(1,II),RSPN(2,II),II=1,NAP)
  WRITE(108,640)
  640 FORMAT(30X,'PERIOD ',10X,'SPECTRUM VALUE',/
& ' 30X,',10X)
  WRITE(108,907) (RSPN(1,II),RSPN(2,II),II=1,NAP)
  907 FORMAT(30X,G14.6,10X,G14.6)
C
DO 20 J=1,NCNDE
  CALL TINFO(EVAL(J),TMP(3),NAP,RSPN)
  IF (TEST(OPTION,'ACC',.FALSE.)) THEN
    ACC=TMP(J)*ASCALE
    DIS=TMP(J)*ASCALE*((6.2831853/EVAL(J))**2)
  ELSE IF (TEST(OPTION,'DISP',.FALSE.)) THEN
    DIS=TMP(J)*ASCALE
    ACC=TMP(J)*ASCALE*((6.2831853/EVAL(J))**2)
  ENDP
  RA(J,1)=RA(J,1)+ACC*CX
  RA(J,2)=RA(J,2)+ACC*CY
  RA(J,3)=RA(J,3)+ACC*CZ
  RD(J,1)=RD(J,1)+DIS*CX
  RD(J,2)=RD(J,2)+DIS*CY
  RD(J,3)=RD(J,3)+DIS*CZ
  20 CONTINUE
  30 CONTINUE
  IF (PRINS) THEN
    WRITE(108,31) 'X'
    WRITE(108,93) (RA(J,1),J=1,NCNDE)
    WRITE(108,31) 'Y'
    WRITE(108,93) (RA(J,2),J=1,NCNDE)
    WRITE(108,31) 'Z'
    WRITE(108,93) (RA(J,3),J=1,NCNDE)
  31 FORMAT(/IX, 'GLOBAL BASE ACCE. -SPECTRUM VALUES IN THE 'A,
& ' DIRECTION ',/)
    WRITE(108,319) 'X'
    WRITE(108,93) (RD(J,1),J=1,NCNDE)
    WRITE(108,319) 'Y'
    WRITE(108,93) (RD(J,2),J=1,NCNDE)
    WRITE(108,319) 'Z'
    WRITE(108,93) (RD(J,3),J=1,NCNDE)
  319 FORMAT(/IX, 'GLOBAL BASE DISP. -SPECTRUM VALUES IN THE 'A,
& ' DIRECTION ',/)
  ENDP
C----- CALCULATE INFLUENCE COEFFICIENT VECTORS --
DO 440 I=1,NDOP
  DO 440 J=1,3
    440 FLU(I,J)=0
C
DO 451 K=1,3
  OMIT ALL DOP THAT ARE CONSTRAINED TO A MASTER DOP...
  DO 554 NODE=1,NCNDE
    ICOS=ICOS(NODE)
    DO 650 JDIR=1,3
      IF (NOT BTEST(JTFLG(NODE),JDIR+1)) THEN
        I=IDOP(NODE,JDIR)-NCND
        IF (I.GE.1 .AND. I.LE.NFREE) PLU(I,I,K)=COSINE(JDIR,K,ICOS)
      ENDIF
      650 CONTINUE
      554 CONTINUE
      451 CONTINUE
  IF (PRINS) CALL WMATRX(PLU,NFREE,3,'INFLUENCE COEF. MATRIX')
  92 FORMAT('X, DIRECTION OF COSINE ',4X,SS,FS,5)
  & ' I ',SP,FS,5,' J ',FS,5,' K ',SS,5)
  93 FORMAT('X,IP,LOG12 4')
  94 FORMAT('I,X, INPUT RESPONSE SPECTRUM VALUES ',/)
  929 FORMAT('X, DIRECTION OF DISPLACEMENT ',4X,SS,FS,5)
  & ' I ',SP,FS,5,' J ',FS,5,' K ',SS,5)

```



```

C-----
C-- CALCULATE CROSS-MODAL COEFFICIENTS FOR CQC APPROACH --
C-----
IF ( ICOMN.EQ.2. ) THEN
  DO 809 I=1,NMODE
    CHC(I,2)=1
    DO 809 J=1,NMODE
      WJI=EVAL(J)/EVAL(I)
      COEF1=9*(DAMP**2)*(1+WJI)*(WJI**1.5)
      COEF2=(1+WJI**2)**2 + 4*(DAMP**2)*WJI*(1+WJI)**2
      CHC(I,2)=COEF1/COEF2
      CHC(I,2)=CHC(I,2)
    CONTINUE
  809 CONTINUE
C
WRITE(108,10) TITLE(1),TITLE(2)
CALL WHATRX(CHC,NMODE,NMDE,'CROSS-MODAL COF MATRIX')
ENDIF
C-----
C-- CALCULATE GENERALIZED MASS, GM=EVC*B*EVC --
C-----
DO 450 I=1,NMODE
  GM(I)=0
  DO 460 II=1,NFREE
    DISP(II)=0
    DO 470 JJ=1,NFREE
      DISP(JJ)=DISP(II) + B(II,JJ)*EVC(JJ,I)
    CONTINUE
  470 CONTINUE
  460 CONTINUE
  450 CONTINUE
IF (PRINS) CALL WHATRX(GM,NMODE,1,'GENERALIZED MASS MATRIX')
C-----
C-- CALCULATE EFFECTIVE LOAD FACTOR FOR EACH MODE, ELF=EVC*B*PLU --
C-----
DO 455 I=1,NMODE
  DO 457 II=1,NFREE
    ELP(I,K)=0
    DO 465 II=1,NFREE
      DISP(II)=0
      DO 475 JJ=1,NFREE
        DISP(JJ)=DISP(II) + B(II,JJ)*PLU(JJ,K)
      CONTINUE
    475 CONTINUE
    465 CONTINUE
    457 CONTINUE
  455 CONTINUE
IF (PRINS) CALL WHATRX(ELP,NMODE,3,'EFFECTIVE LOAD MATRIX')
C-----
C-- CALCULATE EFFECTIVE MASS FOR EACH MODE IN EACH INDIVIDUAL --
C-- SEISMIC INPUT DIRECTION
C-----
READ EFFECTIVE MASS RATIO LIMIT IN X, Y, Z DIRECTION
NOTE: IF NO OF MODES BASED ON THE LIMIT IS LESS THAN NMODE,
NO. OF MODES BASED ON THIS LIMIT IS USED.
C-----
READ(105,*) XLIM,YLIM,ZLIM
C
WRITE(108,10) TITLE(1),TITLE(2)
WRITE(108,487)
487 FORMAT(1X,'EFFECTIVE MASS IN GCS' TRANSLATIONAL DIRECTIONS',/
& 1X,-----)
C
WRITE(108,987) 'X',XLIM
WRITE(108,987) 'Y',YLIM
WRITE(108,987) 'Z',ZLIM
C
C-- CALCULATE TOTAL STRUCTURAL TRANSLATIONAL MASS
TOTX=0
TOTY=0
TOTZ=0
DO 705 J=L3,L4
  LJ=NOMS(J)
  DO 706 I=1,NMODE
    IF ( NOT BTEST(JTPLG(I),12) ) THEN
      NDEF=IDEP(I,1)
      IF (J.EQ.NDEF) TOTX=TOTX+MASS(I,J)
    ENDIF
    IF ( NOT BTEST(JTPLG(I),13) ) THEN
      NDEF=IDEP(I,2)
      IF (J.EQ.NDEF) TOTY=TOTY+MASS(I,J)
    ENDIF
    IF ( NOT BTEST(JTPLG(I),14) ) THEN
      NDEF=IDEP(I,3)
      IF (J.EQ.NDEF) TOTZ=TOTZ+MASS(I,J)
    ENDIF
  706 CONTINUE
705 CONTINUE
WRITE(108,388) 'X',TOTX
WRITE(108,388) 'Y',TOTY
WRITE(108,388) 'Z',TOTZ
388 FORMAT(1X,'TOTAL STRUCTURAL MASS IN ',A,' DIRECTION IS ',
& 1X,OP,G15.6)
WRITE(108,386)
386 FORMAT(1X,' MODE ', 'GCS X-DIRECTION( ) ',
& 'GCS Y-DIRECTION( ) ',
& 'GCS Z-DIRECTION( ) ')
987 FORMAT(1X,'EFFECTIVE MASS LIMIT IN GCS',A,
& ' DIRECTION IS ',G15.6)
C
TLXIM=0
TYLIM=0
TZLIM=0
MODEX=0
MODEY=0
MODEZ=0
FMXE=0
FMZE=0
FMZE=0
DO 701 I=1,NMODE
  IF (TOTX.EQ.0.) GO TO 707
  FMXE=(ELP(I,1)**2/GM(I))/TOTX
  TLXIM=TLXIM+FMXE
  IF (X LIM.NE.0.AND.MODEX.EQ.0.AND.TLXIM.GE.XLIM) MODEX=I
  IF (TOTY.EQ.0.) GO TO 709
  FMZE=(ELP(I,2)**2/GM(I))/TOTY
  TYLIM=TYLIM+FMZE
  IF (Y LIM.NE.0.AND.MODEY.EQ.0.AND.TYLIM.GE.YLIM) MODEY=I
  IF (TOTZ.EQ.0.) GO TO 709
  FMZE=(ELP(I,3)**2/GM(I))/TOTZ
  TZLIM=TZLIM+FMZE
  IF (Z LIM.NE.0.AND.MODEZ.EQ.0.AND.TZLIM.GE.ZLIM) MODEZ=I
709 WRITE(108,389) 1,FMXE,FMZE,FMZE
389 FORMAT(5X,13,5X,3,G15.6,5X)
701 CONTINUE
C
IMO=MAX(MODEX,MODEY,MODEZ)
IF (IMO.NE.0.AND. IMO.LT. NMODE) THEN
  NMDE=IMO
  WRITE(108,*) '** BASED ON EFFECTIVE MASS LIMIT, FINAL NO OF ',
& 'MODES USED IS ',NMDE,***
ELSE
  NMDE=NMODE
ENDIF
C-----
C-- CALCULATE MAXIMUM DISPLACEMENT RESPONSES --
C-----
DO 566 NN=1,NA
  DO 540 I=1,L6
    TMP(I)=0
    DO 541 II=1,NFREE
      DO 550 J=1,NMODE
        DO 500 II=1,NFREE
          DLOAD(II,1)=EVC(I,II)*ELF(J,NN)*RD(J,NN)/GM(J)
          IF (ICOMN.EQ.2) THEN
            CQC(II,NCONDJ)=DLOAD(II,1)
            ELSE IF (ICOMN.EQ.1) THEN
              TMP(II,NCONDJ)=DLOAD(II,1)**2 + TMP(II,NCONDJ)
            ENDIF
          CONTINUE
        500 CONTINUE
        IF (PRINS) THEN
          WRITE(108,*) 'MODAL DISPLACEMENTS FOR MODE ',J
          WRITE(108,*) ('DISP('),I,') = ',DLOAD(II,1),I-1,NFREE)
        ENDIF
        DO 501 LL=L3,L4
          DISP(LL)=0
        501 CONTINUE
        DO 502 LL=L3,L4
          DISP(LL)=DLOAD(LL,L2,1)
        502 CONTINUE
        C-----
        C-- CALCULATE EQUIVALENT EARTHQUAKE FORCES --
        C-----
        DO 753 I=L5,L6
          DISP(I)=0
          LOAD(I)=0
        753 CONTINUE
        DO 580 I=1,NFREE
          LOAD(I,NCONDJ)=0
          DO 735 JJ=1,NFREE
            LOAD(I,NCONDJ)=LOAD(I,NCONDJ)+A(I,JJ)*DISP(JJ,NCONDJ)
          IF (ICOMN.EQ.1) THEN
            TMP2(I)=TMP2(I)+LOAD(I,NCONDJ)**2
          ELSE IF (ICOMN.EQ.2) THEN
            CQC2(I,J)=LOAD(I,NCONDJ)
          ENDIF
        580 CONTINUE
        IF (PRINS) THEN
          WRITE(108,10) TITLE(1),TITLE(2)
          WRITE(108,605) J,NN
          FORMAT(1X,'TOTAL EQUIVALENT EARTHQUAKE FORCES FOR MODE ',I3,
& ' IN THE SEISMIC COMPONENT # ',I2/)
          WRITE(108,212) (I,NCONDJ,LOAD(I,NCONDJ),I-1,NFREE)
        ENDIF
        605 CONTINUE
        C-----
        C-- CALC RESPONSE FOR CONDENSED OUT DOF --
        C-----
        CALL REACTN(1,BTEST(1BUG,6),NMODE,L1,L2,L3,L4,DISP,DISP,MD,
& STIFF,IMD,NDFP,1,1,0,MAXNCD,IDOF,COORD,ID,TITLE,
& STEPID, FALSE, NCOS,COSINE,JTCOS,JTFLG,SUBRCT)
        CALL MSOLL(3,BTEST(1BUG,6),L4,IMD,NDFP,1,L1,L2,MD,STIFF,DISP,
& WORK, FALSE, MASS,MDMS,1, FALSE,KG,MDKG,1)
        DO 675 I=L1,L2
          IF (ICOMN.EQ.1) THEN
            TMP(I)=DISP(I)**2 + TMP(I)
          ELSE IF (ICOMN.EQ.2) THEN
            CQC(I,2)=DISP(I)
          ENDIF
        675 CONTINUE
        C-----
        C-- CALCULATE MEMBER FORCES --
        C-----
        LSTTYP=0
        DO 358 IELNO=1,NELMT
          DO 357 I=1,12
            RSAF(I)=0
          CALL ELELIB
            (4,LSTTYP,PRINS,IREL, FALSE, NAME,EPSE,EPSE,DAMAGE,DUCPAG,
            IELNO,IELDOP,KGDOF,PGRM,RINPUT,AXIAL,12DISP,12LOAD,HSTCR)
          DO 359 IELNO,I,J) RSAF(I)
        358 CONTINUE
        C-----
        C-- SOLVE FOR REACTIONS ---
        C-----
        CALL REACTN(2,PRINS,NMODE,L1,L4,L5,L6,LOAD,DISP,MD,
& STIFF,IMD,NDFP,1,1,0,MAXNCD,IDOF,COORD,ID,TITLE,
& STEPID, FALSE, NCOS,COSINE,JTCOS,JTFLG,SUBRCT)
        C-----
        C-- SOLVE FOR GLOBAL REACTIONS ---
        C-----
        DO 366 I=1,6
          SUBRCT(I)=0
        CALL REACTN(3,PRINS,NMODE,L1,L4,L5,L6,LOAD,DISP,MD,
& STIFF,IMD,NDFP,1,1,0,MAXNCD,IDOF,COORD,ID,TITLE,
& STEPID, FALSE, NCOS,COSINE,JTCOS,JTFLG,SUBRCT)
        DO 360 I=1,6
          RCT(I,J)=SUBRCT(I)
        IF (PRINS) THEN
          WRITE(108,10) TITLE(1),TITLE(2)
          WRITE(108,607) J,NN
          FORMAT(1X,'REACTION FOR MODE ',I3, ' IN SEISMIC COMP # ',I2/)
          WRITE(108,323) SUBRCT
        ENDIF
        607 CONTINUE
        C-----
        C-- SOLVE FOR REACTIONS ---
        C-----
        IF (ICOMN.EQ.1) THEN
          DO 590 JJ=1,L4
            TMP(JJ)=SQRT(TMP(JJ))
          DO 547 II=1,NFREE
            TMP2(JJ)=SQRT(TMP2(JJ))
          DO 591 I=1,NELMT
            DO 592 J=1,NMODE
              ELEF(I,II,NMDE+NN)=ELEF(I,II,NMDE+NN)+ELEF(I,II,J)**2
              ELEF(I,II,NMDE+NN)=SQRT(ELEF(I,II,NMDE+NN))
            CONTINUE
          592 CONTINUE
          DO 594 I=1,6
            RCT(I,NMDE+NN)=RCT(I,NMDE+NN)+RCT(I,J)**2
            RCT(I,NMDE+NN)=SQRT(RCT(I,NMDE+NN))
          594 CONTINUE
          593 CONTINUE
        ELSE IF (ICOMN.EQ.2) THEN
          DO 828 L=1,L4
            TMP(L)=0
            DO 828 J=1,NMODE
              DO 828 JJ=1,NFREE
                CQC(L,J)=CQC(L,J)*CHC(J,J)*CQC(L,J)
            TMP(L)=SQRT(ABS(TMP(L)))
          828 CONTINUE
          DO 837 L=1,NFREE
            DO 838 JJ=1,NMODE
              TMP2(L)=TMP2(L)+CQC(L,J)*CQC(L,J)*CQC2(L,J)
            CONTINUE
          837 CONTINUE
          DO 595 I=1,NELMT
            DO 595 II=1,12
              SUM=0
              DO 596 J=1,NMDE
                SUM=SUM+ELEF(I,II,J)*CHC(J,J)*ELEF(I,II,J)
              SUM=ABS(SUM)
              ELEF(I,II,NMDE+NN)=SQRT(SUM)
            CONTINUE
          595 CONTINUE
          DO 597 I=1,6
            SUM=0

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DO 598 J=1,MMODE
DO 598 JJ=1,MMODE
598 SUM=SUM+RCT(I,J)*CMC(J,JJ)*RCT(I,JJ)
SUM=ABS(SUM)
597 RCT(I,MMODE+NN)-SQRT(SUM)
CONTINUE
C
ENDIF
C
IF (NA GT 1) THEN
WRITE(108,10) TITLE(1),TITLE(2)
WRITE(108,489) NN
489 FORMAT(1X,'MAXIMUM STRUCTURAL RESPONSES ',
& ' DUE TO SEISMIC COMPONENT # ',I2,/,
& ' 1X, ',/,
& '-----',/)
WRITE(108,913)
913 FORMAT(3(' I D O.P. DISPL ',/,
WRITE(108,212) (I=NCND,TMP(I=NCND),I=1,NFREE)
C
CALCULATE TOTAL ELEMENT FORCES
WRITE(108,10) TITLE(1),TITLE(2)
WRITE(108,984) NN
984 FORMAT(1X,'MAXIMUM ELEMENT FORCES DUE TO SEISMIC COMPONENT # ',
& ' 1X, ',/,
& '-----',/)
PRNO=TRUE
LSTTYP=0
DO 905 I=1,NELMT
IELNO=I
DO 959 II=1,12
RSAP(II)=ELF(I,II,MMODE+NN)
959 CALL ELELIB
& (S,LSTTYP,PRNO,IREL,FALSE,NAME,ESE,EPSE,DAMAGE,DUCPAG,
& IELNO,IELDOP,KDOP,PGEOM,RINPUT,AXIAL,IZDISP,IZLOAD,MSTOR)
905 CONTINUE
C
CALCULATE TOTAL EQUIVALENT EARTHQUAKE FORCES
WRITE(108,10) TITLE(1),TITLE(2)
WRITE(108,184) NN
184 FORMAT(1X,'MAXIMUM EQUIVALENT EARTHQUAKE FORCES ',
& ' DUE TO SEISMIC COMPONENT # ',I2,/,
& ' 1X, ',/,
& '-----',/)
WRITE(108,912)
WRITE(108,212) (I=NCND,TMP(I),I=1,NFREE)
C
CALCULATE TOTAL REACTION AT GCS' ORIGIN
DO 696 I=1,6
SUMRCT(I)=RCT(I,MMODE+NN)
WRITE(108,10) TITLE(1),TITLE(2)
WRITE(108,232) NN
232 FORMAT(1X,'TOTAL REACTIONS AT GCS' ORIGIN ',
& ' DUE TO SEISMIC COMPONENT # ',I2,/,
& ' 1X, ',/,
& '-----',/)
& ' 17X, 'FX',13X, 'FY',13X, 'PZ',13X, 'MX',13X, 'MY',13X, 'MZ',/)
WRITE(108,323) SUMRCT
ENDIF
C
STORE RESPONSE TO DISP ARRAY
DO 200 J=L1,L6
DLOAD(J,NN)=TMP(J)
DO 220 J=L5,L6
DLOAD(J,NN)=0
C
STORE EQUIVALENT EARTHQUAKE FORCES IN EEP ARRAY
DO 291 J=1,NFREE
EFP(J,NN)=TMP2(J)
C
596 CONTINUE
C
CALCULATE TOTAL RESPONSES
DO 237 J=L1,L6
DISP(J)=0
DO 237 I=1,NA
DISP(J)=DISP(J)+DLOAD(J,I)
WRITE(108,10) TITLE(1),TITLE(2)
WRITE(108,608) NN
608 FORMAT(1X,'TOTAL MAXIMUM STRUCTURAL RESPONSES',/,
& ' 1X, ',/,
& '-----',/)
WRITE(108,913)
WRITE(108,212) (I=NCND,DISP(I=NCND),I=1,NFREE)
C
CALCULATE TOTAL EQUIVALENT EARTHQUAKE FORCE
DO 202 J=L1,NFREE
TMP2(J)=0
DO 202 I=1,NA
TMP2(J)=TMP2(J)+EFP(J,I)
WRITE(108,10) TITLE(1),TITLE(2)
WRITE(108,678) NN
678 FORMAT(1X,'TOTAL MAXIMUM EQUIVALENT EARTHQUAKE FORCES',/,
& ' 1X, ',/,
& '-----',/)
WRITE(108,912)
WRITE(108,212) (I=NCND,TMP2(I),I=1,NFREE)
C
C
C
PRINT GLOBAL DISPLACEMENTS --
C
CALCULATE TOTAL ELEMENT FORCES
WRITE(108,10) TITLE(1),TITLE(2)
WRITE(108,980) NN
980 FORMAT(1X,'TOTAL MAXIMUM ELEMENT FORCES',/,
& ' 1X, ',/,
& '-----',/)
PRNO=TRUE
LSTTYP=0
DO 509 I=1,NELMT
IELNO=I
DO 599 II=1,12
RSAP(II)=0
DO 599 J=1,NA
RSAP(II)+RSAP(II)+ELF(I,II,MMODE+J)
599 CALL ELELIB
& (S,LSTTYP,PRNO,IREL,FALSE,NAME,ESE,EPSE,DAMAGE,DUCPAG,
& IELNO,IELDOP,KDOP,PGEOM,RINPUT,AXIAL,IZDISP,IZLOAD,MSTOR)
509 CONTINUE
C
CALCULATE TOTAL REACTION AT GCS' ORIGIN
DO 699 I=1,6
SUMRCT(I)=0
DO 699 J=1,NA
SUMRCT(I)+SUMRCT(I)+RCT(I,MMODE+J)
699 CONTINUE
WRITE(108,10) TITLE(1),TITLE(2)
WRITE(108,322) NN
322 FORMAT(1X,'TOTAL REACTIONS AT GCS' ORIGIN ',
& ' DUE TO SEISMIC COMPONENT # ',I2,/,
& ' 17X, 'FX',13X, 'FY',13X, 'PZ',13X, 'MX',13X, 'MY',13X, 'MZ',/)
WRITE(108,323) SUMRCT
323 FORMAT(' GCS SUMM. ',6G15.7)
C
CALCULATE MAXIMUM INERTIA EARTHQUAKE FORCES --
C
DO 207 NN=1,NA
DO 203 I=1,NFREE
TMP(I)=0
DO 204 J=1,MMODE
DO 205 II=1,NFREE
WORK(II)=0
DO 206 JJ=1,NFREE
WORK(II)+WORK(II)+B(II,II)*EVC(JJ,J)
206 DISP(II)=WORK(II)+ELF(J,NN)*R(J,NN)/GM(J)
205 CONTINUE

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```

P=1.2-15
DO 60 I=N,1,-1
  X=Z(I,J)
  I=I-1
  IF ((I1-N).GT.0) GOTO 50
DO 40 K=1,N
  X=X-B(K,I)*Z(K,J)
  Z(I,J)=W/DL(I)
  IF (ABS(Z(I,J)).GT.P) P=ABS(Z(I,J))
50 CONTINUE
DO 80 I=1,N
  Z(I,J)=Z(I,J)/P
100 CONTINUE
RETURN
END

C
C
C
SUBROUTINE JACOBI(N,A,D,V,B,Z)
DIMENSION A(N,N),D(N),V(N,N),B(N),Z(N)
C
20 DO 21 I=1,N
  DO 22 J=1,N
    V(I,J)=0.0
    Z(I,J)=1.
21 CONTINUE
DO 30 K=1,N
  D(K)=A(K,K)
  B(K)=D(K)
  Z(K)=0.
30 CONTINUE
DO 40 I=1,500
  SM=0.0
  NI=N-1
  DO 50 IP=1,NI
    IP1=IP+1
    DO 50 IQ=IP1,N
      SM=SM+ABS(A(IP,IQ))
      IF (SM) 60,70,60
60 IF (I-4) 80,90,90
80 TRESH=0.2*SM/PLGAT(N)**2
      GOTO 100
90 TRESH=0.0
100 DO 110 IP=1,NI
    IP1=IP+1
    DO 110 IQ=IP1,N
      G=100.0*ABS(A(IP,IQ))
      IF (G LE 1.E-20) G=0.0
      IF (I-4) 120,120,120,140,140,120
130 IF (ABS(D(IQ))-G) ABS(D(IP)) 120,140,120
140 IF (ABS(D(IQ))-G) ABS(D(IQ)) 120,150,120
150 A(IP,IQ)=0.0
      GOTO 110
160 IF (ABS(A(IP,IQ))-TRESH) 110,110,160
      H=D(IQ)-D(IP)
      IF (ABS(H)+G) ABS(H) 170,180,170
180 T=A(IP,IQ)/H
      GOTO 190
190 THETA=0.5*H/A(IP,IQ)
      T=SIGN(1.0,THETA)/(ABS(THETA)+SQRT(1.-THETA*THETA))
      C=1./SQRT(1.0-T*T)
      S=T*C
      TAU=S/(1.-C)
      H=T*A(IP,IQ)
      Z(IP)=Z(IP)+H
      Z(IQ)=Z(IQ)+H
      D(IP)=D(IP)+H
      D(IQ)=D(IQ)+H
      A(IP,IQ)=0.0
      IF (IP) 210,210,201
201 DO 205 K=1,IP2
      G=A(K,IP)
      H=A(K,IQ)
      A(K,IP)=G-S*(H+G*TAU)
      A(K,IQ)=H-S*(G+H*TAU)
205 CONTINUE
      IQ1=IQ-1
      IF (IQ1-IQ1) 211,211,220
211 DO 215 K=IP1,IQ1
      G=A(IP,K)
      H=A(K,IQ)
      A(IP,K)=G-S*(H+G*TAU)
      A(K,IQ)=H-S*(G+H*TAU)
215 CONTINUE
      IQ1=IQ-1
220 IF (IQ1-N) 221,221,230
221 DO 225 K=IQ1,N
      G=A(IP,K)
      H=A(K,IQ)
      A(IP,K)=G-S*(H+G*TAU)
      A(K,IQ)=H-S*(G+H*TAU)
225 CONTINUE
      DO 235 K=1,N
      G=V(K,IP)
      H=V(K,IQ)
      V(K,IP)=G-S*(H+G*TAU)
      V(K,IQ)=H-S*(G+H*TAU)
235 CONTINUE
      DO 240 K=1,N
      B(K)=B(K)+2*(K)
      D(K)=B(K)
      Z(K)=0.0
240 CONTINUE
40 CONTINUE
70 RETURN
END

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REBA0070 INTEGER PHASS,FDAMP,CPUREM,CPUELA
REBA0080 DOUBLE PRECISION BIE,BSE,PSE,EKE,EDD,EDUMMY
REBA0090 COMMON /ZDATA/ TITLE,BUG,IBUG,MAXEFP,MD(25),STEPID,GRAV
REBA0100 COMMON /ZDATA/ IZ,IZREL,CPUREM,CPUELA,ITLCPU,INCCPU,
REBA0110 & NNEW, KNAME,NEWKG,MCOND,KCCOND,DOUBLE,ELSTIC,ABORT,
REBA0120 & NNODE,NDOF,NCOND,NFREE,NRST,MAXNCD,NCOS,NMAT,
REBA0130 & NEMT,NMSS,NLOAD,NCOLN,MAXELD,NELD,IZID,IZIDOF,
REBA0140 & IZCORD,IZCOS,IZCNST,IZPLG,IZCOS,IZMAT,IZELE,IZIELD,
REBA0150 & IZELD,IZXE,IZLM,IZSTIP,IZMD,IZKEG,IZLKG,IZMSS,
REBA0160 & IZDMS,IZMDAT,IZDAMP,IZKGD,IZKG,IZMDKG,IZLOAD,IZDISP,
REBA0170 & IZVEL,IZACC,IZDLOA,IZDOSP,IZDVEL,IZDACC,IZAGTX,IZAGTY,
REBA0180 & IZAGT2,IZDUMP,IZFIB,IZAN,IZAR,PHASS,FDAMP,IUNIT,
REBA0190 & BIE,BSE,PSE,EKE,EDD,EDUMMY,SUBRCT(6),GROUND(9),
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4. Title and Subtitle INRESB-3D-SUPII Program Listing for PC: General Purpose Program for Inelastic Analysis of RC and Steel Building Systems for 3D Static and Dynamic Loads and Seismic Excitations			5. Report Date August 1996
7. Author(s) Franklin Y. Cheng, J.F. Ger, Dan Li and J.S. Yang			6.
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15. Supplementary Notes			13. Type of Report & Period Covered Final
16. Abstract (Limit: 200 words) INRESB-3D-SUPII (PC version) is written in FORTRAN and contains about 30,000 statements. While the PC and supercomputer versions have identical functions, the PC solution capacity is limited by its memory. INRESB-3D-SUPII is a general purpose computer program for analyzing elastic and inelastic building systems subject to static loads, dynamic forces, multicomponent earthquake motion, and pseudo-static cyclic loads. Also the program is capable of calculating inelastic post-buckling behavior of steel members and systems, natural frequency, and free vibration. A joint-based system is used to define the geometry of a structure. Structural members may be elastic 3D prismatic beams, nonlinear RC shear walls, nonlinear springs, inelastic 3D-beam-column elements, finite-segment elements, and non-linear bracing elements. INRESB-3D-SUPII had five main functions. They are 1) elastic static analysis with multiple load cases based on control program SOL01; 2) elastic or inelastic analysis of a structure subject to 3D ground acceleration based on SOL02; 3) calculation of either natural frequencies and mode shape or buckling load and mode shape of an elastic structure based on SOL03; 4) calculation of nonlinear static cyclic response of a given loading pattern which consists of joint loads, imposed displacements, or element loads based on SOL04; and 5) calculation of maximum response of an elastic structure subject to pseudo-dynamic force obtained from the response spectrum based on SOL05. INRESB-3D-SUPII pertains to analysis of building systems above ground level. Any connection with basements is treated as a boundary condition fixed, hinged, or with a spring.			
17. Document Analysis			
a. Descriptors		3D structure	Ground acceleration
		Buckling	Inelastic analysis
		Building system	Mode shape
		Dynamic analysis	Multicomponent earthquake motion
		Finite-segment element	Natural frequency
		FORTRAN	Nonlinear material
			Personal computer
b. Identifiers/Open-Ended Terms			Post-buckling
			Reinforced concrete
			Pseudo-dynamic force
			Response spectrum
			Shear wall
c. COSATI Field/Group			
18. Availability Statement		19. Security Class (This Report) Unclassified	21. No. of Pages 111
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